#### GENERAL STRUCTURAL NOTES

1. GENERA

A. ALL CONSTRUCTION SHALL CONFORM WITH THE PROVISIONS OF THE 2018 INTERNATIONAL RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS.

C. THE CONTRACTOR SHALL PROVIDE ALL SHORING AND BRACING AS REQUIRED TO SUPPORT THE EXISTING STRUCTURE. THE CONTRACTOR SHALL EXAMINE THE EXISTING STRUCTURE TO DETERMINE THE EXTENT OF NECESSARY SHORING AND BRACING. THE CAPACITY AND METHOD USED FOR SHORING AND BRACING SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.

#### 2. FOUNDATIONS

A. FOOTINGS ARE DESIGNED FOR AN ALLOMABLE SOIL BEARING CAPACITY OF 2000 PSF.
FOOTINGS SHALL BEAR ON NATURAL UNDISTURBED SOIL, 1'-O" BELOM ORIGINAL GRADE.
THE BOTTOM OF EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 2'-8" BELOW FINISHED
GRADE. CONTRACTOR TO VERIFY THE ALLOMABLE SOIL PRESSURE IN THE FIELD. IF
FOUND TO BE LESS THAN 2000 PSF, THE FOOTINGS WILL HAVE TO BE REDESIGNED.

#### 3. CAST IN PLACE CONCRETE

A. ALL CONCRETE WORK SHALL CONFORM TO THE LATEST APPROVED (BY LOCAL GOVERNMENT) EDITIONS OF THE FOLLOWING A.C.I. AND A.S.T.M. DOCUMENTS:

CI-301 SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS
CI-318 BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE

B. ALL CONCRETE, EXCEPT AS NOTED, SHALL BE (F'C=3,000 PSI) STONE AGGREGATE CONCRETE AT 28 DAYS. ALL CONCRETE EXPOSED TO THE WEATHER SHALL BE AIR ENTRAINED.

C. SLABS ON GROUND SHALL BE 4" THICK CONCRETE REINFORCED WITH 6"X6" W1.4XW1.4 WWF OVER 6 MIL POLYETHYLENE VAPOR BARRIER AND 4" WASHED GRAVEL UNLESS OTHERWISE NOTED.

#### 4. MASONRY

- A. ALL MASONRY CONSTRUCTION AND MATERIALS USED THEREIN (CONCRETE MASONRY, CLAY MASONRY, MORTAR, GROUT AND STEEL REINFORCEMENT) SHALL CONFORM TO "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-92/ASCE 5-92/TMS 402-92) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1-92/ASCE 6-92/TMS 602-92) IN ALL RESPECTS.
- B. MASONRY BEARING WALLS SHALL CONSIST OF STANDARD HOLLOW UNITS CONFORMING TO ASTM C 90 UNLESS OTHERWISE NOTED. WHERE SOLID UNITS ARE REQUIRED, PROVIDE UNITS CONFORMING TO ASTM C 145.
- C. ALL MORTAR SHALL CONFORM TO THE REQUIREMENTS FOR PROPORTIONS, MIXING, STRENGTH AND APPLICATION FOR PORTLAND CEMENT/LIME TYPE "S" MORTAR AS DESCRIBED IN ACI 530-92.
- D. ALL GROUT FILL IN MASONRY WALLS SHALL CONFORM TO ASTM C 476. SLUMP RANGE 8-11". PLACE GROUT IN 5'-0" MAXIMUM POUR HEIGHTS AND CONSOLIDATE BY MECHANICAL VIBRATION.
- E. PROVIDE 8" DEPTH OF 140 % SOLID MASONRY BELOW ALL JOIST OR SLAB BEARING LINES. PROVIDE 16" HIGH X 16" LONG 100 % SOLID MASONRY BELOW ALL LINTELS AND BEAMS UNLESS NOTED OTHERWISE.
- F. ALL MASONRY WALLS SHALL BE REINFORCED WITH NO. 9 GAGE TRUSS TYPE
  GALVANIZED DUR-O-WALL SPACED VERTICALLY AT 16" O.C. U.N.O. LAP ALL DUR-O-WALL
  6" MINIMUM. PROVIDE CORNER AND TEE PIECES AT ALL INTERSECTIONS.
- G. LOOSE LINTELS FOR MASONRY WALLS SHALL BE FOR EACH 4" WIDTH OF MASONRY ONE STEEL ANGLE AS FOLLOWS:

0'-0" TO 3'-0" 3-1/2" X 3-1/2" X 5/16" 3'-1" TO 5'-0" 4" X 3-1/2" X 5/16" 5'-1" TO 6'-6" 5" X 3-1/2" X 3/8"

6'-7" TO 8'-0" 6" X 3-1/2" X 3/8"A ALL ANGLES SHALL HAVE THEIR SHORT LEG OUTSTANDING AND 6" MINIMUM BEARING.

5. STRUCTURAL STEEL

- A. ALL STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATION A992 (LATEST LOCAL APPROVED). ALL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCOPEANCE WITH EAISC MANUAL, AISC SPECIFICATION AND AISC CODE OF STANDARD PRACTICE.
- B. ALL MELDED CONNECTIONS SHALL BE DONE WITH ETOXX ELECTRODES. SHOP AND FIELD MELDS SHALL BE MADE BY APPROVED CERTIFIED WELDERS AND SHALL CONFORM TO THE AMERICAN MELDING SOCIETY CODE FOR BUILDINGS AMS D1.1. MELDS SHALL DEVELOP THE FULL STRENGTH OF MATERIALS BEING WELDED UNLESS OTHERWISE NOTED.

## 6. MOOD

- A. STRUCTURAL SOLID WOOD RAFTERS, JOISTS, BEAMS AND STUDS SHALL BE HEM FIR #2
  OR SPRUCE PINE FIR #2 SURFACED DRY AT A MAXIMUM OF 19
  ALL LUMBER EXPOSED TO MEATHER SHALL BE PRESSURE TREATED SOUTHERN PINE #2.
  ALL FABRICATION, ERECTION, OTHER PROCEDURES, AND MINIMUM UNIT STRESSES
  SHALL CONFORM TO THE CURRENT "NATIONAL DESIGN SPECIFICATION FOR WOOD
  CONSTRUCTION".
- B. WOOD TRUSSES SHALL BE DESIGNED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION (ANSI/TPI 1) AND COMMENTARY AND RECOMMENDATIONS FOR HANDLING, INSTALLING AND BRACING (PER BCSI) METAL PLATE CONNECTED WOOD TRUSSES (HIB-91) AS PUBLISHED BY THE TRUSS PLATE INSTITUTE AND IN ACCORDANCE WITH THE 1991 EDITION OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION.
- C. WOOD TRUSSES AND ENGINEERED FLOOR JOISTS ARE TO BE DESIGNED BY THE SUPPLIER. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER/ARCHITECT FOR REVIEW. ALL TRUSSES AND JOISTS SHALL BE DESIGNED TO LIMIT THE BEARING STRESS TO 425 psi WHEN MEMBERS BEAR ON STUD WALLS. PROVIDE MEMBERS OF ADEQUATE WIDTH OR METAL CONNECTIONS TO LIMIT STRESSES TO THE SPECIFIED VALUE.
- D. ALL LAMINATED VENEER LUMBER (LVL) OR PARALLEL STRAND LUMBER (PSL) SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: Fb=2600psi, Fv=285psi, E=1,900,000psi, Fc=2510psi(PARALLEL), Fc=750psi(PERPENDICULAR).
- E. ALL DOUBLE MEMBERS SHALL BE NAILED TOGETHER WITH 2 ROWS OF 16d NAILS SPACED AT 12" O.C. ALL TRIPLE MEMBERS SHALL BE NAILED TOGETHER WITH 3 ROWS OF 16d NAILS SPACED AT 12" O.C. NAILED FROM EACH SIDE.

F. PROVIDE DOUBLE JOISTS AT PARALLEL PARTITIONS WHERE PARTITION LENGTH
EXCEEDS 1/3 JOIST SPAN.

- G. ALL NAILS ARE TO BE COMMON WIRE NAILS. NAILING OF ALL FRAMING SHALL BE AS SPECIFIED IN THE CONTRACT DOCUMENTS BUT IN NO CASE SHALL BE LESS THAN THE RECOMMENDED NAILING SCHEDULE CONTAINED IN THE 2018 INTERNATIONAL RESIDENTIAL CODE. ALL MULTIPLE STUD POSTS ARE TO BE NAILED TOGETHER WITH 12d NAILS @ 6" O.C. STAGGERED.
- H. PROVIDE BRIDGING SPACED AT 48" O.C. IN FIRST TWO JOIST, RAFTER OR TRUSS SPACES WHEN FRAMING IS PARALLEL TO EXTERIOR WALL. NAIL SHEATHING (FLOOR, CEILING OR ROOF) TO BRIDGING AND NAIL BRIDGING TO EXTERIOR WALL PLATE. PROVIDE ONE ROW OF BRIDGING BETWEEN ALL FLOOR AND ROOF JOISTS FOR EACH 8"-0" OF SPAN. PROVIDE SOLID BLOCKING OR A CONTINUOUS RIM JOIST AT THE BEARING OF JOISTS, RAFTERS OR TRUSSES ON WOOD PLATES.

I. PROVIDE THE FOLLOWING JAMB STUDS AT ALL BEARING WALL OPENINGS UNLESS NOTED OTHERWISE:

0-3' OPENING 3'-1" - 6'-0" OPENING

6'-1" - 9'-0" OPENING

1 JACK STUD, 1 KING STUD 2 JACK STUDS, 1 KING STUD 2 JACK STUDS, 2 KING STUDS

PROVIDE DOUBLE STUDS AT ALL CORNERS AND BENEATH ALL GIRDER TRUSSES AND WOOD BEAMS UNLESS NOTED OTHERWISE ON PLANS. WOOD BEAMS, GIRDER TRUSSES AND HEADERS SHALL BEAR THE FULL DEPTH OF POSTS AND JACK STUDS.

- J. ALL POSTS (MULTIPLE STUDS OR SOLID POST) SUPPORTING BEAMS, WALL HEADERS OR GIRDER TRUSSES, SHALL BE BLOCKED SOLID FOR THE FULL LENGTH AND WIDTH OF POSTS AT ALL INTERSECTIONS WITH FLOORS AS REQUIRED TO PROVIDE CONTINUOUS SUPPORT TO TOP OF FOUNDATION WALLS OR BEAMS. POSTS SHOWN ON UPPER LEVELS FLOORS SHALL ALSO BE INSTALLED ON THE LOWER LEVELS IN LINE WITH THE POST ABOVE DOWN TO FOUNDATION WALLS OR BEAMS.
- K. ALL FLUSH JOIST TO BEAM OR BEAM TO BEAM CONNECTIONS SHALL BE MADE WITH JOIST OR BEAM HANGERS TO SUPPORT THE LOAD CAPACITY INDICATED ON THE PLANS OR THE FULL CAPACITY OF THE JOIST OR BEAM. HANGERS SHALL BE PROVIDED BY SIMPSON STRONG TIE OR USP LUMBER CONNECTORS. THE SUPPLIER SHALL DESIGN ALL HANGERS FOR THE CAPACITY STATED. INSTALL ALL HANGERS IN STRICT CONFORMANCE TO THE MANUFACTURES INSTRUCTIONS. FILL ALL NAIL OR BOLT HOLES USING THE SPECIFIED NAILS AND BOLTS ONLY.

# MAGNOLIA ACRES LOT 19 THE FONTANA CARUSO HOMES

# VERSION

2018 NORTH CAROLINA STATE BUILDING CODE/ RESIDENTIAL CODE: "REFERENCE TO INTERNATIONAL CODES SHALL MEAN THE NORTH CAROLINA CODES.

SEE PAGE 8 FOR HEADER & GIRDER SPANS/ JACK STUDS & FULL HEIGHT STUDS.

(REFERENCES 2018 IRC)

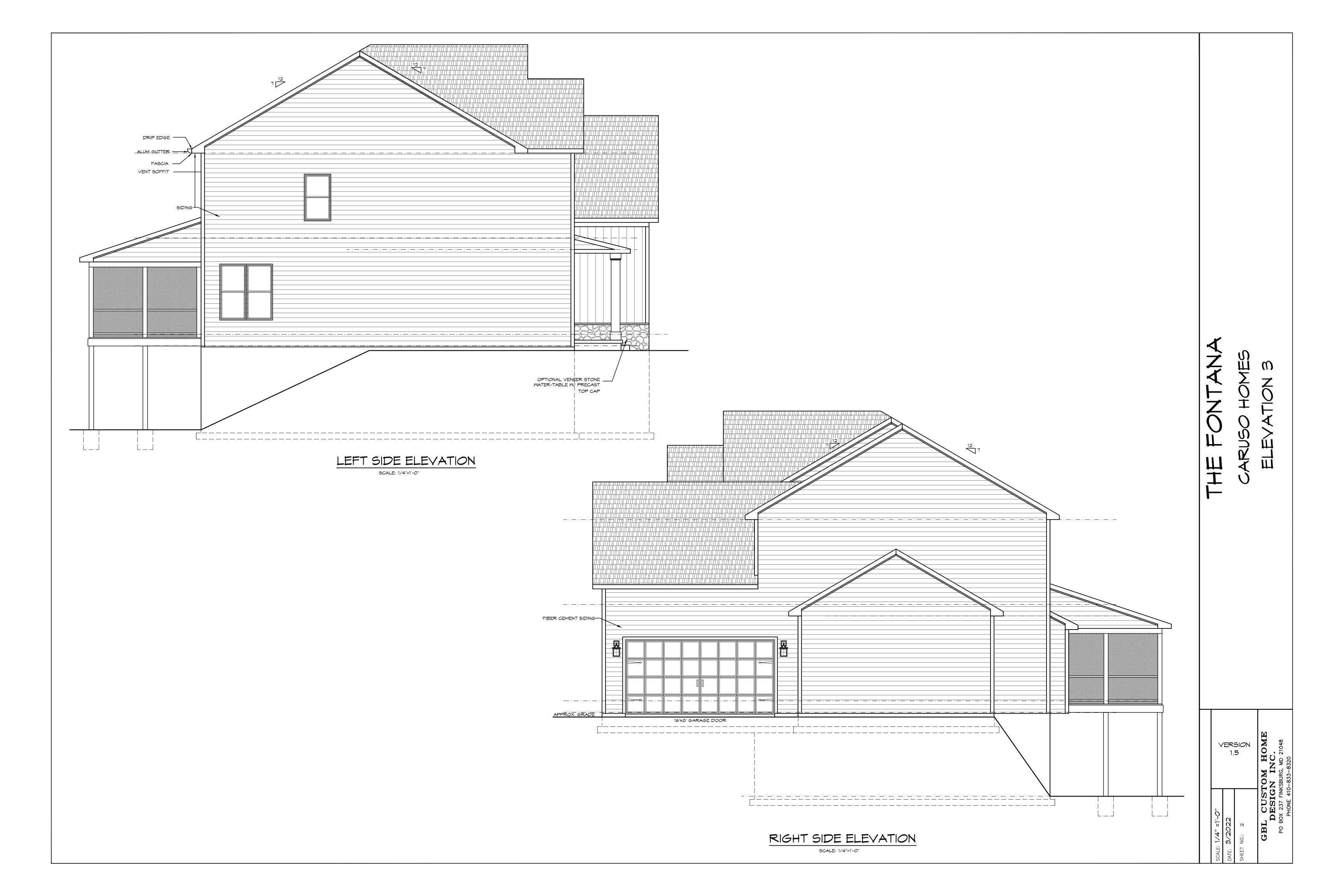
USE PAGE UNLESS NOTED DIFFERENTLY ON PLANS

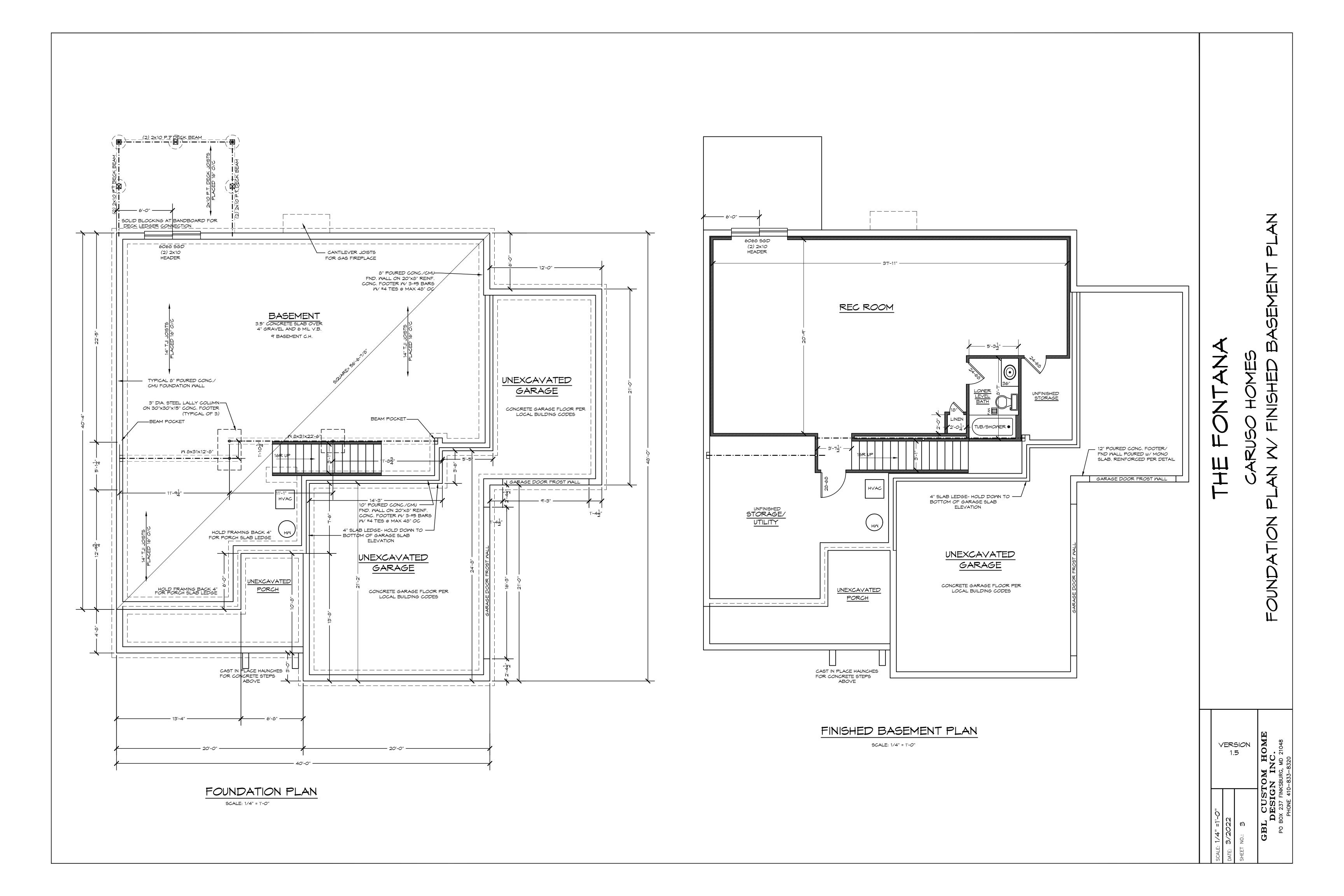
## INDEX

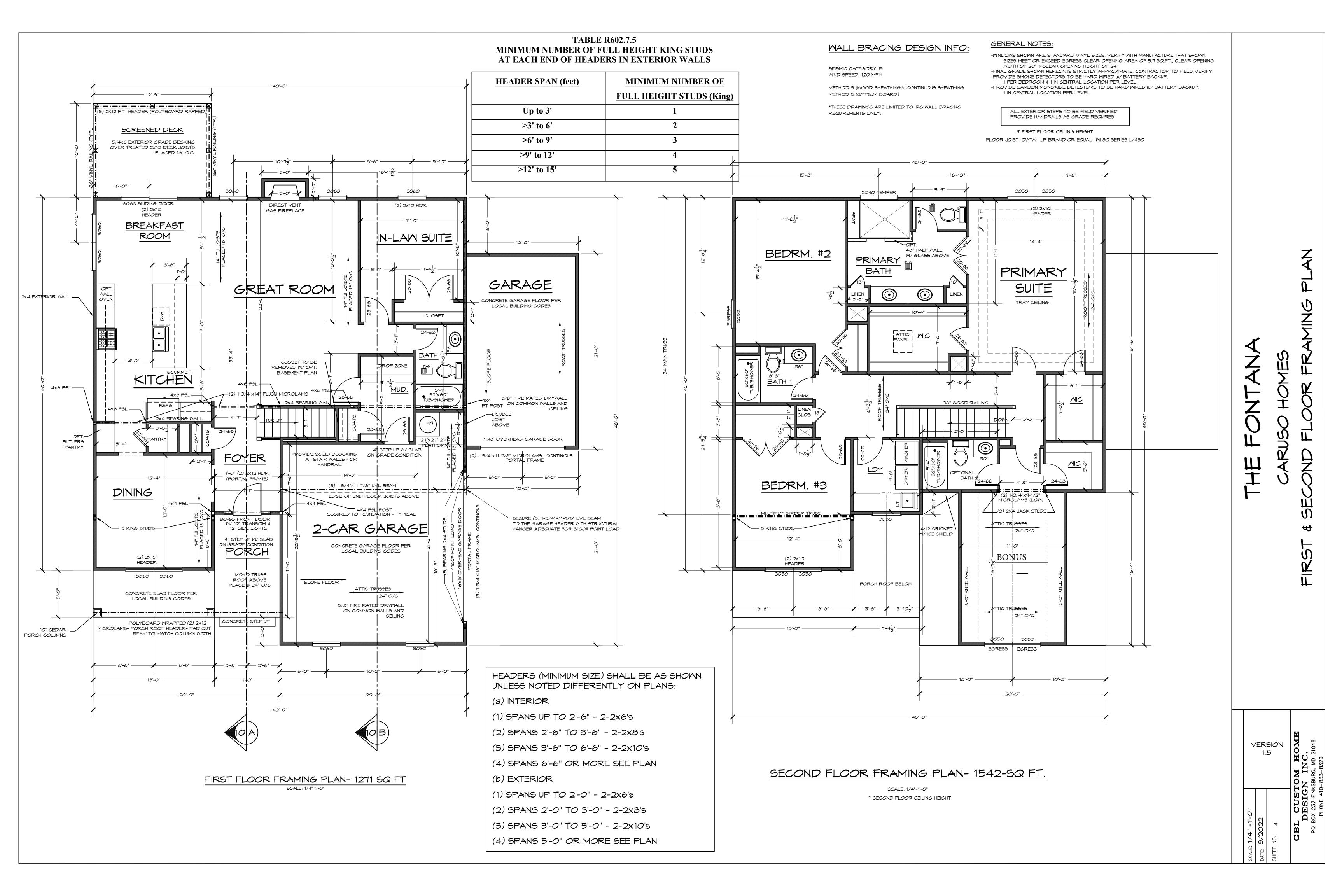
HEATED SQFT-	1
DESCRIPTION	AREA
FIRST FLOOR LIVABLE	1272 SQ.FT.
SECOND FLOOR LIVABLE	1543 SQ.FT.
GRAND TOTAL	2815 SQ.FT.

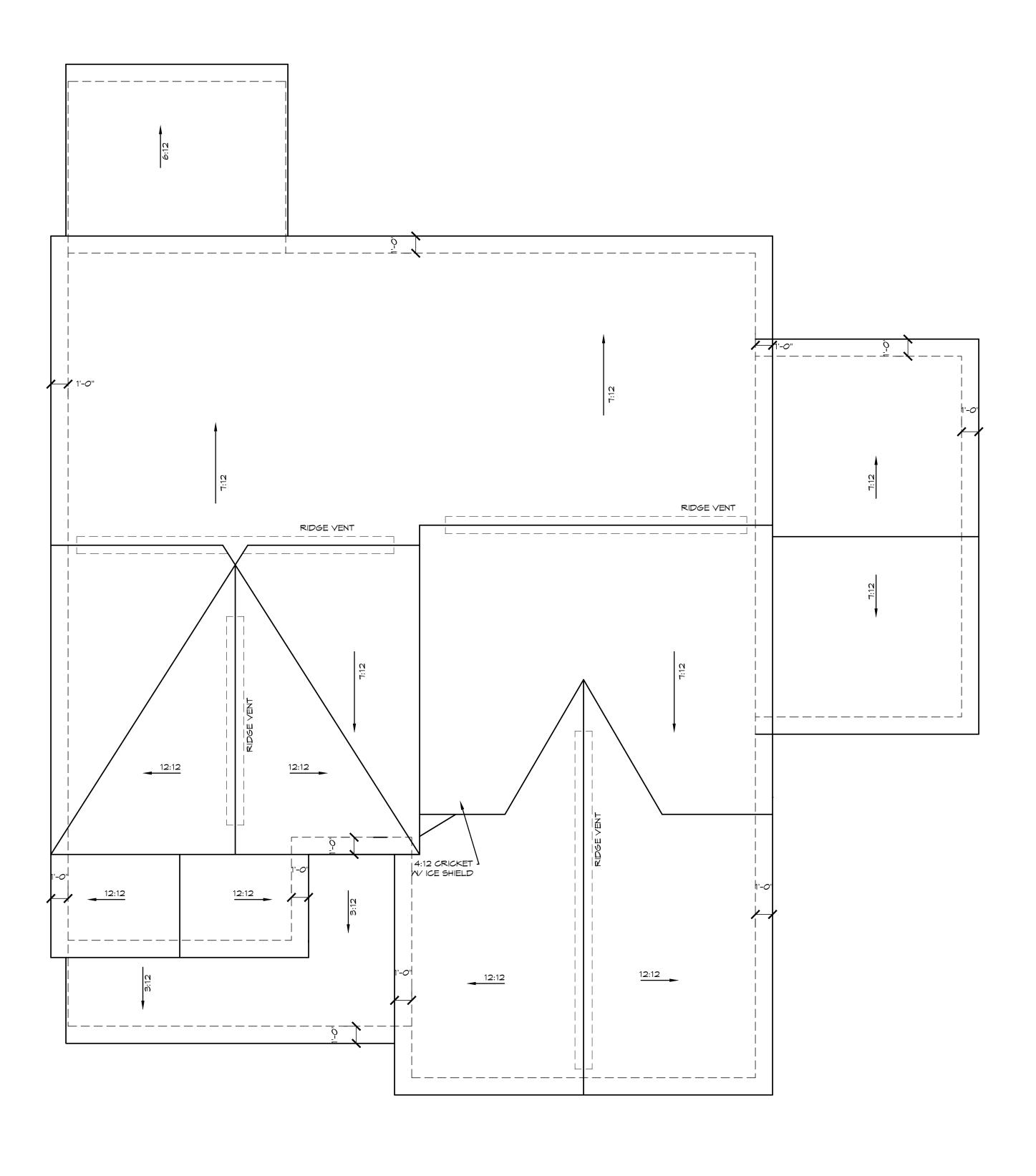
UNHEATED SQFT	Γ- 1
DESCRIPTION	AREA
FRONT PORCH	100 SQ.FT.
REAR DECK	144 SQ.FT.
2 CAR GARAGE	480 SQ.FT.
3RD CAR GARAGE	252 SQ.FT.
GARAGE TOTAL	732 SQ.FT.













