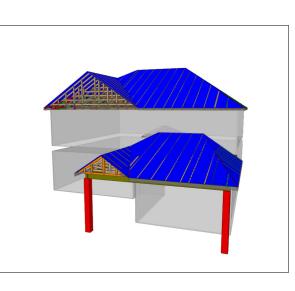


Carter Sanford Component Plant 298 Harvey Faulk Rd Sanford, NC 27332

Phone #:919-775-1450

# **Builder: DR Horton Inc**

# Model: 18 Eagle Creek -Edisto - B



## THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.

2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.

3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.

4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.

5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.

6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.

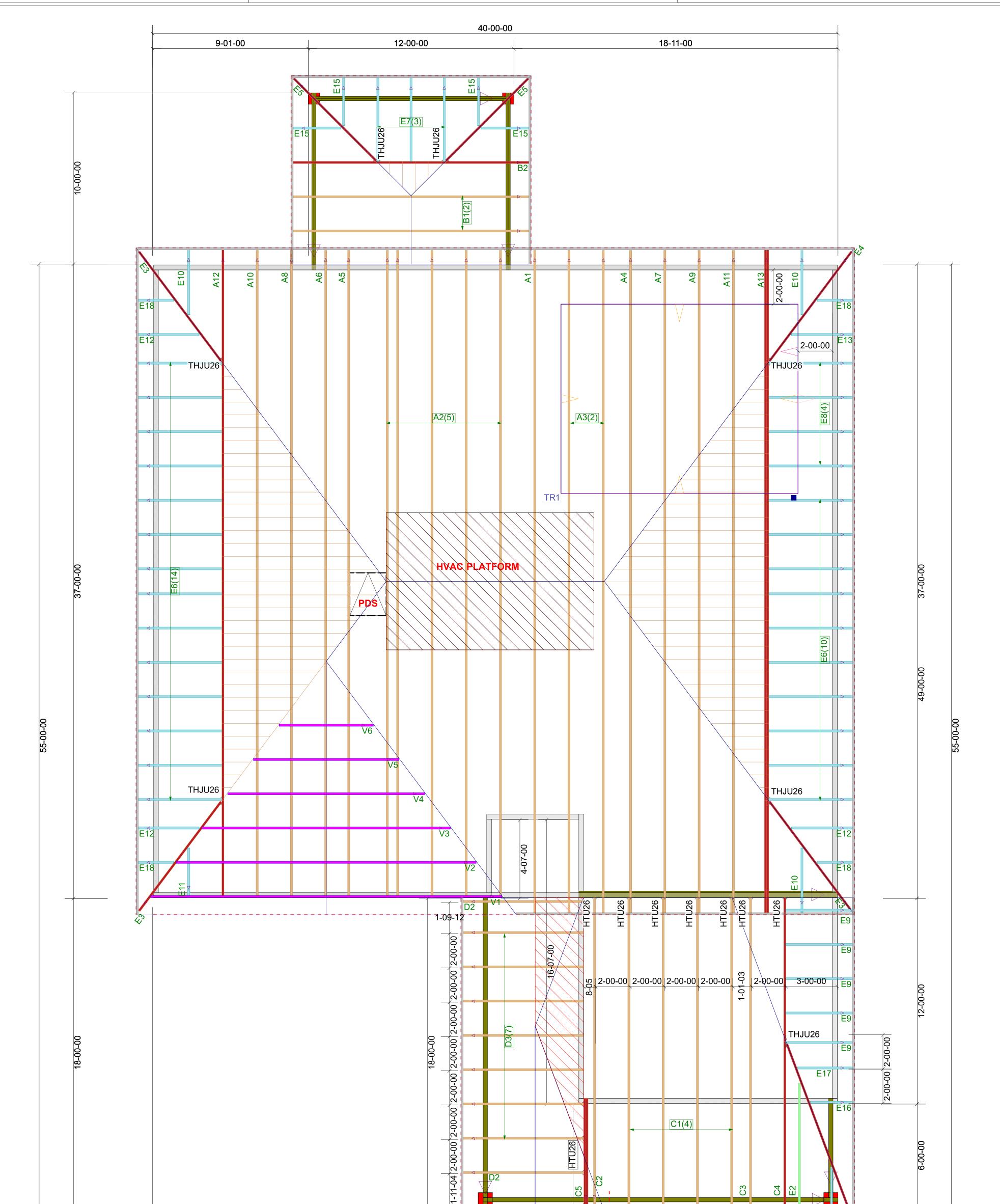
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.

8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.

9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

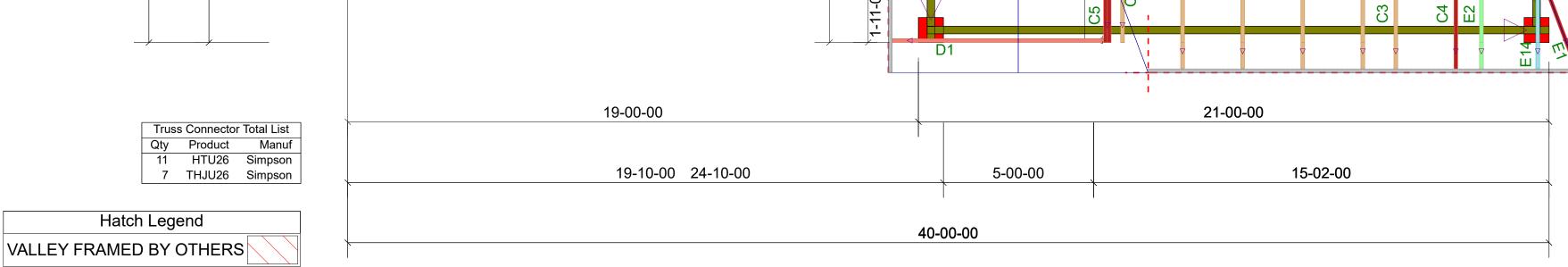
Approved By: \_\_\_\_\_

Date: \_\_\_\_\_





**General Notes:** 



awing Left

\*\* GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS. \*\* DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH. \*\* All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

Date:	DR Horton Inc	R	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss	00/00/00	/00	0/00	
NTS Designer: Designer: Designer: Sheet Num Sheet Num	18 Eagle Creek - Edisto - B	CARTER	design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The disign of the tuss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available	00 00	00/ 1	/00/	Visior
nber:	ROOF PLACEMENT PLAN	Lumber	from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179	Vame Vame	Vame	Name Name	S



RE: 25040187 18 Eagle Creek - Edisto B - Roof Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: DR Horton IncProject Name:25040187Lot/Block: 18Model:Edisto BAddress:Subdivision:Eagle CreekCity:State:

# General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2021/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.7 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 47 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	172941905	A1	4/23/2025	21	172941925	D1	4/23/2025
2	172941906	A2	4/23/2025	22	172941926	D2	4/23/2025
3	172941907	A3	4/23/2025	23	172941927	D3	4/23/2025
4	172941908	A4	4/23/2025	24	172941928	E1	4/23/2025
5	172941909	A5	4/23/2025	25	172941929	E2	4/23/2025
6	172941910	A6	4/23/2025	26	172941930	E3	4/23/2025
7	172941911	A7	4/23/2025	27	172941931	E4	4/23/2025
8	172941912	A8	4/23/2025	28	172941932	E5	4/23/2025
9	172941913	A9	4/23/2025	29	172941933	E6	4/23/2025
10	172941914	A10	4/23/2025	30	172941934	E7	4/23/2025
11	172941915	A11	4/23/2025	31	172941935	E8	4/23/2025
12	172941916	A12	4/23/2025	32	172941936	E9	4/23/2025
13	172941917	A13	4/23/2025	33	172941937	E10	4/23/2025
14	172941918	B1	4/23/2025	34	172941938	E11	4/23/2025
15	172941919	B2	4/23/2025	35	172941939	E12	4/23/2025
16	172941920	C1	4/23/2025	36	172941940	E13	4/23/2025
17	172941921	C2	4/23/2025	37	172941941	E14	4/23/2025
18	172941922	C3	4/23/2025	38	172941942	E15	4/23/2025
19	172941923	C4	4/23/2025	39	172941943	E16	4/23/2025
20	172941924	C5	4/23/2025	40	172941944	E17	4/23/2025

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Carter Components (Sanford, NC)).

Truss Design Engineer's Name: Galinski, John

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Galinski, John



## RE: 25040187 - 18 Eagle Creek - Edisto B - Roof

Trenco 818 Soundside Rd Edenton, NC 27932

## Site Information:

Project Customer: DR Horton Inc Project Name: 25040187 Lot/Block: 18 Subdivision: Eagle Creek Address: City, County: State:

No.	Seal#	Truss Name	Date
41	172941945	E18	4/23/2025
42	172941946	V1	4/23/2025
43	172941947	V2	4/23/2025
44	172941948	V3	4/23/2025
45	172941949	V4	4/23/2025
46	172941950	V5	4/23/2025
47	172941951	V6	4/23/2025

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A1	Common	1	1	Job Reference (optional)	172941905

10-0-12

Loading

TCDL

BCLL

BCDL

WEBS

WEBS

FORCES

WEBS

SLIDER

BRACING

LUMBER

TCLL (roof)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:24 Page: 1 ID:C\_AxBD2r2jGHKNmWKHJBD0zODp6-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f -0-10-8 37-9-8 6-4-3 12-9-9 18-5-8 24-1-7 30-6-13 36-11-0 5-7-15 5-7-15 0-10-8 6-4-3 6-5-6 6-5-6 6-4-3 0-10-8 5x6= 6 5x6 🖌 5x6 34 35 33 36 5 6<sup>12</sup> 3x5 🍃 3x5 👟 37 32 3 9 10 0-10-0 11 1हें|॑॑ 12 18 14 13 23 38 22 20 39 12 4x5= <sup>8x10</sup>**1**5-11-8 4x5= 24-1-7 6x8 ı 6x8 II 13-5-15 13-4-14 24-0-0 23-6-2 12-11-0 12-11-0 0-1-7 7-5-10 12-9-9 18-5-8 20-11-8 23-5-1 29-5-6 36-11-0 7-5-10 5-3-14 2-5-9 2-6-0 2-6-0 2 - 5 - 95-3-14 7-5-10 0-1-1 0-5-14 0-1-1 0-5-14 Scale = 1:71.1 Plate Offsets (X, Y): [5:0-3-0,0-3-0], [7:0-3-0,0-3-0], [13:0-5-0,Edge], [22:0-5-0,Edge] 2-0-0 CSI DEFL in l/defl L/d PLATES GRIP (psf) Spacing (loc) 20.0 Plate Grip DOL 1.15 TC 0.38 Vert(LL) -0.27 17 >999 240 MT20 244/190 Snow (Pf/Pg) 13.9/20.0 Lumber DOL 1.15 BC 0.47 Vert(CT) -0.56 17 >786 180 Rep Stress Incr WB 10.0 YES 0.44 Horz(CT) 0.07 10 n/a n/a 0.0 IRC2021/TPI2014 Matrix-MSH Code 10.0 Weight: 260 lb FT = 20% 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. TOP CHORD 2x4 SP 2400F 2.0E II; Exp B; Enclosed; MWFRS (envelope) and C-C 2x6 SP 2400F 2.0E \*Except\* 21-15:2x4 SP BOT CHORD Exterior(2E) -0-10-1 to 2-10-3, Interior (1) 2-10-3 to No.2 18-5-8, Exterior(2R) 18-5-8 to 22-1-13, Interior (1) 2x4 SP No.3 \*Except\* 22-6,13-6:2x4 SP No.2 22-1-13 to 37-9-1 zone; cantilever left and right Left 2x4 SP No.3 -- 2-0-0, Right 2x4 SP No.3 exposed ; end vertical left and right exposed;C-C for -- 2-0-0 members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33 TOP CHORD Structural wood sheathing directly applied or 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 4-0-3 oc purlins. Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum BOT CHORD Rigid ceiling directly applied or 6-0-0 oc DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully bracing. Exp.; Ce=0.9; Cs=1.00; Ct=1.10 1 Row at midpt 5-23, 7-12 4) Unbalanced snow loads have been considered for this REACTIONS (size) 2=0-3-8, 10=0-3-8 design. Max Horiz 2=-98 (LC 13) This truss has been designed for greater of min roof live 5) Max Grav 2=2002 (LC 3), 10=2002 (LC 3) load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on (Ib) - Maximum Compression/Maximum overhangs non-concurrent with other live loads. Tension 6) 200.0lb AC unit load placed on the bottom chord, 18-5-8 TOP CHORD 1-2=0/28, 2-4=-3642/0, 4-6=-3575/0 from left end, supported at two points, 5-0-0 apart. 6-8=-3575/0. 8-10=-3642/0. 10-11=0/28 All plates are 2x4 MT20 unless otherwise indicated. 7) BOT CHORD 2-23=0/3179, 20-23=0/3009, 18-20=0/2355 \* This truss has been designed for a live load of 20.0psf 8) 14-18=0/2355, 12-14=0/3009, 10-12=0/3180, on the bottom chord in all areas where a rectangle 19-21=-171/0, 17-19=-171/0, 16-17=-171/0, 3-06-00 tall by 2-00-00 wide will fit between the bottom 15-16=-171/0 chord and any other members, with BCDL = 10.0psf. 4-23=-273/134, 5-23=-182/210, 9)

All bearings are assumed to be SP 2400F 2.0E LOAD CASE(S) Standard

NOTES 1)

Unbalanced roof live loads have been considered for this design.

14-16=-135/0, 19-20=-135/0

5-22=-500/257, 21-22=0/1451, 6-21=0/1589,

6-15=0/1589, 13-15=0/1451, 7-13=-500/257, 7-12=-182/210, 8-12=-273/134, 17-18=-90/0, The Providence of the second s ORTH THURSDAY WANTED SEAL 2867 . GA mm

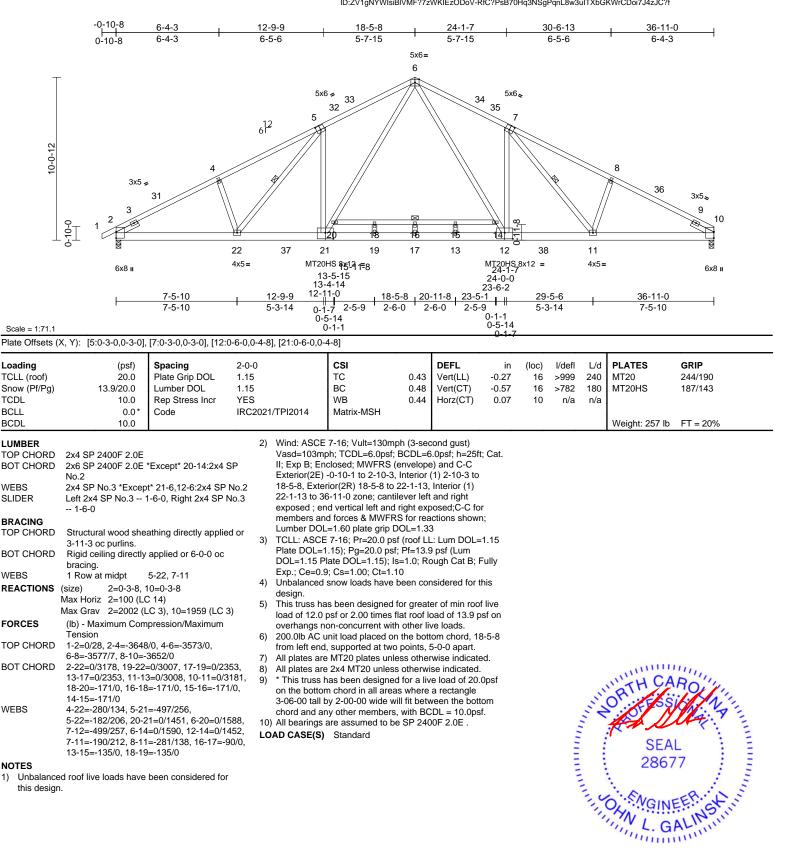
April 23,2025



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A2	Common	5	1	Job Reference (optional)	172941906

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:25 ID:ZV1gNYWIsiBIVMF?7zWKIEzODoV-RfC?PsB70Hg3NSgPgnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



April 23,2025



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A3	Roof Special	2	1	Job Reference (optional)	172941907

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:57 ID:cZs3pE5jKefsBqV50PturfzODp3-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

plional)	
es, Inc. Tue Apr 22 13:52:25	Page: 1
XbGKWrCDoi7J4zJC?f	

						ISBQVOUPt	unzodp3-Ric?	PSB70Hq3i	vəgeqnitaw	JUII ADGAV	VICD017J42JC?1	
	-0-10-8	3-2-4 7-	<u>-9-12</u>	<u>13-2-15</u> 5-5-3	13-5-12 18-5-8		24-1-7 5-7-15		<u>30-6-13</u> 6-5-6		<u>36-11-0</u> 6-4-3	37-9-8
T	0-10-8	J-Z- <del>1</del> -		6 <sup>12</sup>	0-2-13 4-11-12 5x6 = 35 36 65 36	5x6= 7	37 38	5x6 <b>≈</b> 3 ∕ 8	0-0-0		0-4-0	0-10-8
10-0-12	8×11 8×12	3	3x5 5			\ 		84		2x4 # 9	3x5 39 10	
	_⇔⇔E '29	<u></u> 286 ♀⊥	25		23 2 20	18				₩		
	0 8	4x5=	4x5=	I	5x10 = 21 5x6 = 4x5 =	19 2x4 <b>I</b>		14 · ·	40	13 4x5=		
		5x6 II			5x6 = 4x5 = 4x5 = 4x5 =	2x4 II 2x4 II		8x10=		47.3 =		5x10=
		2x4 <b>I</b>			15-1 <del>2×4</del> "			4-1-7 -0-0				
	. 2	-3-83-2-4 7.	-9-12	13-4-0	14-8-10	5-8,20-1		6-2	9-5-6		36-11-0	
			4-7-8	5-6-4	0-3-8 1-2-14 2-6			5	i-3-14		7-5-10	—
Scale = 1:77.9					1-1-2		0-5	5-14				
Plate Offsets (	(X, Y): [2:Edge,0-3-0],	[3:0-6-12,0-2-1	15], [6:0-3-0,0	-3-0], [8:0-3-0	0,0-3-0], [11:Edge,0-	2-9], [14:	0-5-0,0-4-8], [ T	24:0-5-0,0	)-2-12], [27	7:0-3-0,0-1	1-8]	
Loading TCLL (roof) Snow (Pf/Pg) TCDL	(psf) 20.0 13.9/20.0 10.0	Spacing Plate Grip DC Lumber DOL Rep Stress In	1.15 ncr YES		CSI TC BC WB	0.46 0.97 0.79	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.21 -0.43 0.22	24-25 >9 17-18 >9	defl L/d 999 240 999 180 n/a n/a	MT20	<b>GRIP</b> 244/190
BCLL BCDL	0.0* 10.0	Code	IRC20	021/TPI2014	Matrix-MSH						Weight: 260	lb FT = 20%
FORCES TOP CHORD BOT CHORD WEBS	2x4 SP No.2 *Excep 2.0E, 6-23:2x4 SP N 2400F 2.0E 2x4 SP No.3 *Excep Right 2x4 SP No.3 - Structural wood she 3-0-14 oc purlins, e Rigid ceiling directly bracing. (size) 11=0-3-8, Max Horiz 29=-111 ( Max Grav 11=1981 (lb) - Maximum Com Tension 1-2=0/33, 2-3=-492/ 4-5=-4489/0, 5-7=-3 9-11=-3617/0, 11-12 28-29=0/1697, 27-2i 26-27=0/5334, 25-2i 23-24=0/589, 6-24=- 19-21=0/2331, 15-13 11-13=0/3162, 22-2; 18-20=-162/0, 17-16 2-25=-1412/29, 5-25 8-14=-559/251, 8-13 9-13=-263/134, 7-16 7-24=0/1859, 4-26=1 3-29=-2104/0, 20-21 15-17=-110/0, 22-23 ed roof live loads have	lo.3, 23-14,14-1 tt* 14-7:2x4 SP 1- - 2-6-0 athing directly a xcept end vertic applied or 2-2-( 29=0-3-8 LC 13) (LC 3), 29=199( pression/Maxim 13, 3-4=-5938/( 645/0, 7-9=-355 2=0/28, 2-29=-67 6=0/5334, 24-25 -331/155, 21-23 9=0/2331, 13-15 2=-162/0, 16-17: 3=-169/299, 3=-169/299, 3=-169/299, 3=-169/299, 1-280/0, 18-19: 3=-1189/0, 21-22	11:2x6 SP No.2 applied or cals. 0 oc 0 (LC 3) num 0, 559/0, 37/62 =0/5114, 559/3967, 3=0/1071, 559/3967, 3=0/1071, 559/2909, 2=-162/0, 964/76, 3=0/1230, 153/0, =-100/0, 2=0/1542	<ul> <li>II; Exp B; Exterior(1 18-5-8, E</li> <li>22-1-13 t</li> <li>exposed members</li> <li>Lumber I</li> <li>TCLL: A3</li> <li>Plate DO DOL=1.1</li> <li>Exp.; Ce:</li> <li>Unbaland</li> <li>design.</li> <li>This trus:</li> <li>load of 12</li> <li>overhang</li> <li>200.0lb A</li> <li>from left</li> <li>All plates</li> <li>* This tru on the bo</li> <li>3-60.00 t</li> <li>chord an</li> <li>Bearings</li> <li>SP 24000</li> </ul>	3mph; TCDL=6.0psi Enclosed; MWFRS 2E) -0-10-1 to 2-10-3 ixterior(2R) 18-5-8 tt o 37-9-1 zone; canti ; end vertical left ans and forces & MWFI OCL=1.60 plate grip SCE 7-16; Pr=20.0 psi 5 Plate DOL=1.15); e0.9; CS=1.00; Ct=1 zed snow loads have s has been designed 2.0 psf or 2.00 times s non-concurrent with AC unit load placed of end, supported at tw are zx4 MT20 unlets ss has been designed thore in all are all by 2-00-00 wide to d any other member are assumed to be: F 2.0E . (S) Standard	(envelop 3, Interior 22-1-13 lever left d right ex, RS for ree DOL=1.3 sf (roof L 3; F)=13. ls=1.0; R 10 e been co d for great flat roof I th other li on the bot ro points, ss otherw ed for a line as where will fit betts s, with BC	e) and C-C (1) 2-10-3 to Interior (1) and right bossed;C-C for actions shown 3 L: Lum DOL= 9 psf (Lum ough Cat B; F nsidered for the er of min roof oad of 13.9 pr ve loads. tom chord, 18 5-0-0 apart. ise indicated. re load of 20.0 a rectangle ween the bott CDL = 10.0psl	r ;; fully his fon sf on s-5-8 Opsf om		and a second sec	ORTH C SE 280 OKNGI	AR SIDE AL 677 NEFERSEL MULTURE GALMSITURE Pril 23,2025
											A	pril 23,2025
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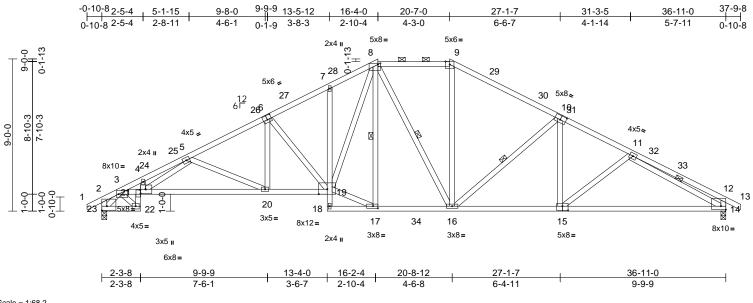


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof
25040187	A4	Нір	1	1	I72941908 Job Reference (optional)

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:25 ID:YyzqEw6zsGvZQ8eT7qvMw4zODp1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale =	- 1.68 2

	[2:Edge,0-3-0], [3:0-6-8,0-2-11], [6:0-3-0,0-3-0], [8:0-1-12,0-0-12], [10:0-4-0,0-3-0], [14:Edge,0-2-8], [15:0-4-0,0-3-0], [17:0-3-8,0-1-8], [21:0-3-8,0-3-0],
Plate Offsets (X, Y):	[21:0-0-8,0-3-12]

