

Carter Sanford Component Plant 298 Harvey Faulk Rd Sanford, NC 27332

Phone #:919-775-1450

Builder: DR Horton Inc

Model: 89 Eagle Creek -Norman - D



THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.

2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.

3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.

4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.

5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.

6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.

7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.

8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.

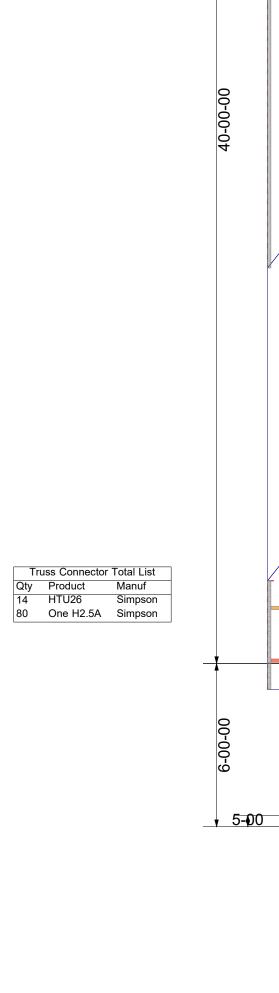
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

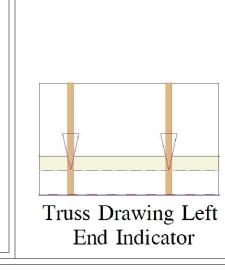
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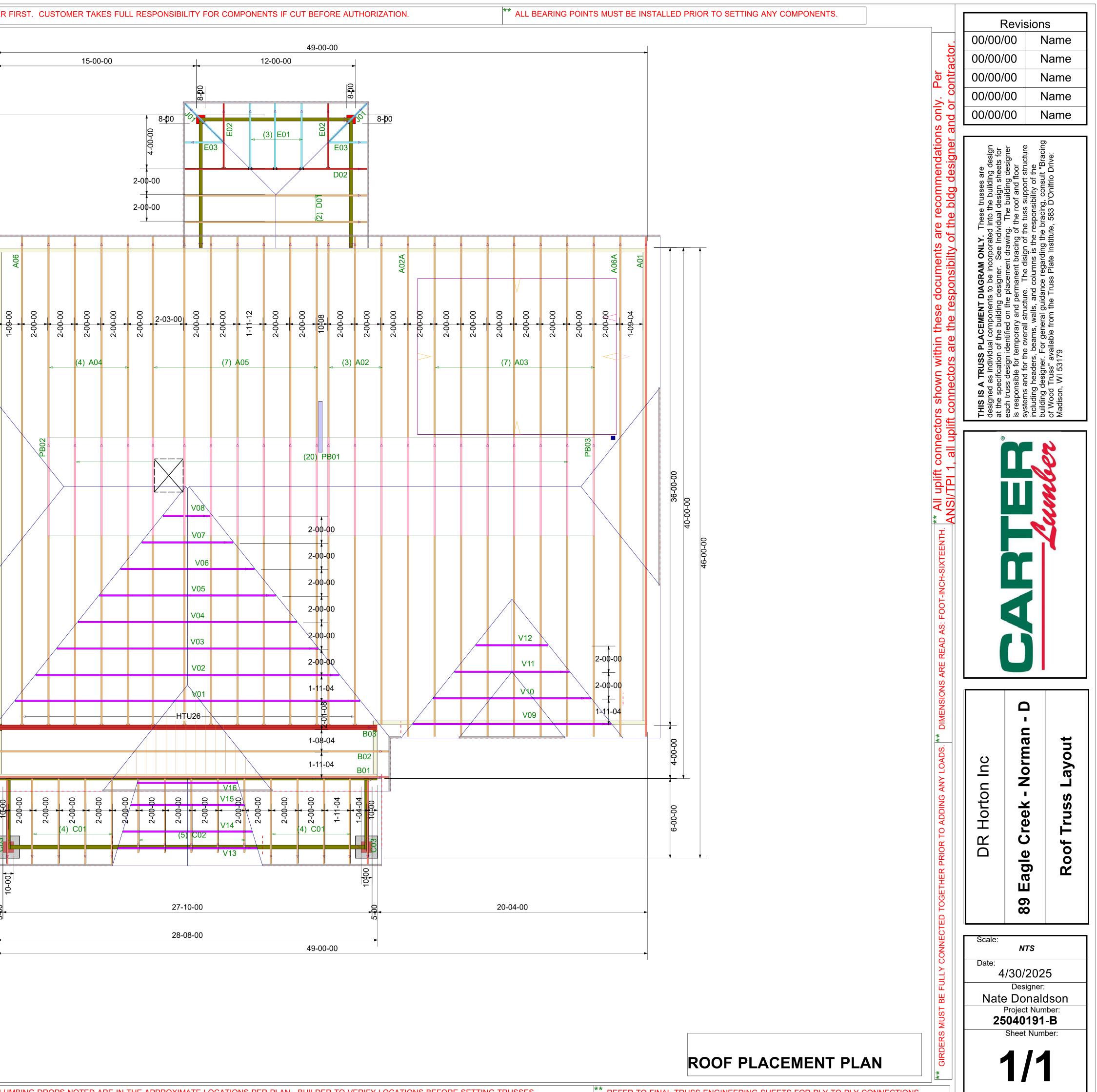


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** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.