THE FONTANA
CARUSO HOMES
ROOF FLAN

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	2022	ľ	BL CUSTOM DESIGN I

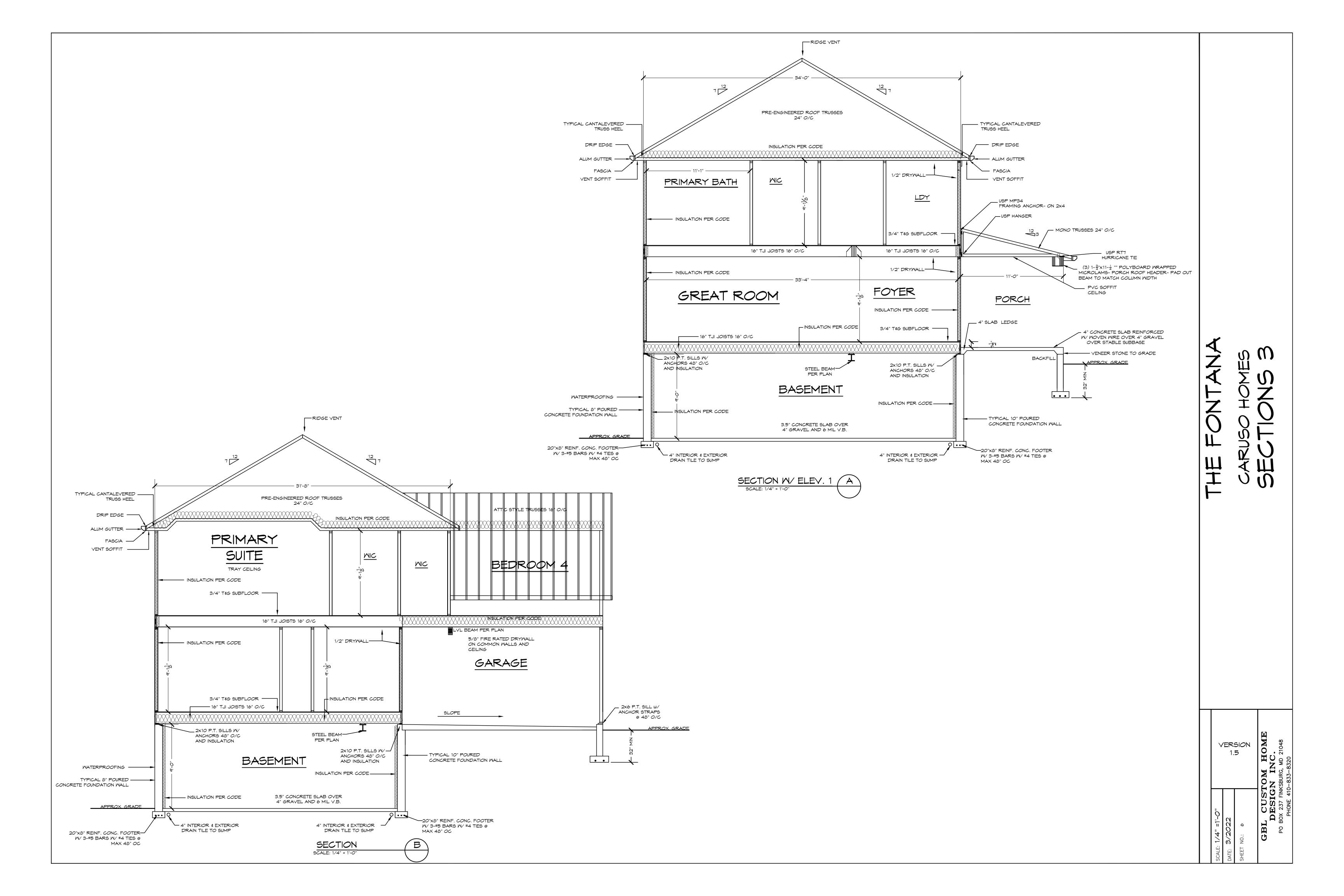


TABLE R404.1.2(3) MINIMUM VERTICAL REINFORCEMENT FOR 8-INCH (203 MM) NOMINAL FLAT CONCRETE BASEMENT WALLS

		MINIMUM VERTICAL REINFORCEMENT-BAR SIZE AND SPACING (INCHES)					
MAXIMUM	MAXIMUM	HEIGHE (SEE	S AND DESIGN LA	TERAL SOIL			
UNSUPPORTED	UNBALANCED		PER FOOT OF DE				
MALL HEIGHT (FEET)	BACKFILL	GW GP SW SP	GM, GC, SM,	SC ML-CL AND			
"X"	"~"	30	SM-SC, AND ML	INORGANIC CL			
_ ^	ı		45	60			
	4	NR	NR	NR			
	5	NR	NR	NR			
8	6	NR	NR	6@37			
	7	NR	6@36	6@35			
	පි	6@41	6@35	6@26			
	4	NR	NR	NR			
	5	NR	NR	NR			
g	6	NR	NR	6@35			
	7	NR	6@35	6@32			
	ව	6@36	6@32	6@23			
	9	6@35	6@25	6@18			
	4	NR	NR	NR			
	5	NR	NR	NR			
	6	NR	NR	6@35			
10	٦	NR	6@35	6@29			
	8	6@35	6@29	6@21			
	9	6@34	6@22	6@16			
	10	6@27	6@17	6@13			

FOR SI: 1 INCH= 25.4 MM; 1 FOOT= 304.8MM; 1 POUND PER SQUARE FOOT PER FOOT= 0.1571 kPa^2/M, 1 POUND PER SQUARE INCH= 6.895 kPa. NR= NOT REQUIRED

- a. SOIL CLASSES ARE IN ACCORDANCE WITH THE UNIFIED SOIL CLASSIFICATION SYSTEM. REFER TO TABLE R405.1. b. TABLE VALUES ARE BASED ON REINFORCING BARS WITH A MINIMUM YIELD
- STRENGTH OF 60,000 PSI CONCRETE WITH A MINIMUM SPECIFIED COMPRESSIVE STRENGTH OF 2,500 PSI AND VERTICAL REINFORCEMENT BEING LOCATED AT THE CENTERLINE OF THE WALL. SEE SECTION R404.1.3.3.7.2.
- c. VERTICAL REINFORCEMENT WITH A YIELD STRENGTH OF LESS THAN 60,000 PSI AND/OR BARS OF A DIFFERENT SIZE THAN SPECIFIED IN THE TABLE ARE PERMITTED IN ACCORDANCE WITH SECTION R404.1.3.3.7.6 AND TABLE R404.1.2(9)
- d. NR INDICATES NO VERTICAL REINFORCEMENT IS REQUIRED e. DEFLECTION CRITERION IS L/240, WHERE L IS THE HEIGHT OF THE BASEMENT WALL
- IN INCHES F. INTERPOLATION IS NOT PERMITTED
- q. WHERE WALLS WILL RETAIN 4 FEET OR MORE OF UNBALANCED BACKFILL, THEY SHALL BE LATERALLY SUPPORTED AT THE TOP AND BOTTOM BEFORE BACKFILLING.
- h. SEE SECTION R404.1.3.2 FOR MINIMUM REINFORCEMENT REQUIRED FOR BASEMENT WALLS SUPPORTING ABOVE-GRADE CONCRETE WALLS.
- SEE TABLE R608.3 FOR TOLERANCE FROM NOMINAL THICKNESS PERMITTED FOR FLAT WALLS.
- . THE USE OF THIS TABLE SHALL BE PROHIBITED FOR SOIL CLASSIFICATIONS NOT SHOWN.

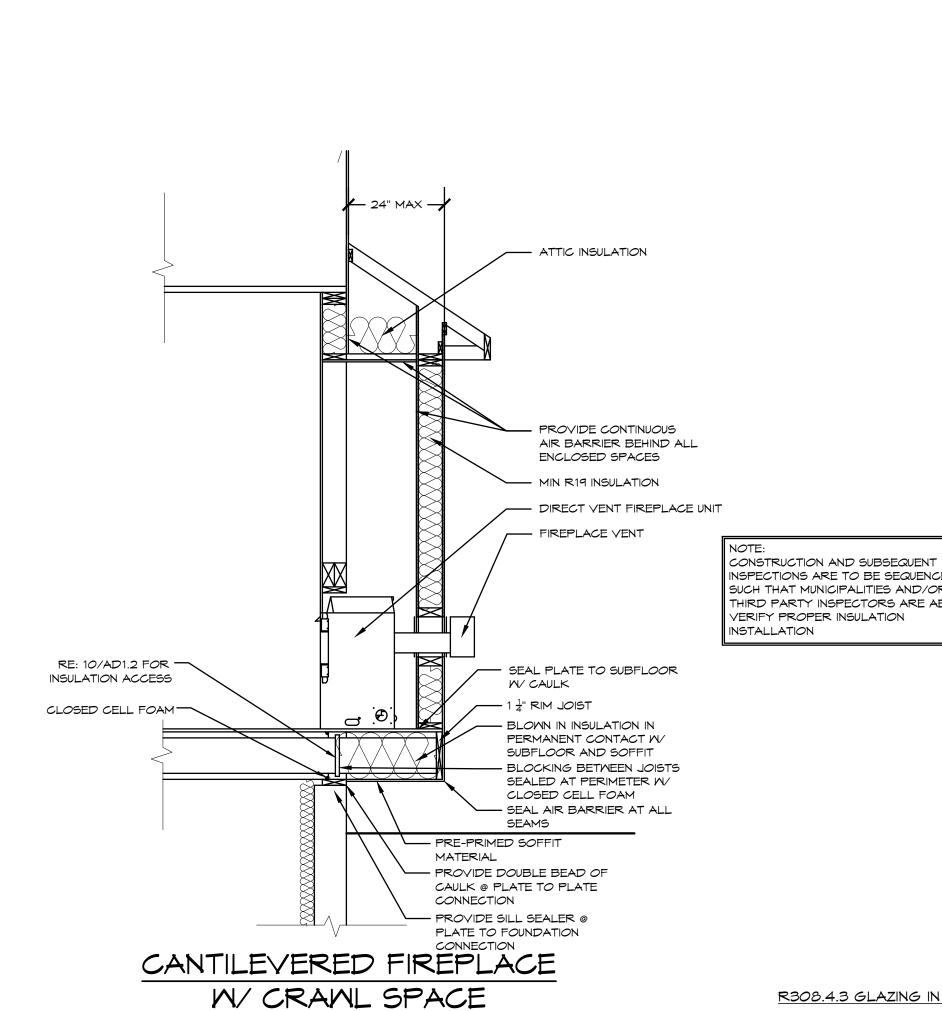
#### TABLE R404.1.2(1) MINIMUM HORIZONTAL REINFORCEMENT FOR CONCRETE BASEMENT WALLS

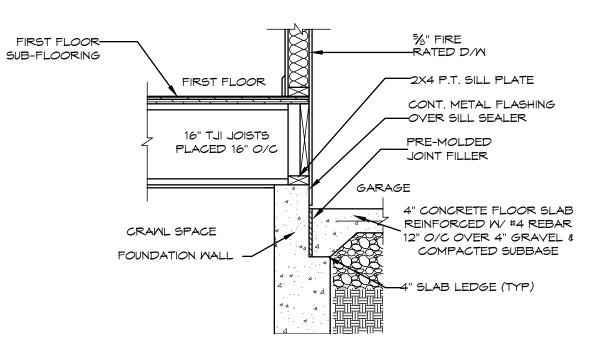
	CONTAL NEIN CHOCKENT FOR CONCINE DASEMENT MALES
MAXIMUM UNSUPPORTED HEIGHT OF BASEMENT WALL (FEET)	LOCATION OF HORIZONTAL REINFORCEMENT
<u>≼</u> 8	ONE NO. 4 BAR WITHIN 12 INCHES OF THE TOP OF THE WALL STORY AND ONE NO. 4 BAR NEAR MID-HEIGHT OF THE WALL STORY.
>8	ONE NO. 4 BAR WITHIN 12 INCHES OF THE TOP OF THE WALL STORY AND ONE NO. 4 BAR NEAR THIRD POINTS IN THE WALL STORY.

FOR SI: 1 INCH= 25.4 MM, 1 FOOT= 304.8 MM, 1 POUND PER SQUARE INCH= 6.895 KPa. a. HORIZONTAL REINFORCEMENT REQUIREMENTS ARE FOR REINFORCING BARS WITH A MINIMUM YIELD STRENGTH OF 40,000 PSI AND CONCRETE WITH A MINIMUM CONCRETE COMPRESSIVE STRENGTH OF 2,500 PSI.

b. SEE SECTION R404.1.2.2 FOR MINIMUM REINFORCEMENT REQUIRED FOR FOUNDATION WALLS SUPPORTING ABOVE-GRADE CONCRETE WALLS.

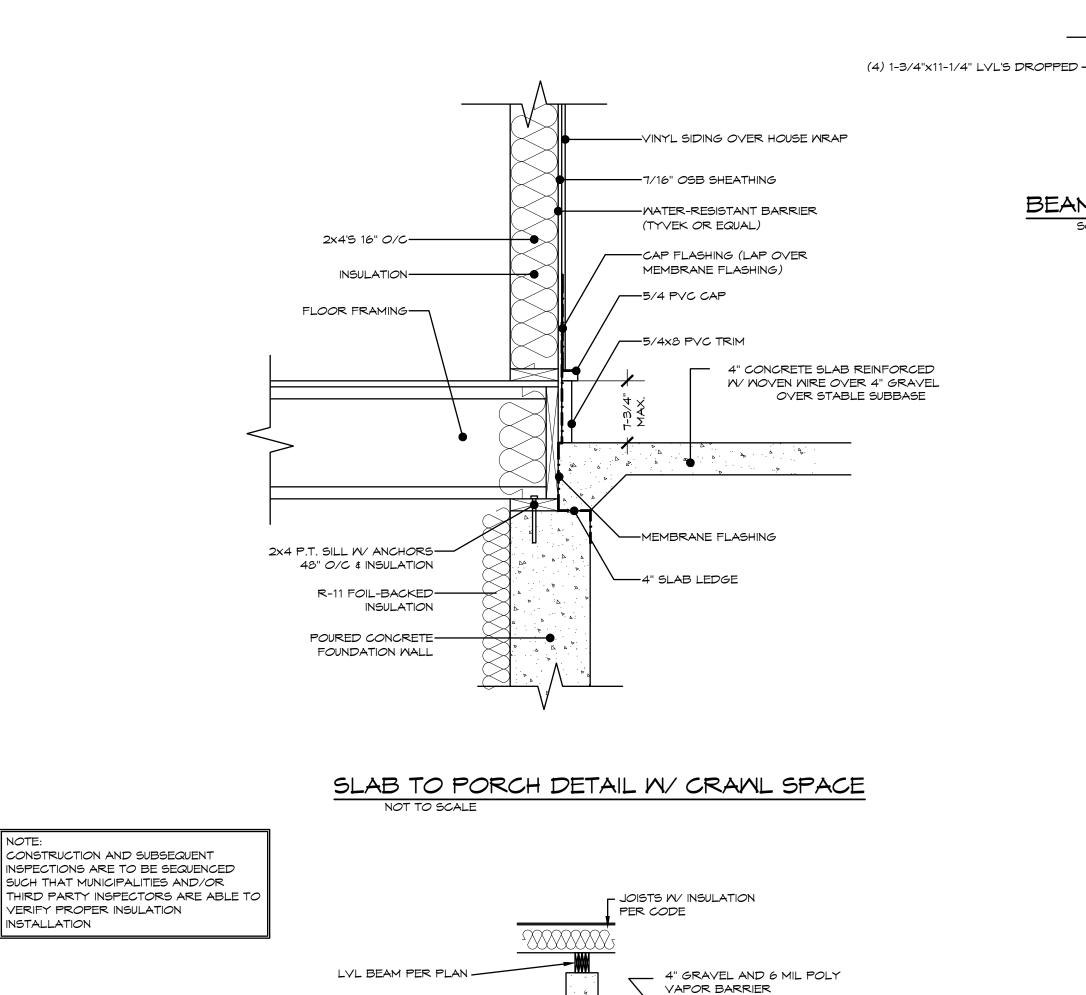
- 1. THE PLACEMENT OF HORIZONTAL AND VERTICAL REINFORCING STEEL SHALL BE MADE IN ACCORDANCE WITH THE GOVERNING PROVISIONS OF THE INTERNATIONAL RESIDENTIAL CODE (IRC) AND AS NOTED ON THE DRAWINGS. THE PLACEMENT OF REINFORCING STEEL UNDER THE IRC SHALL CONFORM TO THE PROVISIONS AS ESTABLISHED BY THE INTERNATIONAL BUILDING CODE (IBC) IN ACCORDANCE WITH THE AMERICAN CONCRETE INSTITUTE (ACI) 318 BUILDING CODE FOR STRUCTURAL CONCRETE, LATEST EDITION.
- 1. REINFORCING STEEL SHALL CONFORM TO ASTM DESIGNATION A-615, GRADE 60.
- 1. REINFORCING BARS SHALL BE LAPPED A MINIMUM OF 40 BAR DIAMETERS WHEN SPLICED, EXCEPT AS NOTED ON THE DRAWINGS OR SUPERCEDED BY OTHER CODE REQUIREMENTS.
- 1. HORIZONTAL AND VERTICAL REINFORCEMENT AT ALL WALL CORNERS, DOORWAY AND WINDOW OPENINGS AND OTHER OPENINGS WITHIN THE FOUNDATION WALLS ARE TO BE CONSTRUCTED WITH MINIMUM 12-INCH HOOKED ENDS. THE HOOKED ENDS SHALL BE LAPPED TO THE REINFORCEMENT WITHIN THE ADJOINING WALL SECTIONS OR SURROUNDING THE OPENINGS WITHIN THE FOUNDATION WALL.
- 1. HORIZONTAL AND VERTICAL REINFORCMENT AT DOORWAY, WINDOW AND OTHER OPENINGS WITHIN FOUNDATION WALLS SHALL BE CONFIGURED AS REQUIRED TO FORM LINTELS TO SUPPORT OVERLYING APPLIED LOADS.
- 1. SHOP DRAWINGS OR OTHER INFORMATION SHALL BE PREPARED IDENTIFYING THE LOCATION AND PLACEMENT OF REINFORCING STEEL WITHIN THE FOUNDATION WALLS FOR REVIEW AND APPROVAL BY THE ARCHITECT, ENGINEER, CONTRACTOR OR OWNER'S REPRESENTATIVE.

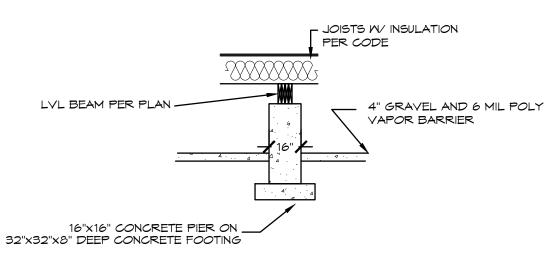




NOT TO SCALE







## CRANL SPACE BEAM PIER DETAIL

## R308.4.3 GLAZING IN WINDOWS

GLAZING IN AN INDIVIDUAL FIXED OR OPERABLE PANEL THAT MEETS ALL OF THE FOLLOWING CONDITIONS SHALL BE CONSIDERED TO BE A HAZARDOUS LOCATION:

- 1. THE EXPOSED AREA OF AN INDIVIDUAL PANE IS LARGER THAN 9 SQUARE FEET (0.836 M2),
- 2. THE BOTTOM EDGE OF THE GLAZING IS LESS THAN 18 INCHES (457 MM) ABOVE THE FLOOR,
- 3. THE TOP EDGE OF THE GLAZING IS MORE THAN 36 INCHES (914 MM) ABOVE THE FLOOR;
- 4. ONE OR MORE WALKING SURFACES ARE WITHIN 36 INCHES (914 MM), MEASURED HORIZONTALLY AND IN A STRAIGHT LINE, OF THE GLAZING.
- EXCEPTIONS:
- DECORATIVE GLAZING.
- 2. WHERE A HORIZONTAL RAIL IS INSTALLED ON THE ACCESSIBLE SIDE(S) OF THE GLAZING 34 TO 38 INCHES (864 TO 965 MM) ABOVE THE WALKING SURFACE. THE RAIL SHALL BE CAPABLE OF WITHSTANDING A HORIZONTAL LOAD OF 50 POUNDS PER LINEAR FOOT (730 N/M) WITHOUT CONTACTING THE GLASS AND HAVE A CROSS-SECTIONAL HEIGHT OF NOT LESS THAN 11/2 INCHES (38 MM).
- 3. OUTBOARD PANES IN INSULATING GLASS UNITS AND OTHER MULTIPLE GLAZED PANELS WHERE THE BOTTOM EDGE OF THE GLASS IS 25 FEET (1620 MM) OR MORE ABOVE GRADE, A ROOF, WALKING SURFACES OR OTHER HORIZONTAL [WITHIN 45 DEGREES (0.79 RAD) OF HORIZONTAL] SURFACE ADJACENT TO THE GLASS EXTERIOR.

## R308.6.2 MATERIALS

- THE FOLLOWING TYPES OF GLAZING SHALL BE PERMITTED TO BE USED:
- 1. LAMINATED GLASS WITH NOT LESS THAN A 0.015-INCH (0.38 MM) POLYVINYL BUTYRAL INTERLAYER FOR GLASS PANES 16 SQUARE FEET (1.5 M2) OR LESS IN AREA LOCATED SUCH THAT THE HIGHEST POINT OF THE GLASS IS NOT MORE THAN 12 FEET (3658 MM) ABOVE A WALKING SURFACE OR OTHER ACCESSIBLE AREA; FOR HIGHER OR LARGER SIZES, THE INTERLAYER THICKNESS SHALL BE NOT LESS THAN 0.030 INCH (0.76 MM).
- 2. FULLY TEMPERED GLASS.
- 3. HEAT-STRENGTHENED GLASS.
- 4. WIRED GLASS.
- 5. APPROVED RIGID PLASTICS.

SECTION R310 EMERGENCY ESCAPE AND RESCUE OPENINGS

## R310.1 EMERGENCY ESCAPE AND RESCUE OPENING REQUIRED

BASEMENTS, HABITABLE ATTICS AND EVERY SLEEPING ROOM SHALL HAVE NOT LESS THAN ONE OPERABLE EMERGENCY ESCAPE AND RESCUE OPENING. WHERE BASEMENTS CONTAIN ONE OR MORE SLEEPING ROOMS, AN EMERGENCY ESCAPE AND RESCUE OPENING SHALL BE REQUIRED IN EACH SLEEPING ROOM. EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL OPEN DIRECTLY INTO A PUBLIC WAY, OR TO A YARD OR COURT THAT OPENS TO A PUBLIC WAY.

SILL PLATE W/ INSULATION

TO ADJUST HEIGHT

- FOUNDATION MALL

BEAM POCKET DETAIL

LVL BEAM (W/ HOUSE WRAP) GROUTED INTO

MIN. 4"X8"X12" DEEP BEAM POCKET

USE STEEL SHIMS AS REQUIRED

EXCEPTION: STORM SHELTERS AND BASEMENTS USED ONLY TO HOUSE MECHANICAL EQUIPMENT NOT EXCEEDING A TOTAL FLOOR AREA OF 200 SQUARE FEET (18.58 M2).

R310.1.1 OPERATIONAL CONSTRAINTS AND OPENING CONTROL DEVICES

EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL BE OPERATIONAL FROM THE INSIDE OF THE ROOM WITHOUT THE USE OF KEYS, TOOLS OR SPECIAL KNOWLEDGE. WINDOW OPENING CONTROL DEVICES COMPLYING WITH ASTM F2090 SHALL BE PERMITTED FOR USE ON WINDOWS SERVING AS A REQUIRED EMERGENCY ESCAPE AND RESCUE OPENING.

## R310.2 EMERGENCY ESCAPE AND RESCUE OPENINGS

EMERGENCY ESCAPE AND RESCUE OPENINGS SHALL HAVE MINIMUM DIMENSIONS AS SPECIFIED IN THIS SECTION.

## R310.2.1 MINIMUM OPENING AREA

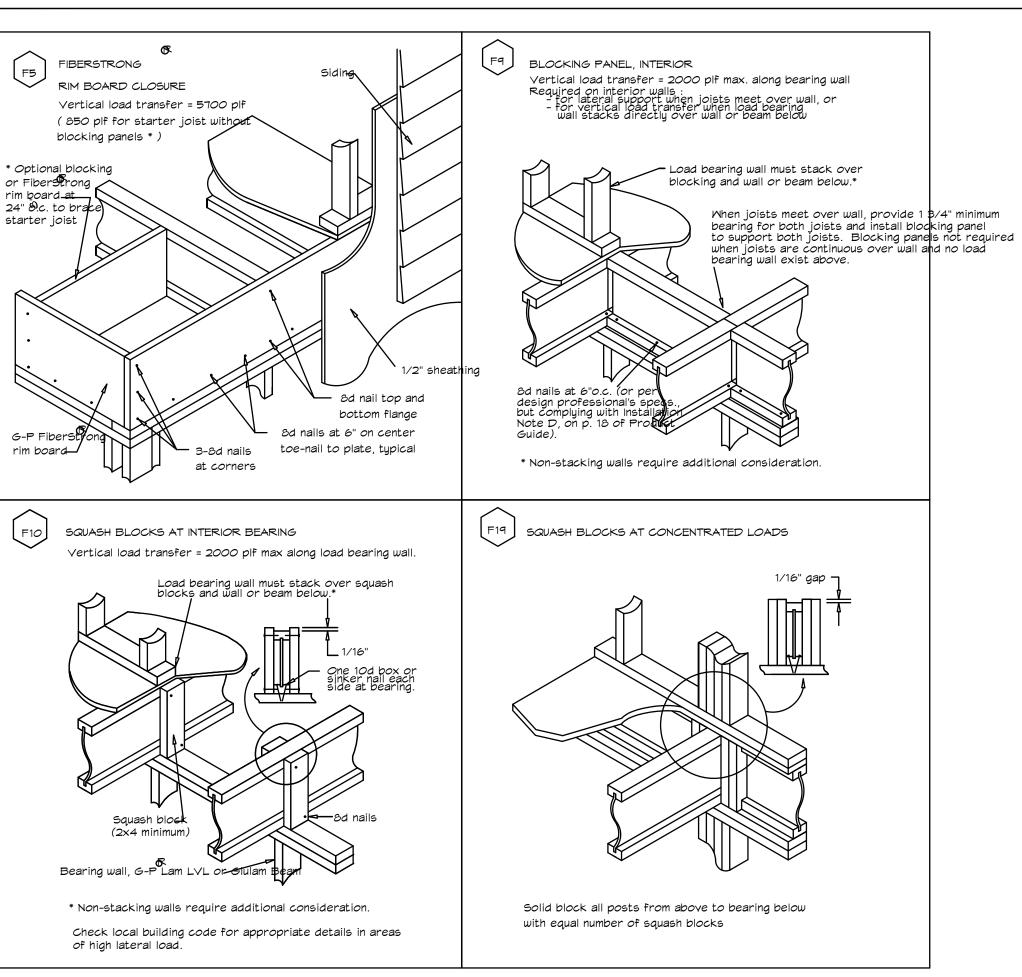
EMERGENCY AND ESCAPE RESCUE OPENINGS SHALL HAVE A NET CLEAR OPENING OF NOT LESS THAN 5.7 SQUARE FEET (0.530 M2). THE NET CLEAR OPENING DIMENSIONS REQUIRED BY THIS SECTION SHALL BE OBTAINED BY THE NORMAL OPERATION OF THE EMERGENCY ESCAPE AND RESCUE OPENING FROM THE INSIDE. THE NET CLEAR HEIGHT OPENING SHALL BE NOT LESS THAN 24 INCHES (610 MM) AND THE NET CLEAR WIDTH SHALL BE NOT LESS THAN 20 INCHES (508 MM).