Plate Offsets (	X, Y): [21:0-0-8,0-3-1	2]											
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES IRC202 <sup>-</sup>	1/TPI2014	CSI TC BC WB Matrix-MSH	0.78 0.80 0.75	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.22 -0.40 0.22	(loc) 20-21 20-21 14	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 254 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD	SP No.3 2x4 SP No.3 Structural wood she	*Except* 22-21,7-18:2x athing directly applied of cept end verticals, and -10 max.): 8-9.	(4 2)	this design. Wind: ASCE Vasd=103mj II; Exp B; En Exterior(2E) 16-4-0, Exte 20-7-0 to 25- cantilever lef	roof live loads have 7-16; Vult=130mp ph; TCDL=6.0psf; E closed; MWFRS (e -0-10-1 to 2-10-3, 1 rior(2E) 16-4-0 to 2 9-10, Interior (1) 2 t and right exposed	h (3-seo 3CDL=6 nvelopo Interior 0-7-0, E 5-9-10 t d ; end v	cond gust) 6.0psf; h=25ft e) and C-C (1) 2-10-3 to Exterior(2R) o 37-9-1 zono vertical left an	; Cat. e; id					
	Rigid ceiling directly bracing. 1 Row at midpt (size) 14=0-3-8, Max Horiz 23=-103 (	applied or 10-0-0 oc 8-17, 8-16, 10-16, 11- 23=0-3-8	3)	for reactions DOL=1.33 TCLL: ASCE Plate DOL=1 DOL=1.15 P	d;C-C for members shown; Lumber D( 7-16; Pr=20.0 psf 1.15); Pg=20.0 psf; late DOL=1.15); Is:	OL=1.60 (roof Ll Pf=18.9 =1.0; Re	) plate grip .: Lum DOL= ) psf (Lum pugh Cat B; F	1.15					
FORCES	(lb) - Maximum Com Tension		,	Unbalanced	9; Cs=1.00; Ct=1.10 snow loads have b			his					
TOP CHORD	1-2=0/33, 2-3=-396/4 4-5=-5669/316, 5-7= 7-8=-2981/338, 8-9= 9-11=-3020/268, 11-	3668/286, 2084/278,	5) 6) 4 7)	load of 12.0 overhangs n Provide adeo	as been designed for psf or 2.00 times fla on-concurrent with quate drainage to p nas been designed	at roof l other li revent	oad of 13.9 p ve loads. water ponding	sf on g.				WH CA	ROUL
BOT CHORD	22-23=-107/1610, 2 3-21=-185/4593, 20-	1-22=-77/1447, ·21=-227/3989, 8-19=0/31, 7-19=-199/8	.,	on the bottor 3-06-00 tall t chord and ar All bearings	n chord in all areas by 2-00-00 wide wil by other members, are assumed to be urlin representation	where I fit betw with BC SP 240	a rectangle veen the botto DL = 10.0pst 00F 2.0E .	om f.			VIII	of the	N.S.
WEBS	6-20=0/637, 6-19=-9 8-19=-139/1692, 8-1 8-16=-161/178, 9-16 10-15=0/330, 11-15= 11-14=-2276/162, 4-	5=-5/678, 10-16=-800/9 =-78/90, 3-23=-1994/13	o, <sup>o</sup> , <sup>19,</sup> LC 39, LC		ation of the purlin a d.			DICE		11110	S. S	SEA 2867	EER ST.
NOTES												China L. G	in the second se

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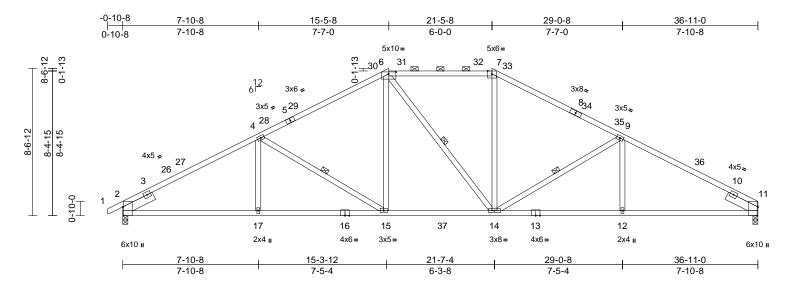
818 Soundside Road Edenton, NC 27932

April 23,2025

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A5	Нір	1	1	Job Reference (optional)	172941909

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:26 ID:NeOxdcb3RYxuDHi9TDdkYVzODoP-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:67

### Plate Offsets (X, Y): [2:0-6-1,Edge], [6:0-5-0,0-1-7], [11:0-6-1,Edge]

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-MSH	0.89 0.97 0.29	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.22 -0.39 0.14	(loc) 15-17 15-17 11	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 202 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING TOP CHORD WEBS REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD WEBS	No.2 2x4 SP 2400F 2.0E No.2 2x4 SP No.3 Left 2x6 SP 2400F 2 SP 2400F 2.0E 2- Structural wood she 3-2-2 oc purlins, exc 2-0-0 oc purlins (2-2 Rigid ceiling directly bracing. 1 Row at midpt (size) 2=0-3-8, ' Max Horiz 2=84 (LC Max Grav 2=1740 (I (lb) - Maximum Com Tension 1-2=0/28, 2-4=-3186 6-7=-2168/282, 7-9 9-11=-3180/253 2-17=-153/2746, 15 14-15=-52/2094, 12: 11-12=-164/2740 4-17=0/254, 4-15=-7	athing directly applied cept -0 max.): 6-7. applied or 2-2-0 oc 4-15, 6-14, 9-14 11=0-3-8 12) _C 50), 11=1701 (LC pression/Maximum 6/252, 4-6=-2537/269, -2528/270, -17=-153/2746,	e6 d or 3) 4) 50) 50) 6) 7) , 8) 9)	Vasd=103mj II; Exp B; En Exterior(2E) 15-5-8, Exte to 21-5-8, Ex 26-8-2 to 36- exposed ; er members an Lumber DOL TCLL: ASCE Plate DOL=1 DOL=1.15 P Exp.; Ce=0.3 Unbalanced design. This truss ha load of 12.0 overhangs n Provide adee * This truss h on the bottor 3-06-00 tall t chord and ar All bearings Graphical pu		BCDL=6 envelope Interior 20-8-2, I or 26-8-2, Ver left a right exp S for rea OL=1.3 f (roof LL Pf=18.5 =1.0; Rc 0, Lu=50 been cor for great at roof k o ther lip prevent v t for a liv s where II fit betw with BC o SP 24C	:.0psf; h=25ft a) and C-C (1) 2-10-3 to (1) 2-10-3 to (1) 2-10-3 to (1) 2-10-3 to (1) 2-10-3 to (1) 2-0 and right sosed; C-C for ctions shown 3 :: Lum DOL= 9 psf (Lum bugh Cat B; F 0-0-0 isidered for t er of min roop a do f 13.9 p ve loads. water pondin e load of 20. a rectangle veen the bott :DL = 10.0ps bt depict the	o-8-2 r r; f1.15 Fully his f live sof on g. 0psf com f.				ORTH CA	
NOTES												2867	1 : E

1) Unbalanced roof live loads have been considered for this design.

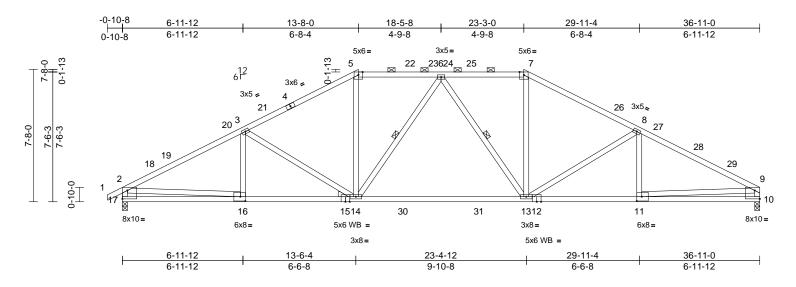


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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A6	Нір	1	1	Job Reference (optional)	172941910

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:26 ID:ColDufgq1Oi2xC9JqUk8nmzODoJ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:66.7

Plate Offsets (2	X, Y): [10:Edge,0-5-1	3], [11:0-3-8,0-3-0], [1	6:0-3-8,	0-3-0], [17:Edg	e,0-5-13]								
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	1 1	2-0-0 1.15 1.15 YES IRC202	21/TPI2014	CSI TC BC WB Matrix-MSH	0.62 0.52 0.80	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.30 -0.52 0.07	(loc) 13-14 13-14 13	l/defl >999 >840 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 217 lb	<b>GRIP</b> 244/190 FT = 20%
	2x4 SP No.3 Structural wood shee 4-6-9 oc purlins, ex 2-0-0 oc purlins (5-6 Rigid ceiling directly bracing.	applied or 10-0-0 oc 6-14, 6-13 17=0-3-8 C 14)	or 1 3; 4	Vasd=103m II; Exp B; En Exterior(2E) 13-8-0, Exte 18-10-10 to Interior (1) 2 right expose for members Lumber DOL 01 CLL: ASCE Plate DOL=1 DOL=1.15 P Exp.; Ce=0.9	7-16; Vult=130mp bh; TCDL=6.0psf; I closed; MWFRS (e -0-10-1 to 2-10-3, rior(2R) 13-8-0 to 1 23-3-0, Exterior(2R) 8-5-10 to 36-9-4 zc d; end vertical left and forces & MW =1.60 plate grip D ; 7-16; Pr=20.0 psf; late DOL=1.15); Is b; Cs=1.00; Ct=1.1 snow loads have t	BCDL=6 envelope Interior 18-10-10 23-3-( one; can and righ FRS for OL=1.33 ( (roof LI Pf=18.9 =1.0; Ro 0, Lu=50	.0psf; h=25ft and C-C (1) 2-10-3 to b, Interior (1) to 28-5-10, tillever left an t exposed;C reactions sho :: Lum DOL= p psf (Lum pugh Cat B; F D-0-0	d -C own; 1.15 Fully					
FORCES	(lb) - Maximum Com Tension		5)	) This truss ha	is been designed f psf or 2.00 times fl								
TOP CHORD	1-2=0/33, 2-3=-3080 5-6=-2210/265, 6-7= 7-8=-2574/258, 8-9= 2-17=-1784/216, 9-1	-3086/244,	6) 7)	overhangs n Provide adeo * This truss l	on-concurrent with quate drainage to p nas been designed n chord in all areas	other li prevent for a liv	ve loads. water ponding e load of 20.0	g.					
BOT CHORD	16-17=-102/747, 14- 13-14=-108/2254, 11	-16=-176/2672,		3-06-00 tall I chord and ar	by 2-00-00 wide wi	ll fit betv with BC	veen the bott DL = 10.0ps					"ATH CA	ROL
WEBS NOTES	6-14=-336/86, 6-13=	593/115, 5-14=-5/76 333/86, 7-13=-9/776 =-52/109, 2-16=-74/1	, 943,	Graphical pu		does no	ot depict the	size			N. N.	SEA	L

1) Unbalanced roof live loads have been considered for this design.

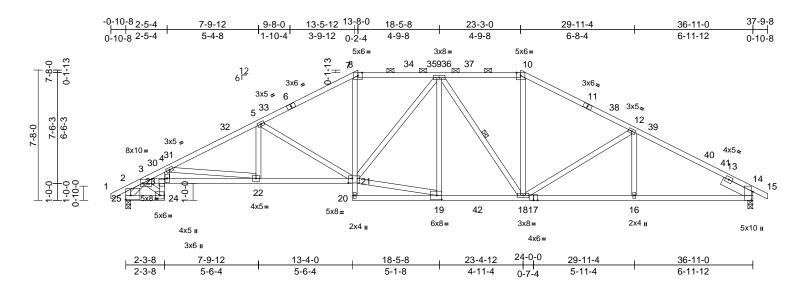


April 23,2025

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A7	Нір	1	1	Job Reference (optional)	

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:26 ID:zXfysy8s9BH8HbN2oyS3YizODp\_-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale =	1:67.8
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Plate Offsets (2	X, Y): [2:Edge,0-3-0],	[3:0-6-8,0-2-15], [14	:0-6-1,Edg	ge], [19:0-3-8,0	-3-0], [21:0-2-12,0-	·3-4], [2	3:0-0-8,0-1-1	2]					
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-MSH	0.63 0.98 0.93	DEFL Vert(LL) Vert(CT) Horz(CT)		(loc) 22-23 21-22 14	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 230 lb	<b>GRIP</b> 244/190 FT = 20%
	2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E * SP No.3 2x4 SP No.3 Right 2x6 SP 2400F Structural wood sheat 3-3-15 oc purlins, ep 2-0-0 oc purlins (5-3 Rigid ceiling directly bracing. 1 Row at midpt (size) 14=0-3-8, Max Horiz 25=-86 (L0 Max Grav 14=1699 ( (lb) - Maximum Com Tension 1-2=0/33, 2-3=-420/5 4-5=-3847/289, 5-7= 7-8=-2441/283, 8-9= 9-10=-2189/272, 10- 12-14=-3075/246, 14 2-25=-513/103 24-25=-55/4823, 22= 21-22=-159/3380, 20	2.0E 2-0-0 athing directly applie xcept end verticals, a -2 max.): 8-10. applied or 10-0-0 oc 9-18 25=0-3-8 C 13) (LC 50), 25=1715 (LC pression/Maximum 51, 3-4=-5795/344, -2859/279, -2472/280, 12=-2537/264, 4-15=0/28, 24=-70/1455, 23=-280/5006,	d or and 3) C 50) 4) 5) 6) 7) 8)	this design. Wind: ASCE Vasd=103m II; Exp B; En Exterior(2E) 13-8-0, Exte 18-10-10 to : Interior (1) 2 right expose for members Lumber DOL TCLL: ASCE Plate DOL=1.15 P Exp.; Ce=0.5 Unbalanced design. This truss ha load of 12.0 overhangs n Provide adea * This truss h on the bottor 3-06-00 tall t chord and ar All bearings	roof live loads have 7-16; Vult=130mpl bh; TCDL=6.0psf; E closed; MWFRS (e -0-10-1 to 2-10-3, 1 rior(2R) 13-8-0 to 1 23-3-0, Exterior(2R 8-5-10 to 37-9-1 zo d; end vertical left. and forces & MWF =1.60 plate grip DC 7-16; Pr=20.0 psf; late DOL=1.15); Is- b; Cs=1.00; Ct=1.10; snow loads have b Is been designed for psf or 2.00 times fit fon-concurrent with quate drainage to p as been designed n chord in all areas by 2-00-00 wide will y other members, are assumed to be rin representation	h (3-sec 3CDL=6 nvelope neterior 8-10-10 ) 23-3-0 ne; can and righ FRS for DL=1.33 (roof LL Pf=18.5 =1.0; Rc 0, Lu=50 keen cor or great at roof k other line for a live with BC SP 240	cond gust) .0psf; h=25ft .0psf; h=25ft .0psf; h=25ft .0psf; h=25ft .1p2-10-3 to .1p2-10-3 to .1p2-10-3 to .1p3 (1)2-10-3 to .1p3 (Lum .1p3 (Lum DUgh Cat B; F) .0-0 .1p3 (Lum DUgh Cat B; F) .0-0 .1p3 (Lum Dugh Cat B; F) .0-0 .1p3 (Lum .1p3 (Lum DUgh Cat B; F) .0-0 .1p3 (Lum .0p3 (2)2 .1p3 (Lum .0p3 (2)2 .1p3 (2)2	; Cat. d -C own; 1.15 Fully his f live sf on g. 0psf om f.			Auto	SEA 2867	ROMAR
WEBS	19-20=-12/143, 18-1 16-18=-141/2641, 14 4-22=-1647/134, 5-2 5-21=-1131/119, 9-1 3-24=-1781/100, 10- 9-18=-369/55, 12-18 19-21=-55/2170, 9-2 3-25=-1887/123	4-16=-141/2641 2=0/521, 9=-275/72, -18=-7/727, 5=-594/104, 12-16=0/	206,	or the orienta bottom chord DAD CASE(S)		long the	top and/or				annun a	SEA 2867	T ER P
NOTES												N L.G	111111

April 23,2025

Page: 1

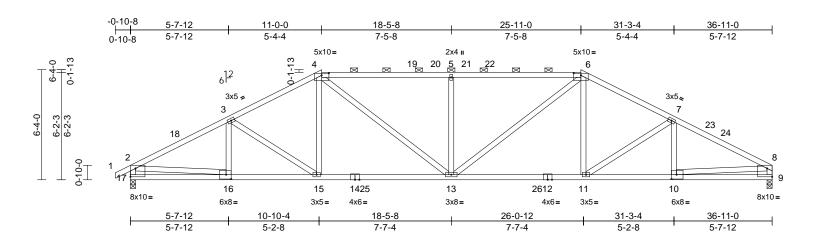


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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A8	Нір	1	1	Job Reference (optional)	172941912

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:27 ID:9BtzJLh4Z?ymBWJixvmctBzODoH-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



#### Scale = 1:66.3

Plate Offsets (2	X, Y): [4:0-5-0,0-1-7],	[6:0-5-0,0-1-7], [9:Ed	ge,0-5-13	8], [10:0-3-8,0-	3-0], [16:0-3-8,0-3-0	0], [17:8	Edge,0-5-13]						
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL	(psf) 20.0 18.9/20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-MSH	0.77 0.36 0.80	DEFL Vert(LL) Vert(CT) Horz(CT)		(loc) 11-13 11-13 9	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0											Weight: 211 lb	FT = 20%
LUMBER       2x4 SP ≥400F 2.0E       2x4 SP ≥400F 2.0E       Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.         BOT CHORD       2x4 SP ≥400F 2.0E       II; Exp B; Enclosed; MWFRS (envelope) and C-C         WEBS       2x4 SP ≥400F 2.0E       II; Exp B; Enclosed; MWFRS (envelope) and C-C         BRACING       TOP CHORD       Structuration of the structuratio of the structurat													
FORCES	( )	pression/Maximum	4)	Unbalanced				his					
TOP CHORD	4-5=-3044/295, 5-6= 6-7=-2714/259, 7-8=	-2925/235,	5)	This truss ha load of 12.0 overhangs n	as been designed fo psf or 2.00 times fla on-concurrent with puate drainage to p	at roof le other li	oad of 13.9 p ve loads.	sf on					
2-17=-1774/206, 8-9=-1698/165       6)       Provide adequate drainage to prevent water ponding.         BOT CHORD       16-17=-82/561, 15-16=-181/2548, 13-15=-109/2362, 11-13=-106/2362, 10-11=-166/2557, 9-10=-55/444       7)       * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.         WEBS       3.16=-101/77, 3-15=-373/86, 4-15=0/473, 4-13=-53/813, 5-13=-769/159, 6-13=-51/813, 6-11=0/481, 7-11=-388/88, 7.10=-112/80, 2-16=-100/2039, 8-10=-123/2150       All bearings are assumed to be SP 2400F 2.0E.       Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or													
WEBS NOTES	4-13=-53/813, 5-13=	-373/86, 4-15=0/473, -769/159, 6-13=-51/8 88/88, 7-10=-112/80, 0=-123/2150	13, 8)	All bearings Graphical pu	ny other members, nare assumed to be irlin representation ation of the purlin al	SP 240 does no	OF 2.0E . ot depict the s				VILL	A M	TIN ST
1) Unbalance	ed roof live loads have	been considered for	LC	DAD CASE(S)	Standard					=		SEA	L 🕴 E 🗌

1) Unbalanced roof live loads have been considered for this design.



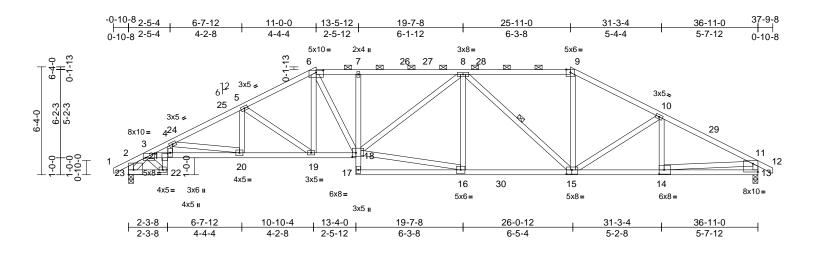
April 23,2025

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A9	Нір	1	1	Job Reference (optional)	

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:27 ID:vwnjHeA6hoXsXvXRwNVXd7zODoy-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

2 13:52:27 Page: 1



Scale = 1:67.6

Plate Offsets (	(X, Y): [2:Edge,0-3-4],	[3:0-6-8,0-3-3], [6:0-	5-0,0-1-7]	, [13:Edge,0-5	-13], [14:0-3-8,0-3	-0], [15:0	-3-12,0-3-0]	, [18:0-2-	8,0-2-12	2], [21:0-	-0-8,0-1	1-12]	
<b>Loading</b> TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-MSH	0.80 1.00 0.77	DEFL Vert(LL) Vert(CT) Horz(CT)		(loc) 16-17 16-17 13	l/defl >999 >970 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 235 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER FOP CHORD SOT CHORD WEBS BRACING FOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.2 2x4 SP No.2 *Excep No.3, 3-18:2x4 SP N 2x4 SP No.3 *Excep Structural wood shea 2-3-6 oc purlins, ext 2-0-0 oc purlins (2-1 Rigid ceiling directly bracing, Except: 1-4-12 oc bracing: 24	o.1 t* 16-18:2x4 SP No.2 athing directly applie cept end verticals, ar 1-5 max.): 6-9. applied or 10-0-0 oc 0-21. 8-15 23=0-3-8 C 13) [LC 50), 23=1670 (LC pression/Maximum 46, 3-4=-5437/365, -3140/280,	2 d or nd 3) C <sup>50)</sup> 4) 5)	this design. Wind: ASCE Vasd=103mg II; Exp B; En Exterior(2E) 11-0-0, Exter 16-2-10 to 22 Interior (1) 3 exposed ; en members an Lumber DOL TCLL: ASCE Plate DOL=1 DOL=1.15 P Exp.; Ce=0.5 Unbalanced design. This truss ha load of 12.0 overhangs n	roof live loads hav 7-16; Vult=130mp bh; TCDL=6.0psf; closed; MWFRS (i -0-10-1 to 2-10-3, ior(2R) 11-0-0 to 5-11-0, Exterior(2F 1-3-4 to 37-9-1 zon d vertical left and d forces & MWFR: =1.60 plate grip D 7-16; Pr=20.0 psf; late DOL=1.15); Is b; Cs=1.00; Ct=1.1 snow loads have t s been designed f por or 2.00 times fil on-concurrent with wate drainage to to	ch (3-sec BCDL=6 envelope Interior 1 16-2-10, R) 25-11. he; canti right exp S for rea OL=1.33 f (roof LL Pf=18.9 = 1.0; RG D, Lu=50 peen cor for great at roof k	cond gust) .0psf; h=25ff and C-C 1) 2-10-3 to Interior (1) 0 to 31-3-4, lever left and osced; C-C fo ctions showr b :: Lum DOL= 0 psf (Lum Dugh Cat B; F )-0-0 isidered for t er of min roo' ad of 13.9 p ve loads.	t; Cat. r r ; :1.15 Fully his f live ssf on					F I = 20%
BOT CHORD WEBS	6-7=-3188/296, 7-8= 8-9=-2352/257, 9-10 10-11=-2891/235, 11 11-13=-1760/206 22-23=-103/1484, 21 3-21=-276/4495, 20- 19-20=-191/3389, 18 17-18=0/109, 7-18=- 14-16=-127/2911, 13 5-19=-896/111, 6-19 16-18=-124/2719, 8- 16=-246/103, 8-15 10-15=-395/84, 10-1 3-23=-1886/134, 11- 3-22=-1718/118, 4-2 4-20=-1396/110	=-2656/254, I-12=0/37, 2-23=-47( I-22=-84/1409, 21=-300/4687, 3-19=-98/2739, 452/105, 16-17=0/22 3-14=-47/485 =-20/649, 6-18=-76/ 18=-13/334, =-829/54, 9-15=-6/8- 4=-95/79, 14=-95/2079,	8) 9) 27, <sup>889,</sup> LC 41,	* This truss h on the bottor 3-06-00 tall h chord and ar All bearings Graphical pu		I for a liv s where II fit betv with BC s SP No. does no	e load of 20. a rectangle veen the bott DL = 10.0ps 2. ot depict the s	Opsf tom .f.			State State	SEA 2867	ER. Kunn

April 23,2025

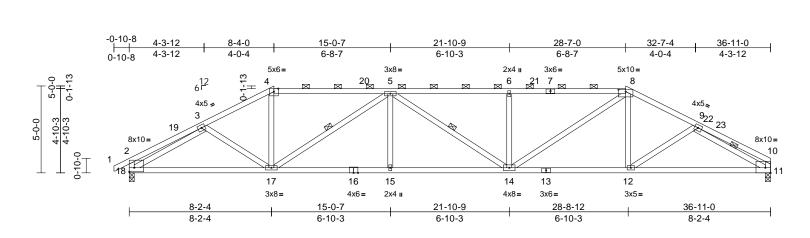


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Job		Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
2504018	87	A10	Нір	1	1	Job Reference (optional)	172941914

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:27 ID:1Q3d04KGdnA0av0xBcEaftzODol-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



#### Scale = 1:66.3

	(, 1). [2.2090,0 2 12	], [8:0-5-0,0-1-7], [10 I	0.Luge,0-0	0]	1								
bading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.58	Vert(LL)		14-15	>999	240	MT20	244/190
now (Pf/Pg)	18.9/20.0	Lumber DOL	1.15		BC	0.96	Vert(CT)	-0.45	14-15	>974	180		
DL	10.0	Rep Stress Incr	YES		WB	0.98	Horz(CT)	0.15	11	n/a	n/a		
CLL	0.0*	Code	IRC202	1/TPI2014	Matrix-MSH								
CDL	10.0											Weight: 203 lb	FT = 20%
JMBER			2)	Wind: ASCE	7-16; Vult=130mp	h (3-seo	cond gust)						
OP CHORD	2x4 SP No.2 *Excep	ot* 4-7,7-8:2x4 SP 24	400F		ph; TCDL=6.0psf;			; Cat.					
	2.0E				closed; MWFRS (								
OT CHORD	2x4 SP No.2				-0-10-1 to 2-10-3,								
EBS	2x4 SP No.3				or(2R) 8-4-0 to 13								
RACING					xterior(2R) 28-7-0			)					
OP CHORD	Structural wood she	athing directly applie	ed or		6-9-4 zone; cantile								
	3-5-7 oc purlins, ex	cept end verticals, a	nd		nd vertical left and								
	2-0-0 oc purlins (3-9	-0 max.): 4-8.			d forces & MWFR			1;					
OT CHORD	Rigid ceiling directly	applied or 2-2-0 oc	0		_=1.60 plate grip D								
	bracing.		3)		7-16; Pr=20.0 ps			1.15					
EBS	1 Row at midpt	5-17, 5-14, 9-11			1.15); Pg=20.0 psf								
EACTIONS	(size) 11=0-3-8,	18=0-3-8			Plate DOL=1.15); Is			ully					
	Max Horiz 18=64 (LC	C 14)	4		9; Cs=1.00; Ct=1.1								
	Max Grav 11=1464	,	C 2) <sup>4)</sup>		snow loads have I	been cor	isidered for t	nis					
ORCES	(lb) - Maximum Com		· ,	design.	a haan daalamadd			live					
NOLO	Tension	pression/maximum	5)		as been designed f psf or 2.00 times f								
OP CHORD	1-2=0/37, 2-3=-483/	55 3-42520/234			ion-concurrent with			51 011					
	4-5=-2244/234, 5-6=	, ,	6)		quate drainage to			~					
	6-8=-3389/314, 8-9=		7)		has been designed								
	9-10=-413/33, 2-18=				m chord in all area			opsi					1.
OT CHORD	17-18=-206/2057, 15	,	11/00					~ m					in the
	14-15=-193/3377, 12				by 2-00-00 wide winde winde winder winder winder begin winder w		veen the both	om				N'TH UA	Roile
	11-12=-195/2063	L 11- 120/2201,	0)		are assumed to be		2				1	A	14/11
EBS	3-17=-104/260, 4-17	/=-4/769. 5-17=-136	4/96, <u>8)</u>		urlin representation						20	- FOS	DY.V.
	5-15=0/132, 5-14=-8	,	, ,		ation of the purlin a			5128			55	IN IL	13:1
	8-14=-93/1362, 8-12			bottom chor		aony the				1		p	< · ·
	3-18=-2044/209, 9-1	,	,									OR HES	r 1
DTES			L	DAD CASE(S)	Sianuaru					=		O L/ (	- :
	d as a filling to a dar to aver	been considered fo	-									2867	7 :

1) Unbalanced roof live loads have been considered for this design.



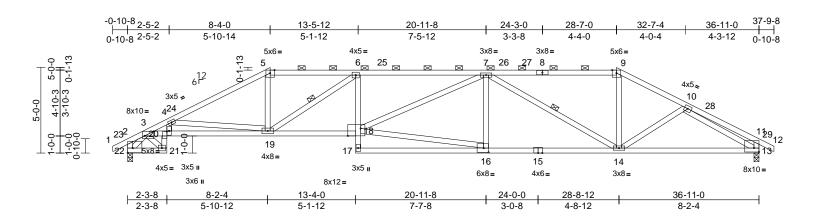
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	Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
1	25040187	A11	Нір	1	1	I7 Job Reference (optional)	2941915

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:27 ID:8NIhcv45ZLX?ZgwuShLfIRzODp4-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



#### Scale = 1:67.4

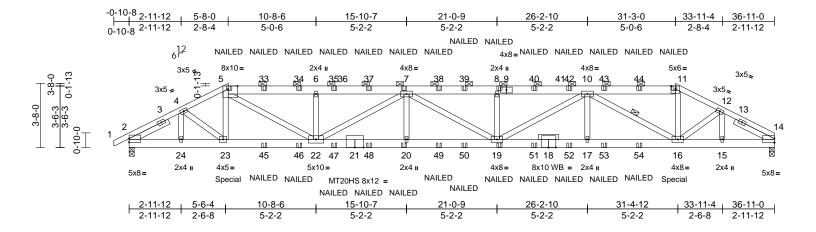
Plate Offsets (X, Y): [2:Edge,0-3-4], [3:0-6-8,0-3-3], [13:Edge,0-2-12], [16:0-3-8,0-3-0], [18:0-9-0,Edge], [20:0-0-8,0-1-10]													
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2027	1/TPI2014	CSI TC BC WB Matrix-MSH	0.60 0.87 0.98	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.28 -0.53 0.26	(loc) 17 16-17 13	l/defl >999 >836 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 214 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD WEBS REACTIONS FORCES TOP CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Unbalance	2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E SP No.3 2x4 SP No.3 *Excep Structural wood shea 3-8-6 oc purlins, exi 2-0-0 oc purlins (3-5 Rigid ceiling directly bracing, 1 Row at midpt (size) 13=0-3-8, Max Horiz 22=-62 (L Max Grav 13=1524 ( (lb) - Maximum Com Tension 1-2=0/37, 2-3=-335// 4-5=-3246/267, 5-6= 6-7=-4229/351, 7-9= 9-10=-2528/235, 10- 2-22=-463/105, 11-1 21-22=-98/1215, 20- 3-20=-332/3859, 19- 18-19=-211/4261, 17 16-17=0/264, 14-16= 13-14=-153/2043 5-19=-10/1146, 6-19 16-18=-181/3251, 7- 7-16=-354/124, 7-14 10-14=-99/283, 10-1 4-20=0/678, 4-19=-1 3-22=-1489/118, 3-2	applied or 10-0-0 oc 6-19, 7-14 22=0-3-8 C 13) (LC 2), 22=1522 (LC 2 pression/Maximum 55, 3-4=-4596/389, -2901/273, -2255/232, 11=-539/62, 11-12=0, 3=-467/117, -21=-75/1155, -20=-366/4015, 7-18=0/78, 6-18=0/37, =-1654/114, -18=-51/826, =-1654/114, -18=-51/826, =-1654/114, -18=-51/826, =-1417/95, 9-14=0/72, 3=-1969/197, 504/233, 1=-1424/105	or 3) 2) 4) 5) /37, <sup>6)</sup> 5, 9) 5, 9) LC	Vasd=103mj II; Exp B; En Exterior(2E) 8-4-0, Exteri to 28-7-0, Ex 33-9-10 to 3 exposed ; er members an Lumber DOL TCLL: ASCE Plate DOL=1 DOL=1.15 P Exp.; Ce=0.3 Unbalanced design. This truss ha load of 12.0 overhangs n Provide adee * This truss f on the bottor 3-06-00 tall t chord and ar All bearings Graphical pu		SCDL=6 invelope interior 5-12, In 0 33-9- <sup>2</sup> ver left a ight exp S for rea OL=1.3: (Iroof LL Pf=18.5 =1.0; Re 0, Lu=50 been cor or great at roof li other li orevent v for a liv s where I fit betw SP 240 does no	.0psf; h=25ft and C-C (1) 2-10-3 to terior (1) 13-5 (0) Interior (1) and right oosed; C-C for ctions showr 3 : Lum DOL= b psf (Lum ough Cat B; F 0-0-0 isidered for the er of min roof bad of 13.9 p ve loads. water ponding e load of 20.1 a rectangle veen the botth 0F 2.0E. bt depict the s	5-12 r r; 1.15 fully his five sf on g. Dpsf om				ORTH CA ORTH CA SEA 2867	ROLL REPORT
		been considered for									11	NN L. G	ALINSTITU

April 23,2025

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A12	Hip Girder	1	1	Job Reference (optional)	172941916

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:29 ID:Km\_HUTQfzx20w\_3H5asDRLzODoe-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:65.9

Plate Offsets (X, Y): [2:Edge.0-2-9], [5:0-6-6.Edge], [14:Edge.0-2-13]

Loading         (psf)         Spacing         2-0-0         CSI         Image: CSI and the construction of the construction device (s) is the responsibility of the construction of the construction device (s) is the responsibility	Plate Offsets (	X, Y): [2:Edge,0-2-9]	, [5:0-6-6,Edge], [14:	Edge,0-2	-13]								-	
TOP CHORD         2x4 SP 2400F 2.0E *Except* 5-9,9-11:2x6 SP 2400F 2.0E         5-22=-269/2546, 6-22=-634/172, 7-29=-34/33, 8-19=-451/153, 10-19=-32/321, 8         (0.148*x3.25*) toe-nails per NDS guidlines.           BOT CHORD         2x6 SP 2400F 2.0E         7-22=-1220/140, 7-20=0/177, 7-19=-34/33, 8-19=-451/153, 10-19=-132/1218, 10-17=-0/150, 10-16=-2560/276, 10-17=0/150, 10-16=-2560/276, 12-5, 22-5, 22-7, 19-7, 19-10, 16-10:2x4 SP No.2         (0.148*x3.25*) toe-nails per NDS guidlines.           WEBS         2x4 SP No.3 *Except*         10-17=0/150, 10-16=-2560/276, 11-16=-88/1381, 12-16=-110/795, 10-19=-34/33,	TCLL (roof) Snow (Pf/Pg) TCDL BCLL	20.0 18.9/20.0 10.0 0.0*	Plate Grip DOL Lumber DOL Rep Stress Incr	1.15 1.15 NO	21/TPI2014	TC BC WB	0.44	Vert(LL) Vert(CT)	-0.30 -0.59	19-20 19-20	>999 >746	240 180	MT20 MT20HS	244/190 187/143
<ul> <li>DTHERS 24 9F No.3 = 2-6-0, Right 2x4 SP No.3 = -2-6-0, Right 2x4 SP No.3 = -2-6-0</li> <li>NOTES</li> <li>NOTES</li></ul>	TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING TOP CHORD WEBS REACTIONS FORCES TOP CHORD	2400F 2.0E 2x6 SP 2400F 2.0E 2x4 SP No.3 *Excep 22-5,22-7,19-7,19-1 2x4 SP No.3 Left 2x4 SP No.3 Left 2x4 SP No.3 2-6-0 Structural wood she 4-0-1 oc purlins, ex 2-0-0 oc purlins (3- Rigid ceiling directly bracing. 1 Row at midpt (size) 2=0-3-8, Max Horiz 2=34 (LC Max Uplift 2=-192 (I Max Grav 2=2184 ( (lb) - Maximum Con Tension 1-2=0/32, 2-4=-334 5-6=-5649/604, 6-7: 7-8=-6684/700, 8-11 10-11=-3444/385, 1 12-14=-3356/347 2-24=-305/2896, 23 22-23=-371/3492, 2 19-20=-685/6687, 1 16-17=-559/5641, 1	pt* 0,16-10:2x4 SP No.2 2-6-0, Right 2x4 SP I eathing directly applie cept 10-3 max.): 5-11. y applied or 10-0-0 oc 10-16 14=0-3-8 2 (5) 10-2 (5), 14=-191 (LC 7 LC 2), 14=2133 (LC 2) npression/Maximum 6/347, 4-5=-3849/422 =-5640/601, 0=-6684/700, 1-12=-3849/421, 1-24=-305/2896, 10-22=-685/6687, 7-19=-559/5641,	x6 SP 2 No.3 1 1 ed or 2 c 3 () 2) 4 5 3, 6 7 8 7 8 9 1	<ul> <li>IOTES</li> <li>) Unbalanced this design.</li> <li>) Wind: ASCE Vasd=103mg II; Exp B; En and right exp Lumber DOL=1</li> <li>) TCLL: ASCE Plate DOL=1.15 P Exp.; Ce=0.5</li> <li>) Unbalanced design.</li> <li>i) This truss ha load of 12.0 overhangs n</li> <li>i) Provide adea</li> <li>) All plates are so the bottom 3-06-00 tall te chord and are so the bottom 3-06-00 tall te chord and are so and the bottom 3-06-00 tall to chord and are so and the arings and the analysis of the area and the analysis of the advector t</li></ul>	5-22=-269/2546, 6- 7-22=-1220/140, 7- 8-19=-451/153, 10- 10-17=0/150, 10-16 11-16=-88/1381, 12 12-15=-388/58 roof live loads have 7-16; Vult=130mp bh; TCDL=6.0psf; E closed; MWFRS (e posed; end vertical =1.60 plate grip De ; 7-16; Pr=20.0 psf; late DOL=1.15); Is- b; Cs=1.00; Ct=1.10; snow loads have b as been designed for psf or 2.00 times fla on-concurrent with quate drainage to pa b MT20 plates unlee has been designed n chord in all areas by 2-00-00 wide will by other members. are assumed to be hanical connection a capable of withstat b uplift at joint 2.	22=-63 20=0/1 19=-13 6=-2560 2-16=-1 e been of h (3-sec 3CDL=6 anvelope left and OL=1.3; (roof LL Pf=18.5 =1.0; Rc O, Lu=50 been cor or great at roof la other librorevent ' so other librorevent ' so other librorevent ' SP 240 (by oth anding 1 does no	4/172, 77, 7-19=-34/33 2/1218, /276, 10/795, considered for cond gust) 3.0psf; h=25ft; C a); cantilever lef d right exposed; 3 :: Lum DOL=1. <sup>-</sup> Dysf (Lum Dugh Cat B; Ful D-0-0 nsidered for this er of min roof lix D-0-0 nsidered for this er of min roof lix D-0-0 sei loads. water ponding. wise indicated. e load of 13.9 psf ve loads. water ponding. wise indicated. e load of 20.0p: a rectangle veen the bottom 10F 2.0E . ers) of truss to 91 lb uplift at jc bt depict the siz	Cat. ft ; 15 lly s ve on sf n	(0.1 13) Har pro lb d lb u of s othe 14) In ti of ti LOAD ( 1) De Inc Ur	148"x3.2 nger(s) c vided su lown and up at 31- such con ers. he LOAI he truss <b>CASE(S</b> ead + Sr crease= niform Li Vert: 1- oncentra	5") toe or other fficient d 51 lb -2-4 on nectior D CASI are no D Star how (ba 1.15 bads (II 5=-48, ited Los	s 3-10d (0.148" -nails per NDS r connection de' t to support con- up at 5-8-0, an bottom chord. n device(s) is th E(S) section, loc ted as front (F) ndard alanced): Lumbo b/ft) 5-11=-58, 11-1: ads (lb)	x3") or 3-12d guidlines. vice(s) shall be centrated load(s) 269 d 269 lb down and 51 The design/selection e responsibility of ads applied to the face or back (B). er Increase=1.15, Plat 4=-48, 25-29=-20

April 23,2025

Page: 1



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Continued on page 2

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A12	Hip Girder	1	1	Job Reference (optional)	172941916

Vert: 5=-31 (B), 23=-264 (B), 7=-27 (B), 20=-19 (B),
8=-27 (B), 19=-19 (B), 16=-264 (B), 11=-31 (B),
33=-27 (B), 34=-27 (B), 35=-27 (B), 37=-27 (B),
38=-27 (B), 39=-27 (B), 40=-27 (B), 42=-27 (B),
43=-27 (B), 44=-27 (B), 45=-19 (B), 46=-19 (B),
47=-19 (B), 48=-19 (B), 49=-19 (B), 50=-19 (B),
51=-19 (B), 52=-19 (B), 53=-19 (B), 54=-19 (B)

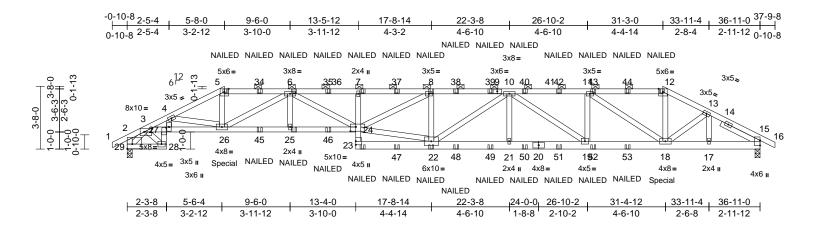
Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:29 ID:Km\_HUTQfzx20w\_3H5asDRLzODoe-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	A13	Hip Girder	1	2	Job Reference (optional)	172941917

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:30 ID:8wLYIXUQZnpAevWRRqzdhczODoY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:67.2

Y): [2:Edge 0-3-4]	[3:0-5-4 0-2-7] [24:	0-3-12 0-3	-0] [27·0-0-8	0-2-01									
(psf) 20.0 18.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO		CSI TC BC WB Matrix-MSH	0.26 0.73 0.71	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.29 -0.55 0.18	(loc) 7 7 15	l/defl >999 >799 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 463 I	<b>GRIP</b> 244/190	
No.3, 23-20,20-15:2: 2x4 SP No.3 *Excep No.2 Right 2x4 SP No.2 Structural wood shee 6-0-0 oc purlins, ext 2-0-0 oc purlins (5-1 Rigid ceiling directly bracing. size) 15=0-3-8, 4x Horiz 29=-45 (L 4x Uplift 15=-209 ( 4x Grav 15=2174 ( (lb) - Maximum Com Tension 1-2=0/37, 2-3=-466/ 4-5=-5425/747, 5-6= 6-7=-8747/1100, 7-8 8-10=-6657/806, 10- 11-12=-3464/419, 12 3-27=-704/5456, 26- 25-26=-935/7303, 24 23-24=0/172, 7-24=- 22-23=-124/1071, 2' 19-21=-727/6445, 18	x6 SP 2400F 2.0E t* 22-24,29-2:2x4 SI • 2-6-0 athing directly applie cept end verticals, an 0-1 max.): 5-12. applied or 10-0-0 oc 29=0-3-8 C 9) LC 7), 29=-255 (LC (LC 2), 29=2199 (LC pression/Maximum 71, 3-4=-6620/851, -4928/686, B=-8494/1070, 11=-5351/635, 2-13=-3879/455, 5-16=0/32, 2-29=-58 7-28=-218/1686, :27=-744/5708, 4-25=-935/7303, :350/106, 1-22=-727/6445, B-19=-580/5351,	SP P ed or nd N c 1) 8) ; 2) 2) 11/75 4) 5)	OTES 2-ply truss t (0.131"x3") Top chords oc. Bottom choi 0-9-0 oc, 2x Web conner All loads ardc except if no CASE(S) se provided to unless othe Unbalancec this design. Wind: ASCE Vasd=103m II; Exp B; Ei and right ex Lumber DO TCLL: ASCE Plate DOL= DOL=1.15 F Exp.; Ce=0.	6-26=-2751/348, 6 6-24=-174/1654, 2 8-24=-285/2008, 8 10-22=-60/278, 10 10-19=-1329/181, 11-18=-2296/281, 13-18=-126/816, 1 3-29=-2226/274, 3 4-27=-82/908 o be connected tog nails as follows: connected as follow connected as follows: connected as follows: connected as follows: connected as follows: connected as follows: connected as follows: considered equal ted as front (F) or b tection. Ply to ply co distribute only load rwise indicated. I roof live loads hav E 7-16; Vult=130mp ph; TCDL=6.0psf; nclosed; end vertica L=1.60 plate grip D E 7-16; Pr=20.0 psf 1.15); Pg=20.0 psf Plate DOL=1.15); Is 9; Cs=1.00; Ct=1.1	-25=0/18 2-24=-6 -22=-13 -21=0/20 11-19=- 12-18=- 3-17=-4 -28=-200 ws: 2x4 - 28=-200 ws: 2x4 - 29=-200 ws: 2x4 - 29=-200 ws: 2x4 - 200 ws: 2	81, 78/5764, 42/279, 55, 13/830, 103/1388, 17/73, 87/288, 41 10d 1 row at 0-9- 44 - 1 row at 0-9- 44 - 1 row at 0-9- 0 cc. at 0-9-0 oc. 4 to all plies, acc in the LC s have been as (F) or (B), considered fo ond gust) .); cantilever 1 ight expose 3 : Lum DOL=' psf (Lum ugh Cat B; F)-0-0	DAD r Cat. eft d; 1.15 ully	loa ove 8) Prc 9) * T on 3-0 chc 10) Bea Joi 11) Prc bea 15 12) Gra or 1 bot 13) "N/	d of 12.0 erhangs ovide add his truss the bottk 6-00 tall ord and a arings ar nt 15 SP ovide me aring pla and 255 aphical p the orien tom cho AILED" in 148"x3.2	) psf or non-co equate has be or cho by 2-0 any oth re assuu 2400F chanic te capa lb uplit urlin re tation or rd. ndicate 5") toe	2.00 times flat neurrent with o drainage to paid en designe pfo erd in all areas v 10-00 wide will f er members. Imed to be: Joir 7.2.0E. al connection (l able of withstan ft at joint 29. apresentation d of the purlin alo es 3-10d (0.148 -nails per NDS	roof load of 13. ther live loads. event water pon or a live load of where a rectang it between the t at 29 SP 2400F by others) of tru ding 209 lb upli bes not depict t ing the top and/ (x3") or 3-12d guidlines.	.9 psf on Iding. 20.0psf gle bottom 2.0E , iss to ift at joint he size
	(psf) 20.0 18.9/20.0 10.0 0.0* 10.0 2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E No.3, 23-20,20-15:2 2x4 SP No.3 *Excep No.2 Right 2x4 SP No.2 - Structural wood she 6-0-0 oc purlins, ex 2-0-0 oc purlins, ex 2-0 oc purl	(psf) 20.0 18.9/20.0 18.9/20.0 10.0 10.0 0.0* 10.0 2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E *Except* 28-27:2x4 No.3, 23-20,20-15:2x6 SP 2400F 2.0E 2x4 SP No.3 *Except* 22-24,29-2:2x4 SI No.2 Right 2x4 SP No.2 2-6-0 Structural wood sheathing directly applied 6-0-0 oc purlins, except end verticals, a 2-0-0 oc purlins (5-10-1 max.): 5-12. Rigid ceiling directly applied or 10-0-0 oc bracing. size) 15=0-3-8, 29=0-3-8 1ax Uplift 15=-209 (LC 7), 29=-255 (LC 1ax Grav 15=2174 (LC 2), 29=2199 (LC (lb) - Maximum Compression/Maximum Tension 1-2=0/37, 2-3=-466/71, 3-4=-6620/851, 4-5=-5425/747, 5-6=-4928/686, 6-7=-8747/1100, 7-8=-8494/1070, 8-10=-6657/806, 10-11=-5351/635, 11-12=-3464/419, 12-13=-3879/459,	$\begin{array}{c c} (psf)\\ 20.0\\ 18.9/20.0\\ 18.9/20.0\\ 10.0\\ 10.0\\ 0.0^*\\ 10.0\\ \end{array} \begin{array}{c c} Spacing\\ Plate Grip DOL\\ 1.15\\ Lumber DOL\\ 1.15\\ Rep Stress Incr\\ NO\\ Code\\ IRC202\\ \end{array}$	$\begin{array}{c c c c c c c c c c c c c c c c c c c $	20.0 18.9/20.0 10.0Piate Grip DOL Lumber DOL1.15 LSTC BC BC10.01.001.15BC10.00.0* 10.0CodeIRC2021/TPI2014Matrix-MSH10.0CodeIRC2021/TPI2014Matrix-MSH10.00.0* 10.0CodeIRC2021/TPI2014Matrix-MSH10.0CodeIRC2021/TPI2014Matrix-MSH2x4 SP 2400F 2.0E 2x4 SP 2400F 2.0E $6-26=-2751/348, 6$ $6-24=-174/1654, 2$ $8-24=-285/2008, 8$ $10-22=-60/278, 10$ $10-19=-1329/181, 13-18=-126/816, 10-19=-1329/181, 13-18=-126/816, 10-29=-2226/274, 3No.2Rigid ceiling directly applied or6-0-0 oc purlins, (5-10-1 max); 5-12.NOTESRigid ceiling directly applied or6-0-0 oc purlins (5-10-1 max); 5-12.NOTESRigid ceiling directly applied or 10-0-0 ocbracing.NOTES11 25=-3426/271, 7-5-6-4928/686,6-7=-8747/1100, 7-8=-849/1070,8-10=-6657/806, 10-11=-5351/635,11-12=-3464/419, 12-133879/459,13-15=-3326/371, 15-16=0/32, 2-29=-581/75NOTES11 -12=-3464/419, 12-133879/459,13-15=-3326/371, 15-16=0/32, 2-29=-581/752-29=-245/1796, 27-28=-218/1686,3-27=-704/5456, 26-27=-744/5708, 22-28=-581/750,2-23==124/1071, 21-22=-727/6445, 18-19=-580/5351, 17-18=-293/2886, 15-17=-293/2886Nick ASCE 7-16; Vult=130mpt, Yasg=100, pairYasg=103mph, TCDL=6.0psf;I; Exp B; Enclosed; MWFRS (and right exposed; end vertica, and right expos$		(psf)         Spacing         2-0-0         CSI         DEFL           18.9/20.0         Lumber DOL         1.15         TC         0.26           10.0         Code         IRC2021/TPI2014         Matrix-MSH         DEFL           2x4 SP 2400F 2.0E         Code         IRC2021/TPI2014         Matrix-MSH         Derce 1           2x4 SP 2400F 2.0E         Except* 28-27:2x4 SP         6-24=-174/1654, 22-24=-678/5764, 8-25=0/181, 6-24=-285/2008, 8-22=-1342/279, 10-22=-60/278, 10-21=0/205, 10-19=-1329/181, 11-19=-13/830, 11-18=-2296/281, 12-18=-103/1388, 13-18=-126/816, 13-17=-447/73, 3-28=-2087/288, 6-0-0 co purlins, except end verticals, and 2-0-0 oc purlins, except end vertical, and 1-2-0/37, 2-3=-466/71, 3-4=-6620/851, 4-2=-784/5706, 1-2-2-74/57466, 2-2-2-7-24/574, 5-6=-4228/686, 6-7-8-7474/170, 7-8=-4294/172, 7-24=-380/172, 7-24=-380/173, 1-16=-032, 2-29=-581/75, 22-2=-935/7303, 24-25=-935/7303, 23-22-207/74, 5-2=-935/7303, 23-22-207/74, 5-2=-9272/6445, 18-19=-580/5351, 11-2=-23/28686, 1-2-2-2	(ps) 18.9/20.0 18.9/20.0 10.0 0.0*         Spacing (ps) 2.0-0 10.0 0.0*         2-0-0 1.15 (pate Grip DOL 1.15 (pate Grip DOL 2.2021/TPI2014         CSI (pate Grip DOL BC 0.26 (pate Grip DOL 1.15 (pate Grip DOL 1.15 (pate Grip DOL 2.224 SP 2400F 2.0E (pate Grip DOL 2.24 SP 2400F 2.0E (pate Grip DOL 2.	(psf)         Spacing         2-0-0           18.9/20.0         Plate Grip DOL         1.15           18.9/20.0         Lumber DOL         1.15           0.0         10.0         Rep Stress Incr         NO           10.0         Rep Stress Incr         NO           2x4 SP 2400F 2.0E         RC2021/TPI2014         BC         0.71           2x4 SP 2400F 2.0E         WEBS $4-26=-994/121, 5-26=-255/2085, 7         7           2x4 SP 2400F 2.0E         WEBS         4-26=-994/121, 5-26=-255/2085, 7         7           2x4 SP No.3 "Except" 22-24,29-2:2x4 SP         6-24=-174/1654, 22-24=-678/5764, 0         ovid           8/24 SP No.3 "Except" 22-24,29-2:2x4 SP         6-24=-275/3/248, 6-25=0/181, 0         ovid           8/24 SP No.3 "Except" 22-24,29-2:2x4 SP         10-22=-60/278, 10-21=0/205, 9         9         *T           8/24 SP No.3 "Except" 22-24,29-2:2x4 SP         10-22=-60/278, 10-21=0/205, 9         9         *T           8/21 SP No.2 - 2-60         11-15=-2296/278, 10-21=0/205, 9         9         *T           8/21 SP No.3 "Except" 22-24, 29-2:2x4 SP         10-22=-60/278, 10-21=0/205, 9         9         *T           8/21 SP No.2 - 2-60         10-19=-1329/181, 11-19=-13/830, 0         on         11           8/21 SP No.2 - 2-60     $	$\begin{array}{ c c c c c c c c c c c c c c c c c c c$	(psf)         Spacing         2-0-0         CSI         TC         0.26           18.9/20.0         Plate Grip DOL         1.15         TC         0.26         Vert(LL)         -0.29         7         >999         240           10.0         10.0         0.0*         TC         0.26         Vert(LL)         -0.29         7         >999         240           10.0         0.0*         TC         0.26         Vert(CT)         -0.55         7         >799         180           0.0*         0.0*         TC         0.0         No         No         No         No         Plate Grip DOL         1.15         BC         0.73         Wert(CT)         -0.55         7         >799         180           2x4 SP 2400F 2.0E         WEBS         4-26=-994/121, 5-26=-255/2085,         7)         This truss has bee         load of 12.0 ps or         overhangs non-co         overhords 11-19=-1329/181, 11-1	(psf)         Spacing         2-0-0         CSI         DEFL         in         (loc)         //deft         L/d         PLATES           18.9/20.0         Lumber DOL         1.15         TC         0.26         Derv         Vert(LL)         -0.29         7         >999         240         MT20           18.9/20.0         Lumber DOL         1.15         BC         0.73         Vert(LL)         -0.29         7         >999         240           0.0         0.0*         Code         IRC2021/TPI2014         Matrix-MSH         Derv         0.18         15         n/a         n/a           2x4 SP 2400F 2.0E         Kecept* 28-27:2x4 SP         6-24=-717/1548, 6-25=-0/181,         6-24=-714/1654, 6-25=-0/181,         10.0         12.0 psf or 2.00 times flat         overhangs non-concurrent with or           2x4 SP No.3, 23-20, 20-15:2x6 SP 2400F 2.0E         % 24-285/2008, 8-22=-1342/279,         8         Provide adequate drainage to provi	Cst         Spacing         2-0-0           (ps)         Spacing         2-0-0