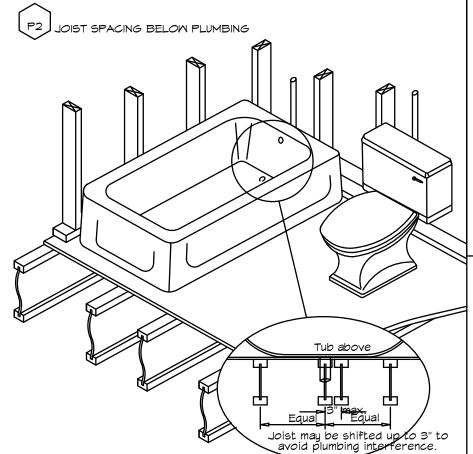
EXCEPTION: GRADE FLOOR OR BELOW GRADE OPENINGS SHALL HAVE A NET CLEAR OPENING OF NOT LESS THAN 5 SQUARE FEET (0.465 M2).

## R310.2.2 MINDOW SILL HEIGHT

WHERE A WINDOW IS PROVIDED AS THE EMERGENCY ESCAPE AND RESCUE OPENING, IT SHALL HAVE A SILL HEIGHT OF NOT MORE THAN 44 INCHES (1118 MM) ABOVE THE FLOOR; WHERE THE SILL HEIGHT IS BELOW GRADE, IT SHALL BE PROVIDED WITH A WINDOW WELL IN ACCORDANCE WITH SECTION R310.2.3.

VERSION 1.5





LVL FASTENING SCHEDULE

Maximum Uniform Load Applied to Either or Both Outside Pieces

(Pounds per lineal foot)

		,,		,				
	16d N	NAILS	1/2" B	OLTS	SCREMS (Note 9)			
Pieces	A	m	O	Ω	Ш	F		
Member	2 ROMS 12" o.c.	3 RONS 12" o.c. 2	2 RONS 4" o.c. staq	2 ROMS 3.12" o.c.		2 ROWS 12" O.C.		
2	505	760	505	1015	500	995		
з	380	570	380	760	375	745		
4	Not Do	ام ما خانمه	240	675	220	665		

1. Confirm adequacy of the beam (depth and number of pieces) for carrying the designated load. 2. Stress level for nail and bolt values is 100%. Increases of 15% for snow loaded or 25% for non-snow loaded roof conditions are permitted. 3. Top and bottom row of connectors should be 2" from edge.

4. Bolt holes are to be the same diameter as the bolt. Every bolt must extend through the full thickness of the member. Use washers under head and nut. 5. For three-piece member, specified nailing is from the each side. 6. To minimize rotation, four-piece members should only be used when loads are applied

to both sides, or completely across the top of the member. 7. Four-piece members must be bolted or attached with 6" screws from both sides.

8. Floor joists must be attached with approved metal hangers. 9. Screws are USP MS series or Simpson Strong-Tie SDS installed per manufacturer instructions. 4 Not Permitted 340 675 330 665 10 Screws for 3-ply and 4-ply members must be from both sides of beam.

1 3/4" Multiple Plies	1 3/4" Multiple Plies 1 3/4" + 3 1/2"							
2 PLY 3 PLY 4 PLY 4 F Connection)(3B Connection) (4C Connection) (4E Con		2 PLY TWO ROWS 12" (4E Connection)	D.C.					
		© 0 12" c						
A (Nails) & F (Screws) TMO ROMS 12" O.C.	B (Nails) THREE ROWS 12" O.C.	C (Bolts) & E (Screws) TMO ROMS 24 IN. O.C. STAGG	ERED					
* 12" O.C.	12" O.C.	© 24" o.c. © ©	0					

2018 IECC CODE COMPLIANCE

R301.1 CLIMATE ZONE 4

R401.2 COMPLIANCE METHOD: MANDATORY AND PRESCRIPTIVE PROVISIONS

R402.1.1 VAPOR RETARDER

WALL ASSEMBLIES IN THE THERMAL BUILDING ENVELOPE SHALL COMPLY WITH THE VAPOR RETARDER REQUIREMENTS OF SECTION R702.7 OF THE IRC CODE, 2018 EDITION

R402.1.2 ATTIC INSULATION: RAISED HEEL TRUSSES:

R402.1.2 WOOD FRAME WALL: R-20 OR R13+R5 CONTINUOUS INSULATION

R402.1.2 BASEMENT WALL INSULATION: R-13/R-10 FOIL FACED CONTINUOUS, UNINTERRUPTED BATTS

R402.1.2 CRAWL SPACE WALL INSULATION: R-13/R-10 FOIL FACED CONTINUOUS BATTS FULL HEIGHT EXTENDING FROM FLOOR ABOVE TO FINISH GRADE LEVEL AND THEN

VERTICALLY OR HORIZONTALLY AN ADDITIONAL 2'-0".

R402.1.2 FLOOR INSULATION OVER UNCONDITIONED SPACE: R-19 BATT INSULATION

R402.1.2 MINDOM U-VALUE / SHGC .40 (SHGC)

.35 (U-VALUE)

R402.2.10 SLAB ON GRADE FLOORS LESS THAN 12" BELOW GRADE: R-10 RIGID FOAM BOARD UNDER SLAB EXTENDING

EITHER 2'-0" HORIZONTALLY OR 2'-0" VERTICALLY.

R402.2.4 ATTIC ACCESS: ATTIC ACCESS SCUTTLE WILL BE WEATHERSTRIPPED AND INSULATED R-49.

R402.4 BUILDING THERMAL ENVELOPE (AIR LEAKAGE): EXTERIOR WALLS AND PENETRATIONS WILL BE SEALED

PER THIS SECTION OF THE 2018 IECC WITH CAULK, GASKETS, WEATHERSTRIPPING OR AN AIR BARRIER OF SUITABLE MATERIAL.

R402.4.12 BUILDING THERMAL ENVELOPE TIGHTNESS TEST: BUILDING ENVELOPE SHALL BE TESTED AND VERIFIED AS HAVING AN AIR LEAKAGE RATE OF NOT EXCEEDING 3 AIR CHANGES PER HOUR. TESTING SHALL BE CONDUCTED

IN ACCORDANCE WITH ASTM E 779 OR ASTM E 1827 WITH (BLOWER DOOR) AS A PRESSURE OF 0.2 INCHES M.G. (50 PASCALS). TESTING SHALL BE SIGNED BY THE PARTY CONDUCTING THE TEST AND PROVIDED TO THE BUILDING INSPECTOR.

R402.4.2 FIREPLACES:

NEW WOOD BURNING MASONRY FIREPLACES WILL HAVE TIGHT-

FITTING FLUE DAMPERS AND OUTDOOR COMBUSTION AIR.
FIRE PLACE DOORS SHALL BE LISTED AND LABLED IN ACCORDANCE WITH UL 127 (FACTORY BUILT FIREPLACE) AND UL 907 (MASONRY FIREPLACE)

R402.4.4 ROOMS CONTAINING FUEL BURNING APPLIANCES WHERE OPEN COMBUSTION AIR DUCTS PROVIDE COMBUSITON AIR TO OPEN COMBUSTION FUEL BURNING APPLICANCES, THE APPLIANCES AND COMBUSITON AIR SHALL BE LOCATED OUTSIDE THE BUILDING THERMAL ENVELOPE TO ENCLOSED IN A ROOM ISOLATED FROM THE THERMAL ENVELOPE.
EXCEPTION: DIRECT VENT APPLIANCES WITH BOTH INTAKE AND EXHAUST PIPES INSTALLED
CONTINOUS THE OUTSIDE. FIREPLACES AND STOVES COMPLYING WITH SECTION R402.4.2 AND

SECTION R1006 OF THE IRC. R402.4.5 RECESSED LIGHTING:

RECESSED LUMINARIES INSTALLED IN THE BUILDING THERMAL ENVELOPE SHALL BE SEALED TO LIMIT AIR LEAKAGE.

R403.1.1 THERMOSTAT: ALL DWELLING UNITS WILL HAVE AT LEAST (1) PROGRAMMABLE THERMOSTAT FOR EACH SEPARATE HEATING AND COOLING SYSTEM

PER 2018 IECC SECTION 403.1.1

R403.1.2 WHERE A HEAT PUMP SYSTEM HAVING SUPPLEMENTARY ELECTRIC RESISTANCE HEAT IS USED THE THERMOSTAT SHALL PREVENT THE SUPPLEMENTARY HEAT FROM COMING ON WHEN HEAT PUMP CAN MEET HEATING LOAD.

R403.3.1 MECHANICAL DUCT INSULATION:

SUPPLY AND RETURN DUCTS IN ATTIC R-8 MINIMUM, R-6 WHEN LESS THAN 3" SUPPLY AND RETURN DUCTS OUTSIDE OF CONDITIONED SPACE R-8 MINIMUM ALL OTHER DUCTS EXCEPT THOSE LOCATED COMPLETELY INSIDE THE

BUILDING THERMAL ENVELOPE R-6 MINIMUM. DUCTS LOCATED UNDER CONCRETE SLABS MUST BE R-6 MINIMUM.

R403.3.2 DUCT SEALING:

ALL DUCTS, AIR HANDLERS, FILTER BOXES WILL BE SEALED. JOINTS AND SEAMS WILL COMPLY WITH SECTION M1601.4.1 OF THE IRC.

> A DUCT TIGHTNESS TEST ("DUCT BLASTER" DUCT TOTAL LEAKAGE TEST) WILL BE PERFORMED ON ALL HOMES AND SHALL BE VERIFIED BY EITHER A POST CONSTRUCTION TEST OR A ROUGH-IN TEST. DUCT TIGHTNESS IS NOT REQUIRED IF THE AIR HANDLER AND

ALL DUCTS ARE LOCATED WITHIN THE CONDITIONED SPACE. R403.6 MECHANICAL VENTILATION:

OUTDOOR (MAKE UP AND EXHAUSTS) AIR DUCTS TO BE PROVIDED WITH AUTOMATIC OR GRAVITY DAMPER THAT CLOSE WHEN THE VENTILATION

SYSTEM IS NOT OPERATING. R403.6.1 WHOLE HOUSE MECHANICAL VENTILATION SYSTEM FAN EFFICIENCY TO COMPLY

MITH TABLE R403.6.1 R403.7 EQUIPMENT SIZING SHALL COMPLY WITH R403.7.

R404.1 LIGHTING EQUIPMENT: A MINIMUM OF 75 % OF ALL LAMPS (LIGHTS) MUST BE

HIGH-EFFICACY LAMPS.

THE CONTRACTOR ALSO RESPONSIBLE FOR GENERATING CERTIFICATE OF COMPLIANCE AND AFFIXING TO ELECTRICAL PANEL OR WITHIN 6' OF THE PANEL AND BE READILY VISABLE.

VERSION

OIPDERS 1115				30				GROUND SNOW LOAD (psf) <sup>e</sup> 50 70												
HEADERS	SIZE	30 Building width <sup>c</sup> (feet)								70										
SUPPORTING		12	-	24		36		12		24		36		12		val.	24		36	6
	1-2 × 6	Span <sup>†</sup>	NJ <sup>d</sup>	Span <sup>f</sup> 3-1	NJ <sup>d</sup>	Span <sup>f</sup> 2-7	NJ <sup>d</sup>	Span <sup>f</sup> 3-5	NJ <sup>d</sup>	Span <sup>f</sup>	NJ <sup>d</sup>	Span <sup>f</sup> 2-3	NJ <sup>d</sup>	Span <sup>f</sup> 3-0	NJ <sup>d</sup>		Span <sup>f</sup>	NJ <sup>d</sup>	Span <sup>f</sup> 2-0	
	1-2 × 8	5-1	2	3-11	2	3-3	2	4-4	2	3-4	2	2-10	2	3-10	2		3-0	2	2-6	
	1-2 × 10	6-0	2	4-8	2	3-11	2	5-2	2	4-0	2	3-4	3	4-7	2	2	3-6	3	3-0	
	1-2 × 12	7-1	2	5-5	2	4-7	3	6-1	2	4-8	3	3-11	3	5-5	2	2	4-2	3	3-6	
	2-2 × 4	4-0	1	3-1	1	2-7	1	3-5	1	2-7	1	2-2	1	3-0	1	1	2-4	1	2-0	
oof and ceiling	2-2 × 6	6-0	1	4-7	1	3-10	1	5-1	1	3-11	1	3-3	2	4-6	1	191	3-6	2	2-11	
HEADER.	2-2 × 8	7-7	1	5-9	1	4-10	2	6-5	1	5-0	2	4-2	2	5-9	1	1	4-5	2	3-9	
	2-2 × 10	9-0	1	6-10	2	5-9	2	7-8	2	5-11	2	4-11	2	6-9	2		5-3	2	4-5	
ROOF AND CEILING	2-2 × 12 3-2 × 8	10-7 9-5	2	8-1 7-3	2	6-10 6-1	2	9-0	2	6-11	2	5-10 5-3	2	8-0 7-2	2	1	6-2 5-6	2	5-2 4-8	
	3-2 × 10	11-3	1	8-7	1	7-3	2	9-7	1	7-4	2	6-2	2	8-6	1	1	6-7	2	5-6	H
	3-2 × 12	13-2	1	10-1	2	8-6	2	11-3	2	8-8	2	7-4	2	10-0	2	2	7-9	2	6-6	
	4-2 × 8	10-11	1	8-4	1	7-0	1	9-4	1	7-2	1	6-0	1	8-3	1	1	6-4	1	5-4	
	4-2 × 10	12-11	1	9-11	1	8-4	1	11-1	1	8-6	1	7-2	2	9-10	1	1	7-7	2	6-4	
	4-2 × 12	15-3	1.	11-8	- 1	9-10	2	13-0	1	10-0	2	8-5	2	11-7	1	1	8-11	2	7-6	
	1-2 × 6	3-3	1	2-7	2	2-2	2	3-0	2	2-4	2	2-0	2	2-9	2	2	2-2	2	1-10	
	1-2 × 8	4-1	2	3-3	2	2-9	2	3-9	2	3-0	2	2-6	3	3-6	2		2-9	2	2-4	
	1-2 × 10	4-11	2	3-10	2	3-3	3	4-6	2	3-6	3	3-0	3	4-1	2		3-3	3	2-9	
oof, ceiling and	1-2 × 12 2-2 × 4	5-9 3-3	2	4-6 2-6	3	3-10 2-2	3	5-3 3-0	2	4-2 2-4	3	3-6 2-0	3	4-10 2-8	3	1	3-10	3	3-3 1-10	
one center-	2-2 × 4 2-2 × 6	4-10	(1) (1)	3-9	9	3-3	2	3-0 4-5	1	3-6	2	3-0	2	4-1	9	1	3-3	2	1-10 2-9	
bearing floor	2-2 × 8	6-1	1	4-10	2	4-1	2	5-7	2	4-5	2	3-9	2	5-2	2	2	4-1	2	3-6	
	2-2 × 10	7-3	2	5-8	2	4-10	2	6-8	2	5-3	2	4-5	2	6-1	2	2	4-10	2	4-1	t
	2-2 × 12	8-6	2	6-8	2	5-8	2	7-10	2	6-2	2	5-3	3	7-2	2	2	5-8	2	4-10	
ROOF, CEILING AND ONE FLOOR (CENTER BEARING)	3-2 × 8	7-8	(1)	6-0	1	5-1	2	7-0	1	5-6	2	4-8	2	6-5	1	1	5-1	2	4-4	
(CENTER BEARING)	3-2 × 10	9-1	1	7-2	2	6-1	2	8-4	1	6-7	2	5-7	2	7-8	2		6-1	2	5-2	
	3-2 × 12	10-8	2	8-5	2	7-2	2	9-10	2	7-8	2	6-7	2	9-0	2	2	7-1	2	6-1	
	4-2 × 8 4-2 × 10	8-10 10-6	117	6-11 8-3	2	5-11 7-0	1	8-1 9-8	.1	6-4 7-7	2	5-5 6-5	2	7-5 8-10	1	1	5-11 7-0	2	5-0 6-0	
	4-2 × 10	12-4	1	9-8	2	8-3	2	11-4	2	8-11	2	7-7	2	10-4	2	2	8-3	2	7-0	
	1-2 × 6	2-11	2	2-3	2	1-11	2	2-9	2	2-1	2	1-9	2	2-7	2		2-0	2	1-8	ŀ
	1-2 × 8	3-9	2	2-10	2	2-5	3	3-6	2	2-8	2	2-3	3	3-3	2	2	2-6	3	2 -2	t
	1-2 × 10	4-5	2	3-5	3	2-10	3	4-2	2	3-2	3	2-8	3	3-11	2	2	3-0	3	2-6	ľ
	1-2 × 12	5-2	2	4-0	3	3-4	3	4-10	3	3-9	3	3-2	4	4-7	3	3	3-6	3	3-0	
Roof, ceiling nd one clear-	2-2 × 4	2-11	1	2-3	1	1-10	1	2-9	1	2-1	1	1-9	1	2-7	1	1	2-0	1	1-8	
span floor	2-2 × 6	4-4	.1	3-4	2	2-10	2	4-1	1	3-2	2	2-8	2	3-10	1	1	3-0	2	2-6	
$\wedge$	2-2 × 8	5-6	2	4-3	2	3-7	2	5-2	2	4-0	2	3-4	2	4-10	2		3-9	2	3-2	
	2-2 × 10 2-2 × 12	6-7 7-9	2	5-0	2	4-2 4-11	3	6-1 7-2	2	4-9 5-7	2	4-0	3	5-9 6-9	2		4-5 5-3	3	3-9 4-5	+
	3-2 × 8	6-11	1	5-3	2	4-11	2	6-5	1	5-0	2	4-2	2	6-1	1	1	4-8	2	4-0	H
ROOF, CEILING AND ONE FLOOR (CLEAR SPAN)	3-2 × 10	8-3	2	6-3	2	5-3	2	7-8	2	5-11	2	5-0	2	7-3	2	2	5-7	2	4-8	t
	3-2 × 12	9-8	2	7-5	2	6-2	2	9-0	2	7-0	2	5-10	2	8-6	2	2	6-7	2	5-6	
	4-2 × 8	8-0	1	6-1	1	5-1	2	7-5	1	5-9	2	4-10	2	7-0	1	1	5-5	2	4-7	
	4-2 × 10	9-6	1	7-3	2	6-1	2	8-10	1	6-10	2	5-9	2	8-4	1	1	6-5	2	5-5	
	4-2 × 12	11-2	2	8-6	2	7-2	2	10-5	2	8-0	2	6-9	2	9-10	2	2	7-7	2	6-5	
	1-2 × 6	2-8	2	2-1	2	1-10	2	2-7	2	2-0	2	1-9	2	2-5	2		1-11	2	1-8	
	1-2 × 8	3-5	2	2-8	2	2-4	3	3-3	2	2-7	2	2-2	3	3-1	2		2-5	3	2-1	
	1-2 × 10 1-2 × 12	4-0	3	3-2 3-9	3	2-9 3-2	3 4	3-10 4-6	3	3-1	3	2-7 3-1	3	3-8 4-3	3		2-11 3-5	3	2-5	
Roof, ceiling	1-2 × 12 2-2 × 4	2-8	1	2-1	1	1-9	1	2-6	1	2-0	1	1-8	1	4-3 2-5	1	1	1-11	1	1-7	
nd two center- bearing floors	2-2 × 6	4-0	1	3-2	2	2-8	2	3-9	1	3-0	2	2-7	2	3-7	1	1	2-10	2	2-5	
	2-2 × 8	5-0	2	4-0	2	3-5	2	4-10	2	3-10	2	3-3	2	4-7	2	2	3-7	2	3-1	-
	2-2 × 10	6-0	2	4-9	2	4-0	2	5-8	2	4-6	2	3-10	3	5-5	2	2	4-3	2	3-8	
	2-2 × 12	7-0	2	5-7	2	4-9	3	6-8	2	5-4	3	4-6	3	6-4	2	2	5-0	3	4-3	
ROOF, CEILING AND	3-2 × 8	6-4	1	5-0	2	4-3	2	6-0	1	4-9	2	4-1	2	5-8	2		4-6	2	3-10	
ROOF, CEILING AND TWO FLOORS (CENTER BEARING)	3-2 × 10	7-6	2	5-11	2	5-1	2.	7-1	2	5-8	2	4-10	2	6-9	2		5-4	2	4-7	
	3-2 × 12	8-10	2	7-0	2	5-11	2	8-5	2	6-8	2	5-8	3	8-0	2	2	6-4	2	5-4	
	4-2 × 8 4-2 × 10	7-3 8-8	(L)	5-9 6-10	2	4-11 5-10	2	6-11 8-3	2	5-6 6-6	2	4-8 5-7	2	6-7 7-10	2	2	5-2 6-2	2	4-5 5-3	-
	4-2 × 10	10-2	2	8-1	2	6-10	2	9-8	2	7-8	2	6-7	2	9-2	2		7-3	2	6-2	
	1-2 × 6	2-3	2	1-9	2	1-5	2	2-3	2	1-9	2	1-5	3	2-2	2		1-8	2	1-5	
	1-2 × 8	2-10	2	2-2	3	1-10	3	2-10	2	2-2	3	1-10	3	2-9	2		2-1	3	1-10	Ì
	1-2 × 10	3-4	2	2-7	3	2-2	3	3-4	3	2-7	3	2-2	4	3-3	3	3	2-6	3	2-2	
Roof, ceiling,	1-2 × 12	4-0	3	3-0	3	2-7	4	4-0	3	3-0	4	2-7	4	3-10	3	3	3-0	4	2-6	
and two clear-	2-2 × 4	2-3	1	1-8	1	1-4	1	2-3	্ৰ	1-8	1	1-4	1	2-2	1	1	1-8	1	1-4	
span floors	2-2 × 6	3-4	1	2-6	2	2-2	2	3-4	2	2-6	2	2-2	2	3-3	2		2-6	2	2-1	
	2-2 × 8	4-3	2	3-3	2	2-8	2	4-3	2	3-3	2	2-8	2	4-1	2		3-2	2	2-8	-
-	2-2 × 10 2-2 × 12	5-0 5-11	2	3-10 4-6	2	3-2 3-9	3	5-0 5-11	2	3-10 4-6	3	3-2 3-9	3	4-10 5-8	2		3-9 4-5	3	3-2 3-9	
	2-2 × 12 3-2 × 8	5-11	1	4-6 4-0	2	3-9	2	5-11	2	4-6	2	3-9	2	5-8 5-1	2		4-5 3-11	2	3-9	1
ROOF, CEILING AND TWO FLOORS (CLEAR SPAN)	3-2 x 10	6-3	2	4-9	2	4-0	2	6-3	2	4-9	2	4-0	2	6-1	2		4-8	2	4-0	-
A SCHOOL STATE STATE	3-2 × 12	7-5	2	5-8	2	4-9	3	7-5	2	5-8	2	4-9	3	7-2	2		5-6	3	4-8	-
	4-2 × 8	6-1	1	4-8	2	3-11	2	6-1	1	4-8	2	3-11	2	5-11	1	1	4-7	2	3-10	
	4-2 × 10	7-3	2	5-6	2	4-8	2	7-3	2	5-6	2	4-8	2	7-0	2	2	5-5	2	4-7	
	4-2 × 12	8-6	2	6-6	2	5-6	2	8-6	2	6-6	2	5-6	2	8-3	2	2	6-4	2	5-4	
																				and a

TABLE R602.7(1)GIRDER SPANS<sup>a</sup>AND HEADER SPANS<sup>a</sup>FOR EXTERIOR BEARING WALLS (Maximum spans for Douglas fir-larch, hem-fir, Southern pine and spruce-pine-fir<sup>b</sup>and required number of jack studs)

For SI: 1 inch = 25.4 mm, 1 pound per square foot = 0.0479 kPa.

- a. Spans are given in feet and inches.b. Spans are based on minimum design properties for No. 2 grade lumber of Douglas fir-larch, hem-fir, Southern pine, and spruce-pine-fir.
- c. Building width is measured perpendicular to the ridge. For widths between those shown, spans are permitted to be interpolated.

  d. NJ = Number of jack studs required to support each end. Where the number of required jack studs equals one, the header is permitted to be supported by an approved framing anchor attached to the full-height wall stud and to the header.
- e. Use 30 psf ground snow load for cases in which ground snow load is less than 30 psf and the roof live load is equal to or less than 20 psf.

  f. Spans are calculated assuming the top of the header or girder is laterally braced by perpendicular framing. Where the top of the header or girder is not laterally braced (for example, cripple studs bearing on the header), tabulated spans for headers consisting of 2 × 8, 2 × 10, or 2 × 12 sizes shall be multiplied by 0.70 or the header or

girder shall be designed.

SCALE: 1/4" =1'-0"

DATE: 3/2022

SHEET NO.: 9

GBL CUSTOM HOME

DESIGN INC.
PO BOX 237 FINKSBURG, MD 21048
PHONE 410-833-8320

TABLE R602.7(2)GIRDER SPANS<sup>a</sup>AND HEADER SPANS<sup>a</sup>FOR INTERIOR BEARING WALLS (Maximum spans for Douglas fir-larch, hem-fir, southern pine and spruce-pine-fir<sup>b</sup>and required number of jack studs) BUILDING Width<sup>c</sup> (feet) HEADERS AND GIRDERS 12 36 SIZE SUPPORTING  $NJ^d$  $NJ^d$  $NJ^d$ Spane Spane Spane 2-2 × 4 4-1 2-10 2-4 2-2 × 6 6-1 1 4-4 3-6 1 2 2-2 x 8 7-9 5-5 4-5 9-2 2 2 2-2 × 10 1 6-6 5-3 2-2 × 12 10-9 1 7-7 6-3  $3-2 \times 8$ 9-8 1 6-10 5-7 1 One floor only 3-2 × 10 11-5 8-1 6-7 2 2 3-2 × 12 13-6 1 9-6 7-9  $4-2 \times 8$ 11-2 7-11 6-5 4-2 × 10 13-3 1 9-4 7-8 4-2 x 12 15-7 9-0 11-0 2-2 × 4 2-7 1-11 1-7 2  $2-2 \times 6$ 3-11 1 2-11 2 2-5  $2-2 \times 8$ 5-0 3-8 2 3-1 2 2-2 × 10 5-11 2 4-4 3-7 2 2-2 x 12 6-11 5-2 4-3 2 Two floors  $3-2 \times 8$ 6-3 4-7 2 3-10 7-5 5-6 2 3-2 × 10 4-6 8-8 2 2 2 3-2 × 12 6-5 5-4 7-2  $4-2 \times 8$ 5-4 4-5  $4-2 \times 10$ 8-6 6-4 2 5-3

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm.

- a. Spans are given in feet and inches.
- b. Spans are based on minimum design properties for No. 2 grade lumber of Douglas fir-larch, hem-fir, Southern pine, and spruce-pine-fir.
- c. Building width is measured perpendicular to the ridge. For widths between those shown, spans are permitted to be interpolated.
- d. NJ = Number of jack studs required to support each end. Where the number of required jack studs equals one, the header is permitted to be supported by an approved framing anchor attached to the full-height wall stud and to the header.