### April 23,2025

Page: 1



Continued on page 2 Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev, 1/2/2023 BEFORE USE. WARNING Design valid for use only with MTek connectors. This design is based only upon parameters and property incorporate this design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	170044047	
25040187	A13	Hip Girder	1	2	Job Reference (optional)	172941917	

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:30

ID:8wLYIXUQZnpAevWRRqzdhczODoY-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 2

Carter Components (Sanford, NC), Sanford, NC - 27332,

14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 295 Ib down and 89 lb up at 5-8-0, and 33 lb down at 13-5-12, and 269 lb down and 42 lb up at 31-2-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-2=-48, 2-5=-48, 5-12=-58, 12-16=-48, 28-29=-20, 24-27=-20, 23-30=-20

Concentrated Loads (lb)

Vert: 5=-19 (F), 24=-22 (F), 7=-23 (F), 26=-292 (F),

6=-17 (F), 25=-30 (F), 22=-22 (F), 8=-23 (F),

18=-269 (F), 12=-28 (F), 34=-17 (F), 35=-17 (F),

37=-23 (F), 38=-23 (F), 39=-23 (F), 40=-23 (F),

42=-23 (F), 43=-23 (F), 44=-23 (F), 45=-30 (F),

46=-30 (F), 47=-22 (F), 48=-22 (F), 49=-22 (F), 50=-22 (F), 51=-22 (F), 52=-22 (F), 53=-22 (F)

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	B1	Common	2	1	Job Reference (optional)	172941918

Loading

TCDL

BCLL

BCDL

WFBS

FORCES

WEBS

2)

3)

4)

NOTES 1)

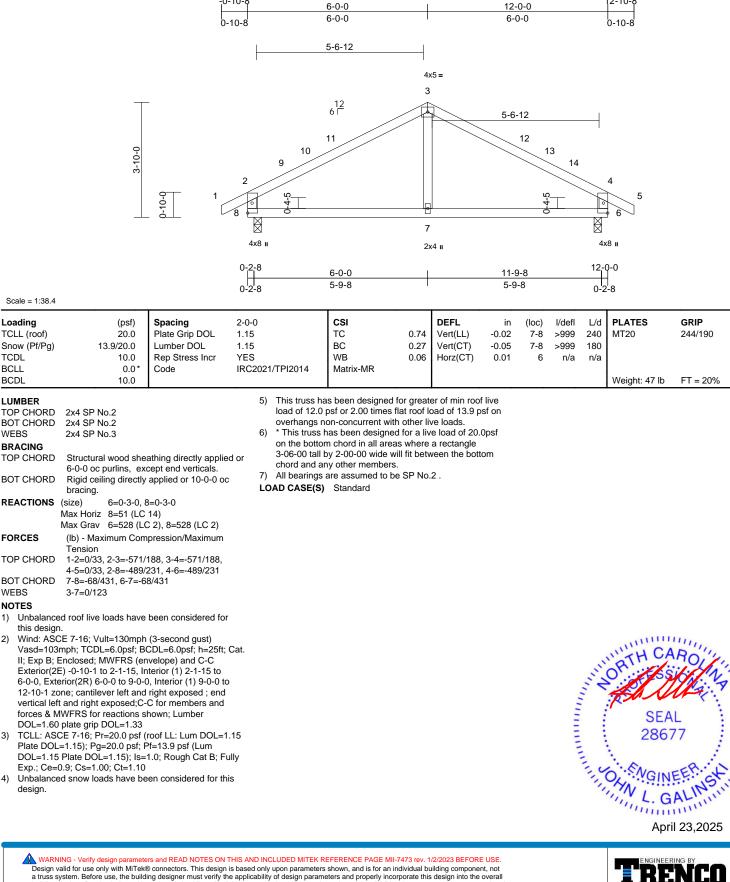
LUMBER

-0-10-8

Run: 8 73 S. Feb 19 2025 Print: 8 730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:31 ID:N6L5VzBkS6fj836dU40mALzODox-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

12-10-8



building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932

MANNING THE

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	B2	Hip Girder	1	1	Job Reference (optional)	172941919

Loading

TCDL

BCLL

BCDL

WEBS

SLIDER

BRACING

FORCES

WEBS

NOTES

1)

2)

3)

4)

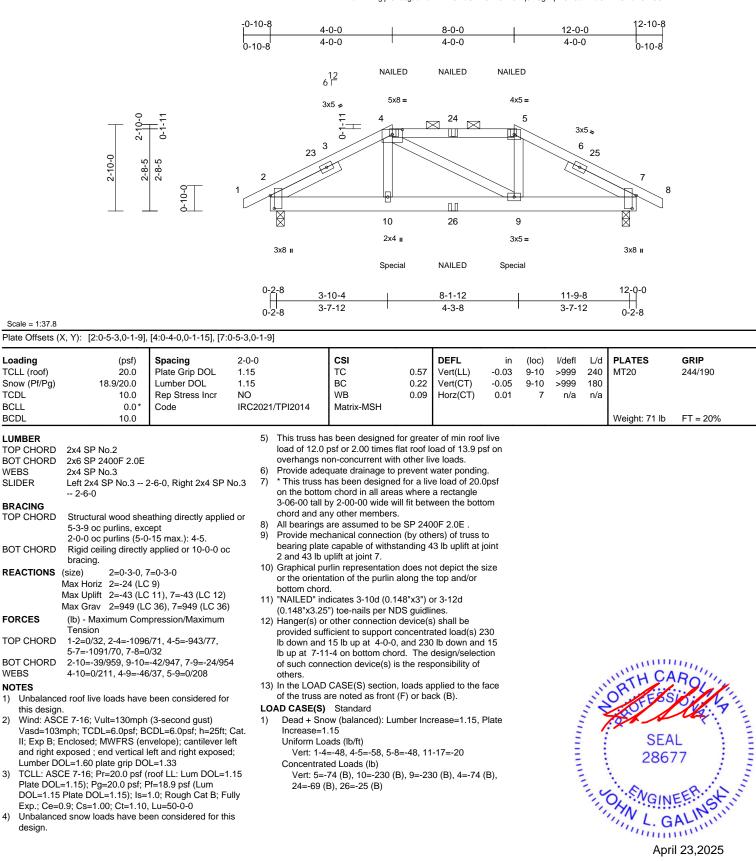
design.

LUMBER

TCLL (roof)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:31 ID:dNRLWgijKJ4coguuVcHrPPzODoG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



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Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	C1	Monopitch	4	1	Job Reference (optional)	172941920

12-2-4

6-0-0

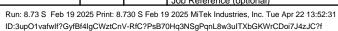
Carter Components (Sanford, NC), Sanford, NC - 27332,

-0-10-8

0-10-8

6-2-4

6-2-4



18-0-8

5-10-4

17



4-11-2

0-2-6 ⊟

7

3x5 =

2x4 II 6 -

3x5 = 3x5 = 12 3 Г 5 16 4 3x5 = E 3 Fq-15 2 -9- $\bigotimes$ × 8 9 5x6 = 3x5 = 2x4 II 0-3-8 6-2-4 12-2-4 18-0-8 5-10-12 5-10-4 6-0-0

Scale = 1:40.6 Plate Offsets (X, Y): [8:0-3-0,0-3-0]

ĥ

5-0-4

Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15		CSI TC	0.43	DEFL Vert(LL)	in -0.02	(loc) 9-14	l/defl >999	L/d 240	PLATES MT20	<b>GRIP</b> 244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15		BC	0.40	Vert(CT)	-0.05	9-14	>999	180	101120	244/100
TCDL	10.0/20.0	Rep Stress Incr	YES		WB	0.46	Horz(CT)	0.00	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/T	PI2014	Matrix-MSH	0.10		0.01					
BCDL	10.0											Weight: 87 lb	FT = 20%
BOT CHORD WEBS BRACING TOP CHORD BOT CHORD	TOP CHORD       2x4 SP No.2         BOT CHORD       2x4 SP No.2         WEBS       2x4 SP No.3         BRACING       Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.         BOT CHORD       Structural wood sheathing directly applied or 10-0-0 oc bracing.         REACTIONS       (size)       2=0-3-0, 7= Mechanical, 9=0-3-8 Max Horiz         Max Uplift       2=-136 (LC 14)         Max Grav       2=320 (LC 2), 7=496 (LC 22), 9=693 (LC 2)				s been designed for sef or 2.00 times fla on-concurrent with as been designed on chord in all areas y 2-00-00 wide will y other members. assumed to be: Jo er(s) for truss to tru- nanical connection capable of withsta at joint 9 and 23 lb Standard	at roof lo other liv for a liv s where I fit betv bint 2 SI uss conr (by oth anding 1	bad of 13.9 p ve loads. e load of 20.1 a rectangle veen the bott P No.2, Joint nections. ers) of truss 6 lb uplift at j	sf on Opsf om t 9 to					
FORCES	(lb) - Maximum Corr	,											
TOP CHORD	,												
BOT CHORD	5-6=-111/76, 6-7=-1 2-9=-146/250, 7-9=-												
WEBS	5-7=-577/116, 3-9=-		'1										115
WEBS	5-8=0/92	551/155, 5-0=-19/57	1,										
NOTES												"TH UA	ROIL
<ol> <li>Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-5 to 2-1-11, Interior (1) 2-1-11 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33</li> <li>TCLL: ASCE 7-16; Pr=20.0 psf; roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10</li> <li>Unbalanced snow loads have been considered for this design.</li> </ol>											Survey States	SEA 286	EEP. St.

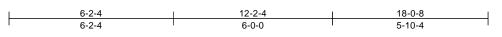
April 23,2025



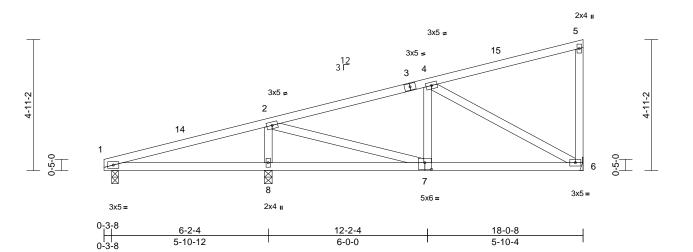
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	C2	Monopitch	1	1	Job Reference (optional)	172941921

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:31 ID:3upO1vafwlf?GyfBf4IgCWztCnV-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Page: 1



#### Scale = 1:43.4

Plate Offsets (X, Y): [7:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0		csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.43	Vert(LL)	-0.02	8-13	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15		BC	0.27	Vert(CT)	-0.05	8-13	>999	180		
TCDL	10.0	Rep Stress Incr	YES		WB	0.46	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC202	1/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 85 lb	FT = 20%
LUMBER			4)		nas been designed			Opsf					
TOP CHORD					n chord in all area								
BOT CHORD					by 2-00-00 wide wi by other members.		ween the botto	om					
WEBS	2x4 SP No.3		5		assumed to be: J		PNo.2 loint	8					
BRACING				SP No.2 .	assumed to be. J		1 10.2 , 30111	0					
TOP CHORD	6-0-0 oc purlins, ex	eathing directly appli	ed or 6)		er(s) for truss to tru	uss coni	nections.						
BOT CHORD		y applied or 10-0-0 o	ic 7		hanical connectior								
	bracing.	, ,,			capable of withst			oint					
REACTIONS	(size) 1=0-3-0,	6= Mechanical, 8=0-	-3-8		t at joint 8 and 1 lb	uplift at	joint 1.						
	Max Horiz 1=133 (L	.C 14)	L	OAD CASE(S)	Standard								
	Max Uplift 1=-1 (LC	11), 6=-17 (LC 15),	8=-11										
	(LC 15)												
	Max Grav 1=268 (L 8=690 (L		,										
FORCES	`	npression/Maximum											
	Tension												
TOP CHORD	1-2=-283/82, 2-4=-	618/123, 4-5=-111/7	6,										
	5-6=-168/91												
BOT CHORD	,												
WEBS	4-6=-582/118, 2-8= 4-7=0/92	-546/153, 2-7=-13/3	50,									, mining	Mun,
NOTES	4-1=0/92											SEA 286	ROUL
	CE 7-16; Vult=130mp	h (3-second quet)									S.	A	in YALL
	Bmph; TCDL=6.0psf; E		; Cat.								22		1. 3'
	Enclosed; MWFRS (e		,								-	GA XI	A CONTRACT
	E) 0-0-0 to 3-0-0, Inte											10.00	
	tilever left and right ex		left							=	:	SEA	∖L : =
	exposed;C-C for mem									- 3		286	77 : E
grip DOL=	for reactions shown; L -1 33		ale							-		200	1 E E
	CE 7-16; Pr=20.0 psf	(roof LL: Lum DOI =	1.15								-	<b>N</b>	1 3
	L=1.15); Pg=20.0 psf;		-								20	S.SNO.	FFR. LS
DOL=1.15	5 Plate DOL=1.15); Is	=1.0; Rough Cat B; F	ully								11	A. GIN	F.F. S.N
	=0.9; Cs=1.00; Ct=1.10										1	L.G	ALICIN
<ol><li>Unbalance</li></ol>	ed snow loads have b	een considered for tl	his									11, 2.0	

Unbalanced snow loads have been considered for this 3) design.



April 23,2025

mmm

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof			
25040187	C3	Half Hip	1	1	Job Reference (optional)	172941922		

4-2-0

4-3-2

Scale = 1:38

Loading

TCDL

BCLL

BCDL

WEBS

BRACING

FORCES

TOP CHORD

BOT CHORD WEBS

DOL=1.33

NOTES

1)

2)

LUMBER

TCLL (roof)

-0-10-8

10=806 (LC 41)

Tension

(lb) - Maximum Compression/Maximum

1-2=0/16, 2-3=-249/109, 3-5=-399/104,

2-10=-167/253, 8-10=-259/570, 7-8=-135/352

3-10=-405/132, 5-8=0/284, 5-7=-520/143,

5-6=-68/76, 6-7=-101/49

Wind: ASCE 7-16; Vult=130mph (3-second gust)

II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-5 to 2-1-11, Interior (1) 2-1-11 to 15-0-0, Exterior(2E) 15-0-0 to 17-10-12 zone; cantilever

left and right exposed ; end vertical left and right

Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully

Exp.: Ce=0.9: Cs=1.00: Ct=1.10. Lu=50-0-0

exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

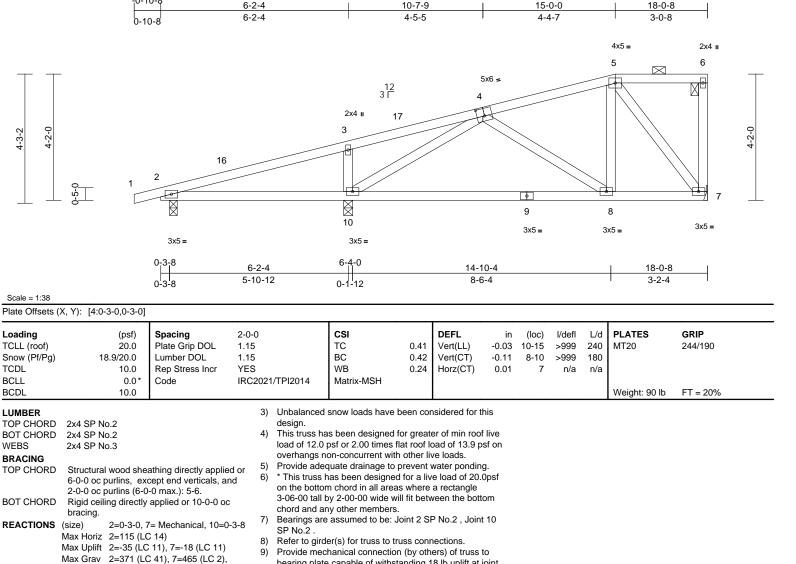
TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15

4-8=-255/149, 4-10=-400/108

Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.

#### Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:31 ID:cPg9Ylb8SUHFIKdf25SXqCzshbc-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



bearing plate capable of withstanding 18 lb uplift at joint 7 and 35 lb uplift at joint 2. 10) Graphical purlin representation does not depict the size

or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



April 23,2025



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	C4	Half Hip Girder	1	1	Job Reference (optional)	172941923

-0-10-8

0-10-8

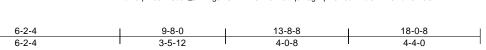
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NAILED

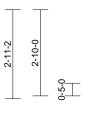
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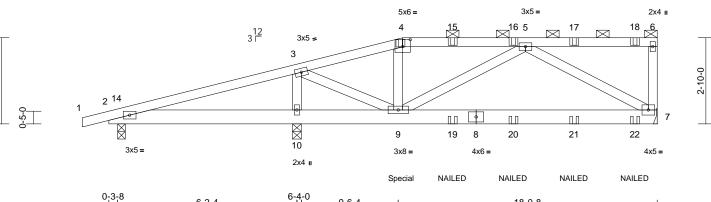
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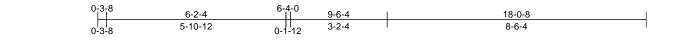
NAILED



NAILED







Scale = 1:37.9

Plate Offsets	(X,	Y):	[4:0-3-0,0-2-12]
---------------	-----	-----	------------------

			-										-		
	ading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
тс	LL (roof)	20.0	Plate Grip DOL	1.15		TC	0.41	Vert(LL)	-0.02	10-13	>999	240	MT20	244/190	
Sn	ow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15		BC	0.34	Vert(CT)	-0.06	7-9	>999	180			
TC	DL	10.0	Rep Stress Incr	NO		WB	0.27	Horz(CT)	0.00	7	n/a	n/a			
BC	LL	0.0*	Code	IRC20	21/TPI2014	Matrix-MSH									
BC	DL	10.0											Weight: 97 lb	FT = 20%	
LU	MBER			5	i) This truss ha	as been designed	for great	er of min roo	f live						
то	P CHORD	2x4 SP No.2			load of 12.0	psf or 2.00 times f	lat roof l	oad of 13.9 p	sf on						
BO	T CHORD	2x6 SP No.2			overhangs n	on-concurrent with	n other li	ve loads.							
WE	BS	2x4 SP No.3		6	<ol> <li>Provide ade</li> </ol>	quate drainage to	prevent	water pondin	g.						
BR	ACING			7	7) * This truss has been designed for a live load of 20.0psf										
	P CHORD	Structural wood she	athing directly applie	ed or		m chord in all area									
		6-0-0 oc purlins, ex	cept end verticals, a			by 2-00-00 wide w		veen the bott	om						
		2-0-0 oc purlins (6-0			<ul><li>chord and any other members.</li><li>8) Bearings are assumed to be: Joint 2 SP No.2, Joint 10</li></ul>										
BO	T CHORD	Rigid ceiling directly	applied or 6-0-0 oc	c	SP No.2 .	e assumed to be: J	10111 2 5	P NO.2 , JOIN	10						
		bracing.			••••••	er(s) for truss to tr	uss con	nections.							
RE	ACTIONS		7= Mechanical, 10=0		10) Provide mechanical connection (by others) of truss to										
		Max Horiz 2=74 (LC			bearing plate capable of withstanding 75 lb uplift at joint										
		Max Uplift 2=-42 (LC			7, 104 lb upl	ift at joint 10 and 4	l2 lb upli	ft at joint 2.							
		10=-104 ( Max Grav 2=302 (LC		、 1	<ol> <li>Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or</li> </ol>										
		10=812 (LC		),			along the	e top and/or							
FO	RCES	(lb) - Maximum Com	,		bottom chore										
FU	RGES	Tension	pression/maximum	1	12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.										
то	P CHORD	1-2=0/16, 2-3=-94/1	13 3-4=-496/85			o") toe-nails per NL r other connection									
10		4-5=-462/91, 5-6=-5				ficient to support of			08 lb						
во	T CHORD	2-10=-121/102, 9-10	,	532		19 lb up at 9-7-2 c								111.	
WE	BS	3-10=-632/118, 3-9=	-87/610, 4-9=-105/5	5,		tion of such conne							"" CI	ND "IL	
		5-9=-184/76, 5-7=-5			responsibility								"ATH UT	NO M	
NO	TES			1		CASE(S) section	, loads a	pplied to the	face			5	O FAS	SKIM'S	
1)	Unbalance	ed roof live loads have	been considered for	r	of the truss a	are noted as front	(F) or ba	ck (B).				33	10/1	14: 7 3	
	this desigr	ז.		L	OAD CASE(S)	Standard						2	7/1	VLY : E	
2)		CE 7-16; Vult=130mph			) Dead + Sn	ow (balanced): Lu	mber Inc	rease=1.15,	Plate				1	. 1 N E .	
		mph; TCDL=6.0psf; B			Increase=1	.15						:	SEA	\L : =	
		Enclosed; MWFRS (er			Uniform Lo	ads (lb/ft)					=	:	286	77 : 2	
		exposed ; end vertical		d;	Vert: 1-4	48, 4-658, 7-1	1=-20						200	// <u>;</u> =	
		OL=1.60 plate grip DO			Concentrat	ed Loads (lb)							<b>1</b>	1 - E	
3)		CE 7-16; Pr=20.0 psf (		.15	Vert: 4=-	5 (B), 9=51 (B), 1	5=-5 (B)	16=-5 (B), 1	7=-5				1. A.	ains	
		=1.15); Pg=20.0 psf; F			-12 (B), 19=-9 (B),	20=-9 (l	3), 21=-9 (B),					O, VGIN	EEL		
		Plate DOL=1.15); Is=	uny	22=-13 (	B)						11	SEA 286	"IN IN		
Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0 4) Unbalanced snow loads have been considered for this				ie									L.G	AL	
4)	design.	Su Show IDaus Have De		10									· · · · · · · ·	mm	
	abbigit.												1998		

April 23,2025

Page: 1

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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

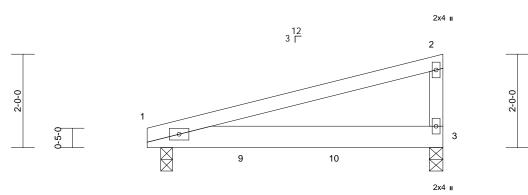
Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	C5	Monopitch Girder	1	2	Job Reference (optional)	172941924

6-4-0

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:32 ID:n5xbi8vbZb39?5DNkFH0pDztCn4-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



3x5 =



#### Scale = 1:24.7

TC Sno TC BC	LL	(psf) 20.0 13.9/20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2021	I/TPI2014	CSI TC BC WB Matrix-MP	0.29 0.33 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.03 -0.06 0.00	(loc) 3-8 3-8 1	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BC	DL	10.0											Weight: 53 lb	FT = 20%
TO BO WE TO BO RE FO	ACING P CHORD T CHORD ACTIONS	2x4 SP No.2 2x6 SP No.2 2x4 SP No.3 Structural wood she 6-0-0 oc purlins, exi Rigid ceiling directly bracing. (size) 1=0-3-0, 3 Max Horiz 1=46 (LC Max Grav 1=714 (LC (Ib) - Maximum Com Tension 1-2=-197/31, 2-3=-1 1-3=-17/221	cept end verticals. applied or 10-0-0 oc 3=0-3-8 10) 2 2), 3=635 (LC 2) pression/Maximum		on the bottom 3-06-00 tall b chord and an All bearings a Hanger(s) or provided suff Ib down at 0 down at 4-0- chord. The c (s) is the resp <b>PAD CASE(S)</b> Dead + Snc Increase=1. Uniform Loa	w (balanced): Lum 15	where fit betw SP No. levice(s incentra at 2-0-1 n at 6-2 such co	a rectangle veen the bott 2 . ) shall be ated load(s) 2 , and 210 lb -4 on bottom nnection dev	om 19 ice					
	TES	1-5=-17/221				ed Loads (lb) 183 (F), 4=-184 (F)	017	1 (E) 1017	5 (E)					
	2-ply truss Top chords follows: 2x4 Bottom cho	to be connected toget s connected with 10d ( 4 - 1 row at 0-9-0 oc. ords connected with 10 6 - 2 rows staggered a	(0.131"x3") nails as 0d (0.131"x3") nails a	IS	ven. 3=-	ιου (Γ), 4=-104 (Γ)	, 9=-17	i (i <sup>-</sup> ), i∪=-17	J (F)				mm	
	except if no CASE(S) s provided to unless othe Wind: ASC	re considered equally oted as front (F) or bar ection. Ply to ply conro distribute only loads erwise indicated. E 7-16; Vult=130mph	ck (B) face in the LO nections have been noted as (F) or (B), (3-second gust)								Ē	View	SEA 2867	ROUNT
4)	II; Exp B; E and right ex Lumber DC TCLL: ASC	nph; TCDL=6.0psf; B6 Enclosed; MWFRS (en xposed ; end vertical I DL=1.60 plate grip DO CE 7-16; Pr=20.0 psf ( =1.15); Pg=20.0 psf; F	velope); cantilever le eft and right exposed L=1.33 roof LL: Lum DOL=1	eft 1;							11111		SEA 2867	L

Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 pst (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

L. GALINS

April 23,2025



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	D1	Common Structural Gable	1	1	Job Reference (optional)	172941925

3-3-15

3-3-15

-0-10-8

0-10-8

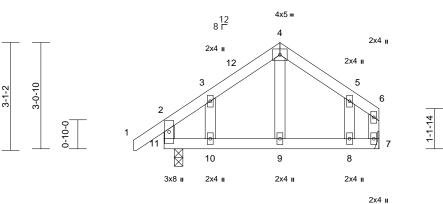
Carter Components (Sanford, NC), Sanford, NC - 27332,

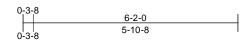
#### Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:32 ID:7VhdcEYOOhPH0eVoXfGC75ztCnX-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

6-2-0

2-10-1







Scale = 1:33.1

Loading TCLL (roof)	(psf) 20.0	<b>Spacing</b> Plate Grip DOL	2-0-0 1.15		CSI TC	0.22	<b>DEFL</b> Vert(LL)	in 0.01	(loc) 9-10	l/defl >999	L/d 240	PLATES MT20	<b>GRIP</b> 244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15		BC	0.16	Vert(CT)	-0.01	9-10	>999	180		
TCDL BCLL	10.0 0.0*	Rep Stress Incr Code	YES IRC2021/1		WB Matrix-MR	0.03	Horz(CT)	0.00	7	n/a	n/a		
BCDL	10.0	Code	IKC2021/	11712014								Weight: 31 lb	FT = 20%
	6-0-0 oc purlins, ex Rigid ceiling directly bracing. (size) 7= Mecha Max Horiz 11=68 (L0 Max Uplift 11=-2 (L0	applied or 10-0-0 oc anical, 11=0-3-0 C 10) C 13)	5) d or 6) 7) 8)	Plate DOL=1 DOL=1.15 Pl Exp.; Ce=0.9 This truss ha load of 12.0 µ overhangs nu Truss to be fi braced again Gable studs * This truss h on the botton 3-06-00 tall b	7-16; Pr=20.0 psi .15); Pg=20.0 psi; ate DOL=1.15); Is ; Cs=1.00; Ct=1.1 s been designed f por concurrent with ully sheathed from st lateral moveme spaced at 2-0-0 on as been designed n chord in all area by 2-00-00 wide wi y other members.	; Pf=13.9 s=1.0; Ro 0 for greate lat roof lo n other lim n one fac ent (i.e. d c. d for a liv s where ill fit betw	e) psf (Lum bugh Cat B; F pad of 13.9 p ve loads. se or securely liagonal web) re load of 20.0 a rectangle	Fully f live sf on , Opsf					
FORCES	Max Grav 7=230 (L0 (lb) - Maximum Com	,. , ,			assumed to be: J er(s) for truss to tr								
TOROLO	Tension	pression/maximum			hanical connection			to					
TOP CHORD	1-2=0/41, 2-3=-222/ 4-5=-225/160, 5-6=- 6-7=-173/74	102, 3-4=-211/165, 210/86, 2-11=-273/1	99, <sup>1</sup>	bearing plate 11.	capable of withst								
BOT CHORD	10-11=-57/161, 9-10 7-8=-57/161	)=-57/161, 8-9=-57/1	61, <b>LOA</b>	AD CASE(S)	Standard								
WEBS	4-9=-35/66, 3-10=-5	6/89, 5-8=-64/112											11.
NOTES												WHILL CA	Dall
,	ed roof live loads have	been considered for									1	ATT	19114
Vasd=103 II; Exp B; E (3E) -0-10 Corner(3E exposed ; members a	n. CE 7-16; Vult=130mph mph; TCDL=6.0psf; B Enclosed; MWFRS (er -0 to 2-2-0, Exterior(2I ) 3-3-15 to 6-0-4 zone end vertical left and ri and forces & MWFRS DL=1.60 plate grip DC	CDL=6.0psf; h=25ft; nvelope) and C-C Co N) 2-2-0 to 3-3-15, ; cantilever left and r ght exposed;C-C for for reactions shown;	rner ght							. CHILLING	No. No.	SEA 2867	L 77
only. For s	gned for wind loads in studs exposed to wind ard Industry Gable En	(normal to the face)										CHAN L. G	ALINSTIT

- exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

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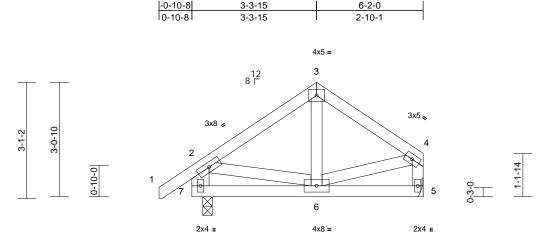
April 23,2025

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	D2	Common	2	1	Job Reference (optional)	172941926

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:32 ID:7VhdcEYOOhPH0eVoXfGC75ztCnX-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:30.7

(psf) 20.0	Spacing	2-0-0										
20.0				CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
	Plate Grip DOL	1.15		TC	0.17	Vert(LL)	0.00	6	>999	240	MT20	244/190
13.9/20.0	Lumber DOL	1.15		BC	0.05	Vert(CT)	0.00	6-7	>999	180		
10.0	Rep Stress Incr	YES		WB	0.05	Horz(CT)	0.00	5	n/a	n/a		
0.0*	Code	IRC2021	/TPI2014	Matrix-MP								
10.0											Weight: 36 lb	FT = 20%
		5) 6)	on the botton 3-06-00 tall b chord and an Bearings are	n chord in all are by 2-00-00 wide w by other members assumed to be:	as where will fit betw s. Joint 7 SF	a rectangle veen the botto P No.2 .	•					
2	10.0 0.0* 10.0 x4 SP No.2 x4 SP No.2 x4 SP No.3 *Excep	10.0 0.0* 10.0Rep Stress Incr Codexx4 SP No.2 xx4 SP No.2 xx4 SP No.3 *Except* 7-2:2x6 SP No.2	10.0 0.0* 10.0 x4 SP No.2 x4 SP No.2 x4 SP No.2 x4 SP No.3 *Except* 7-2:2x6 SP No.2	10.0     Rep Stress Incr     YES       0.0*     Code     IRC2021/TPI2014       10.0     5) * This truss h on the botton 3-06-00 tall b chord and an 6) Bearings are 70 Bearings are	10.0       Rep Stress Incr       YES       WB         0.0*       Code       IRC2021/TPI2014       Matrix-MP         10.0       5) * This truss has been designed on the bottom chord in all are 3-06-00 tall by 2-00-00 wide wide wide wide wide wide wide wide	10.0     Rep Stress Incr Code     YES     WB     0.05       10.0     Rc2021/TPI2014     Matrix-MP       10.0     * This truss has been designed for a liv on the bottom chord in all areas where 3-06-00 tall by 2-00-00 wide will fit betw chord and any other members.       6)     * Except* 7-2:2x6 SP No.2       6)     Bearings are assumed to be: Joint 7 SF Defect to direction (and (a) for true to true open	10.0       Rep Stress Incr Code       YES       WB       0.05       Horz(CT)         10.0       No.1       RC2021/TPI2014       Matrix-MP       Horz(CT)         x4 SP No.2       * This truss has been designed for a live load of 20.0 on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.         6)       Bearings are assumed to be: Joint 7 SP No.2 .         7)       Before to ridox(0) for two to two another on the two to two another on the constraints.	10.0       Rep Stress Incr       YES       WB       0.05       Horz(CT)       0.00         10.0       Code       IRC2021/TPI2014       WB       0.05       Horz(CT)       0.00         x4 SP No.2       X4 SP No.2       5)       * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle       3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.         6)       Bearings are assumed to be: Joint 7 SP No.2.       To prove to true to true to true to comparison	10.0       Rep Stress Incr       YES       WB       0.05       Horz(CT)       0.00       5         0.0*       10.0       Code       IRC2021/TPI2014       WB       0.05       Horz(CT)       0.00       5         x4 SP No.2       x4 SP No.2       5)       * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle       3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.       6)       Bearings are assumed to be: Joint 7 SP No.2.         20       Pafor to girdpr(0) for true to true to graph to page to true to true to graph to page to	10.0     Rep Stress Incr Code     YES     WB     0.05     Horz(CT)     0.00     5     n/a       10.0     10.0     IRC2021/TPI2014     Matrix-MP     Matrix-MP     Horz(CT)     0.00     5     n/a       10.0     50     * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.     5)     * Bearings are assumed to be: Joint 7 SP No.2.       10.0     50     Pedra to ridor(o) for two to true or promotione     10.0     5     n/a	10.0       Rep Stress Incr Code       YES       WB       0.05       Horz(CT)       0.00       5       n/a       n/a         10.0       10.0       IRC2021/TPI2014       Matrix-MP       Matrix-MP       Horz(CT)       0.00       5       n/a       n/a         x4 SP No.2       50       * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.       50       Bearings are assumed to be: Joint 7 SP No.2.         60       Bearings are assumed to be: Joint 7 SP No.2.       70       Before to refore for former to true to to present the present to present to present the present to present t	10.0       Rep Stress Incr       YES       WB       0.05       Horz(CT)       0.00       5       n/a       n/a         10.0       10.0       Code       IRC2021/TPI2014       Matrix-MP       Horz(CT)       0.00       5       n/a       n/a         x4 SP No.2       x4 SP No.2       5)       * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle       3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.       6)       Bearings are assumed to be: Joint 7 SP No.2.         20       Befort to right/ploif for the torup to tune to tune to an expendence       PNo.2.       7)       Refort to right/ploif for the torup to tune to tune to an expendence       Sint 7 SP No.2.

TOP CHORD	Structural wood sheathing directly applie
	6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc
	bracing.

- **REACTIONS** (size) 5= Mechanical, 7=0-3-0 Max Horiz 7=69 (LC 10) Max Uplift 7=-2 (LC 13) Max Grav 5=226 (LC 2), 7=301 (LC 2) FORCES (lb) - Maximum Compression/Maximum Tension 1-2=0/44, 2-3=-230/82, 3-4=-212/80, TOP CHORD 2-7=-325/174, 4-5=-248/105 BOT CHORD 6-7=-85/54, 5-6=-16/18
- WEBS NOTES
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-0 to 2-2-0, Interior (1) 2-2-0 to 3-3-15, Exterior(2E) 3-3-15 to 6-0-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

3-6=-7/40, 4-6=-22/149, 2-6=0/137

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 3) Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live 4) load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- Refer to girder(s) for truss to tru
- Provide mechanical connection (by others) of truss to 8) bearing plate capable of withstanding 2 lb uplift at joint 7.
- LOAD CASE(S) Standard



April 23,2025



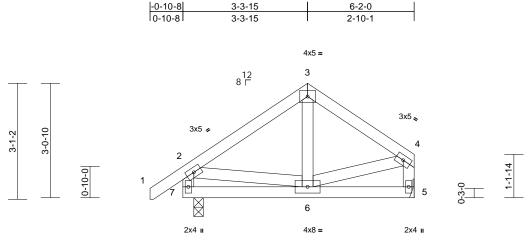
Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	D3	Common	7	1	Job Reference (optional)	172941927

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:32 ID:7VhdcEYOOhPH0eVoXfGC75ztCnX-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

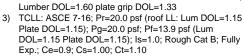
Page: 1





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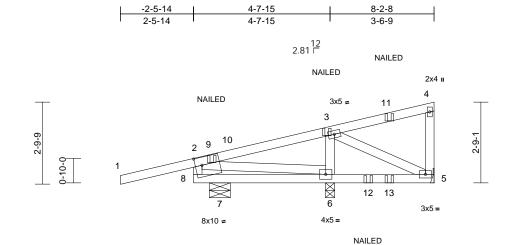
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2	2014	CSI TC BC WB Matrix-MP	0.18 0.06 0.06	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 6 6-7 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 36 lb	<b>GRIP</b> 244/190 FT = 20%
	6-0-0 oc purlins, ex Rigid ceiling directly bracing.	applied or 10-0-0 oc anical, 7=0-3-0 12) 13)	on th 3-06 chor 6) Bear d or 7) Refe 8) Prov bear	he bottom 5-00 tall b rd and an rings are er to girde vide mech ring plate	as been designed as chord in all area y 2-00-00 wide w y other members assumed to be: J er(s) for truss to tr hanical connection capable of withst Standard	is where ill fit betv loint 7 SI russ conr n (by oth	a rectangle veen the botto P No.2 . nections. ers) of truss to	om o					
<ul> <li>this design</li> <li>2) Wind: ASC</li> <li>Vasd=1037</li> <li>II; Exp B; E</li> <li>Exterior(2E</li> <li>right exposs</li> <li>for membe</li> <li>Lumber DC</li> <li>3) TCLL: ASC</li> <li>Plate DOL:</li> <li>DOL=1.15</li> <li>Exp.; Ce=C</li> <li>4) This truss I</li> <li>load of 12.1</li> </ul>	(lb) - Maximum Com Tension 1-2=0/41, 2-3=-235/ 2-7=-321/170, 4-5=- 6-7=-84/53, 5-6=-16 3-6=-5/43, 4-6=-24/ droof live loads have b E 7-16; Vult=130mph mph; TCDL=6.0psf; B Enclosed; MWFRS (er E) -0-10-0 to 2-2-0, Int E) 3-3-15 to 6-0-4 zonu- sed; end vertical left a trs and forces & MWF DL=1.60 plate grip DC DCE 7-16; Pr=20.0 psf; F Plate DOL=1.15); Is= Plate DOL=1.15); Is= Plate DOL=1.15); Is= Net Constant of the signed for 0 psf or 2.00 times fla non-concurrent with o	81, 3-4=-219/83, 253/106 /18 154, 2-6=0/140 been considered for (3-second gust) CDL=6.0psf; h=25ft; tvelope) and C-C erior (1) 2-2-0 to 3-3- e; cantilever left and ind right exposed; C-C RS for reactions shou L=1.33 roof LL: Lum DOL=1 2f=13.9 psf (Lum 1.0; Rough Cat B; Fu r greater of min roof I t roof load of 13.9 psi	Cat. 15, Cwn; .15 illy								and a state of the	SEA 2867	EFRISK ALINSTITUTION 123,2025
												Apri	1 23,2025

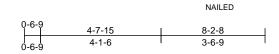


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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E1	Diagonal Hip Girder	1	1	Job Reference (optional)	172941928

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:32 ID:0dj9LjbillyuTjMuEt\_IVvzshcu-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1





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## Plate Offsets (X, Y): [8:0-2-12,0-3-4]

	(, .). <u>L</u> ere = .=,e e .	-1 -1										
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 NO IRC2021/TPI2014	CSI TC BC WB Matrix-MP	0.69 0.44 0.15	Vert(CT)	in 0.01 0.02 0.00	(loc) 6-7 6-7 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 46 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD	2x4 SP No.2 2x4 SP No.3 Structural wood she 6-0-0 oc purlins, ex Rigid ceiling directly bracing. (size) 5= Mecha Max Horiz 7=75 (LC Max Uplift 5=-71 (LC 7=-49 (LC Max Grav 5=78 (LC 7=302 (LC (lb) - Maximum Com Tension	r applied or 6-0-0 oc anical, 6=0-3-12, 7=0- 8) 2 41), 6=-148 (LC 7), 2 7) 42), 6=-481 (LC 18), C 41) apression/Maximum	d or the bo 3-06-00 t chord an 6) Bearings SP No.2 7) Refer to 8) Provide r bearing r 5, 148 lb NDS guid 10) In the LC of the tru LOAD CASE 1) Dead + Increas Uniform	girder(s) for truss to t nechanical connection late capable of withs uplift at joint 6 and 4° 'indicates 2-12d (0.1 tlines. AD CASE(S) section ss are noted as front (S) Standard Snow (balanced): Lu ∋=1.15 Loads (lb/ft)	as where vill fit betv. Joint 7 S russ coni n (by oth tanding 7 9 Ib uplifit 48"x3.25 I, loads a (F) or ba	a rectangle veen the bott P No.2 , Join nections. ers) of truss 1 lb uplift at at joint 7. ") toe-nails p pplied to the ck (B).	to to joint face					
BOT CHORD WEBS NOTES 1) Wind: AS Vasd=100 II; Exp B; and right Lumber D Plate DOI DOL=1.11 Exp.; Ce=	3-4=-47/20, 4-5=-84 7-8=-6/4, 6-7=-73/8,	/24 ,5-6=-327/160 157/363, 2-6=-330/18 CDL=6.0psf; h=25ft; rvelope); cantilever le left and right exposed DL=1.33 (roof LL: Lum DOL=1 Pf=13.9 psf (Lum 1.0; Rough Cat B; Fu	Concen 89 Vert: Cat. oft 1; .15 .15	1-2=-46, 2-4=-46, 5-4 trated Loads (lb) 12=60 (F), 13=-1 (B)						Number of Street	ORTH CA ORTH CA SEA 2861	

design.