4-2 × 12

e. Spans are calculated assuming the top of the header or girder is laterally braced by perpendicular framing. Where the top of the header or girder is not laterally braced by perpendicular framing. Where the top of the header or girder is not laterally braced by perpendicular framing. Where the top of the header or girder is not laterally braced by perpendicular framing. Where the top of the header or girder is not laterally braced by perpendicular framing. girder shall be designed.

2

6-2

2

7-5

## R602.7.5 Supports for headers. INSIGHTS

Headers shall be supported on each end with one or more jack studs or with approved framing anchors in accordance with Table R602.7(1) or R602.7(2). The full-height stud adjacent to each end of the header shall be end nailed to each end of the header with four-16d nails (3.5 inches × 0.135 inches). The minimum number of full-height studs at each end of a header shall be in accordance with Table R602.7.5.

10-1

> INSIGHTS (1)

## TABLE R602.7.5MINIMUM NUMBER OF FULL-HEIGHT STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS<sup>a</sup>

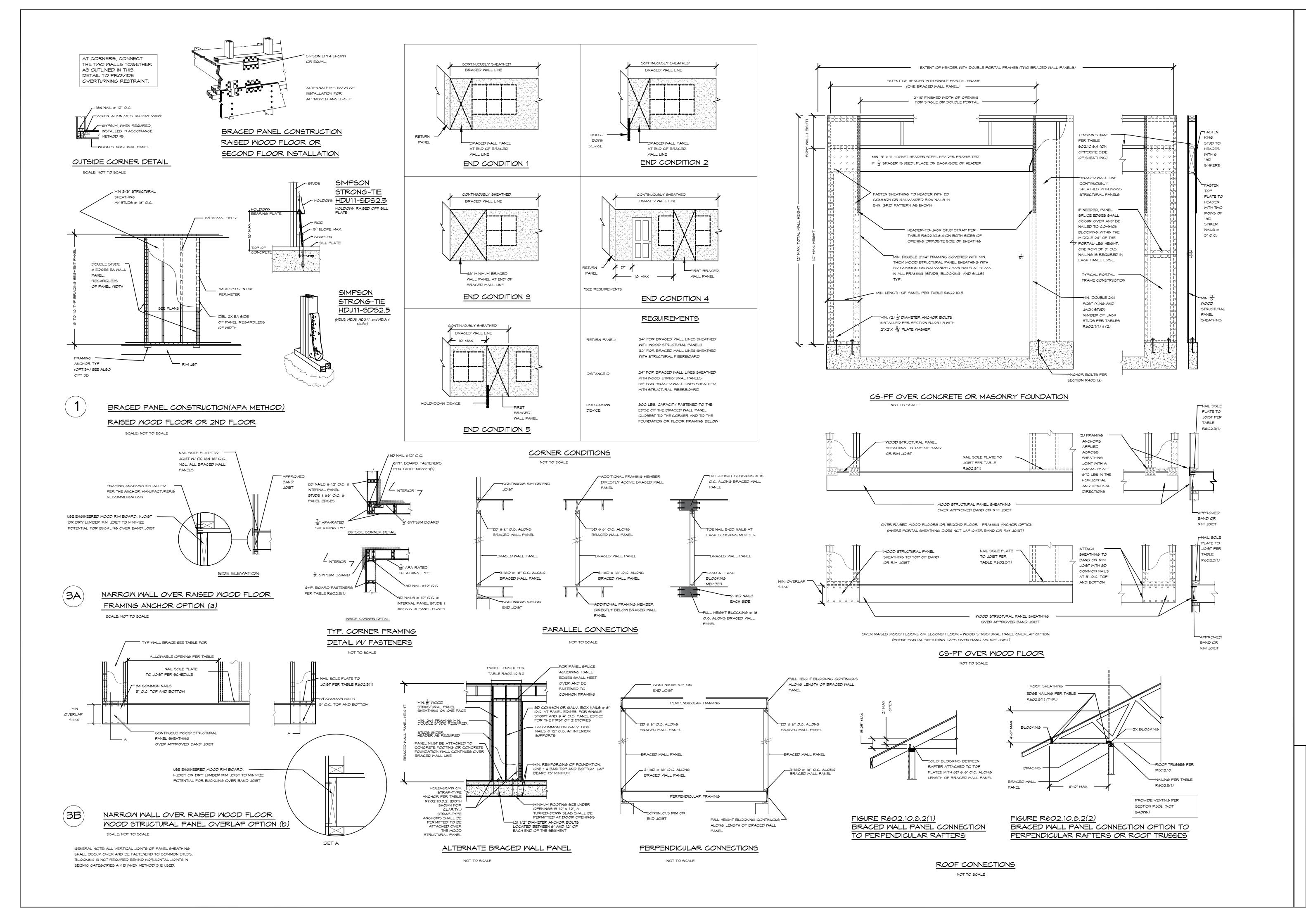
MAXIMUM	ULTIMATE DESIGN WIND SPEED AND EXPOSURE CATEGORY						
HEADER SPAN (feet)	< 140 mph, Exposure B or < 130 mph, Exposure C	≤ 115 mph, Exposure B <sup>b</sup>					
4	1	1					
6	2	1					
8	2	1					
10	3	2					
12	3	2					
14	3	2					
16	4	2					
18	4	2					

For SI: 1 foot = 304.8 mm, 1 mîle per hour = 0.447 m/s.

- a. For header spans between those given, use the minimum number of fullheight studs associated with the larger header span.
- b. The tabulated minimum number of full-height studs is applicable where jack studs are provided to support the header at each end in accordance with Note d of Table R602.7(1), the minimum number of full-height studs at
  - each end of a header shall be in accordance with requirements for wind speed < 140 mph, Exposure B.

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PHONE 410-833-8320



CARUSO HOMES

NALL BRACING NOTES

3/2022

No.: 11

GBL CUSTOM HOME

DESIGN INC.

PO BOX 237 FINKSBURG, MD 21048

PHONE 410-833-8320



1900 AM DRIVE, SUITE 201, QUAKERTOWN, PA 18951 (215) 804 - 4449 www.kse-eng.com

# **FONTANA** VERSION 1.5 MAGNOLIA ACRES LOT 19 RALEIGH, NORTH CAROLINA

THESE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH AND COORDINATED WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS. THIS COORDINATION IS NOT THE RESPONSIBILITY OF THE STRUCTURAL ENGINEER OF RECORD (SER). SHOULD ANY DISCREPANCIES BECOME APPARENT, THE CONTRACTOR SHALL NOTIFY KSE ENGINEERING, P.C. BEFORE CONSTRUCTION BEGINS. IT IS THE INTENT OF THE ENGINEER LISTED ON THESE DOCUMENTS THAT THESE DOCUMENTS BE ACCURATE, PROVIDING LICENSED PROFESSIONALS CLEAR INFORMATION. EVERY ATTEMPT HAS BEEN MADE TO PREVENT ERROR. THE BUILDER AND ALL SUBCONTRACTORS ARE REQUIRED TO REVIEW ALL OF THE INFORMATION CONTAINED IN THESE DOCUMENTS PRIOR TO THE COMMENCEMENT OF ANY WORK. THE ENGINEER IS NOT RESPONSIBLE FOR ANY PLAN ERRORS, OMISSIONS, OR MISINTERPRETATIONS UNDETECTED AND NOT REPORTED TO THE ENGINEER PRIOR TO CONSTRUCTION. ALL CONSTRUCTION MUST BE IN ACCORDANCE TO THE INFORMATION FOUND IN THESE DOCUMENTS.

## DESIGN SPECIFICATIONS:

DESIGN BUILDING CODE (REFERRED TO HEREIN AS 'THE BUILDING CODE'):

2024 NORTH CAROLINA RESIDENTIAL CODE

#### DESIGN LIVE LOADS:

- ROOF = 20 PSF (LOAD DURATION FACTOR=1.25)
- UNINHABITABLE ATTICS WITH LIMITED STORAGE = 20 PSF (WHERE SPECIFIED ON PLANS)
- HABITABLE ATTICS AND ATTICS SERVED WITH FIXED STAIRS = 30 PSF
- FLOOR = 40 PSF
- FLOOR (SLEEPING AREAS) = 30 PSF
- DECK = 40 PSF ■ BALCONY = 40 PSF
- STAIRS = 40 PSF

## DESIGN DEAD LOADS:

- ROOF TRUSS = 17 PSF (TC=7, BC=10)
- FLOOR TRUSS = 15 PSF (TC=10, BC=5)
- FLOOR JOIST = 10 PSF
- STANDARD BRICK = 40 PSF
- QUEEN ANNE BRICK = 25 PSF
- TILE = 10 PSF (WHERE NOTED ON PLANS)

\*NOTE: STRUCTURAL FRAMING HAS NOT BEEN DESIGNED FOR GRANITE, MARBLE OR OTHER MATERIALS HEAVIER THAN THE ABOVE LOADING UNLESS SPECIFICALLY NOTED ON PLANS.\*.

#### DESIGN WIND LOADS:

- ULTIMATE WIND SPEED = 120 MPH
- EXPOSURE CATEGORY = B

ASSUMED SOIL BEARING CAPACITY = 2000 PSF

ASSUMED LATERAL SOIL PRESSURE = 60 PCF

FROST DEPTH = 12"

SEISMIC DESIGN CATEGORY = B

ENGINEERED LUMBER SHALL HAVE THE FOLLOWING MINIMUM DESIGN VALUES:

• TJI 210 SERIES (SERIES AND SPACING PER PLANS)

- LSL: E=1,550,000 PSI,  $F_B=2,325$  PSI,  $F_V=310$  PSI,  $F_C=900$  PSI
- LVL: E=2,000,000 PSI,  $F_B=2,600$  PSI,  $F_V=285$  PSI,  $F_C=750$  PSI ■ PSL: E=2,000,000 PSI, F<sub>B</sub>=2,900 PSI, F<sub>V</sub>=290 PSI, F<sub>C</sub>=625 PSI



Version Sheet

Magnolia Fontana Cover Project #: 108-24004 Designed By: AAM

Checked By: KRK

Issue Date: 3/24/25 Re-Issue: 4/14/25 Scale:



#### GENERAL STRUCTURAL NOTES:

- 1. THE DESIGN PROFESSIONAL WHOSE SEAL APPEARS ON THESE DRAWINGS IS THE STRUCTURAL ENGINEER OF RECORD (SER) FOR THIS PROJECT. THE SER BEARS THE RESPONSIBILITY OF THE PRIMARY STRUCTURAL ELEMENTS AND THE PERFORMANCE OF THIS STRUCTURE NO OTHER PARTY MAY REVISE, ALTER, OR DELETE ANY STRUCTURAL ASPECTS OF THESE CONSTRUCTION DOCUMENTS WITHOUT WRITTEN CONSENT OF KSE ENGINEERING, P.C. OR THE SER. FOR THE PURPOSES OF THESE CONSTRUCTION DOCUMENTS, THE SER AND KSE ENGINEERING SHALL BE CONSIDERED THE SAME ENTITY.
- THE STRUCTURE IS ONLY STABLE IN ITS COMPLETED FORM. THE CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY BRACING DURING CONSTRUCTION TO STABILIZE THE STRUCTURE.
- 3. THE SER IS NOT RESPONSIBLE FOR CONSTRUCTION SEQUENCES, METHODS, OR TECHNIQUES IN CONNECTION WITH THE CONSTRUCTION OF THIS STRUCTURE. THE SER WILL NOT BE HELD RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CONFORM TO THE CONTRACT DOCUMENTS, SHOULD ANY NON-CONFORMITIES OCCUR.
- 4. THE SER DOES NOT CERTIFY DIMENSIONAL ACCURACY OR ARCHITECTURAL LAYOUT INCLUDING ROOF GEOMETRY. THE SER ASSUMES NO LIABILITY FOR CHANGES MADE TO THESE PLANS BY OTHERS, OR FOR CONSTRUCTION METHODS, OR FOR ANY DEVIATION FROM THE PLANS. THE SER SHALL BE NOTIFIED PRIOR TO CONSTRUCTION IF ANY DISCREPANCIES ARE NOTED ON THE PLANS.
- 5. ANY STRUCTURAL ELEMENTS OR DETAILS NOT FULLY DEVELOPED ON THE CONSTRUCTION DRAWINGS SHALL BE COMPLETED UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER. THESE SHOP DRAWINGS SHALL BE SUBMITTED TO KSE ENGINEERING FOR REVIEW BEFORE ANY CONSTRUCTION BEGINS. THE SHOP DRAWINGS WILL BE REVIEWED FOR OVERALL COMPLIANCE AS IT RELATES TO THE STRUCTURAL DESIGN OF THIS PROJECT. VERIFICATION OF THE SHOP DRAWINGS FOR DIMENSIONS, OR FOR ACTUAL FIELD CONDITIONS, IS NOT THE RESPONSIBILITY OF THE SER OR KSE ENGINEERING, P.C.
- 6. VERIFICATION OF ASSUMED FIELD CONDITIONS IS NOT THE RESPONSIBILITY OF THE SER. THE CONTRACTOR SHALL VERIFY THE FIELD CONDITIONS FOR ACCURACY AND REPORT ANY DISCREPANCIES TO KSE ENGINEERING, P.C. BEFORE CONSTRUCTION BEGINS.
- 7. THE SER IS NOT RESPONSIBLE FOR ANY SECONDARY STRUCTURAL ELEMENTS OR NON-STRUCTURAL ELEMENTS, EXCEPT FOR THE ELEMENTS SPECIFICALLY NOTED ON THE STRUCTURAL DRAWINGS.
- 8. THIS STRUCTURE AND ALL CONSTRUCTION SHALL CONFORM TO ALL APPLICABLE SECTIONS OF THE BUILDING CODE AND ANY LOCAL CODES OR RESTRICTIONS.
- 9. DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS. ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS OTHERWISE NOTED.
- 10. PROVIDE MOISTURE PROTECTION AND FLASHING PER ARCHITECTURAL DETAILS.

- 1. FOUNDATIONS SHALL BE CONSTRUCTED IN ACCORDANCE WITH CHAPTER 4 OF THE BUILDING CODE
- 2. CONTRACTOR IS SOLELY RESPONSIBLE FOR VERIFYING THE SUITABILITY OF THE SITE SOIL CONDITIONS AT THE TIME OF CONSTRUCTION. THE BUILDER SHALL FURNISH ANY AND ALL REPORTS RECEIVED FROM THE GEOTECHNICAL ENGINEER ON THE STUDY OF THE PROPOSED SITE TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR.
- 6. MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS TO BE AS SPECIFIED IN THE BUILDING CODE.
- 4. THE SER HAS NOT PERFORMED A SUBSURFACE INVESTIGATION. VERIFICATION OF THE ASSUMED VALUE IS THE RESPONSIBILITY OF THE OWNER OR THE CONTRACTOR. SHOULD ANY ADVERSE SOIL CONDITION BE ENCOUNTERED, THE SER MUST BE CONTACTED BEFORE PROCEEDING.
- 5. THE BOTTOM OF ALL FOOTINGS SHALL EXTEND BELOW THE FROST LINE FOR THE REGION IN WHICH THE STRUCTURE IS TO BE CONSTRUCTED, BUT NOT LESS THAN A MINIMUM OF 12" BELOW GRADE. ALL FOOTINGS TO HAVE A MINIMUM PROJECTION OF 2" ON EACH SIDE OF FOUNDATION WALLS. MAXIMUM FOOTING PROJECTION SHALL NOT EXCEED THE THICKNESS OF THE FOOTING.
- 6. WOOD SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WITH 1/2" ANCHOR BOLTS WITH MINIMUM 7" EMBEDMENT, SPACED A MAXIMUM of 6'-0" o.c. install minimum 2 anchor bolts per section, 12" MASONRY MAXIMUM FROM CORNERS. 1/2" DIAMETER x 8" LONG SIMPSON TITEN HD OR USP SCREW-BOLT+ SCREWS MAY BE SUBSTITUTED ON A 1 FOR 1 BASIS.
- 7. ANY FILL SHALL BE PLACED UNDER THE DIRECTION OR RECOMMENDATION OF A LICENSED PROFESSIONAL ENGINEER. THE RESULTING SOIL SHALL BE COMPACTED TO A MINIMUM OF 95% MAXIMUM DRY DENSITY.
- 8. EXCAVATIONS OF FOOTINGS SHALL BE LINED TEMPORARILY WITH A 6 MIL POLYETHYLENE MEMBRANE IF PLACEMENT OF CONCRETE DOES NOT OCCUR WITHIN 24 HOURS OF EXCAVATION.
- 9. NO CONCRETE SHALL BE PLACED AGAINST ANY SUBGRADE CONTAINING WATER, ICE, FROST, OR LOOSE MATERIAL.
- 10. PROVIDE FOUNDATION WATERPROOFING AND DRAIN WITH POSITIVE SLOPE TO OUTLET AS REQUIRED BY SITE CONDITIONS (SEE ARCHITECTURAL PLANS AND DETAILS).
- 11. NONE OF THE FOUNDATION DESIGNS IN THESE DOCUMENTS ARE SUITABLE FOR INSTALLATION IN SHRINK/SWELL CONDITIONS. REFER TO GEOTECHNICAL ENGINEER FOR APPROPRIATE DESIGN.
- 12. LOTS SHALL BE GRADED TO DRAIN SURFACE WATER AWAY FROM FOUNDATION WALLS. THE GRADE SHALL FALL A MINIMUM OF 6 INCHES WITHIN THE FIRST TEN FEET.
- 13. CRAWL SPACE TO BE GRADED LEVEL AND CLEAR OF ALL DEBRIS. 14. PROVIDE MINIMUM 6 MIL APPROVED VAPOR BARRIER. ALL JOINTS TO BE LAPPED MINIMUM 12" AND SEALED.

#### CONCRETE & REINFORCING

- 1. CONCRETE DESIGN BASED ON ACI 318 AND ACI 318.1 OR ACI 332. CONCRETE SHALL HAVE A NORMAL WEIGHT AGGREGATE AND A MINIMUM COMPRESSIVE STRENGTH (f'c) = 3,000 PSI MINIMUM AT 28 DAYS PER CODE (VARIES W/ WEATHER), UNLESS OTHERWISE NOTED ON THE PLAN.
- CONCRETE SHALL BE PROPORTIONED, MIXED, AND PLACED IN ACCORDANCE WITH THE LATEST EDITIONS OF ACI 318: "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" AND ACI 301: "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS".
- AIR ENTRAINED CONCRETE MUST BE USED FOR ALL STRUCTURAL ELEMENTS EXPOSED TO FREEZE/THAW CYCLES AND DEICING CHEMICALS. AIR ENTRAINMENT AMOUNTS (IN PERCENT) SHALL BE WITHIN -1% TO +2% OF 5% FOR FOOTINGS AND EXTERIOR SLABS
- NO ADMIXTURES SHALL BE ADDED TO ANY STRUCTURAL CONCRETE WITHOUT WRITTEN PERMISSION OF THE SER. WATER ADDED TO CONCRETE ON SITE SHALL NOT EXCEED THAT ALLOWED BY THE MIX
- 5. CONCRETE SLABS-ON-GRADE SHALL BE CONSTRUCTED IN ACCORDANCE WITH ACI 302.1R: "GUIDE FOR CONCRETE SLAB AND SLAB CONSTRUCTION".
- 6. CONTROL OR SAW CUT JOINTS (CUT OR TOOLED) SHALL BE SPACED IN INTERIOR SLABS-ON-GRADE AT A MAXIMUM OF 15'-0" O.C. AND IN EXTERIOR SLABS-ON-GRADE AT A MAXIMUM OF 10'-0" UNLESS OTHERWISE NOTED. CARE SHALL BE TAKEN TO AVOID RE-ENTRANT CORNERS.
- CONTROL OR SAW CUT JOINTS SHALL BE PRODUCED USING CONVENTIONAL CUT OR TOOLED PROCESSES WITHIN 4 TO 12 HOURS AFTER THE SLAB HAS BEEN FINISHED.
- 8. ALL WELDED WIRE FABRIC (W.W.F.) FOR CONCRETE SLABS-ON-GRADE SHALL BE PLACED AT MID-DEPTH OF SLAB. THE W.W.F. SHALL BE SECURELY SUPPORTED DURING THE CONCRETE POUR. FIBROUS CONCRETE REINFORCEMENT, OR POLYPROPYLENE FIBERS MAY BE USED IN LIEU OF W.W.F. APPLICATION OF POLYPROPYLENE FIBERS PER CUBIC YARD OF CONCRETE SHALL BE PER MANUFACTURER AND COMPLY WITH ASTM C1116, ANY LOCAL BUILDING CODE REQUIREMENTS AND SHALL MEET OR EXCEED CURRENT INDUSTRY STANDARD.
- POLYPROPYLENE REINFORCING TO BE 100% VIRGIN, CONTAINING NO REPROCESSED OLEFIN MATERIALS AND SPECIFICALLY MANUFACTURED FOR USE AS CONCRETE SECONDARY REINFORCEMENT.
- 10. STEEL REINFORCING BARS SHALL BE NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60.
- 11. DETAILING, FABRICATION, AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 315: "MANUAL OF STANDARD PRACTICE FOR DETAILING CONCRETE STRUCTURES".
- 12. HORIZONTAL FOOTING AND WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90° BENDS, OR CORNER BARS WITH
- THE SAME SIZE/SPACING AS THE HORIZONTAL REINFORCEMENT. 13. PROVIDE REINFORCEMENT LAP AS NOTED BELOW, UNLESS NOTED OTHERWISE:
- #4 BARS 30" LENGTH #5 BARS - 38" LENGTH
- #6 BARS 45" LENGTH 14. WHERE REINFORCING DOWELS ARE REQUIRED, THEY SHALL BE EQUIVALENT IN SIZE AND SPACING TO THE VERTICAL REINFORCEMENT. THE DOWEL SHALL EXTEND 48 BAR DIAMETERS VERTICALLY AND 20 BAR DIAMETERS INTO THE FOOTING. SEE KSE FOUNDATION DETAILS.
- 15. WHERE FOOTING BOTTOMS ARE TO BE STEPPED AT SLOPING GRADE CONDITIONS, PROVIDE CONTINUOUS REINFORCING WITH Z BARS (TO MATCH FOOTING REINFORCING) AS REQUIRED.
- 16. BAR SUPPORT ACCESSORIES SHALL BE PROVIDED IN ACCORDANCE WITH THE LATEST ACI MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES, EXCEPT THAT REINFORCING SHALL BE CHAIRED ON THE BOTTOM AND/OR THE SIDES ON BOLSTERS SPACED NOT MORE THAN 4 FEET ON CENTER. NO ROCKS, CMU, CLAY TILE. OR BRICK SHALL BE USED TO SUPPORT REINFORCING.
- 17. FOR GRADE SUPPORTED SLABS, SLAB REINFORCING SHALL BE HELD IN PLACE BY BAR SUPPORTS AND ACCESSORIES AS DESCRIBED IN THE CRSI MANUAL OF STANDARD PRACTICE. BAR SUPPORTS SHALL BE SPACED A MAXIMUM OF 4'-0" O.C. BOTH WAYS IN STRAIGHT LINES ON THE MESH GRID.

- 1. ALL MASONRY SHALL CONFORM TO ASTM C-90, F'm=1500 PSI. ALL BRICK SHALL CONFORM TO ASTM C-216, F'm=1500 PSI. ALL MORTAR SHALL BE TYPE 'S' (TYPE 'M' BELOW GRADE) AND CONFORM TO ASTM C-270. COARSE GROUT SHALL CONFORM TO ASTM C-476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH OF 2,000
- 2. ALL MASONRY WORK SHALL BE IN ACCORDANCE WITH "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" ACI 530/ASCE 5/TMS 402 AND "SPECIFICATIONS FOR MASONRY STRUCTURES" ACI 530.1/ ASCE 6/TMS 602.
- THE UNSUPPORTED HEIGHT OF SOLID MASONRY PIERS SHALL NOT EXCEED TEN TIMES THEIR LEAST DIMENSION. UNFILLED HOLLOW PIERS MAY BE USED IF THE UNSUPPORTED HEIGHT IS NOT MORE THAN FOUR 2. RAFTERS SHALL BE SUPPORTED BY PURLINS AND PURLIN BRACES TIMES THEIR LEAST DIMENSION.
- 4. EACH CRAWL SPACE PIER SHALL BEAR IN THE MIDDLE THIRD OF ITS RESPECTIVE FOOTING AND EACH GIRDER SHALL BEAR IN THE MIDDLE THIRD OF THE PIERS. PILASTERS TO BE BONDED TO PERIMETER FOUNDATION WALL.
- 5. TOP COURSE OF MASONRY SHALL BE GROUTED SOLID.
- 6. HORIZONTAL WALL JOINT REINFORCEMENT SHALL BE STANDARD 9 GAGE GALVANIZED LADDER OR TRUSS TYPE SPACED AT 16" O.C., UNLESS SHOWN OTHERWISE ON THE DRAWINGS.
- SPLICED WIRE REINFORCEMENT SHALL BE LAPPED AT LEAST 6" AND CONTAIN AT LEAST ONE CROSS WIRE OF EACH PIECE OF REINFORCEMENT WITHIN THE 6". LAP WITH STANDARD 'T' AND 'L' SHAPED PIECES AT INTERSECTIONS AND CORNERS.