 This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

April 23,2025

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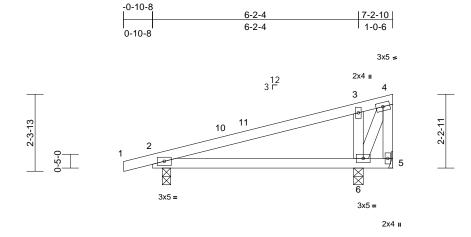
TREENCO A MITek Affiliate 818 Soundside Road Edenton, NC 27932

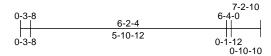
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Job	Truss Truss Type		Qty	Ply	18 Eagle Creek - Edisto B - Roof		
25040187	E2	Jack-Closed	1	1	Job Reference (optional)	172941929	

#### Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:32 ID:biF0qZZ09?X8eo4\_5NnRgJztCnW-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:34.6

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL	(psf) 20.0 13.9/20.0 10.0 0.0*	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MP	0.46 0.35 0.13	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.04 -0.08 0.01	(loc) 6-9 6-9 2	l/defl >999 >982 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0										Weight: 30 lb	FT = 20%
	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she 6-0-0 oc purlins, ex Rigid ceiling directly bracing. (size) 2=0-3-0, § Max Horiz 2=57 (LC Max Uplift 2=-26 (LC 6=-1 (LC Max Grav 2=284 (LC (LC 22)	cept end verticals. applied or 10-0-0 or 5= Mechanical, 6=0- 14) 3 11), 5=-104 (LC 22 15)	ed or c -3-8 2), 5) * This tru on the bo 3-06-00 t chord and SP No.2 7) Refer to ( 8) Provide r bearing p 5, 1 bu pp	has been designed 20 psf or 2.00 times s non-concurrent wit ss has been designe ttom chord in all aree all by 2-00-00 wide wide any other members are assumed to be: yirder(s) for truss to the handle of withs lift at joint 6 and 26 ll (S) Standard	flat roof I h other Ii d for a liv as where vill fit betv s. Joint 2 S russ com on (by oth tanding 1	oad of 13.9 p ve loads. ve load of 20. a rectangle veen the bott P No.2, Join nections. uers) of truss 104 lb uplift a	osf on Opsf tom t 6 to					
FORCES	(lb) - Maximum Com Tension	pression/Maximum										
TOP CHORD BOT CHORD	1-2=0/16, 2-3=-167/ 4-5=-52/2 2-6=-99/160, 5-6=-3	, ,										
WEBS	3-6=-511/379, 4-6=-	180/332										
Vasd=103 II; Exp B; F Exterior(2I 7-0-14 zor vertical lef forces & M DOL=1.60 2) TCLL: AS( Plate DOL DOL=1.15 Exp.; Ce=0	CE 7-16; Vult=130mph mph; TCDL=6.0psf; B Enclosed; MWFRS (er E) -0-10-5 to 2-1-11, Ir e; cantilever left and r t and right exposed;C- WFRS for reactions s plate grip DOL=1.33 CE 7-16; Pr=20.0 psf ( =1.15); Pg=20.0 psf; F Plate DOL=1.15); Is= 0.9; Cs=1.00; Ct=1.10 ed snow loads have be	CDL=6.0psf; h=25ft; ivelope) and C-C iterior (1) 2-1-11 to ight exposed ; end C for members and hown; Lumber roof LL: Lum DOL= 2f=13.9 psf (Lum 1.0; Rough Cat B; F	1.15 Fully							and a state of the	SEA SEA 286	EER. St.

April 23,2025

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E3	Diagonal Hip Girder	3	1	Job Reference (optional)	172941930

3-3-0

3-3-0

-1-5-8

1-5-8

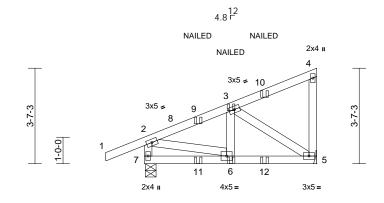
Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:32 ID:crNVO2IOKtoRjRIMVTgt1EzODoo-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

6-6-0

3-3-0

Page: 1



NAILED



#### Scale = 1:43.4

00010 = 1.40.4												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	0.00	6	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	-0.01	5-6	>999	180		
TCDL BCLL	10.0 0.0*	Rep Stress Incr	NO IRC2021/TPI2014	WB	0.11	Horz(CT)	0.00	5	n/a	n/a		
BCLL BCDL	10.0	Code	IRC2021/1PI2014	Matrix-MP							Weight: 40 lb	FT = 20%
	10.0	I.			1-1-1-7-0						Wolght. To ib	
LUMBER TOP CHORD	2x4 SP No.2			are assumed to be: girder(s) for truss to								
BOT CHORD	2x4 SP No.2			mechanical connecti			0					
WEBS	2x4 SP No.3			plate capable of with								
BRACING				lb uplift at joint 5.								
TOP CHORD	Structural wood she		ed or 9) "NAILEI NDS qu	" indicates 2-12d (0.	148"x3.2	o") toe-nails pe	er					
	6-0-0 oc purlins, ex		40) 1- 4 14	DAD CASE(S) sectio	n loads a	nnlied to the f	ace					
BOT CHORD	Rigid ceiling directly bracing.	applied or 10-0-0 c		iss are noted as fron			400					
REACTIONS	5	anical, 7=0-4-13	LOAD CAS	(S) Standard								
	Max Horiz 7=104 (L0	,		Snow (balanced): L	umber Inc	rease=1.15, F	Plate					
	Max Uplift 5=-33 (LC	,		e=1.15								
	Max Grav 5=280 (LC	C 18), 7=354 (LC 2)		1 Loads (lb/ft) 1-2=-48, 2-4=-48, 5	-720							
FORCES	(lb) - Maximum Com Tension	npression/Maximum		trated Loads (lb)	-7=-20							
TOP CHORD	2-7=-330/50, 1-2=0/ 3-4=-73/27, 4-5=-93		Vert	6=-3 (F), 3=-12 (F),	11=3 (B),	12=-1 (B)						
BOT CHORD WEBS	6-7=-102/14, 5-6=-5 2-6=0/257, 3-6=0/53											
NOTES	,	,										
	CE 7-16; Vult=130mph	(3-second gust)										
	mph; TCDL=6.0psf; B											11111
	Enclosed; MWFRS (er										WAH CA	ARO
	exposed ; end vertical OL=1.60 plate grip DC		ed;							N	R	Service 1
	CE 7-16; Pr=20.0 psf (		1.15							22	U. FLOS	DK. V.S.
	=1.15); Pg=20.0 psf; I									2	<b>4</b> 1	Maj: 2
	Plate DOL=1.15); Is=		Fully						-		14.00	N N E -
	0.9; Cs=1.00; Ct=1.10		L						=	:	SEA	AL : =
<ol> <li>Unbalance design.</li> </ol>	ed snow loads have be	een considered for t	nis						= =		SEA 286	77 : E
	has been designed fo	r greater of min root	flive									11 - E - E -
	.0 psf or 2.00 times fla		sf on							-	N	- 1. E
0	non-concurrent with		0{							24	O SNGIN	FERICE
	s has been designed f tom chord in all areas		upsr							11	YA	in S'in
	Il by 2-00-00 wide will	0	om								11. L.G	AL
	any other members.		-								111111	EEP. St.
											Anr	il 23 2025

- 3) Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live 4) load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- \* This truss has been designed for a live load of 20.0psf 5) on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

818 Soundside Road Edenton, NC 27932

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April 23,2025

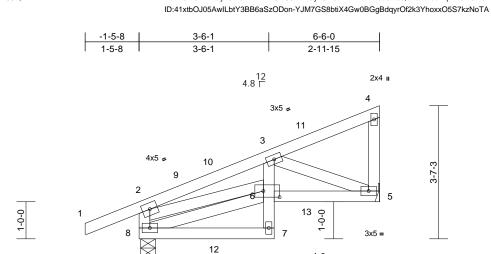
Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E4	Diagonal Hip Girder	1	1	Job Reference (optional)	172941931

3-7-3

# Run: 8.73 E May 9 2024 Print: 8.730 E May 9 2024 MiTek Industries, Inc. Wed Apr 23 16:10:28

4x8 =

Page: 1



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3x5 =



2x4 🛛

Scale = 1:31.2

Plate Offsets (X, Y): [6:0-5-4,0-2-0]

	7, 1). [0.0-3-4,0-2-0]												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0		2-0-0 1.15 1.15 NO IRC2021	I/TPI2014	CSI TC BC WB Matrix-MP	0.24 0.20 0.18	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.01 -0.02 0.01	(loc) 7 7-8 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 43 lb	<b>GRIP</b> 244/190 FT = 20%
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Wind: ASC Vasd=103 II; Exp 8; f and right e Lumber DO 2) TCLL: ASC Plate DOL DOL=1.15 Exp.; Ce=0 3) Unbalance design. 4) This truss load of 12.	2x4 SP No.2 *Excep 2x4 SP No.3 Structural wood she 6-0-0 oc purlins, exx Rigid ceiling directly bracing. (Ib/size) 5=223/ Mi Max Horiz 8=93 (LC Max Uplift 5=-50 (LC Max Grav 5=294 (LC (Ib) - Maximum Com Tension 2-8=-323/69, 1-2=0// 3-4=-58/24, 4-5=-86	athing directly applied cept end verticals. applied or 10-0-0 oc echanical, 8=306/0-4 8) 2 8), 8=-45 (LC 7) C 18), 8=362 (LC 2) apression/Maximum 44, 2-3=-492/70, /16 , 3-6=-23/111, /423, 3-5=-479/100 (3-second gust) CDL=6.0psf; h=25ft; C hvelope); cantilever left left and right exposed; 9L=1.33 roof LL: Lum DOL=1.1 Pf=13.9 psf (Lum 1.0; Rough Cat B; Full een considered for this r greater of min roof liv t roof load of 13.9 psf of	8) 13 9) LO 1) :at. t 5 y y	on the bottor 3-06-00 tall b chord and ar Refer to gird Provide mec bearing plate 8 and 50 lb u Hanger(s) or provided suff down and 16 up at 3-3-4, 8 lb down an up at 3-6-1, bottom chore device(s) is t In the LOAD of the truss a <b>DAD CASE(S)</b> Dead + Sno Increase=1 Uniform Loz Vert: 1-22 Concentrate	w (balanced): Lum 15	where I fit betw iss conr (by oth unding 4 device(so oncentra d 44 lb 4-6-6 ct oncentra d 44 lb ction of others. loads al =) or ba	a rectangle veen the bott nections. ers) of truss 5 lb uplift at ) shall be ated load(s) 3 down and 37 n top chord, lb down and up at 4-6-6 of such connect oplied to the ck (B). rease=1.15, 6=-20	to joint 36 lb 7 lb and 14 lb on stion face				SEA ON MININ	EEP. OC.



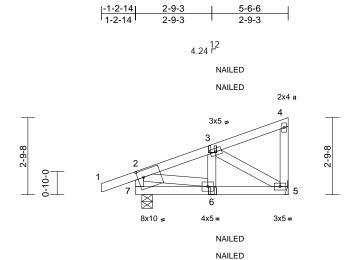
L. GA 111111111 April 23,2025

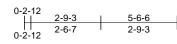
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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E5	Diagonal Hip Girder	2	1	Job Reference (optional)	172941932

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:32 ID:41xtbOJ05AwILbtY3BB6aSzODon-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:41.7

Plate Offsets (X, Y): [7:0-2-8,0-3-0]

Plate Grip DOL1.19Lumber DOL1.11Rep Stress IncrNO	5	CSI TC BC WB Matrix-MP	0.19 0.09 0.10	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 6 6-7 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 32 lb	<b>GRIP</b> 244/190 FT = 20%
eathing directly applied or xcept end verticals. y applied or 10-0-0 oc 28) 211), 7=-38 (LC 7) C 18), 7=315 (LC 18) mpression/Maximum y/34, 2-3=-275/2, 8/14 7/230 13, 3-5=-265/18 h (3-second gust) 3CDL=6.0psf; h=25ft; Cat. envelope); cantilever left left and right exposed; OL=1.33 (roof LL: Lum DOL=1.15 Pf=13.9 psf (Lum =1.0; Rough Cat B; Fully 0 even considered for this or greater of min roof live at roof load of 13.9 psf on other live loads.	on the bottor 3-06-00 tall to chord and ar 6) Bearings are 7) Refer to gird 8) Provide mec bearing plate 7 and 8 lb up 9) "NAILED" int (0.148"x3.25 10) In the LOAD of the truss ar LOAD CASE(S) 1) Dead + Snc Increase=1 Uniform Lo Vert: 1-2 Concentrat	n chord in all areas by 2-00-00 wide will by other members. assumed to be: Jo er(s) for truss to tru hanical connection capable of withsta lift at joint 5. dicates 3-10d (0.14 ") toe-nails per ND: CASE(S) section, 1 tre noted as front (F Standard by (balanced): Lurr .15 ads (lb/ft) =-48, 2-4=-48, 5-7= ed Loads (lb)	where I fit betw bint 7 SI iss conr (by oth unding 3 8"x3") of S guidli loads a =) or ba aber Inc	a rectangle veen the botto P No.2 . lections. ers) of truss t 8 lb uplift at j or 2-12d nes. oplied to the to ck (B).	ro ooint				SEA 286	EEP. CAMPANY
	Plate Grip DOL 1.11 Lumber DOL 1.11 Rep Stress Incr NO Code IRC Code IRC eathing directly applied or xcept end verticals. y applied or 10-0-0 oc anical, 7=0-4-10 2.8) 11), 7=-38 (LC 7) .C 18), 7=-315 (LC 18) mpression/Maximum //34, 2-3=-275/2, 8/14 7/230 3, 3-5=-265/18 h (3-second gust) 3COL=6.0psf; h=25ft; Cat. invelope); cantilever left left and right exposed; DL=1.39 psf (Lum =1.0; Rough Cat B; Fully or een considered for this or greater of min roof live at roof load of 13.9 psf on	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2021/TPI2014 5) * This truss f on the bottor 3-06-00 tall t chord and ar 6) Bearings are 7) Refer to gird 8) Provide mec 20 anical, 7=0-4-10 20	Plate Grip DOL       1.15       TC         Lumber DOL       1.15       BC         wyb       WB       Matrix-MP         Code       IRC2021/TPI2014       Matrix-MP         5) * This truss has been designed on the bottom chord in all areas 3-06-00 tall by 2-00-00 wide wil chord and any other members.         eathing directly applied or xcept end verticals. y applied or 10-0-0 oc       5)         anical, 7=0-4-10       Bearings are assumed to be: Jc         c. 18), 7=315 (LC 18) mpression/Maximum       9) "NAILED" indicates 3-10d (0.14 (0.148"x3.25") toe-nails per ND         10) In the LOAD CASE(S) section, of the truss are noted as front (f LOAD CASE(S) Standard       9)         10) In the LOAD CASE(S) Standard       1)         11) Zasecond gust) 33, 3-5=-265/18       100 In the LOAD CASE(S) Standard         11) Group Cat B; Fully Deeen considered for this       Vert: 6=-1 (F=0, B=0)         h (3-second gust) 3COL=6.0psf; h=25ft; Cat. invelope); cantilever left       Vert: 6=-1 (F=0, B=0)         h (3-second gust) 3COL=5.0psf (Lum =1.0; Rough Cat B; Fully Deeen considered for this       Vert: 6=-1 (F=0, B=0)	Plate Grip DOL       1.15       TC       0.19         Lumber DOL       1.15       BC       0.09         Rep Stress Incr       NO       Matrix-MP         Code       IRC2021/TPI2014       Matrix-MP         5) * This truss has been designed for a liv on the bottom chord in all areas where 3-06-00 tall by 2-00-00 wide will fit betw chord and any other members.         eathing directly applied or xcept end verticals. y applied or 10-0-0 oc       6         anical, 7=0-4-10       7         C 18), 7=315 (LC 7)       7         C 18), 7=315 (LC 18)       9         mpression/Maximum       10         V/34, 2-3=-275/2, 8/14       9         V/34, 2-3=-275/2, 8/14       10         V/34, 2-3=-275/2, 8/14       10         Docd call spit and right exposed; QL=1.33       0Lall spit and right exposed; QL=1.33         Group LL: Lum DOL=1.15       Profile and right exposed; QL=1.39 psf (Lum =1.0; Rough Cat B; Fully 0         een considered for this       or greater of min roof live at roof load of 13.9 psf on	Plate Grip DOL       1.15       TC       0.19       Vert(LL)         Lumber DOL       1.15       BC       0.09       Wert(CT)         Rep Stress Incr       NO       Matrix-MP       Horz(CT)         Code       IRC2021/TPI2014       Matrix-MP       Horz(CT)         5)       * This truss has been designed for a live load of 20.0 on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bott chord and any other members.       6) Bearings are assumed to be: Joint 7 SP No.2.         7       Refer to girder(s) for truss to truss connections.       Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 5.         9       "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.         10)       In the LOAD CASE(S) section, loads applied to the i of the truss are noted as front (F) or back (B).         10/At 2-3=-275/2, 8/14       Dead + Snow (balanced): Lumber Increase=1.15, 1         10/At 2-3=-265/18       Uniform Loads (lb/t)         11       Vert: 1-2=-48, 2-4=-48, 5-7=-20         12       Concentrated Loads (lb)         12       Vert: 6=-1 (F=0, B=0)         14       Yer: 6=-1 (F=0, B=0)         15       Vert: 6=-1 (F=0, B=0)         16       Now Glopa)         17       Read of 13.9 psf	Plate Grip DOL1.15TC0.19Vert(LL)0.00Rep Stress IncrNOWB0.10Horz(CT)0.00CodeIRC2021/TPI2014WB0.10Horz(CT)0.00Matrix-MPWB0.10Matrix-MPHorz(CT)0.005)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.5)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.eathing directly applied or tx applied or 10-0-0 oc7)Refer to girder(s) for truss to truss connections.anical, 7=0-4-10 C 18), 7=315 (LC 7) C 18), 7=315 (LC 7) C 18), 7=315 (LC 7) Matimum9)NAILED* indicates 3-10d (0.148*x3") or 2-12d (0.148*x3.25") toe-nails per NDS guidlines.10)In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).LOAD CASE(S) Vart: 1==48, 2-4=-48, 5-7=-20Concentrated Loads (Ib) Vert: 6=-1 (F=0, B=0)h (3-second gust) 30GDL=6.0psf; h=25ft; Cat. rowelope); cantilever left left and right exposed; OL=1.33 (roof LL: Lum DOL=1.15Pf=13.9 psf (Lum =1.0; Rough Cat B; Fully 0een considered for thisor greater of min roof live at roof load of 13.9 psf on	Plate Grip DOL1.15TC0.19Vert(LL)0.006Lumber DOL1.15BC0.09Vert(CT)0.006-7Rep Stress IncrNOWB0.10Matrix-MPVert(CT)0.005CodeIRC2021/TPI2014Matrix-MPMatrix-MPVert(CT)0.0055) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.6)Bearings are assumed to be: Joint 7 SP No.2 .eathing directly applied or xcept end verticals. y applied or 10-0-0 oc7)Refer to girder(s) for truss to truss connections.anical, 7=0-4-10 C 8)9)"NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.9) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).LOAD CASE(S)Standard1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.1510/10rm Loads (lb/ft) Vert: 1-2=-48, 2-4=-48, 5-7=-20 Concentrated Loads (lb)	Plate Grip DOL1.15TC0.19Vert(LL)0.006>999Lumber DOL1.15BC0.09Vert(CT)0.006-7>999MoMatrix-MPMB0.10Matrix-MPVert(CT)0.006-7>999Horz(CT)0.005n/aCodeIRC2021/TPI2014Matrix-MPVert(CT)0.005n/a5)* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.6)Bearings are assumed to be: Joint 7 SP No.2 .eathing directly applied or xcept end verticals. y applied or 10-0-0 oc7)Refer to girder(s) for truss to truss connections.anical, 7=0-4-10 C 8)9)"NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.9)"NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.10)In the LOAD CASE(S) Section, loads applied to the face of the truss are noted as front (F) or back (B).LOAD CASE(S)Standard1)Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (lb/ft) Vert: 1-2=-48, 2-4=-48, 5-7=-20(230Concentrated Loads (lb) Dead	Plate Grip DOL       1.15       TC       0.19       Vert(LL)       0.00       6       >999       240         Lumber DOL       1.15       BC       0.09       Vert(CT)       0.00       6-7       >999       180         Rep Stress Incr       NO       Matrix-MP       Matrix-MP       Vert(CT)       0.00       5       n/a       n/a         state Grip DOL       1.15       BC       0.09       Vert(CT)       0.00       6-7       >999       180         Code       IRC2021/TPI2014       Matrix-MP       Matrix-MP       Vert(CT)       0.00       5       n/a       n/a         statistic statisti	Plate Grip DOL       1.15       TC       0.19       Vert(LL)       0.00       6       >999       240         Martix-MP       WB       0.10       Wr(CT)       0.00       5       n/a       n/a         Weight: 32.lb       Matrix-MP       WB       0.10       Horz(CT)       0.00       5       n/a       n/a         Solution       IRC2021/TPI2014       Matrix-MP       Weight: 32.lb       Weight: 32.lb       Weight: 32.lb         Solution       Solution       Solution       Solution       Solution       Solution       Solution       Solution       Solution       Weight: 32.lb         Solution       Solution<

April 23,2025

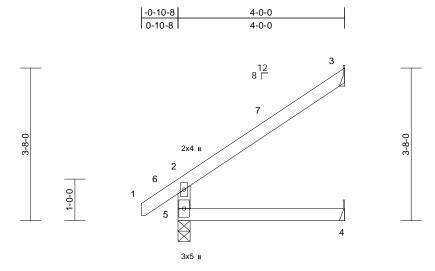
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and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E6	Jack-Open	24	1	Job Reference (optional)	172941933

#### Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:33 ID:CGiMI1FV1yQss\_ZnqL7APczODor-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





4-0-0

#### Scale = 1:27.7

00010 = 1.27.7							·						
Loading TCLL (roof) Snow (Pf/Pg) TCDL	(psf) 20.0 13.9/20.0 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES		CSI TC BC WB	0.34 0.23 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.02 -0.02 -0.02	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCLL BCDL	0.0* 10.0	Code	IRC2021	/TPI2014	Matrix-MR							Weight: 16 lb	FT = 20%
	5=0-3-8 Max Horiz 5=73 (LC Max Uplift 3=-46 (LC Max Grav 3=108 (LC 5=218 (LC	cept end verticals. applied or 10-0-0 o anical, 4= Mechanica 13) C 13) C 29), 4=47 (LC 29), C 2)	ed or 6) 7) c al, LO	on the botton 3-06-00 tall I chord and and Bearings are Refer to gird Provide med	has been designed m chord in all areas by 2-00-00 wide wil ny other members. a assumed to be: , , ler(s) for truss to tru- hanical connection a capable of withsta Standard	s where Il fit betv Joint 5 \$ Jss coni (by oth	a rectangle veen the bott SP No.2 . nections. ers) of truss	to					
FORCES	(lb) - Maximum Com Tension 2-5=-191/98, 1-2=0/												
BOT CHORD	4-5=0/0	11, 2 0= 01/00											
Vasd=103 II; Exp B; E Exterior(2E zone; cant and right e	CE 7-16; Vult=130mph imph; TCDL=6.0psf; B Enclosed; MWFRS (er E) -0-10-0 to 2-2-0, Int liever left and right ex exposed;C-C for memt or reactions shown; Lu	CDL=6.0psf; h=25ft; hvelope) and C-C erior (1) 2-2-0 to 3-1 posed ; end vertical bers and forces &	1-4 left								New York	OR CASE	ROCKAT

- grip DOL=1.33 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live 3) load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

and some solution of the second se SEAL 28677 L. GA (((()))))))

April 23,2025



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E7	Jack-Open	3	1	Job Reference (optional)	172941934

4-0-0 4-0-0

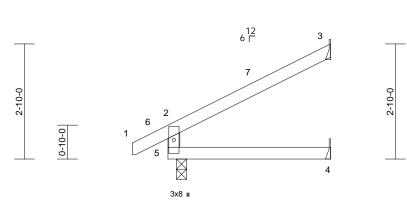
-0-10-8 0-10-8

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:33 ID:CGiMI1FV1yQss\_ZnqL7APczODor-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1