#### WOOD FRAMING

- 1. SOLID SAWN WOOD FRAMING MEMBERS SHALL CONFORM TO THE SPECIFICATIONS LISTED IN THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION": (NDS). UNLESS OTHERWISE NOTED, ALL WOOD FRAMING MEMBERS ARE DESIGNED TO
- SPRUCE-PINE-FIR (SPF) WITH THE FOLLOWING MINIMUM DESIGN
- $E=1,400,000 \text{ PSI}, F_b=875 \text{ PSI}, F_v=135 \text{ PSI}$ 1.1. FRAMING: SPF #2.
- 1.2. PLATES: SPF #2. 1.3. STUDS: SPF STUD GRADE.
- 2. WALL STUD SPACING, (MAXIMUM 10' NOMINAL PLATE HEIGHT): 1 & 2 STORY EXTERIOR AND INTERIOR BEARING:
- 2x4 @ 16" O.C. OR 2x6 @ 24" O.C., U.N.O. BOTTOM OF 3 STORIES EXTERIOR AND INTERIOR BEARING: 2x6 @ 16" O.C., U.N.O.
- INTERIOR NON-BEARING: 2x @ 24" O.C., U.N.O.
- 3. ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE TREATED SOUTHERN YELLOW PINE #2 OR BETTER.
- 4. ANCHOR SILL PLATES IN ACCORDANCE W/ GENERAL STRUCTURAL NOTES. 5. ALL BEAMS SPECIFIED ARE MINIMUM SIZES ONLY. LARGER MEMBERS MAY
- BE SUBSTITUTED AS NEEDED FOR EASE OF CONSTRUCTION. 6. NAILS SHALL BE COMMON WIRE NAILS UNLESS OTHERWISE NOTED.
- 7. BOLT HOLES AND LEAD HOLES FOR LAG SCREWS SHALL BE IN ACCORDANCE WITH NDS SPECIFICATIONS.
- 8. INDIVIDUAL STUDS FORMING A COLUMN SHALL BE ATTACHED WITH (2) ROWS 10d NAILS @ 6" O.C. STAGGERED. THE STUD COLUMN SHALL BE FULLY BLOCKED AT ALL FLOOR LEVELS TO ENSURE PROPER LOAD TRANSFER. WALL SHEATHING SHALL BE NAILED TO EDGE OF EACH STUD.
- 9. FACE NAIL ALL MULTI-PLY BEAMS AND HEADERS WITH (2) ROWS 16d COMMON NAILS @ 16" O.C., STAGGERED, OR PER MANUFACTURER'S SPECIFICATIONS FOR ENGINEERED LUMBER. APPLY NAILING FROM BOTH FACES FOR (3) OR MORE PLIES.
- 10. FASTEN 4-PLY BEAMS WITH (1)  $\frac{1}{2}$ " DIAMETER THROUGH BOLT W/ NUTS AND WASHERS AT 12" O.C. STAGGERED TOP AND BOTTOM, 1/2" MINIMUM EDGE DISTANCE. (UNLESS OTHERWISE NOTED)
- 11. ALL BEAMS AND HEADERS SHALL HAVE (1)2x JACK STUD & (1)2x KING STUD UNLESS OTHERWISE NOTED. THE NUMBER OF STUDS INDICATED ON PLANS ARE THE TOTAL NUMBER OF JACK STUDS REQUIRED, UNLESS OTHERWISE NOTED.
- 12. PROVIDE KING STUDS AT EACH END OF HEADERS AS NOTED BELOW. (1) STUD UP TO 6' OPENING (2) STUDS UP TO 8' OPENING (3) STUDS UP TO 9' OPENING
- 13. ALL BEAMS TO BE CONTINUOUSLY SUPPORTED LATERALLY AND SHALL BEAR FULL WIDTH ON THE SUPPORTING WALLS OR COLUMNS INDICATED WITH A MINIMUM OF TWO STUDS, UNLESS OTHERWISE NOTED. ALL BEAM
- SPLICES SHALL OCCUR OVER SUPPORTS. 14. SOLID BLOCKING TO BE PROVIDED AT ALL POINT LOADS THROUGH FLOOR LEVELS TO THE FOUNDATION OR TO OTHER STRUCTURAL COMPONENTS. 15. ALL LUMBER SPECIFIED ON DRAWINGS IS INTENDED FOR DRY USE ONLY
- (MOISTURE CONTENT <19%) UNLESS OTHERWISE NOTED 16. ALL WATERPROOFING AND FIRE SAFETY SYSTEMS ARE THE RESPONSIBILITY OF THE CONTRACTOR AND ARE TO BE DESIGNED AND
- DETAILED BY OTHERS. 17. ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIAMETER SHALL HAVE STUD PROTECTION SHIELDS. ALL HOLES OVER 1" IN DIAMETER FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 OR USP STS1 STUD SHOES, TYPICAL, UNLESS OTHERWISE NOTED.
- 18. BEARING WALLS SHALL BE SHEATHED ON NOT LESS THAN ONE SIDE WITH OSB OR GYPSUM BOARD. BRIDGING SHALL BE INSTALLED NOT GREATER THAN 4 FEET APART MEASURED VERTICALLY FROM EITHER END OF THE STUD IN LIEU OF SHEATHING.
- 19. DIAGONAL BRACING SHALL BE INSTALLED AT EACH END OF BASEMENT BEARING WALLS AND NOT MORE THAN 20' ON CENTER.

#### EXTERIOR WOOD FRAMED DECKS:

- DECKS ARE TO BE FRAMED IN ACCORDANCE WITH APPLICABLE BUILDING CODES AND AS REFERENCED ON THE STRUCTURAL PLANS, EITHER THROUGH CODE REFERENCES OR CONSTRUCTION DETAILS. 2. PRESERVATIVE TREATED WOOD FRAMING TO BE SOUTHERN YELLOW PINE #2 OR BETTER.
- 3. GUARD RAILS REQUIRED AT DECKS. DESIGN BY OTHERS TO MEET MINIMUM CODE REQUIREMENTS.
- 4. PROVIDE DECK LATERAL LOAD AND BRACING CONNECTIONS PER BUILDING

## RAFTER FRAMED ROOF CONSTRUCTION:

PROVIDE 2x4x4'-0" RAFTER TIES AT 48" O.C. AS SHOWN ON THE PLAN. PURLIN BRACES SHALL NOT BEAR ON ANY CEILING JOIST, STRONGBACK OR HEADER UNLESS SPECIFICALLY

SHOWN ON PLAN. RAFTERS MAY BE SPLICED AT PURLIN LOCATIONS.

- 3. CEILING JOISTS SHALL HAVE LATERAL SUPPORT W/ 1x4 FLAT BRACING ON TOP EDGE OF JOIST AT LOOSE JOIST ENDS (WHERE JOISTS NOT FASTENED TO RAFTERS) OR FULL DEPTH BLOCKING. FASTEN END OF BRACING TO RAFTER OR GABLE END FRAMING.
- 4. FASTEN RAFTER AND CEILING JOIST WITH (6) 12d NAILS UNLESS OTHERWISE NOTED.
- 5. PROVIDE VERTICAL 2x6 STRONGBACKS AT CEILING JOISTS @ 8'-0" O.C. TIE STRONGBACK ENDS TO GABLE STUDS OR RAFTERS WHERE POSSIBLE. PROVIDE BLOCKING BETWEEN TOP PLATES AND STRONGBACKS. PROVIDE 2x4 FLAT FASTENED TO EACH JOIST WITH (2) 12d NAILS. FASTEN STRONGBACK TO 2x4 FLAT WITH 12d NAILS @ 12" O.C. AND FASTENED TO EACH JOIST WITH (1) 12d TOENAIL.

## WOOD TRUSSES (FLOOR & ROOF)

- 1. THE WOOD TRUSS MANUFACTURER/FABRICATOR IS RESPONSIBLE FOR THE DESIGN OF THE WOOD TRUSSES. SUBMIT SEALED SHOP DRAWINGS AND SUPPORTING CALCULATIONS TO THE SER FOR REVIEW PRIOR TO FABRICATION. THE SER SHALL HAVE A MINIMUM OF (5) DAYS FOR REVIEW. THE REVIEW BY THE SER SHALL BE FOR OVERALL COMPLIANCE OF THE DESIGN DOCUMENTS. THE SER SHALL ASSUME NO RESPONSIBILITY FOR THE CORRECTNESS OF THE STRUCTURAL DESIGN FOR THE WOOD TRUSSES.
- 2. THE WOOD TRUSSES SHALL BE DESIGNED FOR ALL REQUIRED LOADINGS AS SPECIFIED IN THE LOCAL BUILDING CODE, THE ASCE STANDARD "MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES." (ASCE 7), AND THE LOADING REQUIREMENTS SHOWN ON THESE SPECIFICATIONS. THE TRUSS DRAWINGS SHALL BE COORDINATED WITH ALL OTHER CONSTRUCTION DOCUMENTS AND PROVISIONS PROVIDED FOR LOADS SHOWN ON THESE DRAWINGS INCLUDING BUT NOT LIMITED TO HVAC EQUIPMENT, PIPING, AND ARCHITECTURAL FIXTURES ATTACHED TO THE TRUSSES.
- 3. THE TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE ANSI/TPI 1: "NATIONAL DESIGN STANDARD FOR METAL PLATE CONNECTED WOOD TRUSS CONSTRUCTION"
- 4. THE TRUSS MANUFACTURER SHALL PROVIDE ADEQUATE BRACING INFORMATION IN ACCORDANCE WITH "BUILDING COMPONENT SAFETY INFORMATION GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING, RESTRAINING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES" (BCSI). THIS BRACING, BOTH TEMPORARY AND PERMANENT, SHALL BE SHOWN ON THE SHOP DRAWINGS. ALSO, THE SHOP DRAWINGS SHALL SHOW THE REQUIRED ATTACHMENTS FOR THE TRUSSES
- THE CONTRACTOR IS RESPONSIBLE FOR INSTALLING TEMPORARY BRACING AND SHORING FOR THE FLOOR AND ROOF TRUSSES AS REQUIRED DURING CONSTRUCTION. AT A MINIMUM, CONTRACTOR SHALL FOLLOW THE REQUIREMENTS OF THE LATEST BCSI. THE CONTRACTOR SHALL KEEP A COPY OF THE BCSI SUMMARY SHEETS ON SITE.
- THE CONTRACTOR IS RESPONSIBLE FOR INSTALLING ALL PERMANENT TRUSS BRACING SHOWN IN THE STRUCTURAL DRAWINGS AND IN THE TRUSS DESIGNS. ALL CONTINUOUS LATERAL BRACING OF WEBS REQUIRES BRACES. REFER TO BCSI SUMMARY SHEET B3 FOR TYPES OF DIAGONAL BRACES TO PROVIDE AT EACH CONTINUOUS LATERAL BRACE LINE. SUCH DIAGONAL BRACES SHALL NOT BE SPACED MORE THAN 20 FEET O.C. DIAGONAL BRACES SHALL BE FASTENED TO EACH TRUSS WEB WITH A MINIMUM OF TWO 10d FACE NAILS. WHERE CONTINUOUS LATERAL BRACING CANNOT BE INSTALLED, DUE TO A MINIMUM OF THREE ADJACENT TRUSSES NOT BEING IDENTICAL, THE CONTRACTOR SHALL COORDINATE WITH THE TRUSS SPECIALTY ENGINEER/MANUFACTURER TO DETERMINE WHAT TYPE OF ALTERNATE BRACE (I.E., T OR L BRACE, ETC.) IS REQUIRED.
- 7. ANY CHORDS OR TRUSS WEBS SHOWN ON THESE DRAWINGS HAVE BEEN SHOWN AS A REFERENCE ONLY. THE FINAL DESIGN OF THE TRUSSES SHALL BE PER THE MANUFACTURER.
- 8. TRUSS LAYOUT AND PLACEMENT BY MANUFACTURER TO COINCIDE WITH THE SUPPORT LOCATIONS SHOWN ON THE SEALED STRUCTURAL DRAWINGS. TRUSS PROFILES TO BE SEALED BY THE TRUSS MANUFACTURER. TRUSS PLANS TO BE COORDINATED WITH THE SEALED STRUCTURAL DRAWINGS.
- 9. TRUSS MANUFACTURER TO PROVIDE REQUIRED UPLIFT CONNECTORS FOR ALL TRUSSES
- 10. PROVIDE SIMPSON H2.5A, USP RT7 OR EQUIVALENT AT EACH TRUSS TO TOP PLATE CONNECTION, UNLESS OTHERWISE NOTED.

#### WOOD STRUCTURAL PANELS:

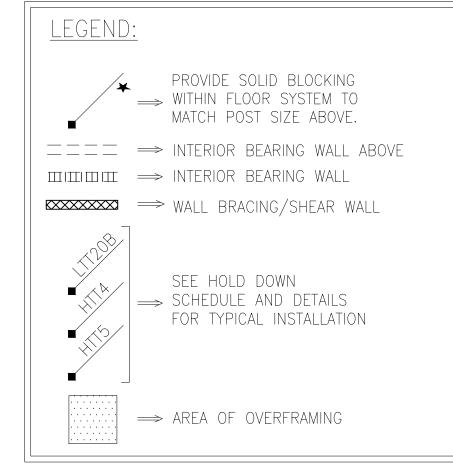
- 1. FABRICATION AND PLACEMENT OF STRUCTURAL WOOD SHEATHING SHALL BE IN ACCORDANCE WITH THE APA DESIGN/CONSTRUCTION GUIDE "RESIDENTIAL AND COMMERCIAL," AND ALL OTHER APPLICABLE APA STANDARDS.
- 2. ALL REQUIRED WOOD SHEATHING SHALL BEAR THE MARK OF THE APA.
- 3. WOOD WALL SHEATHING SHALL COMPLY WITH THE REQUIREMENTS OF LOCAL BUILDING CODES FOR THE APPROPRIATE STATE AS INDICATED ON THESE DRAWINGS. REFER TO WALL BRACING NOTES IN PLAN SET FOR MORE INFORMATION. EXTERIOR WALLS TO BE FULLY SHEATHED USING  $\frac{7}{6}$ " OSB OR PLYWOOD MINIMUM. AT BRACED WALL PANELS, PROVIDE BLOCKING AT ALL SHEET EDGES NOT FALLING ON STUDS OR PLATES.
- ROOF SHEATHING SHALL BE APA RATED SHEATHING EXPOSURE 1 OR 2. ROOF SHEATHING SHALL BE CONTINUOUS OVER TWO SUPPORTS MINIMUM AND ATTACHED TO ITS SUPPORTING ROOF FRAMING WITH 8d NAILS AT 6" O.C. AT PANEL EDGES AND AT 6" O.C. IN PANEL FIELD UNLESS OTHERWISE NOTED ON THE PLANS. SHEATHING SHALL BE APPLIED WITH THE LONG DIRECTION PERPENDICULAR TO FRAMING. SHEATHING SHALL HAVE A SPAN RATING CONSISTENT WITH THE FRAMING SPACING. PROVIDE SUITABLE EDGE SUPPORT BY USE OF PLYWOOD CLIPS OR LUMBER BLOCKING UNLESS OTHERWISE NOTED. PANEL END JOINTS SHALL OCCUR OVER FRAMING. ROOF SHEATHING TO BE  $\frac{7}{16}$ " OSB MINIMUM.
- WOOD FLOOR SHEATHING SHALL BE APA RATED SHEATHING EXPOSURE 1 OR 2. ATTACH SHEATHING TO ITS SUPPORTING FRAMING WITH (1) 10d NAIL AT 6" O.C. AT PANEL EDGES AND AT 12" O.C. IN PANEL FIELD UNLESS OTHERWISE NOTED ON THE PLANS. SHEATHING SHALL BE APPLIED PERPENDICULAR TO FRAMING. SHEATHING SHALL HAVE A SPAN RATING CONSISTENT WITH THE FRAMING SPACING. PROVIDE SUITABLE EDGE SUPPORT BY USE OF T&G PLYWOOD OR LUMBER BLOCKING UNLESS OTHERWISE NOTED. PANEL END JOINTS SHALL OCCUR OVER FRAMING.
- 6. SHEATHING SHALL HAVE A 1/2" GAP AT PANEL ENDS AND EDGES AS RECOMMENDED IN ACCORDANCE WITH THE APA.

#### STRUCTURAL FIBERBOARD PANELS:

- STRUCTURAL FIBERBOARD SHEATHING SHALL ONLY BE USED WHERE SPECIFICALLY NOTED ON THE STRUCTURAL PLANS.
- 2. FABRICATION AND PLACEMENT OF STRUCTURAL FIBERBOARD SHEATHING SHALL BE IN ACCORDANCE WITH THE APPLICABLE AFA
- 3. FIBERBOARD WALL SHEATHING SHALL COMPLY WITH THE REQUIREMENTS OF LOCAL BUILDING CODES FOR THE APPROPRIATE STATE AS INDICATED ON THESE DRAWINGS. REFER TO WALL BRACING NOTES IN PLAN SET FOR MORE INFORMATION.
- 4. SHEATHING SHALL HAVE A 1/8" GAP AT PANEL ENDS AND EDGES AS RECOMMENDED IN ACCORDANCE WITH THE AFA.

- STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AND OF THE MANUAL OF STEEL CONSTRUCTION "LOAD RESISTANCE FACTOR DESIGN" LATEST EDITIONS.
- 2. ALL STEEL SHALL HAVE A MINIMUM YIELD STRESS  $(F_v)$  OF 50 KSI UNLESS OTHERWISE NOTED.
- 3. WELDING SHALL CONFORM TO THE LATEST EDITION OF THE AMERICAN WELDING SOCIETY'S STRUCTURAL WELDING CODE AWA D1.1. ELECTRODES FOR SHOP AND FIELDING WELDING SHALL BE CLASS E70XX. ALL WELDING SHALL BE PERFORMED BY A CERTIFIED WELDER PER THE ABOVE STANDARDS
- 4. ALL STEEL BEAMS TO BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3½" AND FULL FLANGE WIDTH UNLESS OTHERWISE NOTED. BEAMS MUST BE ATTACHED AT EACH END WITH A MINIMUM OF FOUR 16d NAILS OR (2) 1/2" x 4" LAG SCREWS UNLESS OTHERWISE NOTED.
- INSTALL 2x WOOD PLATE ON TOP OF STEEL BEAMS, RIPPED TO MATCH BEAM WIDTH. FASTEN PLATE TO BEAM W/ HILTI X-DNI 52 P8 PINS AT 12" O.C. STAGGERED OR 1/2" DIAMETER BOLTS AT 24"

- 1. ALL METAL HARDWARE AND FASTENERS TO BE SIMPSON STRONG-TIE
- OR APPROVED EQUIVALENT. 2. ALL HARDWARE AND FASTENERS IN CONTACT WITH PRESERVATIVE PRESSURE TREATED LUMBER SHALL BE HOT DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A 153, G-185.
- 3. MANY OF THE NEW PRESSURE TREATED WOODS USE CHEMICALS THAT ARE CORROSIVE TO STEEL. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE TYPE OF WOOD TREATMENT AND SELECT APPROPRIATE CONNECTORS THAT WILL RESIST THE APPLICABLE CORROSIVE CHEMICALS.



BRICK	VENEER LINTEL SC	HEDULE					
SPAN	LINTEL SIZE	END BEARING					
UP TO 3'-0"	3½"×3½"×¾"	4"					
UP TO 6'-3"	5"x3½"x5⁄ <sub>16</sub> " L.L.V.	8"					
UP TO 9'-6"	6"x3½"x5½" L.L.V. 12"						
LINTELS ARE NOT DESIGNED TO BE BOLTED TO HEADERS UNLESS SPECIFIED ON UNIT PLANS.							

SPANS OVER 4'-0" SHALL BE SHORED UP UNTIL CURED.



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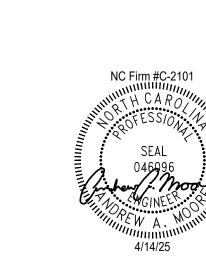
 $\leftarrow$ 108-20000 Designed By: KRK

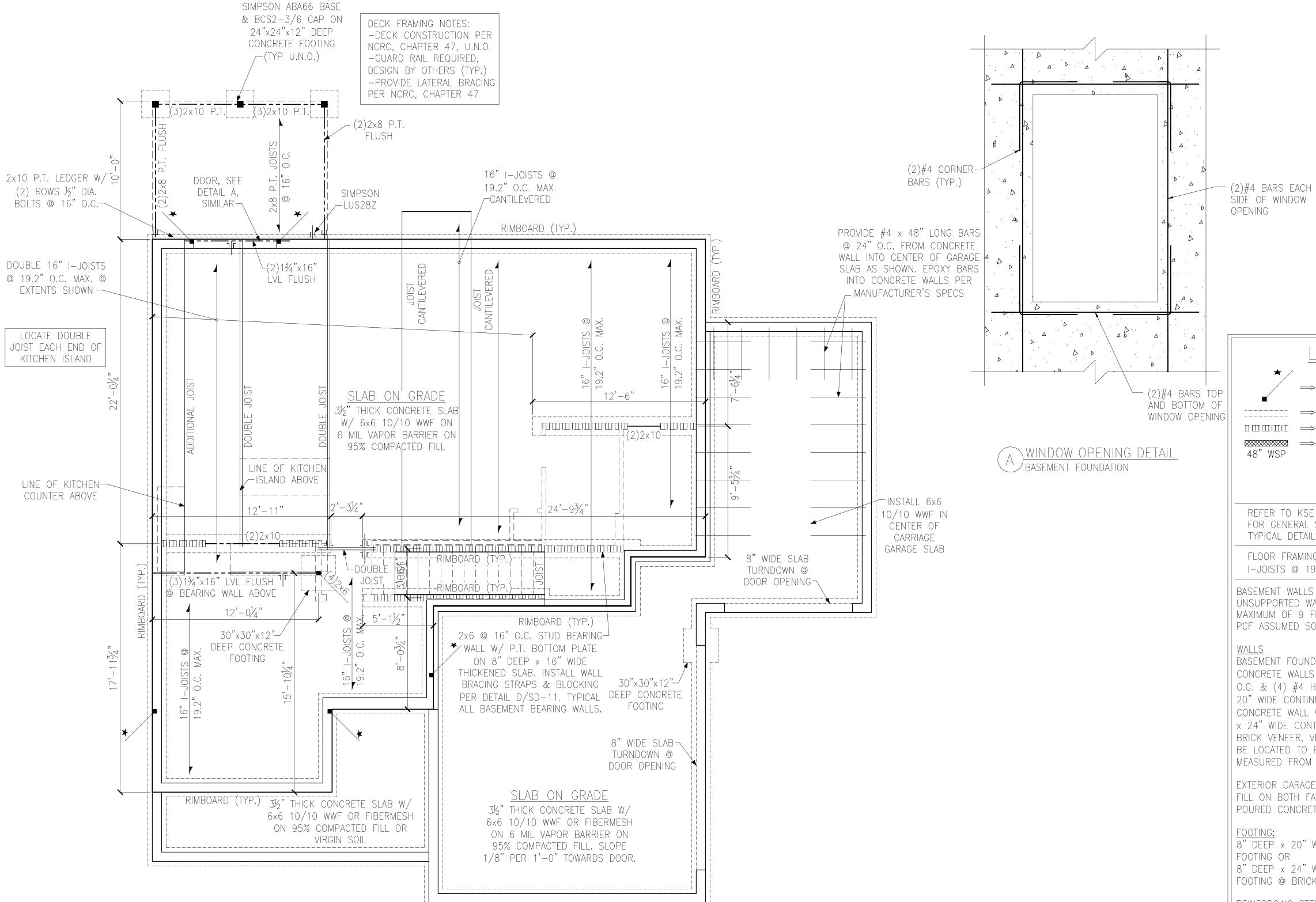
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Re-Issue: Scale: 1/8"=1'-0" @ 11x171/4"=1'-0" @ 22x34

# BASEMENT FOUNDATION PLAN

6x6 P.T. POST W/





LEGEND

PROVIDE SOLID BLOCKING ⇒ WITHIN FLOOR SYSTEM TO MATCH POST SIZE ABOVE.

⇒ BEARING WALL ABOVE ⇒ INTERIOR BEARING WALL ⇒ BRACED WALL PANEL 

(SEE KSE STRUCTURAL DETAILS SET FOR BRACED WALL PANEL SHEATHING FASTENING & BLOCKING DETAILS)

REFER TO KSE STRUCTURAL DETAILS SET FOR GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS

FLOOR FRAMING TO BE 16" DEEP TJI 210 I-JOISTS @ 19.2" O.C. MAXIMUM OR EQUAL

BASEMENT WALLS DESIGNED FOR MAXIMUM UNSUPPORTED WALL HEIGHT OF 10 FEET WITH MAXIMUM OF 9 FEET OF UNBALANCED FILL. 45 PCF ASSUMED SOIL PRESSURE.

<u>WALLS</u>

BASEMENT FOUNDATION WALLS TO BE 10" POURED CONCRETE WALLS W/ #4 VERTICAL BARS @ 16"  $\parallel$  O.C. & (4) #4 HORIZONTAL BARS ON 8" DEEP ightarrow20" WIDE CONTINUOUS CONCRETE FOOTING, 10" CONCRETE WALL W/ 4" BRICK LEDGE ON 8" DEEP x 24" WIDE CONTINUOUS CONCRETE FOOTING @ BRICK VENEER. VERTICAL REINFORCEMENT SHALL BE LOCATED TO PROVIDE A COVER OF 1.25" MEASURED FROM THE INSIDE FACE OF THE WALL.

EXTERIOR GARAGE WALLS AND PORCH WALLS WITH | FILL ON BOTH FACES TO BE 8" UN-REINFORCED POURED CONCRETE

8" DEEP x 20" WIDE CONTINUOUS CONCRETE FOOTING OR

8" DEEP x 24" WIDE CONTINUOUS CONCRETE FOOTING @ BRICK VENEER.

REINFORCING STEEL NOT REQUIRED IN FOOTING UNLESS NOTED OTHERWISE. BUILDER MAY EXCEED THE CODE REQUIREMENTS AT THEIR OWN DISCRETION.

Basement Magnolia A Fontana V 120 M.P.H Raleigh, N

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Project #: 108-24004

Designed By: AAM Checked By: KRK

Plan

Foundation

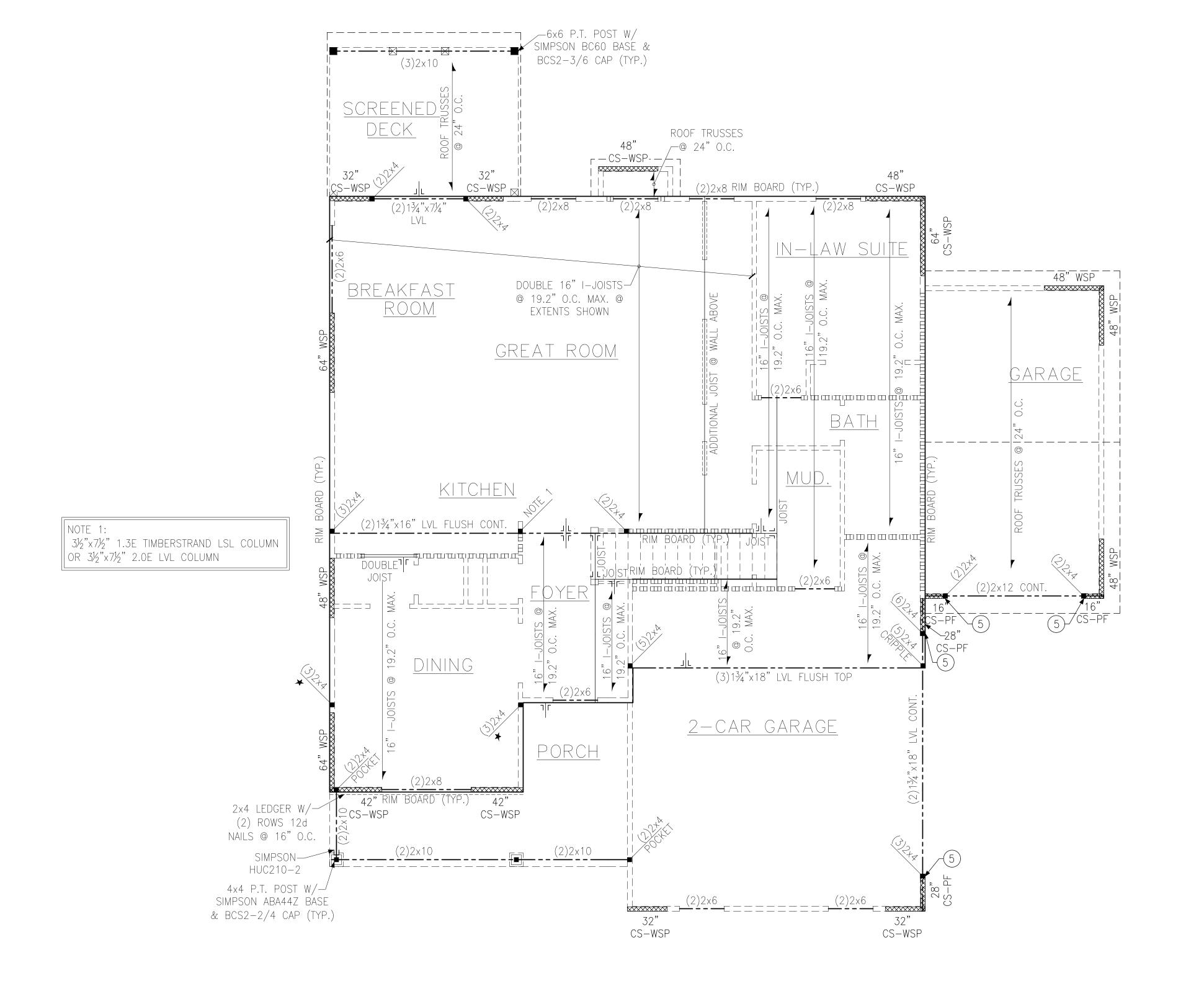
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Issue Date: 3/24/25 Re-Issue: 4/14/25 Scale: 1/8"=1'-0" @ 11x17 1/4"=1'-0" @ 22x34

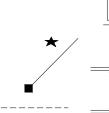
S-





SECOND FLOOR FRAMING PLAN





PROVIDE SOLID BLOCKING

WITHIN FLOOR SYSTEM TO

MATCH POST SIZE ABOVE.

BEARING WALL ABOVE

⇒ INTERIOR BEARING WALL
 ⇒ BRACED WALL PANEL
 (SEE KSE STRUCTURAL DETAILS
 SET FOR BRACED WALL PANEL
 SHEATHING FASTENING &
 BLOCKING DETAILS)

NC Firm #C-2101

REFER TO KSE STRUCTURAL DETAILS SET FOR GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS

PLAN DESIGNED WITH 9' WALL PLATES

FLOOR FRAMING TO BE 16" DEEP TJI 210 I-JOISTS @ 19.2" O.C. MAXIMUM OR EQUAL

#### <u>KEYNOTES:</u>

5 INSTALL TWO PANEL CS-PF PORTAL FRAME PER DETAIL A OR B/SD-4.

Project #: 108-24004

Designed By: AAM
Checked By: KRK
Issue Date: 3/24/25

Plan

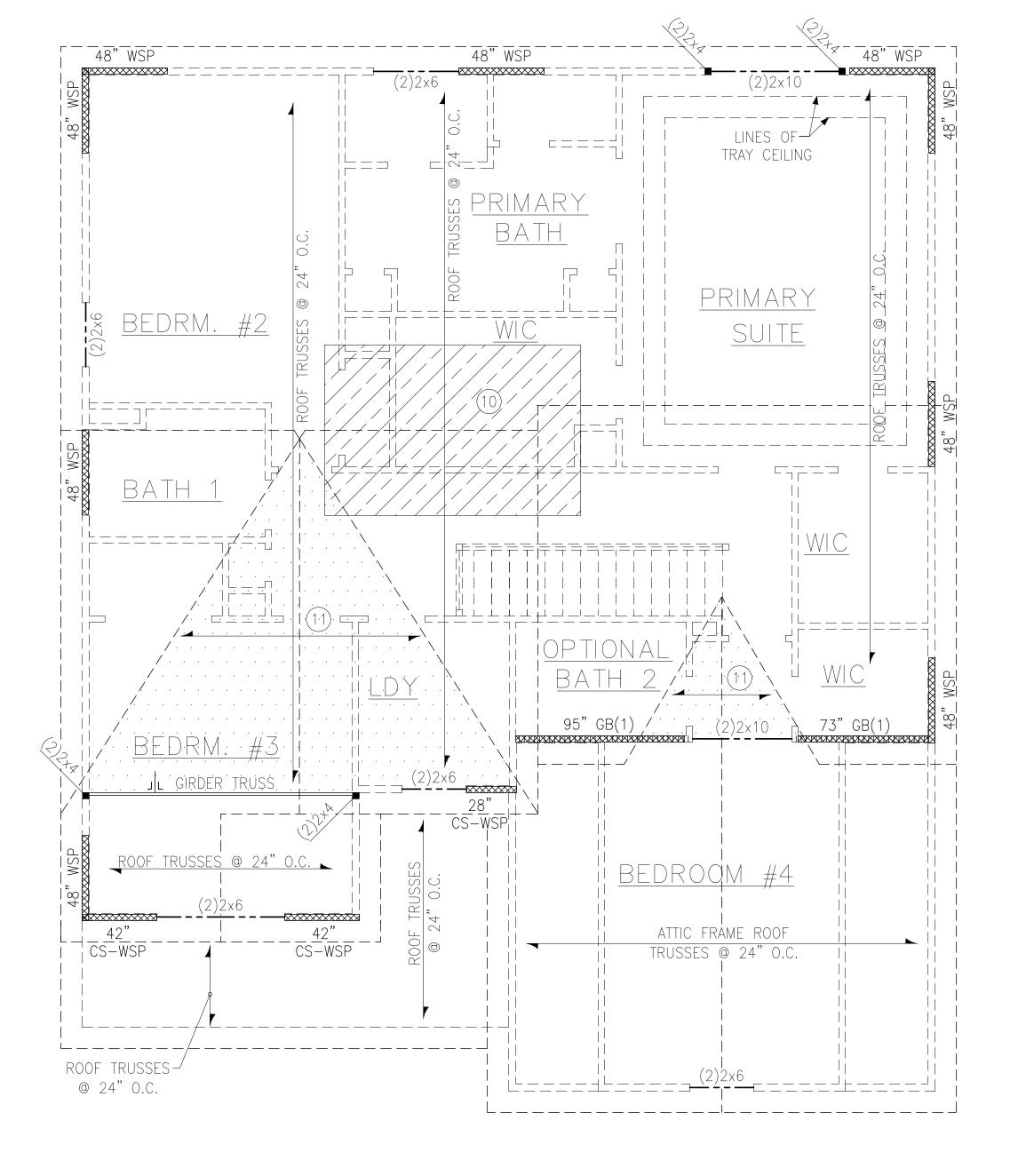
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Floor

Re-Issue: 4/14/25
Scale: 1/8"=1'-0" @ 11x17
1/4"=1'-0" @ 22x34

Carolina

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ROOF FRAMING PLAN

# LEGEND

**\*** / =

PROVIDE SOLID BLOCKING

WITHIN FLOOR SYSTEM TO

MATCH POST SIZE ABOVE.

> INTERIOR BEARING WALL
> BRACED WALL PANEL
(SEE KSE STRUCTURAL DETAILS
SET FOR BRACED WALL PANEL
SHEATHING FASTENING &
BLOCKING DETAILS)

NC Firm #C-2101

REFER TO KSE STRUCTURAL DETAILS SET FOR GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS

PLAN DESIGNED WITH 8' WALL PLATES

## KEYNOTES:

(10) 8'x12' HVAC PLATFORM TRUSSES DESIGNED TO SUPPORT HVAC UNITS.

11) VALLEY SET TRUSSES @ 24" O.C.
OR 2x6 OVERFRAMING @ 24" O.C.
W/ 2x8 RIDGE & VALLEY PLATES
(TYP.)

Project #: 108-24004

Raleigh,

Agents and A

Designed By: AAM
Checked By: KRK
Issue Date: 3/24/25

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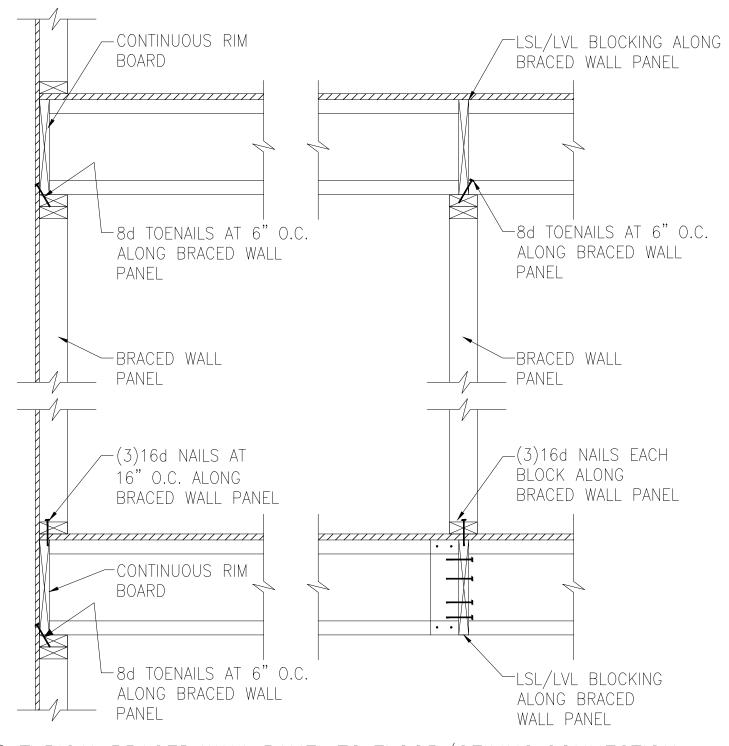
Plan

Framing

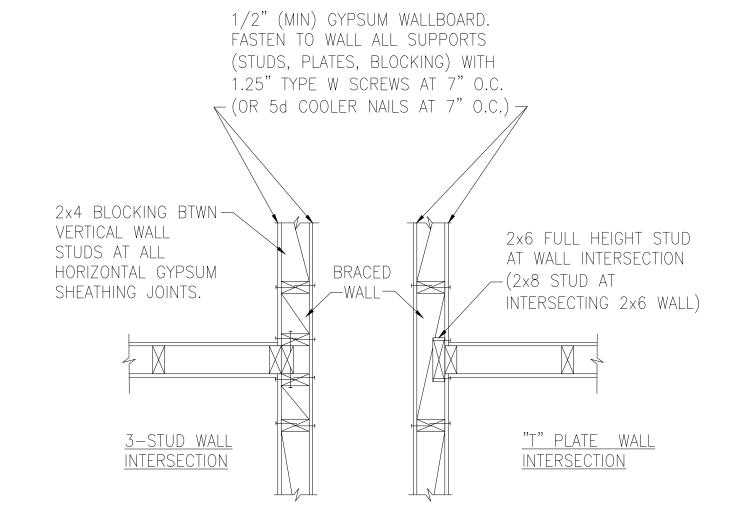
Issue Date: 3/24/25
Re-Issue: 4/14/25
Scale: 1/8"=1'-0" @ 11x17
1/4"=1'-0" @ 22x34

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S - 3

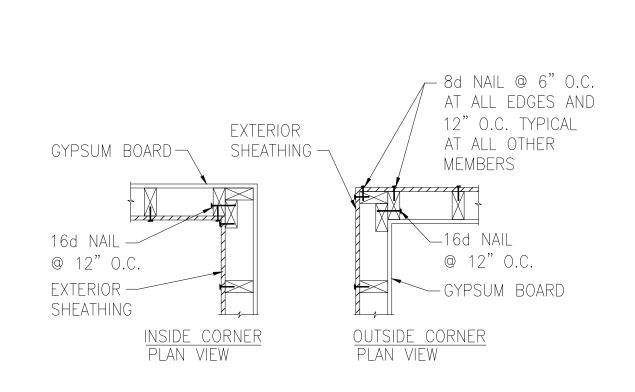


BYPICAL BRACED WALL PANEL TO FLOOR/CEILING CONNECTION BRACED WALL PANELS PERPENDICULAR TO I-JOISTS

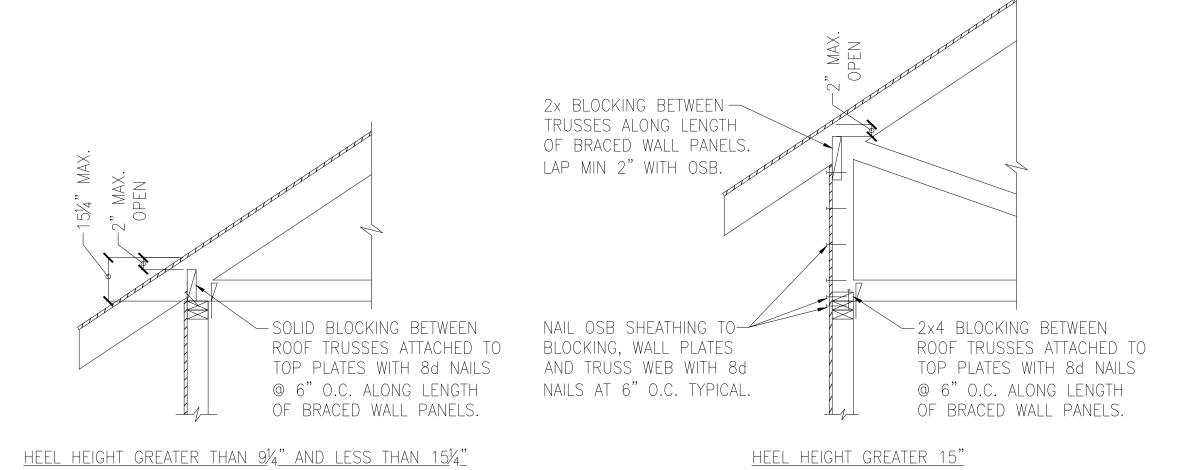


BRACED WALL INTERSECTIONS MAY BE FRAMED USING EITHER THE 3-STUD OR THE T-PLATE METHOD.

METHOD GB(1) AND GB(2) INTERSECTION DETAILS

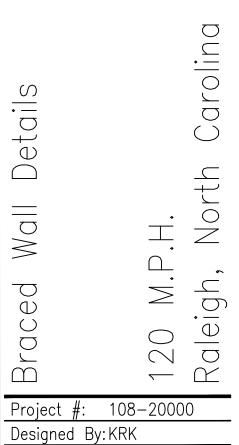






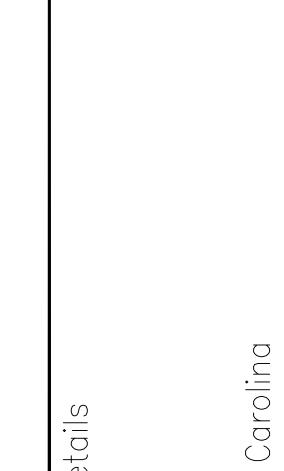
ROOF TRUSS BEARING/BLOCKING AT BRACED WALL PANELS ONLY REQUIRED AT BRACED WALL PANELS





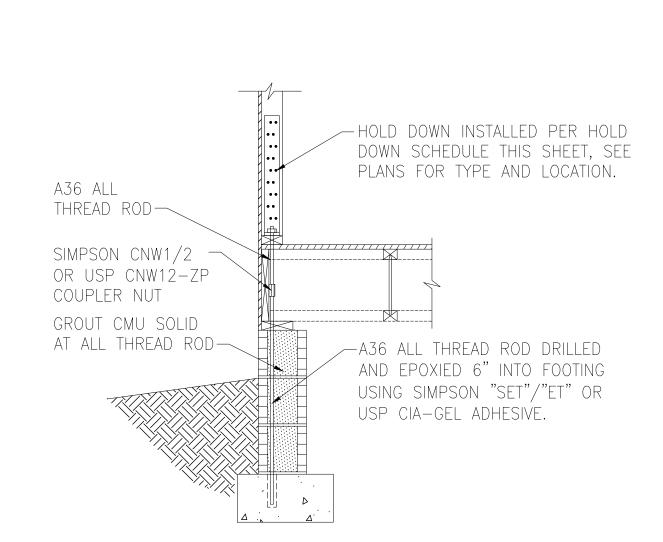
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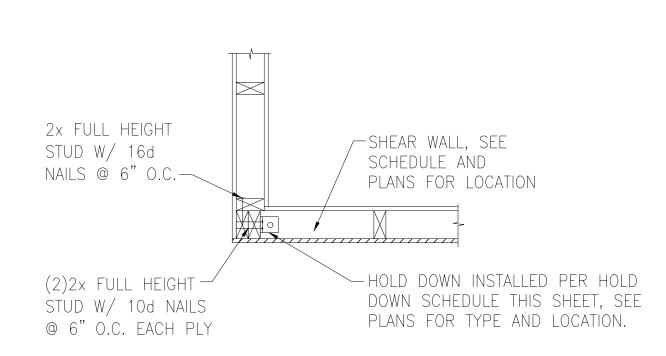




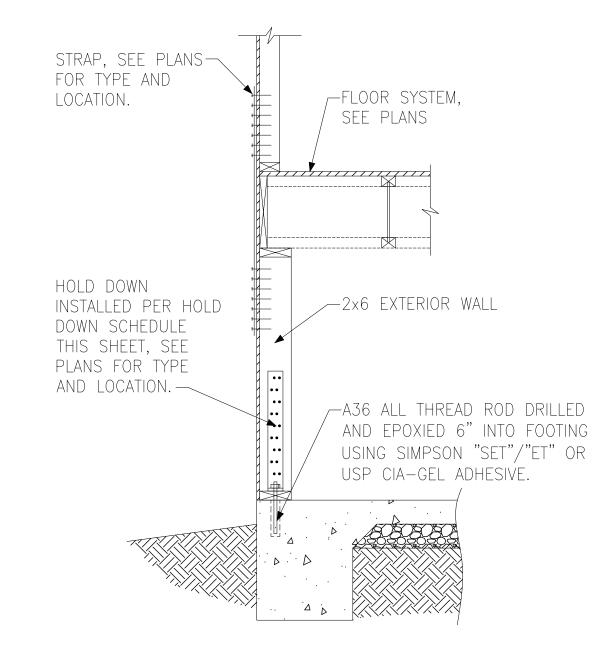
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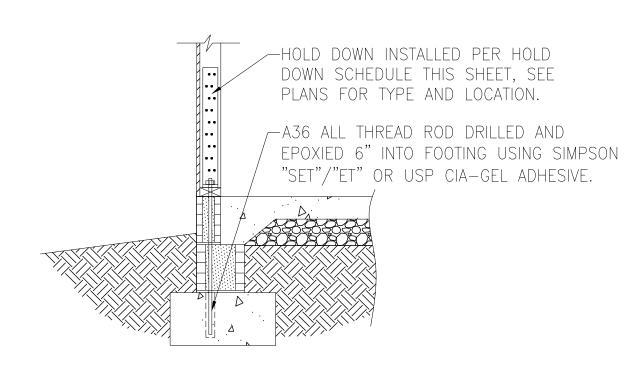
(E)HOLD DOWN AT CRAWL SPACE FOUNDATION



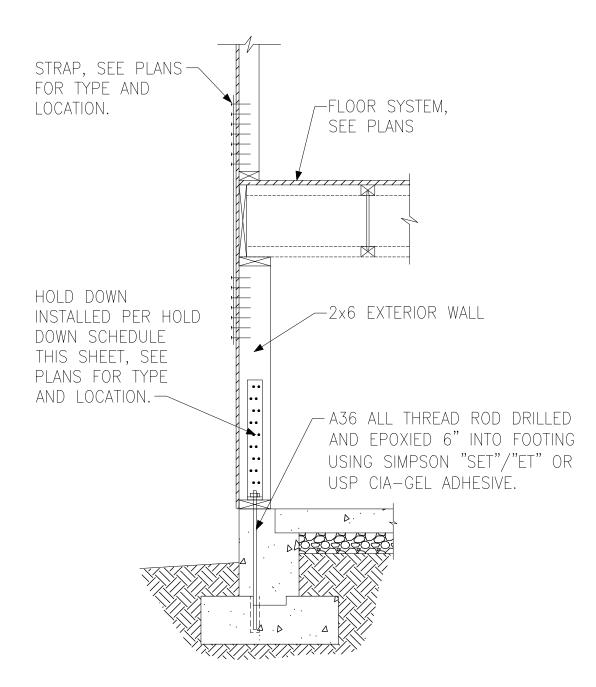
# TYPICAL HOLD DOWN DETAIL



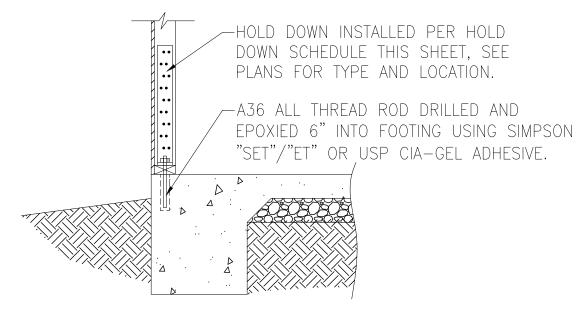
F HOLD DOWN AT BASEMENT FOUNDATION MONOLITHIC TURN-DOWN



HOLD DOWN AT STEMWALL SLAB FOUNDATION



G HOLD DOWN AT BASEMENT FOUNDATION STEM WALL



DHOLD DOWN AT MONOLITHIC SLAB FOUNDATION

HOLD DOWN SCHEDULE HOLD DOWN ALL THREAD ROD FASTENERS USP SIMPSON ½" DIA. (10)10d NAILS LTS20B LTT20B 5⁄8" DIA.  $(18)16dx2\frac{1}{2}$ " LONG NAILS HTT4 HTT16 5⁄8" DIA.  $(26)16dx2\frac{1}{2}$ " LONG NAILS HTT45 HTT5



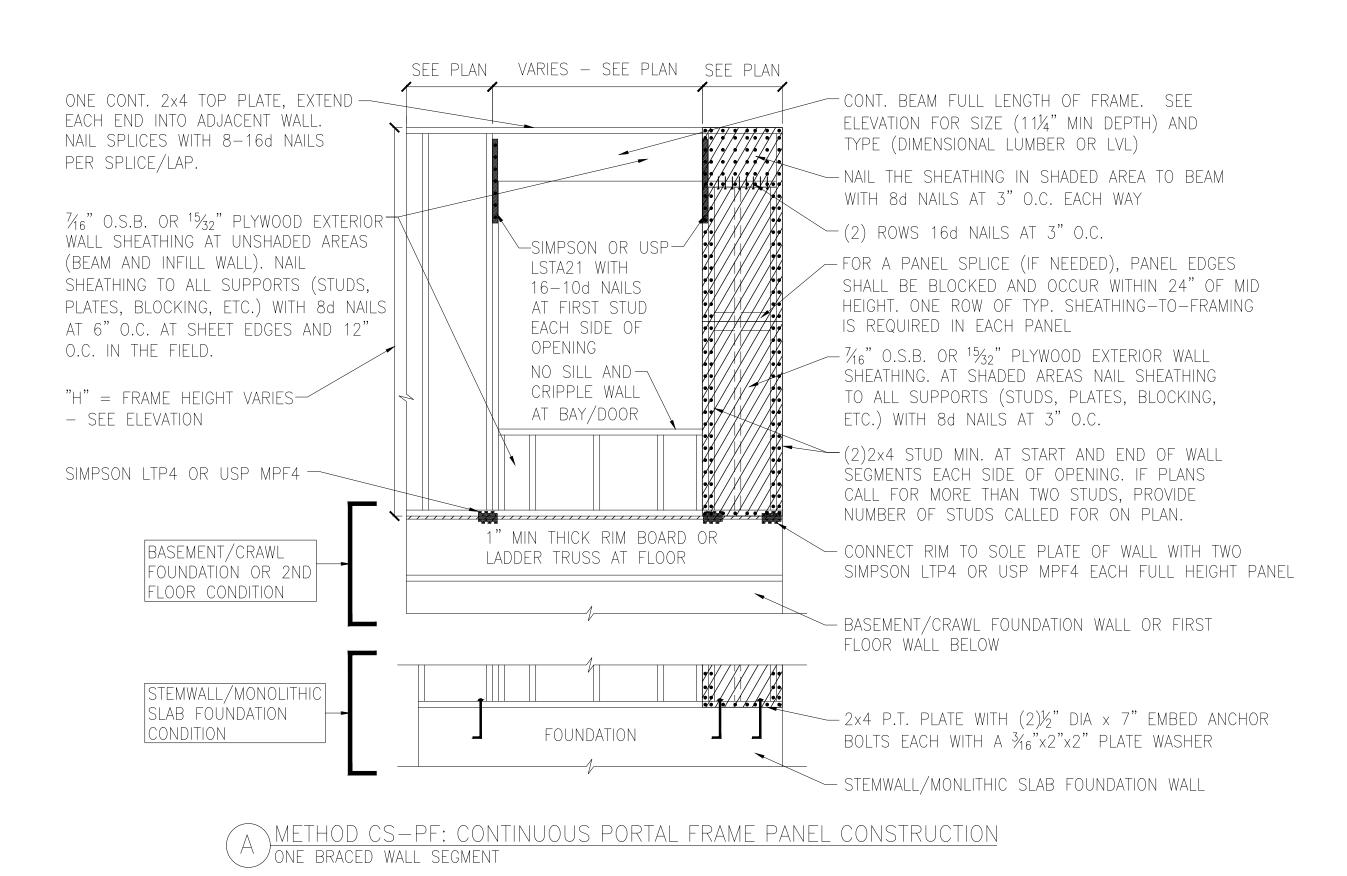


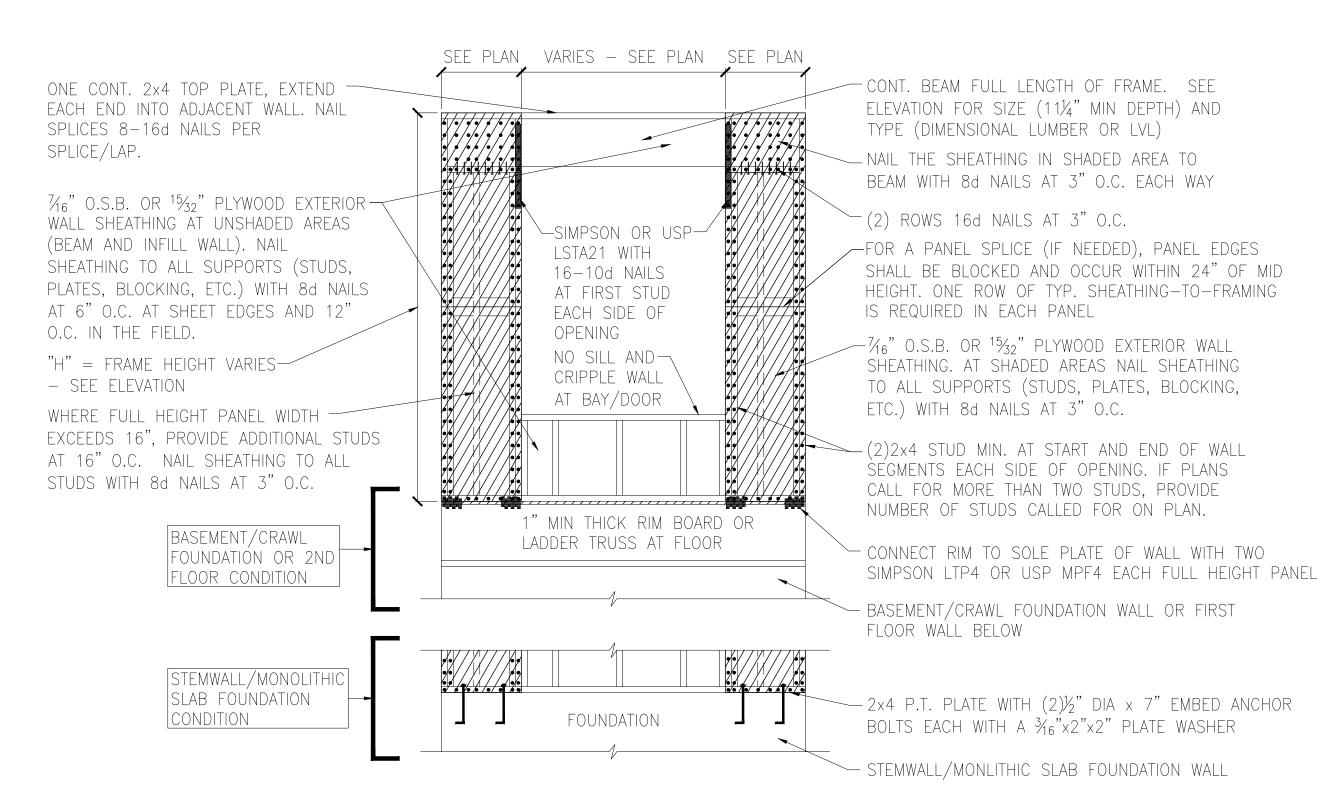
Details  $\square \cup \square \cup \square$ 120 M.P Raleigh,