Scale = 1:28.4

Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15	CSI TC	0.29	DEFL Vert(LL)	in 0.01	(loc) 4-5	l/defl >999	L/d 240	PLATES MT20	<b>GRIP</b> 244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.29	Vert(CT)	-0.02	4-5	>999	180	101120	244/130
TCDL	10.0/20.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR		- (- /						
BCDL	10.0										Weight: 15 lb	FT = 20%
BOT CHORD WEBS BRACING TOP CHORD BOT CHORD REACTIONS	5=0-3-0	cept end verticals. applied or 10-0-0 of nical, 4= Mechanica	on the bo 3-06-00 t chord an 6) Bearings ed or 7) Refer to 8) Provide r bearing p 3.	ss has been designe ttom chord in all are all by 2-00-00 wide w d any other members are assumed to be: jirder(s) for truss to t echanical connecti late capable of withs (S) Standard	as where vill fit betv s. , Joint 5 \$ truss conr on (by oth	a rectangle veen the bott SP No.2 . nections. ers) of truss	om					
I	Max Horiz 5=55 (LC Max Uplift 3=-35 (LC Max Grav 3=127 (LC 5=266 (LC	5 15) C 22), 4=45 (LC 22),										
FORCES	(lb) - Maximum Com Tension	pression/Maximum										
TOP CHORD BOT CHORD	2-5=-236/137, 1-2=0 4-5=0/0	/33, 2-3=-75/50										
NOTES	100,0											
<ol> <li>Wind: ASC Vasd=103r II; Exp B; E Exterior(2E 3-11-4 zon- vertical left forces &amp; M DOL=1.60</li> <li>TCLL: ASC Plate DOL= DOL=1.15 Exp.; Ce=0</li> <li>Unbalance design.</li> <li>This truss b load of 12.0</li> </ol>	E 7-16; Vult=130mph mph; TCDL=6.0psf; B Enclosed; MWFRS (er E) -0-10-1 to 2-1-15, Ir e; cantilever left and r t and right exposed;C- WFRS for reactions s plate grip DOL=1.33 CE 7-16; Pr=20.0 psf ( Plate DOL=1.15); Is= 0.9; Cs=1.00; Ct=1.10 d snow loads have be has been designed fo 0 psf or 2.00 times fla non-concurrent with o	CDL=6.0psf; h=25ft; velope) and C-C iterior (1) 2-1-15 to ight exposed; end C for members and hown; Lumber roof LL: Lum DOL=1 2f=13.9 psf (Lum 1.0; Rough Cat B; F een considered for th greater of min roof i roof load of 13.9 ps	1.15 ully nis							State State	SEA 286	EER. Stummer

818 Soundside Road Edenton, NC 27932

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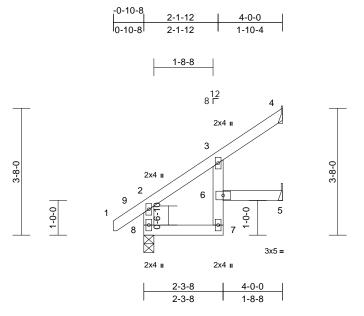


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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof			
25040187	E8	Jack-Open	4	1	Job Reference (optional)	172941935		

### Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:33 ID:CGiMI1FV1yQss\_ZnqL7APczODor-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



#### Scale = 1:33.3

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MR	0.15 0.19 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.02 -0.02 -0.01	(loc) 7 7 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 19 lb	<b>GRIP</b> 244/190 FT = 20%
	4-0-0 oc purlins, ex Rigid ceiling directly bracing. (size) 4= Mecha 8=0-3-8 Max Horiz 8=73 (LC Max Uplift 4=-30 (LC	athing directly applied cept end verticals. applied or 10-0-0 oc anical, 5= Mechanical, 13) C 13), 5=-7 (LC 13) 29), 5=62 (LC 29), 8=	on the bot 3-06-00 ta chord and 5) Bearings a d or 6) Refer to g 7) Provide m bearing pl 4 and 7 lb LOAD CASE(	s has been designe tom chord in all area Il by 2-00-00 wide w any other members are assumed to be: , irder(s) for truss to tr echanical connectio ate capable of withs uplift at joint 5. S) Standard	as where vill fit betw Joint 8 \$ russ conr n (by oth	a rectangle ween the both SP No.2 . nections. iers) of truss t	om to					
TOP CHORD	Tension 2-8=-198/85, 1-2=0/ 3-4=-61/59											
BOT CHORD		/31, 3-6=-21/52, 5-6=	0/0									
Vasd=103r II; Exp B; E Exterior(2E 3-11-4 zon vertical left forces & M DOL=1.60 2) TCLL: ASC Plate DOL= DOL=1.15 Exp.; Ce=C 3) This truss I load of 12.1	Enclosed; MWFRS (er Enclosed; MWFRS (er E) -0-10-0 to 2-0-12, Ir e; cantilever left and r wFRS for reactions s plate grip DOL=1.33 CE 7-16; Pr=20.0 psf ( =1.15); Pg=20.0 psf; F Plate DOL=1.15); Is= Plate DOL=1.15); Is=	CDL=6.0psf; h=25ft; C nvelope) and C-C tterior (1) 2-0-12 to right exposed; end C for members and hown; Lumber roof LL: Lum DOL=1. Pf=13.9 psf (Lum 1.0; Rough Cat B; Ful r greater of min roof lir t roof load of 13.9 psf	15 Ily ve							-	SEA 286	EEP Stur

GA mm April 23,2025

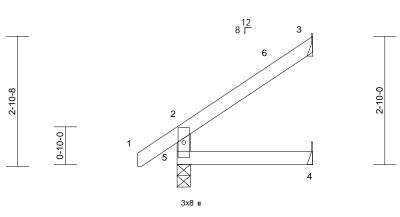
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Schut Information, purplication component of component development properties. and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss Truss Type Qty Ply 18 Eagle Creek - Edisto P				18 Eagle Creek - Edisto B - Roof	
25040187	E9	Jack-Open	5	1	Job Reference (optional)	172941936

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:33 ID:CGiMI1FV1yQss\_ZnqL7APczODor-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Page: 1



3-0-0

Scale	- 1	.25	5

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL	(psf) 20.0 13.9/20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-MR	0.20 0.12 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.01 -0.01 -0.01	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0											Weight: 12 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS BRACING TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she 3-0-0 oc purlins, ex Rigid ceiling directly bracing.	cept end verticals.	5) ed or 6) 7)	on the bottor 3-06-00 tall b chord and ar Bearings are Refer to girde Provide mec bearing plate 3.	has been designe n chord in all area by 2-00-00 wide w by other members assumed to be: , er(s) for truss to t hanical connectio c capable of withs	as where vill fit betw 5. , Joint 5 \$ russ conr on (by oth	a rectangle veen the bott SP No.2 . nections. ers) of truss	om to					
	(size) 3= Mecha 5=0-3-8 Max Horiz 5=57 (LC Max Uplift 3=-34 (LC Max Grav 3=77 (LC (LC 2)	( 13)	",	AD CASE(S)	Standard								
FORCES	(lb) - Maximum Com Tension 2-5=-162/95, 1-2=0/												
BOT CHORD	4-5=0/0												
Vasd=103i II; Exp B; E Exterior(2) zone; canti and right e MWFRS fc grip DOL= 2) TCLL: ASC Plate DOL- DOL=1.15 Exp.; Ce=(	CE 7-16; Vult=130mph mph; TCDL=6.0psf; B Enclosed; MWFRS (er E) -0-10-0 to 2-2-0, Int illever left and right exp exposed; C-C for memb or reactions shown; Lu 1.33 CE 7-16; Pr=20.0 psf ( =1.15); Pg=20.0 psf; F Plate DOL=1.15); Is= 0.9; Cs=1.00; Ct=1.10	CDL=6.0psf; h=25ft; ivelope) and C-C erior (1) 2-2-0 to 2-1 porsed; end vertical bers and forces & imber DOL=1.60 pla roof LL: Lum DOL=1 7f=13.9 psf (Lum 1.0; Rough Cat B; Fi	1-4 eft .15 ully								N. N. N.	OR FERS	ROMAR

This truss has been designed for greater of min roof live 3) load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

ON L. GAL mmm

April 23,2025



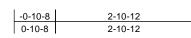
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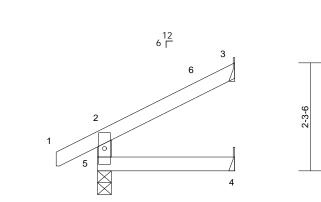
Job	Truss Truss Type Qty Ply 18 Eagle Creek - Edisto B - R		18 Eagle Creek - Edisto B - Roof			
25040187	E10	Jack-Open	3	1	Job Reference (optional)	172941937

2-3-6

0-10-0

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:33 ID:k38\_YhFtGeH0Fq\_bGecxtOzODos-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





2-10-12



Scale = 1:24.3

.oading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
CLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	0.00	4-5	>999	240	-	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	0.00	4-5	>999	180	_	
CDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 11 lb	FT = 20%
UMBER OP CHORD	2x4 SP No.2		on the b	uss has been design ottom chord in all ar	eas where	a rectangle	•					
OT CHORD	2x4 SP No.2			tall by 2-00-00 wide		veen the bott	om					
/EBS	2x4 SP No.3			nd any other membe are assumed to be		P No 2						
	o		7) D-4	girder(s) for truss to								
OP CHORD	Structural wood she 2-10-12 oc purlins, Rigid ceiling directly	except end verticals	s. 8) Provide	mechanical connect	tion (by oth	ers) of truss						
	bracing.	applied of 10-0-0 0	3.		0	•						
EACTIONS	(size) 3= Mecha 5=0-3-8	anical, 4= Mechanica	al, LOAD CAS	E(S) Standard								
	Max Horiz 5=41 (LC	12)										
	Max Uplift 3=-26 (LC	2 15)										
	Max Grav 3=82 (LC (LC 22)	22), 4=29 (LC 22),	5=209									
ORCES	(lb) - Maximum Com Tension	pression/Maximum										
OP CHORD	2-5=-184/117, 1-2=0 4-5=0/0	)/33, 2-3=-52/35										
IOTES												
) Wind: ASC	CE 7-16; Vult=130mph	(3-second gust)										
	mph; TCDL=6.0psf; B		; Cat.									1111
	Enclosed; MWFRS (er										WHY CA	APOUL
	E) -0-10-1 to 2-1-15, Ir ne; cantilever left and r									N	A 1.1.	PULL
	t and right exposed;C-									5.	O'FESS	Kill in
	WFRS for reactions s									32		112.7 -
	plate grip DOL=1.33								1		Render	14: 3
	CE 7-16; Pr=20.0 psf (	roof LL: Lum DOL=	1.15								SEA	u : =
Plate DOL	=1.15); Pg=20.0 psf; F	Pf=13.9 psf (Lum							=		JL/	
	Plate DOL=1.15); Is=		Fully						=		286	// : :
	0.9; Cs=1.00; Ct=1.10										<b>1</b>	1 2
) Unbalance design.	ed snow loads have be	en considered for t	nis							2	1. 0	als S
	has been designed fo	r areater of min roof	live							1,4	SEA 286	EENA
	.0 psf or 2.00 times fla									11	YA	in Si'
	non-concurrent with o										11. L.G	AL
											L. G	mm
											A	1 22 2025

April 23,2025

Page: 1



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E11	Jack-Open	1	1	Job Reference (optional)	172941938

2-10-12

12 6 Г

Carter Components (Sanford, NC), Sanford, NC - 27332,

2-3-6

0-10-0

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:33 ID:8ep6AjHIZZga6Ij9ym9eU1zODop-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

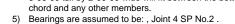
2

Page: 1

2-3-6 1 4 3 3x8 II 2-10-12

Scale = 1:21.6

	-		1							i	
Loading (p	) Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20	· · · ·	1.15	тс	0.17	Vert(LL)	0.00	3-4	>999	240	MT20	244/190
Snow (Pf/Pg) 13.9/20		1.15	BC	0.10	. ,	-0.01	3-4	>999	180		
TCDL 1		YES	WB	0.00	Horz(CT)	-0.01	2	n/a	n/a		
BCLL	0* Code	IRC2021/TPI2014	Matrix-MR		- (- )						
BCDL 1	0									Weight: 10 lb	FT = 20%
2-10-12 oc pur BOT CHORD Rigid ceiling di bracing. <b>REACTIONS</b> (size) 2= N 4=0 Max Horiz 4=3 Max Uplift 2=-2	(LC 12)	7) Provide med bearing plat 2. LOAD CASE(S) ied or s. bc	ler(s) for truss to tru chanical connection e capable of withsta Standard	n (by oth	ers) of truss						
Max Grav 2=8 (LC		4=119									
FORCES (lb) - Maximum Tension	Compression/Maximum	I									
TOP CHORD 1-4=-99/50, 1-2	-54/37										
BOT CHORD 3-4=0/0											
NOTES											
<ol> <li>Wind: ASCE 7-16; Vult=13 Vasd=103mph; TCDL=6.0 II; Exp B; Enclosed; MWFF Exterior(2E) zone; cantilev vertical left and right expos forces &amp; MWFRS for reacti DOL=1.60 plate grip DOL=</li> <li>TCLL: ASCE 7-16; Pr=20.0 Plate DOL=1.15); Pg=20.0 DOL=1.15 Plate DOL=1.15 Exp.; Ce=0.9; Cs=1.00; Ct:</li> <li>Unbalanced snow loads ha design.</li> <li>* This truss has been desig on the bottom chord in all a 3-06-00 tall by 2-00-00 wid chord and any other memb</li> <li>Bearings are assumed to b</li> </ol>	f; BCDL=6.0psf; h=25ff (envelope) and C-C left and right exposed d;C-C for members and as force for members and as f (roof LL: Lum DOL= sf; Pf=13.9 psf (Lum Is=1.0; Rough Cat B; f .10 e been considered for t ed for a live load of 20. eas where a rectangle will fit between the bott 's.	; end I 1.1.15 Fully his Opsf							and states of the states of th	SEA 2867	EER. St.



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April 23,2025

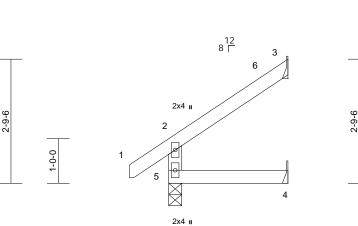


Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof		
25040187	E12	Jack-Open	3	1	Job Reference (optional)	172941939	

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:33 ID:k38\_YhFtGeH0Fq\_bGecxtOzODos-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



2-8-1



Scale = 1:25.8

Scale = 1:25.8												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MR	0.20 0.12 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 -0.01	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 11 lb	<b>GRIP</b> 244/190 FT = 20%
I	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she 2-8-1 oc purlins, ex Rigid ceiling directly bracing. (size) 3= Mecha 5=0-3-8 Max Horiz 5=50 (LC Max Grav 3=68 (LC (LC 2)	cept end verticals. applied or 10-0-0 o inical, 4= Mechanica 10) : 13)	ed or c bearings c c al, <b>b</b> ootheat c <b>b</b> ootheat c <b>b</b> ootheat <b>b</b> o	ss has been designe ittom chord in all are all by 2-00-00 wide v d any other member are assumed to be: girder(s) for truss to nechanical connection late capable of withs (S) Standard	as where will fit betw s. , Joint 5 \$ truss conr on (by oth	a rectangle veen the bott SP No.2 . nections. ers) of truss	tom					
FORCES TOP CHORD BOT CHORD	(lb) - Maximum Com Tension 2-5=-151/89, 1-2=0/ 4-5=0/0											
NOTES 1) Wind: ASC Vasd=1037 II; Exp B; E Exterior(2E zone; canti and right ex MWFRS fo grip DOL=1 2) TCLL: ASC Plate DOL= DOL=1.15 Exp.; Ce=0 3) This truss f load of 12.0	E 7-16; Vult=130mph mph; TCDL=6.0psf; Br Inclosed; MWFRS (er E) -0-10-0 to 2-2-0, Int ilever left and right exj xposed;C-C for memb r reactions shown; Lu	CDL=6.0psf; h=25ft; ivelope) and C-C erior (1) 2-2-0 to 2-7 bosed ; end vertical bers and forces & mber DOL=1.60 pla roof LL: Lum DOL= Pf=13.9 psf (Lum 1.0; Rough Cat B; F r greater of min roof t roof load of 13.9 ps	r-5 left 1.15 fully							and	SEA 286	EER. St.

April 23,2025

Page: 1



Job	Truss	Truss Type	ss Type Qty Ply 18 Eagle Creek - Edisto B - Roof			
25040187	E13	Jack-Open	1	1	Job Reference (optional)	172941940

-0-10-8

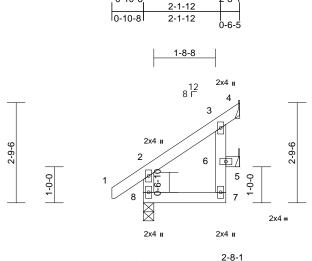
Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8,73 S Feb 19 2025 Print: 8,730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:33 ID:k38\_YhFtGeH0Fq\_bGecxtOzODos-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

-8-

Page: 1







Scale = 1:32

Scale = 1.52												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MR	0.14 0.09 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 7-8 6 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 14 lb	<b>GRIP</b> 244/190 FT = 20%
	2x4 SP No.2 2x4 SP No.2 *Excep 2x4 SP No.3 Structural wood she 2-8-1 oc purlins, exx Rigid ceiling directly bracing. (size) 4= Mecha 8=0-3-8 Max Horiz 8=50 (LC Max Uplift 5=-31 (LC Max Grav 4=34 (LC	athing directly applie cept end verticals. applied or 10-0-0 or inical, 5= Mechanica 10) : 13)	on the t 3-06-00 chord a 5) Bearing ed or 6) Refer to 6) Refer to bearing 5. al, LOAD CAS	uss has been design oottom chord in all are tall by 2-00-00 wide nd any other member s are assumed to be: girder(s) for truss to mechanical connecti plate capable of with <b>E(S)</b> Standard	eas where will fit betw s. , Joint 8 s truss conr on (by oth	a rectangle veen the both SP No.2 . nections. ers) of truss t	om to					
FORCES	(LC 2) (lb) - Maximum Com Tension 2-8=-152/83, 1-2=0/4	41, 2-3=-63/7, 3-4=-										
BOT CHORD	7-8=-47/38, 6-7=-12	/30, 3-6=-63/95, 5-6	6=0/0									
NOTES		(2)										
Vasd=103/ II; Exp B; E Exterior(2E 2-7-5 zone vertical left forces & M DOL=1.60 2) TCLL: ASC Plate DOL: DOL=1.15 Exp.; Ce=C 3) This truss i load of 12.	CE 7-16; Vult=130mph mph; TCDL=6.0psf; Bf Enclosed; MWFRS (en E) -0-10-0 to 2-0-12, In ; cantilever left and rigt t and right exposed;C- IWFRS for reactions s plate grip DOL=1.33 CE 7-16; Pr=20.0 psf; F Plate DOL=1.15); Is= 0.9; Cs=1.00; Ct=1.10 has been designed for 0 psf or 2.00 times flat is non-concurrent with c	CDL=6.0psf; h=25ft; ivelope) and C-C iterior (1) 2-0-12 to ght exposed ; end C for members and hown; Lumber roof LL: Lum DOL= of=13.9 psf (Lum 1.0; Rough Cat B; F r greater of min roof t roof load of 13.9 ps	1.15 'ully live							Super States	SEA 286	ROUTE EER SCL

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818 Soundside Road Edenton, NC 27932

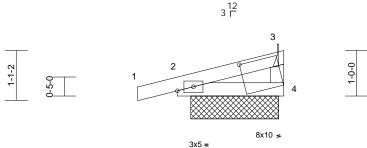
L. GA minin April 23,2025

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E14	Jack-Open	1	1	I7: Job Reference (optional)	2941941

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:33 ID:biF0qZZ09?X8eo4\_5NnRgJztCnW-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

-0-10-8 2-4-0 0-10-8 2-4-0



0-3-8		2-4-0
	2-2-10	
	1-11-2	
0-3-8		0-1-5

Scale = 1:25.3

Plate Offsets (X, Y): [4:1-5-7,0-2-11]

	7, 1). [4.1-3-7,0-2-11]												
Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-MP	0.06 0.07 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4-9 4-9 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 9 lb	<b>GRIP</b> 244/190 FT = 20%
	2x4 SP No.2 2x4 SP No.3 Structural wood shea 2-4-0 oc purlins, exc Rigid ceiling directly bracing.	cept end verticals. applied or 10-0-0 oc 3= Mechanical, 4=1 14) 11), 3=-7 (LC 15)	; -11-2 9) 10	load of 12.0 overhangs n Gable studs * This truss ł on the bottor 3-06-00 tall ł chord and ar Bearings are Refer to gird D) Provide mec bearing plate 2, 7 lb uplift :	is been designed psf or 2.00 times 1 on-concurrent with spaced at 2-0-0 o nas been designed in chord in all area by 2-00-00 wide w ny other members assumed to be: , re(s) for truss to tr hanical connectio o capable of withs at joind 3 and 31 ll d Industry Piggyb nnection to base 1	ilat roof lin n other lin c. d for a liv s where ill fit betw Joint 2 S russ conr n (by oth tanding 3 o uplift at ack Trus	bad of 13.9 p ve loads. e load of 20.1 a rectangle veen the bott SP No.2 . tections. ers) of truss t i 1 b uplift at j joint 2. s Connection	sf on Opsf om to joint					
FORCES	(lb) - Maximum Com Tension	pression/Maximum		consult quali	fied building desig								
TOP CHORD BOT CHORD	1-2=0/16, 2-3=-148/ 2-4=-110/177	111, 3-4=0/0	L	DAD CASE(S)	Standard								
Vasd=103 II; Exp B; F (3E) zone; left and rig MWFRS fr grip DOL= 2) Truss desi only. For see Stand. or consult 3) TCLL: ASS Plate DOL DOL=1.15 Exp.; Ce=	CE 7-16; Vult=130mph mph; TCDL=6.0psf; Bd Enclosed; MWFRS (en ; cantilever left and righ the exposed;C-C for me or reactions shown; Lu 1.33 igned for wind loads in studs exposed to wind ard Industry Gable End qualified building desig CE 7-16; Pr=20.0 psf ( .=1.15); Pg=20.0 psf; [ Plate DOL=1.15); Is= 0.9; Cs=1.00; Ct=1.10 ad snow loads have be	CDL=6.0psf; h=25ft; velope) and C-C Co texposed; end veri embers and forces & mber DOL=1.60 plat the plane of the trus (normal to the face) d Details as applicab gner as per ANSI/TP roof LL: Lum DOL=1 2f=13.9 psf (Lum 1.0; Rough Cat B; Fu	rner tical te ss , ole, vl 1. .15 ully								and a state of the	SEA 286	EEP. Stur

April 23,2025



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E15	Jack-Open	4	1	Job Reference (optional)	172941942

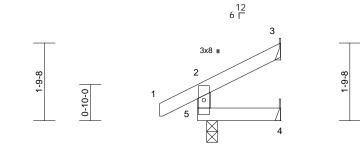
-0-10-8

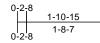
0-10-8

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:33 ID:k38\_YhFtGeH0Fq\_bGecxtOzODos-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





1-10-15

1-10-15

Scale = 1:26.6

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	<b>Spacing</b> Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MR	0.10 0.05 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 8 lb	<b>GRIP</b> 244/190 FT = 20%
	$\begin{array}{l} 2x4 \ \text{SP No.2} \\ 2x4 \ \text{SP No.2} \\ 2x4 \ \text{SP No.3} \\ \\ \hline \\ \text{Structural wood she} \\ 1-10-15 \ \text{oc purlins,} \\ \text{Rigid ceiling directly} \\ \text{bracing.} \\ (size) \qquad 3= \text{Mecha} \\ \hline \\ 5=0-3-0 \\ \\ \text{Max Horiz} \ 5=32 \ (\text{LC} \\ \text{Max Upliff} \ 3=-18 \ (\text{LC} \\ \text{Max Grav} \ 3=44 \ (\text{LC} \\ (\text{LC 22}) \end{array}$	except end verticals. applied or 10-0-0 oc nical, 4= Mechanica 12) : 15)	on the bott 3-06-00 tal chord and 6) Bearings a d or 7) Refer to gi 8) Provide mo bearing pla 3. LOAD CASE(\$	thas been designed om chord in all area by 2-00-00 wide wi any other members. re assumed to be: , der(s) for truss to tru- chanical connection te capable of withst b) Standard	s where Il fit betv Joint 5 S uss conr n (by oth	a rectangle veen the botto SP No.2 . nections. ers) of truss t	o					
FORCES	(Ib) - Maximum Com Tension	pression/Maximum										
TOP CHORD BOT CHORD	2-5=-145/98, 1-2=0/3 4-5=0/0	33, 2-3=-34/22										
Vasd=103i II; Exp B; F Exterior(2E vertical left forces & M DOL=1.60 2) TCLL: ASC Plate DOL DOL=1.15 Exp.; Ce=( 3) Unbalance design. 4) This truss load of 12.	CE 7-16; Vult=130mph mph; TCDL=6.0psf; BG Enclosed; MWFRS (er E) zone; cantilever left t and right exposed;C- IWFRS for reactions si plate grip DOL=1.33 CE 7-16; Pr=20.0 psf; F Plate DOL=1.15); Is= 0.9; Cs=1.00; Ct=1.10 d snow loads have be has been designed for 0 psf or 2.00 times flat non-concurrent with c	CDL=6.0psf; h=25ft; ivelope) and C-C and right exposed; C for members and hown; Lumber roof LL: Lum DOL=1 2f=13.9 psf (Lum 1.0; Rough Cat B; Fu en considered for th greater of min roof 1 toof load of 13.9 ps	end .15 ully is live							and a start of the	SEA 286	TT EER Sturr

# April 23,2025



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E16	Jack-Open	1	1	Job Reference (optional)	172941943

-0-10-0

0-10-0

1

1-7-6

1-7-6

8 Г

3

Carter Components (Sanford, NC), Sanford, NC - 27332,

1-11-1

0-9-11

Run: 8.73 E May 9 2024 Print: 8.730 E May 9 2024 MiTek Industries, Inc. Wed Apr 23 16:12:10 ID:ZhSv\_xhEnnB8C8Y6fw7YTkztCyz-1743ugOCcLVT7IO\_ooYnVhc1dZxXAI15dwYPEGzNoRZ

Page: 1

1-10-9 2 Δ

# 3x8 II

1-7-6

#### Scale = 1:23.4

Loading TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	(psf) 20.0 13.9/20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	<b>CSI</b> TC BC WB Matrix-MR	0.12 0.05 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4-5 4-5 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 8 lb	<b>GRIP</b> 244/190 FT = 20%
	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she 1-7-6 oc purlins, ex Rigid ceiling directly bracing. (Ib/size) 3=24/ Me 5=108/1-6 Max Horiz 5=35 (LC Max Upliff 3=-19 (LC Max Grav 3=34 (LC (LC 2)	cept end verticals. applied or 10-0-0 oc chanical, 4=12/1-6-1 5-14 10) : 13)	<ul> <li>6) Provide mec bearing plate 3.</li> <li>LOAD CASE(S)</li> <li>d or</li> <li>4,</li> </ul>	er(s) for truss to tru hanical connection a capable of withsta Standard	ı (by oth	ers) of truss t						
Vasd=103r II; Exp B; E Exterior(2E vertical left forces & M DOL=1.60 2) TCLL: ASC Plate DOL= DOL=1.15 Exp.; Ce= 3) This truss I load of 12. overhangs 4) * This truss on the bott 3-06-00 tal	(lb) - Maximum Com Tension 2-5=-123/85, 1-2=0/ 4-5=0/0 ZE 7-16; Vult=130mph mph; TCDL=6.0psf; Bi Enclosed; MWFRS (er 2) zone; cantilever left: a and right exposed;C- WFRS for reactions s plate grip DOL=1.33 CE 7-16; Pr=20.0 psf; [ =1.15); Pg=20.0 psf; [ Plate DOL=1.15); Is= Digt or 2.00 times flat non-concurrent with of s has been designed for om chord in all areas II by 2-00-00 wide will any other members.	39, 2-3=-38/26 (3-second gust) CDL=6.0psf; h=25ft; ivelope) and C-C and right exposed ; C for members and hown; Lumber roof LL: Lum DOL=1 Pf=13.9 psf (Lum 1.0; Rough Cat B; Fi r greater of min roof t roof load of 13.9 ps ther live loads. or a live load of 20.0 where a rectangle	end .15 ully live f on psf								SEA 286	EEP. St.