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<u>р</u> Н Project #: 108-20000 Designed By: KRK Checked By: Issue Date: 1/1/20 Re-Issue: Scale: 1/8"=1'-0" @ 11x17 1/4"=1'-0" @ 22x34



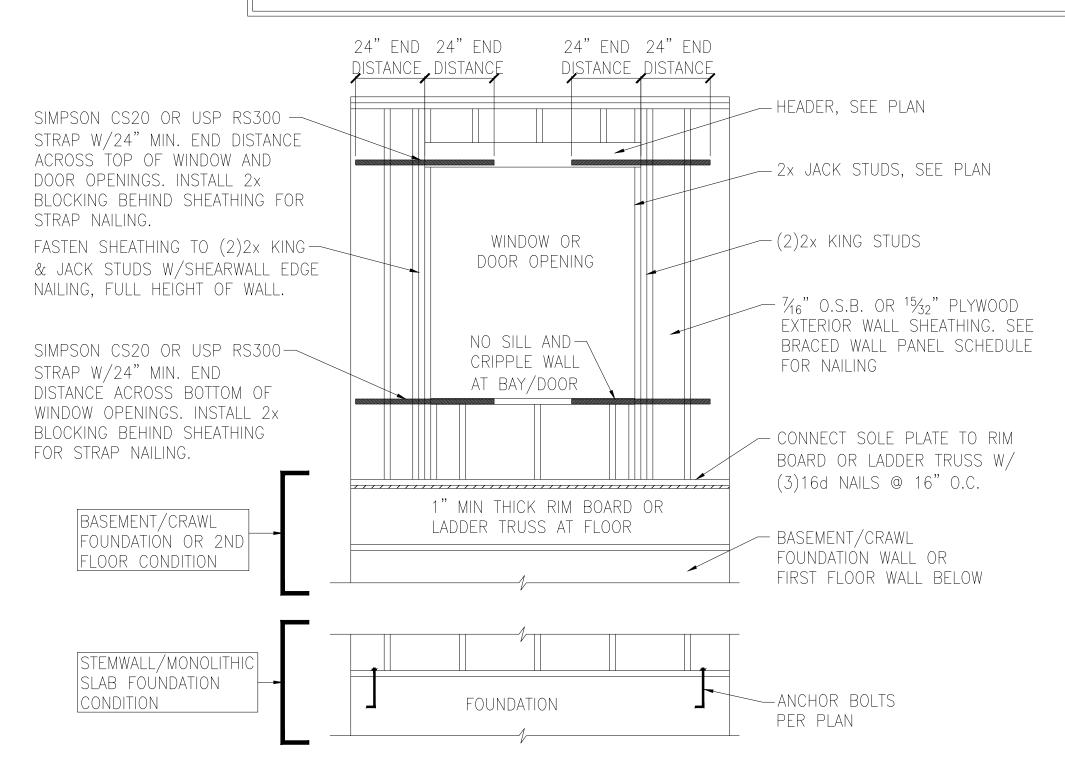


B METHOD CS-PF: CONTINUOUS PORTAL FRAME PANEL CONSTRUCTION TWO BRACED WALL SEGMENTS

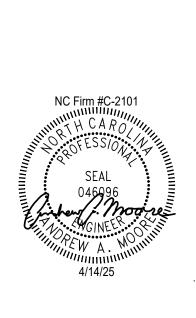
	BRACED WALI	PANEL AN	ID ENGINEERED SHEAR WALL SCHEDULE
PANEL TYPES	PANEL TYPE MATERIAL		FASTENERS
WSP	INTERMITTENT WOOD STRUCTURAL PANEL	7/16" OSB	6D OR 8D COMMON NAILS AT 6" O.C. AT SHEET EDGES AND 12" O.C. (6" O.C. AT INTERIOR WALL LOCATIONS) AT INTERMEDIATE SUPPORTS.  ENGINEERED ALTERNATIVE: 16 GAGE BY 1.75" LONG STAPLES AT 3" O.C. AT SHEET EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS
GB(1)	INTERMITTENT GYPSUM BOARD (SHEATHING ONE FACE OF WALL)	1/2" GYPSUM	1.5" LONG GALV. ROOFING NAILS, 6d COMMON NAILS, OR 1.25" LONG TYPE W DRYWALL SCREWS AT 7" O.C. AT SHEET EDGES AND INTERMEDIATE SUPPORTS.
GB(1)-4	INTERMITTENT GYPSUM BOARD (SHEATHING ONE FACE OF WALL)	1/2" GYPSUM	1.5" LONG GALV. ROOFING NAILS, 6d COMMON NAILS, OR 1.25" LONG TYPE W DRYWALL SCREWS AT 4" O.C. AT SHEET EDGES AND INTERMEDIATE SUPPORTS.
GB(2)	INTERMITTENT GYPSUM BOARD (SHEATHING BOTH FACES OF WALL)	1/2" GYPSUM	1.5" LONG GALV. ROOFING NAILS, 6d COMMON NAILS, OR 1.25" LONG TYPE W DRYWALL SCREWS AT 7" O.C. AT SHEET EDGES AND INTERMEDIATE SUPPORTS.
CS-WSP	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	7/16" OSB	6D OR 8D COMMON NAILS AT 6" O.C. AT SHEET EDGES AND 12" O.C. (6" O.C. AT INTERIOR WALL LOCATIONS) AT INTERMEDIATE SUPPORTS.  ENGINEERED ALTERNATIVE: 16 GAGE BY 1.75" LONG STAPLES AT 3" O.C.  AT SHEET EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS
CS-PF	CONTINUOUS SHEATHED PORTAL FRAME	7/16" OSB	NAILING PER DETAIL
PFH	PORTAL FRAME WITH HOLD DOWNS	7/16" OSB	NAILING PER DETAIL
CS-ESW(1)	ENGINEERED SHEAR WALL, TYPE 1	7/16" OSB	8D COMMON NAILS AT 6" O.C. AT SHEET EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. CONTINUOUS OSB AROUND DOOR/WINDOW OPENINGS
CS-ESW(2)	ENGINEERED SHEAR WALL, TYPE 2	7/16" OSB	8D COMMON NAILS AT 4" O.C. AT SHEET EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. CONTINUOUS OSB AROUND DOOR/WINDOW OPENINGS
CS-ESW(3)	ENGINEERED SHEAR WALL, TYPE 3	7/16" OSB	8D COMMON NAILS AT 3" O.C. AT SHEET EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. CONTINUOUS OSB AROUND DOOR/WINDOW OPENINGS

#### BRACED WALL PANEL NOTES:

- 1. ALL BRACED WALL PANELS, EXCEPT GB(1) & GB(2), SHALL HAVE 2x BLOCKING BETWEEN WALL STUDS AT ALL HORIZONTAL SHEET EDGES.
- 2. PROVIDE NAILING/BLOCKING ABOVE AND BELOW ALL BRACED WALL PANELS PER KSE BRACED WALL DETAILS.
- 3. SHEATH ALL EXTERIOR WALLS OF THE HOUSE WITH 7/6" O.S.B., OR 15/32" PLYWOOD, FASTENED PER IRC. AT EXTERIOR CORNERS, SHEATHING SHALL BE FASTENED PER KSE BRACED WALL DETAILS. AT INTERIOR WALL INTERSECTIONS, FASTEN STUDS & WALL BRACING PER KSE BRACED WALL DETAILS.
- 4. BRACED WALL PANELS AND ENGINEERED SHEAR WALLS ARE PROVIDED PER IRC. PANEL LENGTHS SHOWN ON PLANS ARE THE MINIMUM LENGTH REQUIRED.



WINDOW OR DOOR REINFORCEMENT IN ENGINEERED SHEAR WALL ONLY REQUIRED WHERE SPECIFED ON PLANS





CARUSO HOMES

Wall Notes & Details

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Project #: 108-20000 Designed By: KRK

Designed By: KRK
Checked By:

Issue Date: 1/1/20
Re-Issue:

Scale: 1/8"=1'-0" @ 11x17 1/4"=1'-0" @ 22x34

SD-3

MONOLITHIC SLAB OR BASEMENT FOUNDATION



Frame Details

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NC Firm #C-2101

120 M.P.H Raleigh, N

Carolina

North

Project #: 108-20000

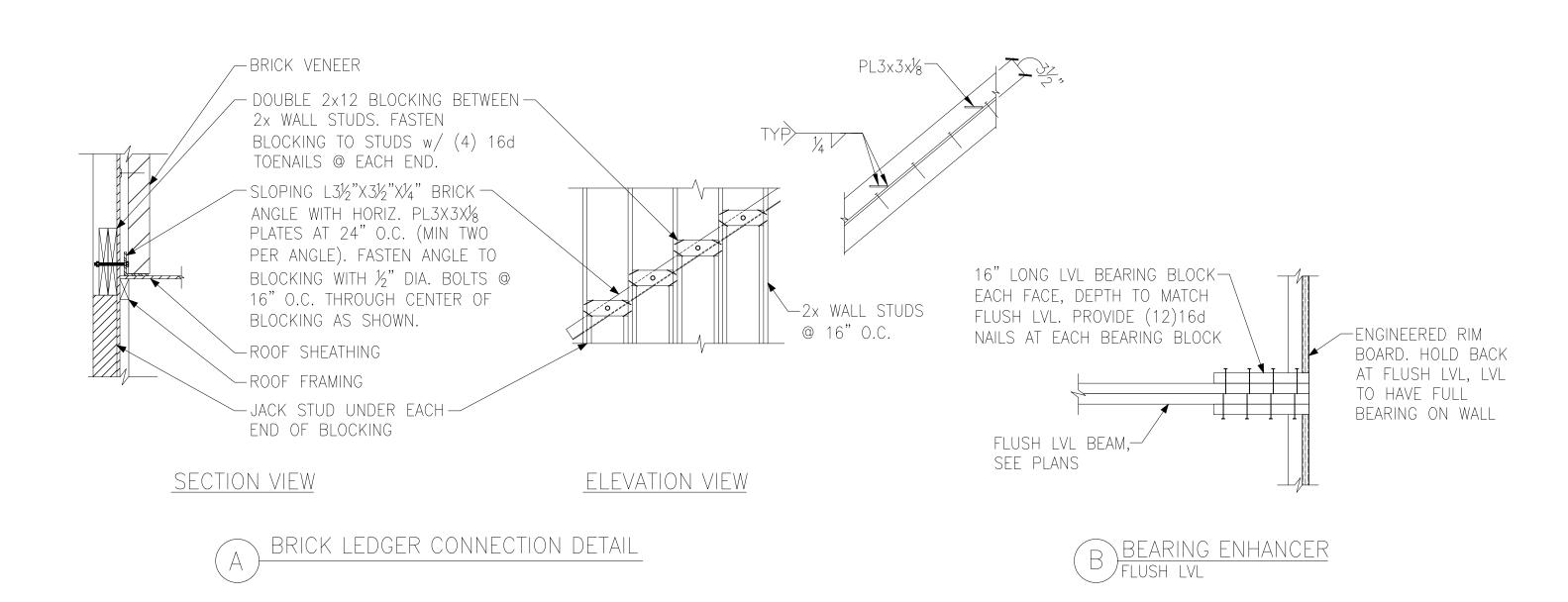
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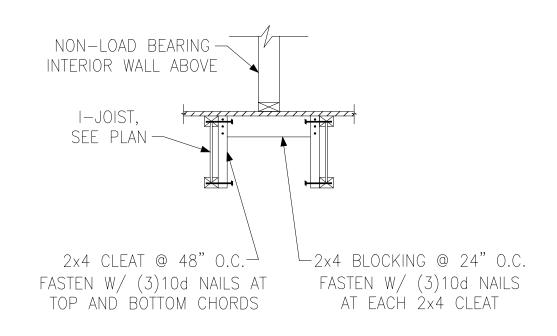
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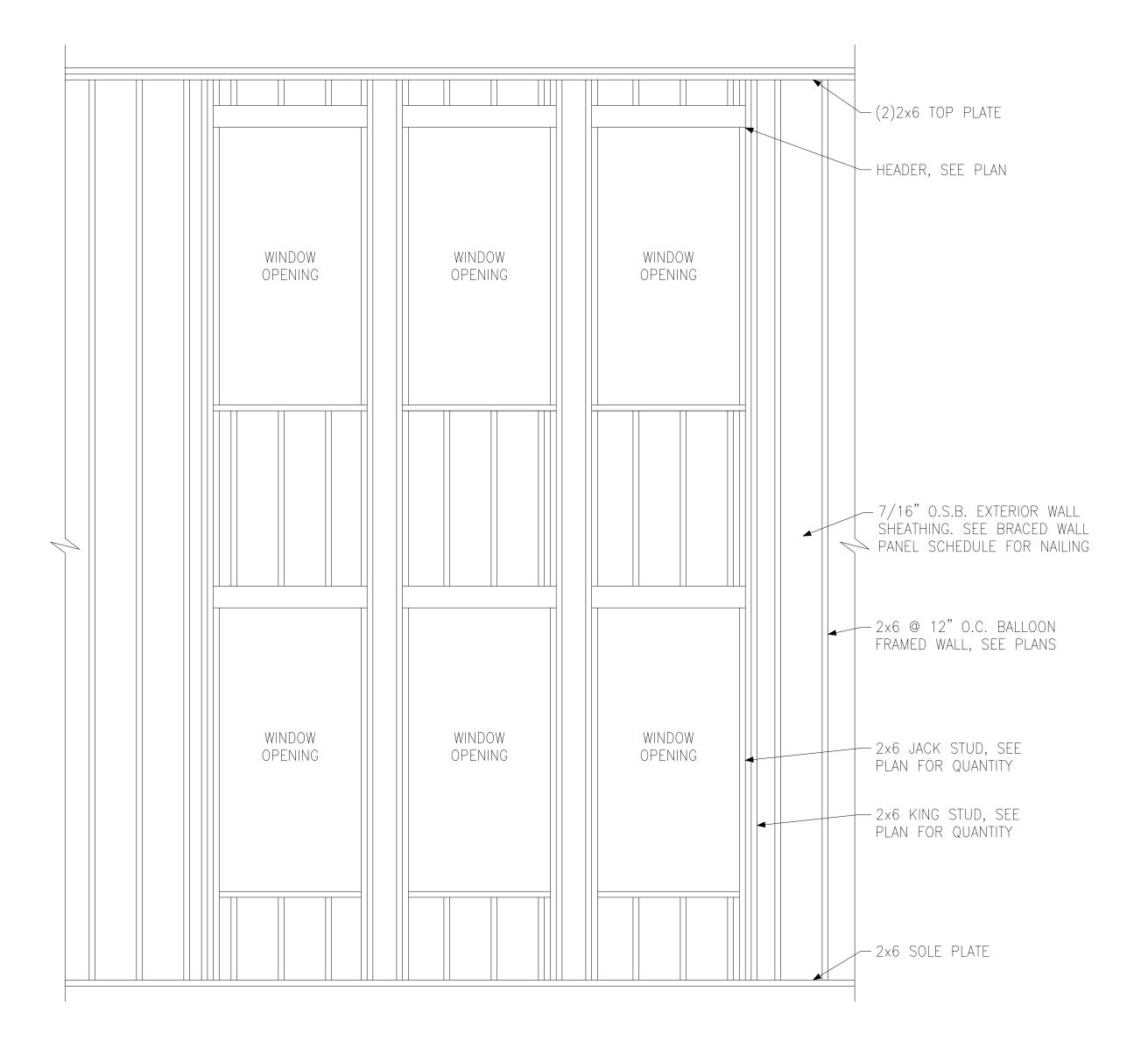
Re-Issue:
Scale: 1/8"=1'-0" @ 11x17
1/4"=1'-0" @ 22x34







I-JOIST LADDER BLOCKING
AS REQUIRED @ PARALLEL WALLS



BALLOON FRAMED WALL DETAIL N.T.S.



Details Framing Miscellaneous

120 M.P Raleigh,

Project #: 108-20000

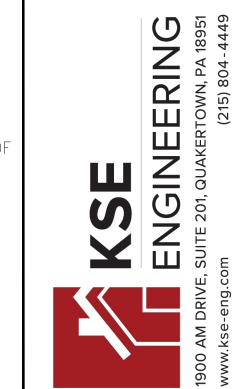
Designed By: KRK

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1/4"=1'-0" @ 22x34

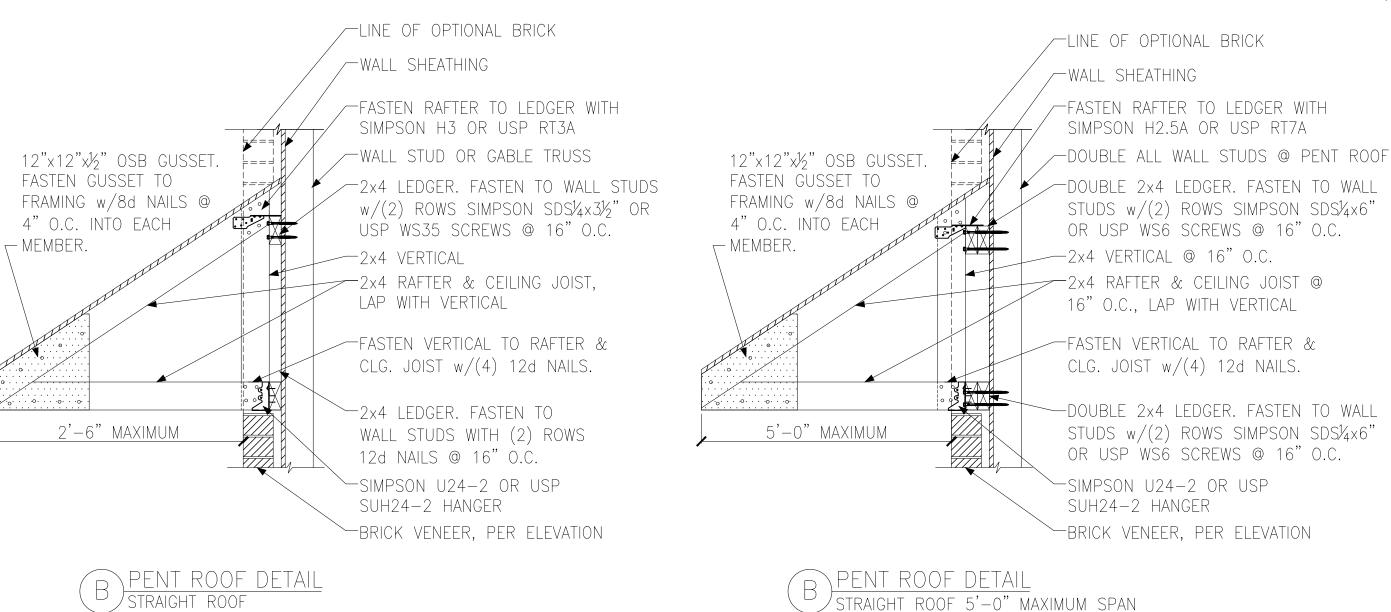
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(5) 10d —

- ROOF TRUSSES AT

24" O.C., SEE PLAN.

PROVIDE WEB MEMBER

BRACING PER TRUSS

MANUFACTURER

NAILS

/STRAIGHT ROOF 5'-0" MAXIMUM SPAN

2×4 BLOCKING BETWEEN TRUSSES WITH (2)10d CONTINUOUS SHEATHING TOENAILS EACH END AT OVERHANG - $2 \times 6$  KICKER AT 6'-0" O.C. WITH  $\longrightarrow$ 2x6 "T" SCAB. NAIL SCAB TO KICKER WITH 10d NAILS AT 6" O.C. KICKER MAY BE OMITTED WHEN HEIGHT OF GABLE END TRUSS IS 4'-0" OR LESS. -(2) SIMPSON GBC OR USP HC520

7/16" OSB WALL-

SHEATHING

GABLE END WALL DETAIL

EACH KICKER

SLOPING L3½"x3½"x¼" BRICK ANGLE WITH HORIZ. PL3x3x1/8 BRICK VENEER — PLATES AT 24" O.C. (MIN TWO PER ANGLE. NAIL TO GIRDER TRUSS WITH 16d NAILS AT 9" O.C. THROUGH PRE-DRILLED —WALL STUD OR GABLE TRUSS PL3x3x1/6-TOENAIL RAFTER TO LEDGER WITH (4) 12d NAILS TYP 1/4 / 2x4 LEDGER. FASTEN TO WALL STUDS w/(2) ROWS SIMPSON SDS $\frac{1}{4}$ x $\frac{3}{2}$ " OR USP WS35 SCREWS @ 16" O.C. 2× WALL -2x4 RAFTER & CEILING JOIST, STUDS, SEE PLAN — LAP AND FACE NAIL WITH (4) 12d NAILS ROOF GIRDER TRUSS TO 12" MAXIMUM 2x4 LEDGER. FASTEN TO WALL SUPPORT DEAD LOAD OF BRICK, LIMIT DEFLECTION OR GABLE TRUSS WITH (2) TO L/600 OR 0.3" MAX., ROWS 12d NAILS @ 16" O.C. SEE PLANS.

2x12 RAFTER @ 24"

O.C. WITH CURVED

RAFTER -

PROFILE CUT INTO

INTO EACH MEMBER.

OSB GUSSET, CUT TO MATCH-

ROOF PROFILE FASTEN GUSSET TO FRAMING w/8d NAILS @ 4" O.C.

2x4 VERTICAL

CURVED ROOF

TRUSS DETAIL

∕2x4 LEDGER. FASTEN TO

WALL STUDS w/(2) ROWS

SIMPSON SDS1/4×31/2" OR USP

WS35 SCREWS @ 16" O.C.

LINE OF OPTIONAL BRICK

FASTEN RAFTER TO LEDGER WITH

2x4 LEDGER. FASTEN TO WALL STUDS

w/(2) ROWS SIMPSON SDS $\frac{1}{4}$  $\times$ 3 $\frac{1}{2}$ " OR

USP WS35 SCREWS @ 16" O.C.

FASTEN VERTICAL TO RAFTER &

CLG. JOIST w/(4) 12d NAILS.

WALL STUDS WITH (2) ROWS

-BRICK VENEER, PER ELEVATION

SIMPSON H3 OR USP RT3A

WALL STUD OR GABLE TRUSS

\_\_2x4 CEILING JOIST @ 24"

O.C., LAP WITH VERTICAL

─2×4 LEDGER. FASTEN TO

12d NAILS @ 16" O.C.

USP SUH24-2 HANGER

-SIMPSON U24-2 OR

-WALL SHEATHING

-2x4 VERTICAL

OSB GUSSET, CUT TO MATCH

INTO EACH MEMBER. —

2x12 RAFTER @24"

O.C. WITH CURVED

PROFILE CUT INTO

RAFTER —

ROOF PROFILE FASTEN GUSSET TO FRAMING w/8d NAILS @ 4" O.C.

2'-6" MAXIMUM

(A) CURVED ROOF

EYEBROW ROOF DETAIL STRAIGHT ROOF

Miscellaneous

Checked By:

Details

raming

20 M.I Raleigh, 108-20000 Designed By: KRK

Carolina

North

Issue Date: 1/1/20 Re-Issue: Scale: 1/8"=1'-0" @ 11x17 1/4"=1'-0" @ 22x34

NC Firm #C-2101

-2x STUD WALL, SEE PLAN AND VERTICALLY AND SHALL SUPPORT NOT MORE THAN 2 SQUARE FEET OF WALL AREA -FLOOR JOIST, SEE PLAN -2x6 STUD WALL W/ P.T. PLATE, SEE PLAN - INSTALL ½" DIA. ANCHOR BOLTS, SEE FOUNDATION NOTES. CONCRETE SLAB, SEE PLAN. —4" GRAVEL FILL COMPACTED SOIL CONCRETE FOOTING, SEE PLAN.

NC Firm #C-2101

oundati OS

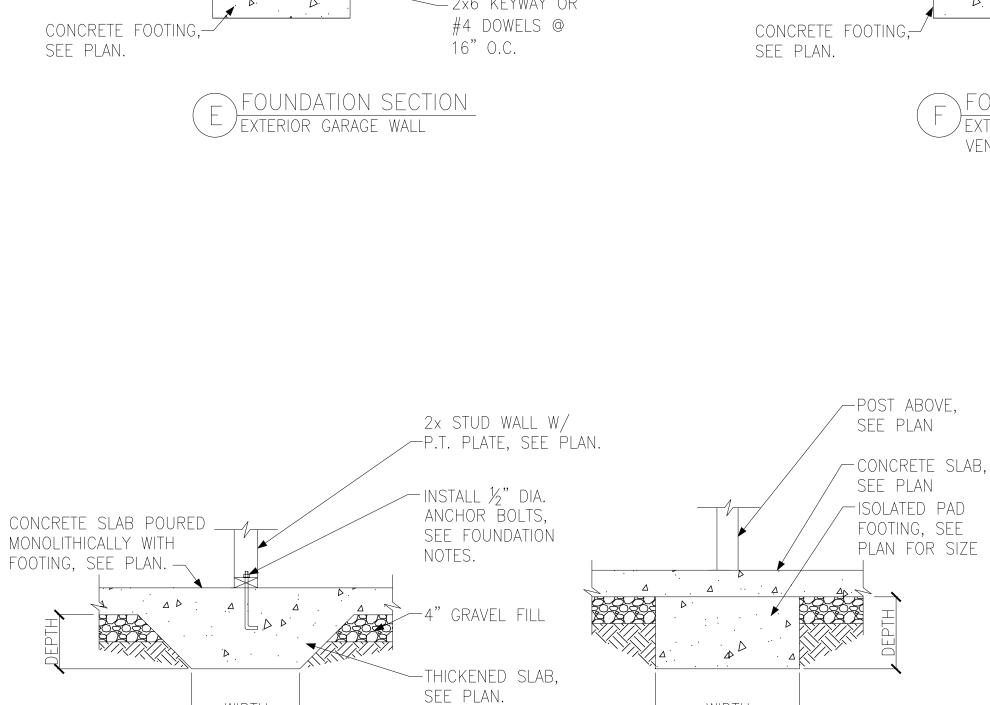
20 M. aleigh, 108-20000

Designed By:KRK

Checked By: Issue Date: 1/1/20 Re-Issue:

Scale: 1/8"=1'-0" @ 11x17 1/4"=1'-0" @ 22x34

SD - 10



WIDTH

ISOLATED PAD FOOTING

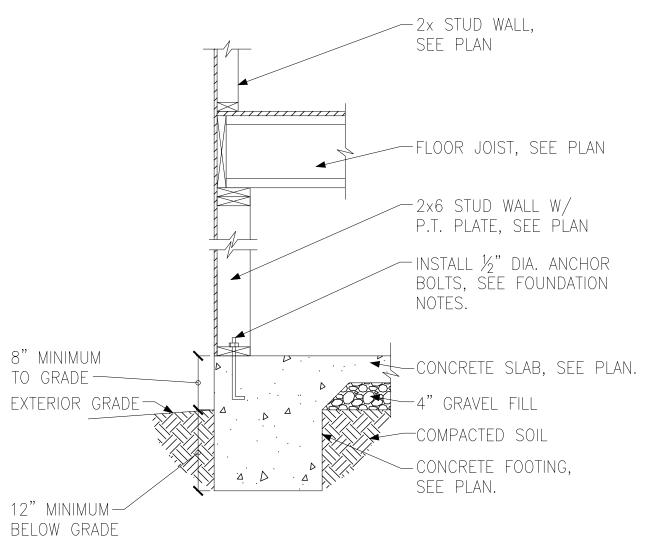
FOUNDATION SECTION

WIDTH

FOUNDATION SECTION

THICKENED SLAB



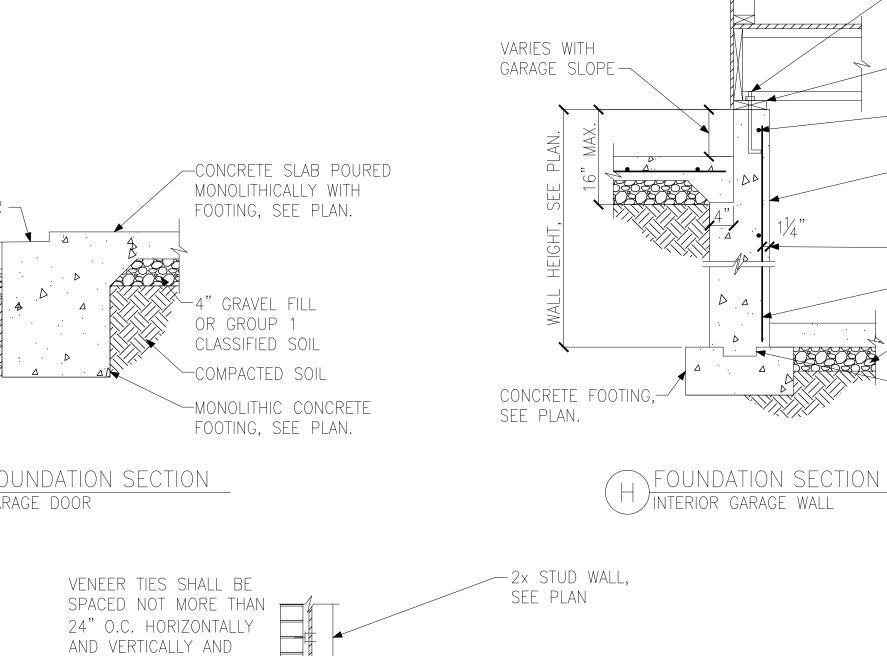


FOUNDATION SECTION

WALKOUT BASEMENT

MASONRY VENEER-8" MINIMUM — TO GRADE EXTERIOR GRADE— 12" MINIMUM-BELOW GRADE





FOUNDATION SECTION H) INTERIOR GARAGE WALL

Details

Carolina

orth

**O** W

IEERING

KERTOWN, PA 18951
(215) 804 - 4449

-2x STUD WALL,

-P.T. SILL PLATE

-INSTALL 1/3" DIA. ANCHOR

BOLTS, SEE FOUNDATION

-PROVIDE (1) HORIZONTAL

BAR w/IN 12" OF TOP

-CONCRETE BASEMENT

INSIDE FACE OF WALL.