April 23,2025



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# F

- 2

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E17	Jack-Open	1	1	Job Reference (optional)	172941944

2-4-6

2-4-6

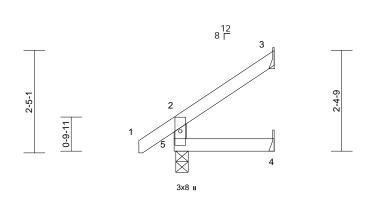
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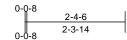
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:34 ID:ZhSv\_xhEnnB8C8Y6fw7YTkztCyz-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:27.2

					_							
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0								-		Weight: 10 lb	FT = 20%
LUMBER			, 0	are assumed to be:								
TOP CHORD	2x4 SP No.2			rder(s) for truss to								
BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.3			echanical connection ate capable of withs								
	2X4 SP IN0.3		3.	ate capable of with	stanuing 2	27 ib upint at j	om					
BRACING TOP CHORD	Structural wood she	athing directly appli		S) Standard								
	2-4-6 oc purlins, ex			-,								
BOT CHORD	Rigid ceiling directly		C									
	bracing.											
REACTIONS	( )	anical, 4= Mechanic	al,									
	5=0-3-8	10)										
	Max Horiz 5=46 (LC Max Uplift 3=-27 (LC	,										
	Max Grav 3=58 (LC		5=155									
	(LC 2)	20), 120 (20 11),	0-100									
FORCES	(lb) - Maximum Con Tension	npression/Maximum										
TOP CHORD	2-5=-140/86, 1-2=0/	/39, 2-3=-55/38										
BOT CHORD	4-5=0/0											
NOTES												
	CE 7-16; Vult=130mph											
	mph; TCDL=6.0psf; B		; Cat.								minin	11111
	Enclosed; MWFRS (er E) zone; cantilever left		ond								"TH CA	Rolly
	t and right exposed;C									Nº.	Ricit	Kik's
	IWFRS for reactions s									22	FFOS	Chilles -
	plate grip DOL=1.33									2	4/1	UN: 2
	CE 7-16; Pr=20.0 psf		1.15								per l	1 N N E
	=1.15); Pg=20.0 psf; I								=		SEA	AL : =
	5 Plate DOL=1.15);		ully						=		286	77 : 5
	has been designed fo		live								200	11 E E
	.0 psf or 2.00 times fla									-	N	1 3
	non-concurrent with									50	S. SNOW	FER. DS
	s has been designed t		Opsf							11	SEA 286	5.5. 5
	tom chord in all areas Il by 2-00-00 wide will		om							1	INI G	ALICIN
	any other members.	in between the bolt	UIII								11111	EER. KALING
	, saler memorie											1 00 0005
											Δnr	コンマンハント

April 23,2025



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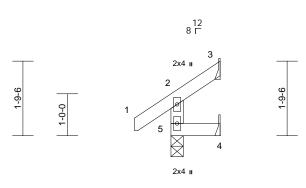
Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	E18	Jack-Open	4	1	Job Reference (optional)	172941945

-0-10-8 1-2-1 0-10-8 1-2-1

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:34 ID:CGiMI1FV1yQss\_ZnqL7APczODor-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:27.5

Loading TCLL (roof) Snow (Pf/Pg)	(psf) 20.0 13.9/20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI TC BC	0.14 0.05	DEFL Vert(LL) Vert(CT)	in 0.00 0.00	(loc) 4-5 4-5	l/defl >999 >999	L/d 240 180	PLATES MT20	<b>GRIP</b> 244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								<b>FT</b> 000/
BCDL	10.0						-	-	-		Weight: 6 lb	FT = 20%
UMBER				s are assumed to be:								
TOP CHORD	2x4 SP No.2			girder(s) for truss to								
BOT CHORD	2x4 SP No.2		,	mechanical connecti plate capable of with		,						
BRACING	2x4 SP No.3			b uplift at joint 3.	stanuing t	o io upint at jo	IIII 4					
TOP CHORD	Structural wood she	athing directly appli		E(S) Standard								
	1-2-1 oc purlins, ex			(-)								
BOT CHORD	Rigid ceiling directly	applied or 10-0-0 o	C									
	bracing.											
REACTIONS	· /	anical, 4= Mechanic	al,									
	5=0-3-8 Max Horiz 5=33 (LC	10)										
	Max Uplift 3=-16 (LC											
	Max Grav 3=16 (LC		5=129									
	(LC 19)											
FORCES	(lb) - Maximum Com	npression/Maximum										
TOP CHORD	Tension 2-5=-120/88, 1-2=0/	41 2-329/23										
BOT CHORD	4-5=0/0	-1, 2 0= 25/25										
NOTES												
	CE 7-16; Vult=130mph	(3-second gust)										
	mph; TCDL=6.0psf; B		; Cat.									1111
	Enclosed; MWFRS (er										W'TH C	ARO
	E) zone; cantilever left t and right exposed;C-									N	R	
	WFRS for reactions s		1							2.5	O FO	State View
	plate grip DOL=1.33									32	4/	1. 7. 7 -
	CE 7-16; Pr=20.0 psf (		1.15								:4	1. 2
	=1.15); Pg=20.0 psf; I								-		SE	AL 🗄 🗄
	Plate DOL=1.15); ls= 0.9; Cs=1.00; Ct=1.10		-ully						=	:	206	
	has been designed fo		flive								200	11 i E
	.0 psf or 2.00 times fla									1	N	1 2
	non-concurrent with									30	S.ENO.	-ERIL S
	s has been designed f		0psf							11	OV GIV	EF. ST.
	tom chord in all areas Il by 2-00-00 wide will		om							1	SEA SEA SEA SEA SEA SEA SEA SEA SEA SEA	ALIMIN
	any other members.	in between the bott	UIII								11111	AL VEER ST.
	,											
											AD	FILZ 3 2025

April 23,2025



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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	V1	Valley	1	1	Job Reference (optional)	172941946

3x5: 6

10-2-10

10-2-10

Carter Components (Sanford, NC), Sanford, NC - 27332,

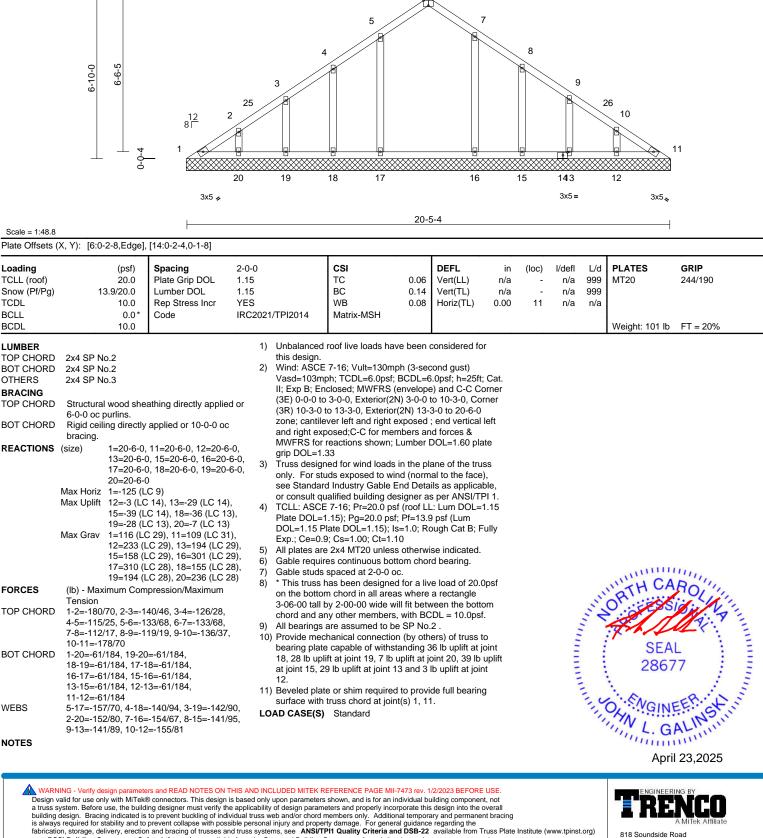
Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:34 ID:gSGkzNG7oFYjU88zO2ePypzODoq-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

20-0-1

9-9-7

Page: 1

Edenton, NC 27932

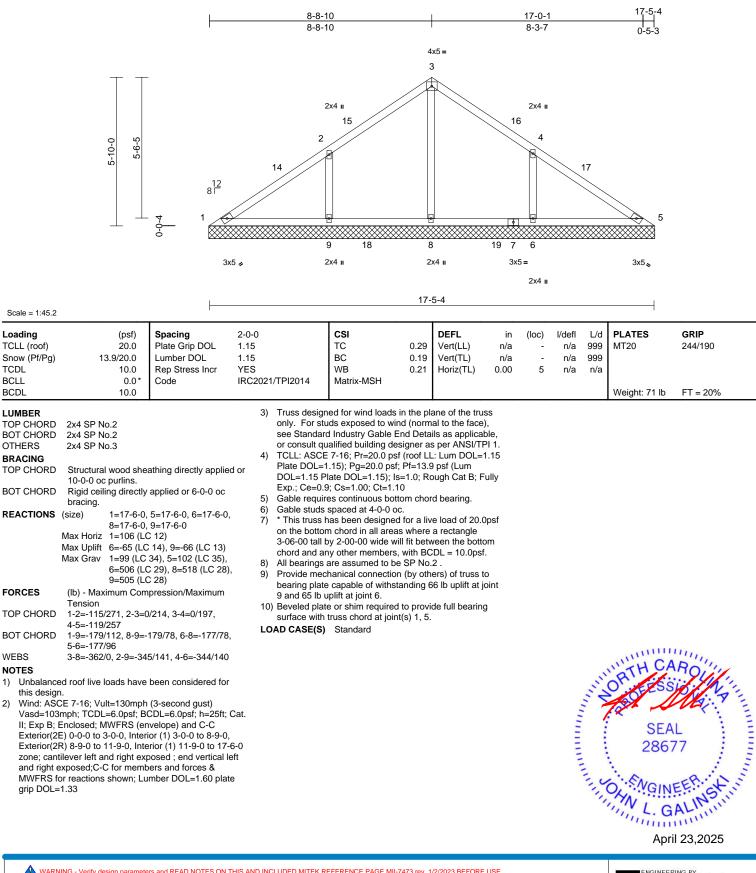


and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	V2	Valley	1	1	Job Reference (optional)	172941947

2)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:34 ID:gSGkzNG7oFYjU88zO2ePypzODoq-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



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818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof
25040187	V3	Valley	1	1	I72941948 Job Reference (optional)

Loading

TCDL

BCLL

BCDL

LUMBER

OTHERS

FORCES

WEBS

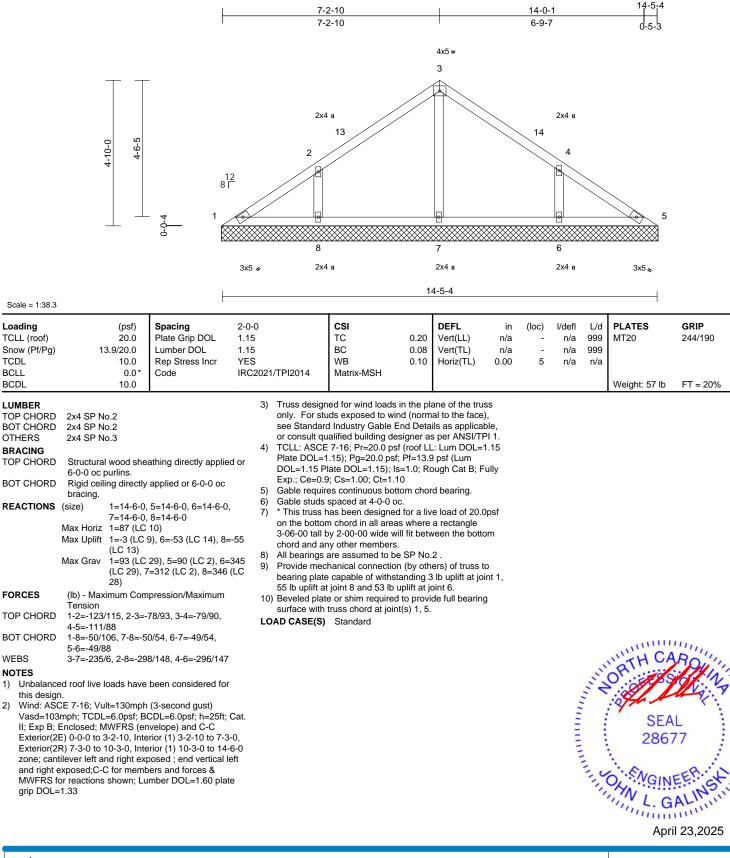
NOTES

1)

2)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:34 ID:gSGkzNG7oFYjU88zO2ePypzODoq-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



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818 Soundside Road

Edenton, NC 27932

Martin Hall

Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	V4	Valley	1	1	Job Reference (optional)	172941949

1)

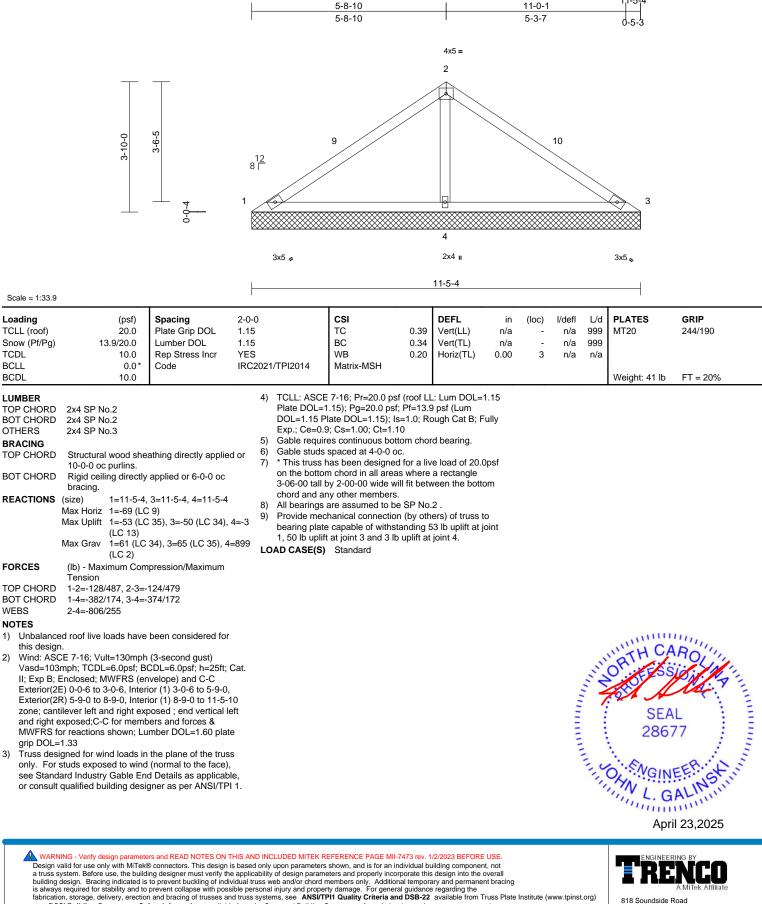
2)

3)

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries. Inc. Tue Apr 22 13:52:34 ID:gSGkzNG7oFYjU88zO2ePypzODoq-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1

11-5-4

Edenton, NC 27932



and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

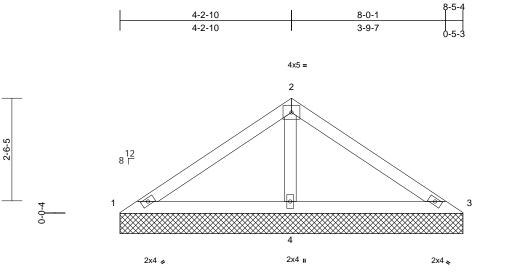
Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	V5	Valley	1	1	Job Reference (optional)	172941950

2-10-0

## Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:34 ID:8ep6AjHIZZga6Ij9ym9eU1zODop-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



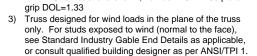
Page: 1



8-5-4

Scale =	1.28.3	

Loading TCLL (roof) Snow (Pf/Pg)	(psf) 20.0 13.9/20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI TC BC	0.23 0.21	<b>DEFL</b> Vert(LL) Vert(TL)	in n/a n/a	(loc) - -	l/defl n/a n/a	L/d 999 999	PLATES MT20	<b>GRIP</b> 244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.10	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 29 lb	FT = 20%
BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	Max Horiz 1=50 (LC Max Uplift 1=-19 (LC Max Grav 1=65 (LC (LC 2)	applied or 6-0-0 oc 3=8-5-4, 4=8-5-4 12) 2 35), 3=-16 (LC 34) 34), 3=69 (LC 35), 4	d or 7) * This truss on the botto 3:06-00 tall chord and a 8) All bearings 9) Provide mer bearing plat	E 7-16; Pr=20.0 psf 1.15); Pg=20.0 psf; Plate DOL=1.15); Is- 9; Cs=1.00; Ct=1.11 res continuous bott spaced at 4-0-0 oc has been designed m chord in all areas by 2-00-00 wide wil ny other members. are assumed to be chanical connection e capable of withsta uplift at joint 3.	Pf=13.9 =1.0; Ro om chor c for a liv s where Il fit betw SP No. (by oth	<ul> <li>a) psf (Lum</li> <li>b) ugh Cat B; F</li> <li>c) d bearing.</li> <li>c) load of 20.0</li> <li>a) rectangle</li> <li>veen the botto</li> <li>2.</li> <li>ers) of truss t</li> </ul>	ully Opsf om o					
FORCES	(lb) - Maximum Corr Tension	pression/Maximum		Otandara								
TOP CHORD	1-2=-105/311, 2-3=-	102/304										
BOT CHORD	1-4=-265/157, 3-4=-											
WEBS	2-4=-519/202											
NOTES												
1) Unbalance	d roof live loads have	been considered for										
this design.											minin	1111.
Vasd=103r II; Exp B; E Exterior(2E Exterior(2R zone; cantil and right ex MWFRS fo grip DOL=1		CDL=6.0psf; h=25ft; hvelope) and C-C ior (1) 3-0-6 to 4-3-0, ior (1) 7-6-7 to 8-5-1 posed ; end vertical I bers and forces & imber DOL=1.60 plat	, 0 eft ie							Vin	SEA 2867	ROJANA NA L 77
only. For s	gned for wind loads in studs exposed to wind ard Industry Gable En qualified building desi	l (normal to the face) d Details as applicab	, ile,							and and a second second	OLYN L. G	EEP. St.





April 23,2025

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Job	Truss	Truss Type	Qty	Ply	18 Eagle Creek - Edisto B - Roof	
25040187	V6	Valley	1	1	Job Reference (optional)	172941951

2-8-10

2-8-10

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Tue Apr 22 13:52:34 ID:8ep6AjHIZZga6Ij9ym9eU1zODop-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

4x5 =

2

5-0-1

2-3-7

5-5-4

3

2x4 💊



þ 1 0-0-4 4 2x4 II 2x4 🍫 5-5-4

1-6-5

1-10-0

12 8 Г

Scale = 1:24.3

Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15	CSI TC	0.08	DEFL Vert(LL)	in n/a	(loc)	l/defl n/a	L/d 999	PLATES MT20	<b>GRIP</b> 244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.00	Vert(TL)	n/a	_	n/a	999	101120	244/130
TCDL	10.0/20.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0	1									Weight: 18 lb	FT = 20%
LUMBER				ids spaced at 4-0-0								
TOP CHORD	2x4 SP No.2		,	ss has been designe			Opsf					
BOT CHORD OTHERS	2x4 SP No.2 2x4 SP No.3			ttom chord in all are all by 2-00-00 wide		0	ന					
BRACING	214 37 110.3			d any other member		veen the bolt	5111					
TOP CHORD	Structural wood she	athing directly appli		gs are assumed to I		2.						
	5-5-4 oc purlins.		<ol><li>9) Provide ı</li></ol>	nechanical connection								
BOT CHORD	Rigid ceiling directly bracing.	applied or 6-0-0 oc	bearing p	late capable of with	standing 2	lb uplift at jo	int 3.					
REACTIONS	0	3=5-5-4, 4=5-5-4	LOAD CASE	(S) Standard								
	Max Horiz 1=-31 (LC	,		. ,								
	Max Uplift 3=-2 (LC											
	Max Grav 1=64 (LC	34), 3=67 (LC 35),	4=332									
	(LC 2)											
FORCES	(lb) - Maximum Con	pression/Maximum										
TOP CHORD	Tension 1-2=-60/129, 2-3=-6	6/122										
BOT CHORD	1-2=-00/129, 2-3=-0											
WEBS	2-4=-248/109	11/00										
NOTES												
1) Unbalance	ed roof live loads have	been considered for	or									
this design											MILLIN	11111
	CE 7-16; Vult=130mph		_								W'TH CA	ROUL
	mph; TCDL=6.0psf; B Enclosed; MWFRS (er		; Cat.							S	Rint	. Aller
	E) zone; cantilever left		. end							20	C APS	Ki Vit
	t and right exposed;C									5	AL A	1 7: 1 -
	IWFRS for reactions s	hown; Lumber									14	1 N N E
	plate grip DOL=1.33								=		SEA	
	igned for wind loads ir studs exposed to wind								=		286	77 : 5
	ard Industry Gable En										: 200	11 E E
	qualified building desi									-	N	1 3
	CE 7-16; Pr=20.0 psf (		1.15							20	OR TH CA SEA 286	FER. LS
	=1.15); Pg=20.0 psf; I									11	4 GIN	S.S.
	Plate DOL=1.15);		-uiiy							-		ALIM
	uires continuous botto										111111	11111
-,												

April 23,2025



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