- VERTICAL REINFORCING

SEE FOUNDATION

-2×6 KEYWAY OR #4

DOWELS @ 16" O.C.

PLAN NOTES.

—4" GRAVEL FILL

WALL, SEE PLANS.

-1¼" CLEAR FROM

SEE PLAN

NOTES.

OF WALL.

FOUNDATION SECTION

VENEER

/exterior wall at porch w/ masonry

-2x STUD WALL,

-INSTALL 1/3" DIA. ANCHOR

BOLTS, SEE FOUNDATION

PROVIDE (1) HORIZONTAL BAR

-1¼" CLEAR FROM INSIDE FACE

VERTICAL REINFORCING — SEE

FOUNDATION PLAN NOTES.

-2x6 KEYWAY OR #4 DOWELS

w/IN 12" OF TOP OF WALL.

CONCRETE BASEMENT

WALL WITH 4" SLAB

LEDGE, SEE PLANS.

-4" GRAVEL FILL

@ 16" O.C.

SEE PLAN

NOTES.

P.T. PLATE

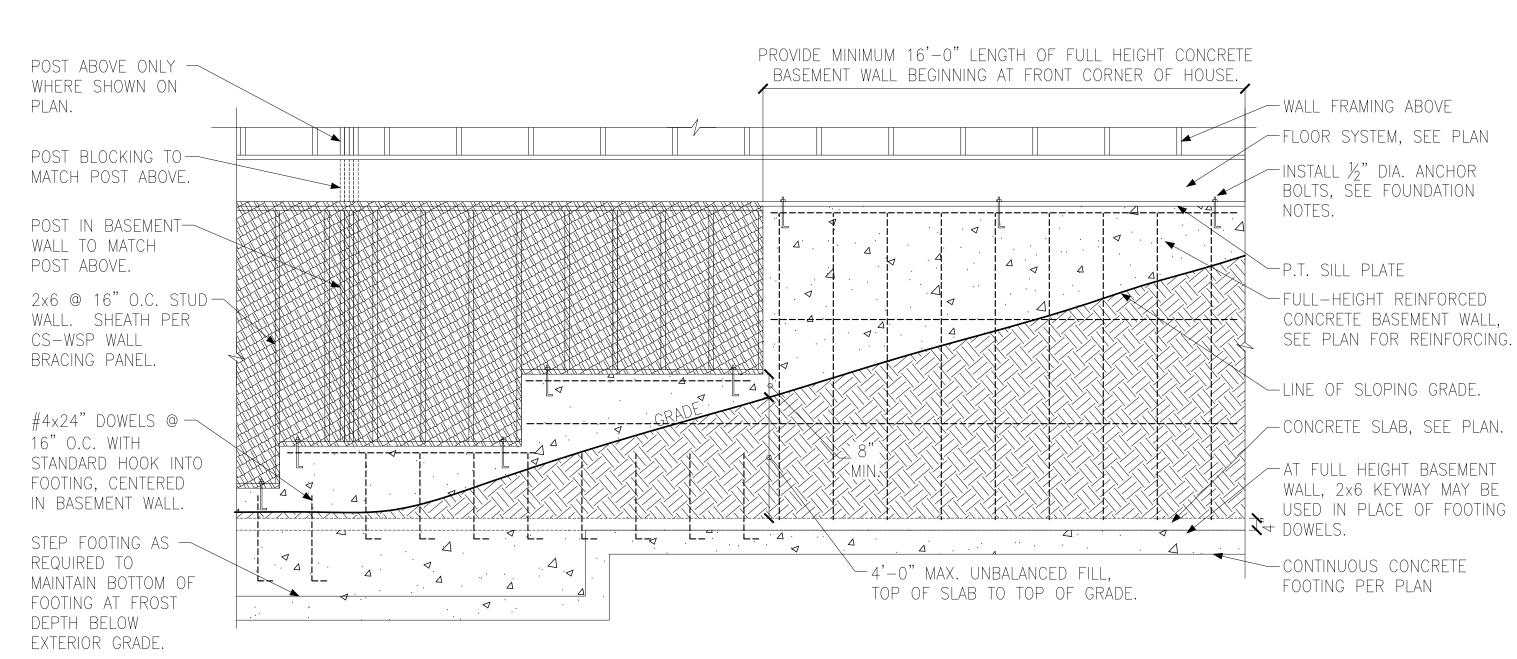
OF WALL.

FOUNDATION SECTION

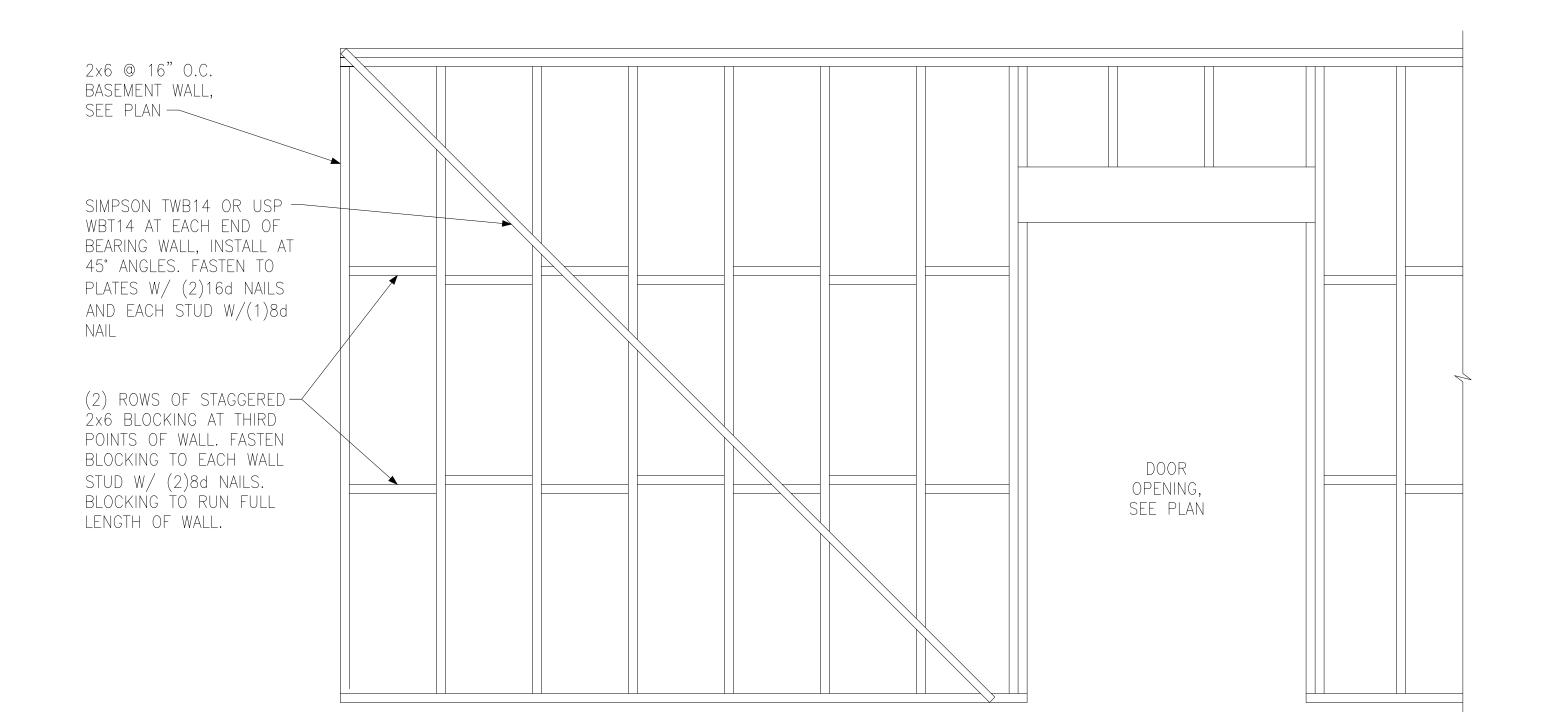
VENEER

STEPPED SIDEWALL @ MASONRY

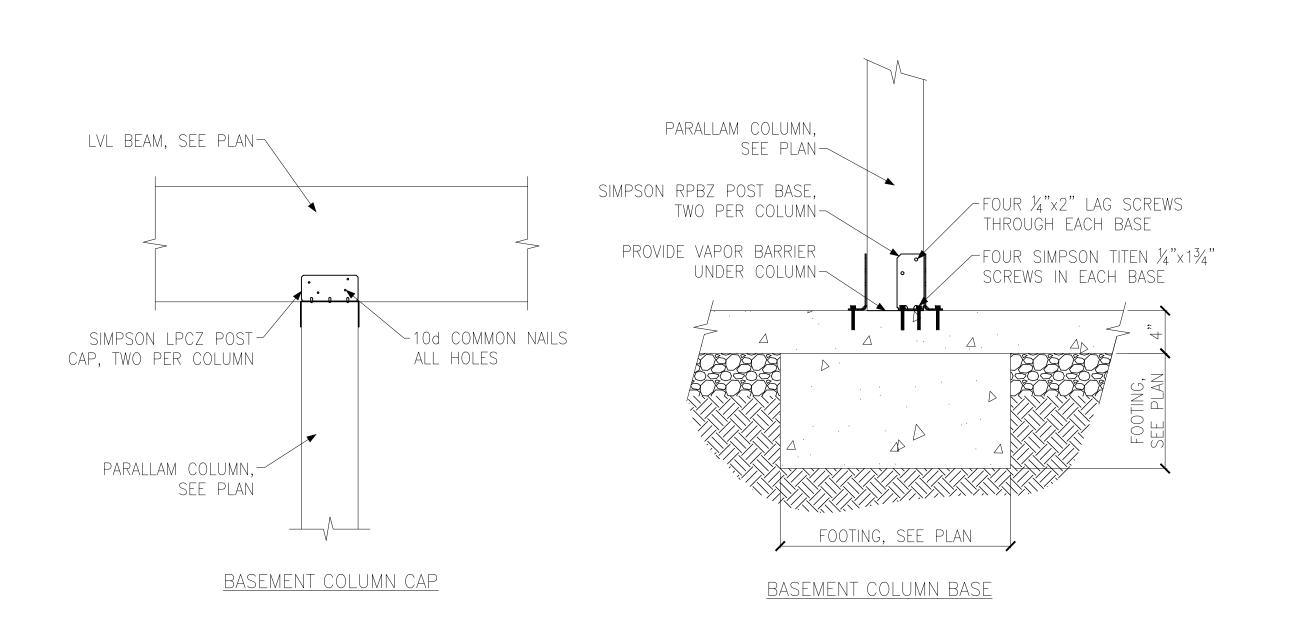
B FOUNDATION SECTION STEPPED SIDEWALL



FOUNDATION ELEVATION A) STEPPED SIDEWALL



D BASEMENT BEARING WALL BRACING DETAIL



(E) BASEMENT COLUMN CAP AND BASE DETAILS



Details  $\mathbb{M}_{Q}$ Foundation Basement

20 M. aleigh,

108-20000 Designed By: KRK

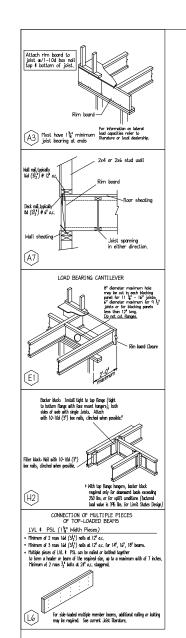
Checked By: Issue Date: 1/1/20

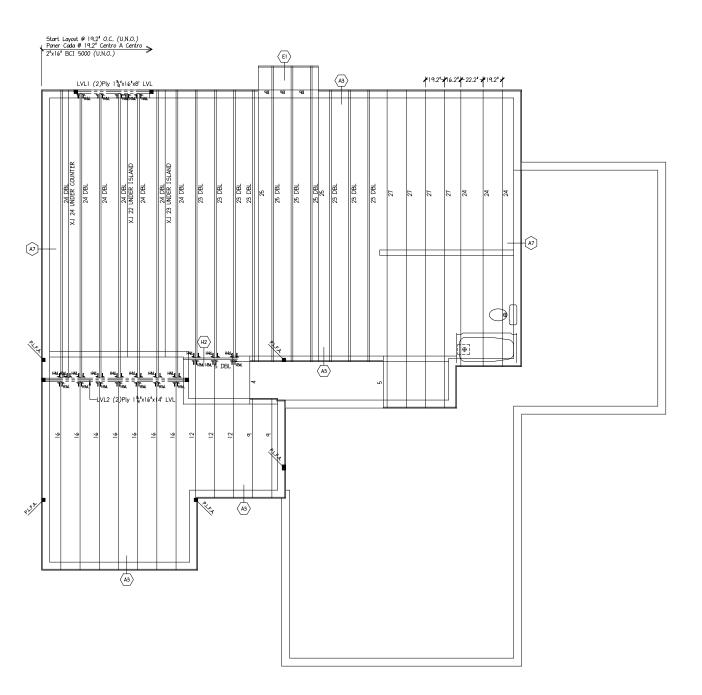
Re-Issue: Scale: 1/8"=1'-0" @ 11x171/4"=1'-0" @ 22x34

Carolina

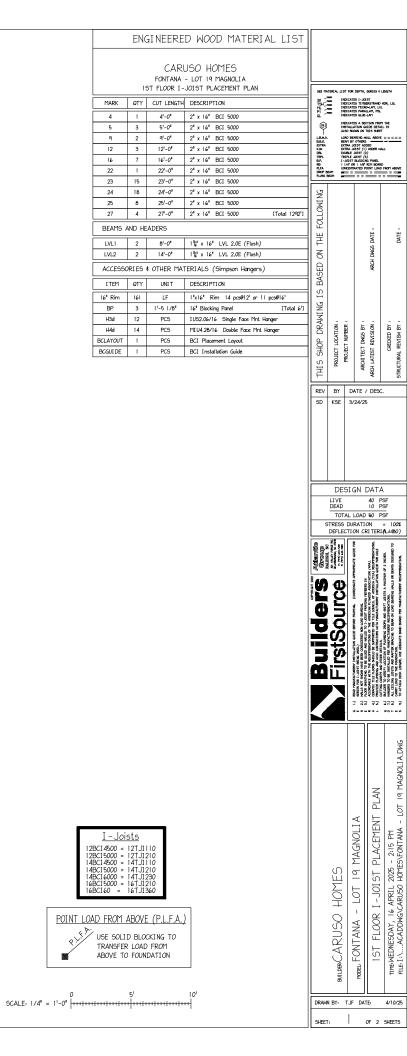
North

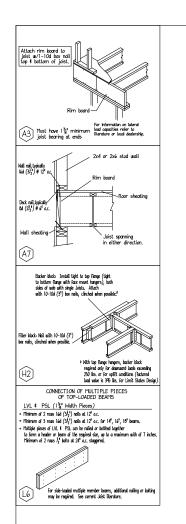
SINEERING , QUAKERTOWN, PA 18951 (215) 804-4449

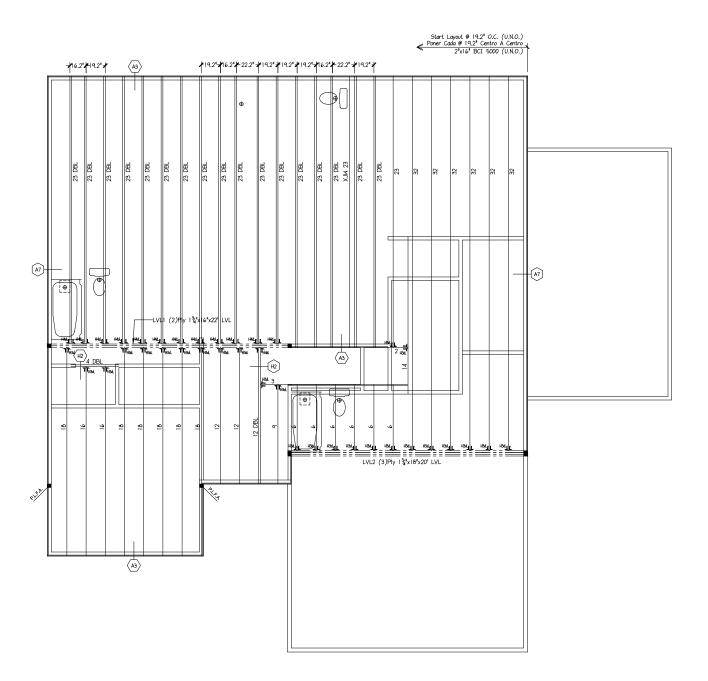




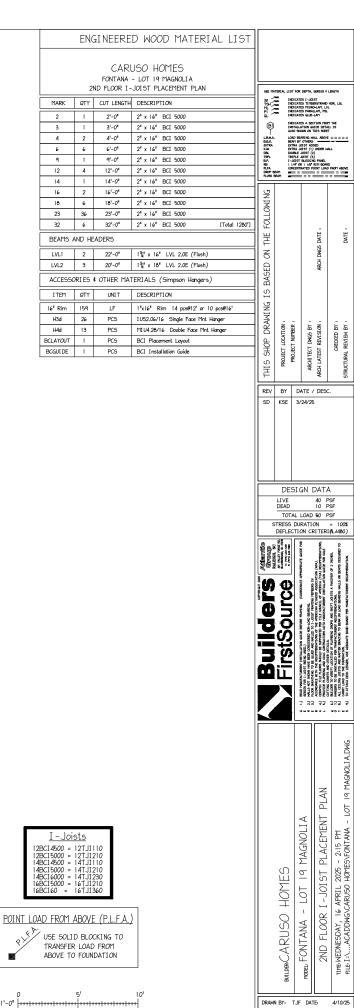
1ST FLOOR I - JOIST PLACEMENT PLAN CARUSO HOMES - FONTANA - LOT 19 MAGNOLIA



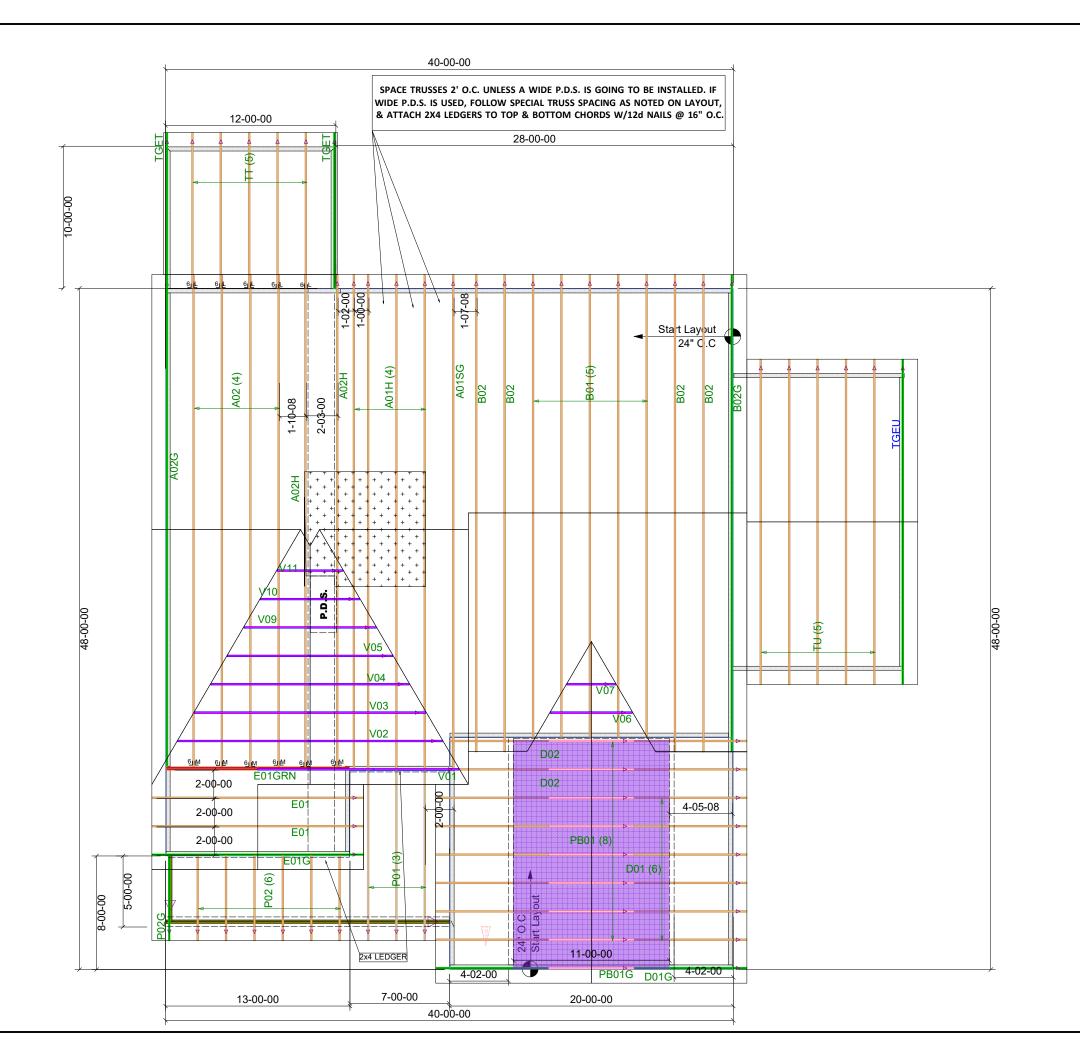




2ND FLOOR I-JOIST PLACEMENT PLAN CARUSO HOMES - FONTANA - LOT 19 MAGNOLIA



SHEET: 2 OF 2 SHEETS





Builders First Source 23 Red Cedar Way Apex, NC 27523 Phone: (919) 363-4956 Fax: (919) 387-8565 https://www.bldr.com

#### **General Notes:**

- Per ANSI/TPI 1-2002 all " Truss to Wall" connections are the responsibility of the Building Designer, not the Truss Manufacturer.

- Dimensions are Feet-Inches- Sixteenths.
   Trusses are to be 24" o.c. unless noted otherwise
- Trusses are not designed to support brick U.N.O.
- Do not cut or modify trusses without first contacting Builders FirstSource.
   Immediately contact Builders FirstSource if

#### trusses are damaged.

- Connection Notes:

   All hangers are to be Simpson or equivalent U.N.O.

   Use Manufacturer's specifications for all hanger
- Use Manufacturer's specifications for all ranges.
  connections U.N.O.
   Use 10d x 1 1/2" Nails in hanger connections to single ply roof girder trusses.

Floor notes:
- Shift truss as required to avoid plumbing traps.
- Installation Contractor and/or Field Supervisor are to verify all dimensions, trap locations, and options prior to

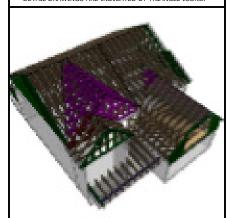
#### **Dimension Notes:**

HANGER LIST

6 HTU26 <u>Мյ (6</u> 3 LUS24 L<sub>I I</sub>4

- Drawing not to scale. Do not scale dimensions

 $\triangle$   $\triangleleft$   $\nabla$   $\triangleright$  LEFT END OF TRUSS AS SHOWN ON TRUSS DETAIL DRAWINGS ARE INDICATED BY TRIANGLE ICONS.



ALL TIE DOWNS H2.5A UNLESS NOTE SPECIAL ITEMS LIST

,							
				MISC MATERIAL			
		C	CARUS	MOH O	IES	5	
FONTANA				ELEV:			2
		М	AGNO	LIA AC	RE	S	
			NC	LOT:			19
				APPWRIGHT #			
• [OPTION 1]			4558473				
• [OPTION 2]			CODE			IRC 2015	
• [OPTION 3]			<u>LOADING:</u>				
				T.C.L.	L.		40 PSF
DESIGNED BY:		H)	/S	T.C.D.	L.		10 PSF
LAYOUT:				B.C.L.	L.		0 PSF
L/Q DATE:	3/31/2025		B.C.D.	L.		5 PSF	
<b>REVISION HISTORY</b>			<u>WIND:</u>				
REV1:	XX/XX/XX		M.P.H	١.	120 MPH		
REV2:	XX/XX/XX		EXPOSURE CATEGORY				
REV3:	3: XX/XX/XX		B (WOODED AREAS/OTHERS)				
PICK TICKET:				JOI	JOB NO:		
041.50				466	ACCT NO:		
SALES	NO:			ALL	ΤD	iU:	

OLUME CEILING

STICK FRAMING