RE: 25040191 89 Eagle Creek - Norman D - Roof Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:Customer: DR Horton IncProject Name:25040191Lot/Block: 89Model: Norman DAddress:Subdivision: EagleCity:State:

Subdivision: Eagle Creek State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014 Wind Code: ASCE 7-16 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.7 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 40 individual, dated Truss Design Drawings and 0 Additional Drawings.

No. 1 2 3	Seal# I68925780 I68925781 I68925782	Truss Name A01 A01A A02	Date 10/17/2024 10/17/2024 10/17/2024	No. 21 22 23	Seal# I68925800 I68925801 I68925802	Truss Name J01 PB01 PB02	Date 10/17/2024 10/17/2024 10/17/2024
3 4	168925783	A02 A02A	10/17/2024	23 24	168925803	PB02 PB03	10/17/2024
5	168925784	A03	10/17/2024	25	168925804	V01	10/17/2024
6	168925785	A04	10/17/2024	26	168925805	V02	10/17/2024
7	168925786	A05	10/17/2024	27	168925806	V03	10/17/2024
8	168925787	A06	10/17/2024	28	168925807	V04	10/17/2024
9	168925788	A06A	10/17/2024	29	168925808	V05	10/17/2024
10	168925789	B01	10/17/2024	30	168925809	V06	10/17/2024
11	168925790	B02	10/17/2024	31	168925810	V07	10/17/2024
12	168925791	B03	10/17/2024	32	168925811	V08	10/17/2024
13	168925792	C01	10/17/2024	33	168925812	V09	10/17/2024
14	168925793	C02	10/17/2024	34	168925813	V10	10/17/2024
15	168925794	C03	10/17/2024	35	l68925814	V11	10/17/2024
16	168925795	D01	10/17/2024	36	168925815	V12	10/17/2024
17	168925796	D02	10/17/2024	37	168925816	V13	10/17/2024
18	168925797	E01	10/17/2024	38	168925817	V14	10/17/2024
19	168925798	E02	10/17/2024	39	168925818	V15	10/17/2024
20	168925799	E03	10/17/2024	40	168925819	V16	10/17/2024

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Carter Components (Sanford, NC)).

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A01	Hip Supported Gable	1	1	Job Reference (optional)	168925780

8-10-0

0 0

Scale = 1:64.1

Loading

TCLL (roof)

Snow (Pf)

LUMBER

TOP CHORD

BOT CHORD

TCDL

BCLL

BCDL

WEBS

OTHERS

Run: 8,73 S Sep 25 2024 Print: 8,730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:39

Page: 1 ID:jgeUCYH2wVfnTG7e_tlm6?zuo40-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 0-10-8 36-9-8 11-9-0 24-2-0 35-11-0 0-10-8 11-9-0 12-5-0 11-9-0 0-10-8 5x6= 5x6= 4**42**5 89 10 13 14 1516 11 7 17 12 81 6 18 5 19 X Ø Ø X X 4⁴³ 4620 3 21 22 23 24 40 39 38 37 36 35 34 32 30 41 33 31 29 28 27 26 25 5x6= 5x6= 35-11-0 Plate Offsets (X, Y): [8:0-4-8,0-2-8], [16:0-4-8,0-2-8], [32:0-3-0,0-3-0], [34:0-3-0,0-3-0] 2-0-0 CSI DEFL in l/defl L/d PLATES GRIP (psf) Spacing (loc) 20.0 Plate Grip DOL 1.15 TC 0.16 Vert(LL) 999 MT20 244/190 n/a n/a 20.0 Lumber DOL 1.15 BC 0.08 Vert(CT) n/a n/a 999 Rep Stress Incr WB 10.0 YES 0.20 Horz(CT) 0.01 24 n/a n/a 0.0 IRC2018/TPI2014 Matrix-MR Code 10.0 Weight: 271 lb FT = 20% Max Grav 24=165 (LC 51), 25=196 (LC 26), NOTES 26=185 (LC 45), 27=228 (LC 45), 2x4 SP No 2 Unbalanced roof live loads have been considered for 1) 28=227 (LC 45), 29=233 (LC 45), 2x4 SP No.2 this design. 30=180 (LC 54), 31=234 (LC 40), Wind: ASCE 7-16; Vult=130mph (3-second gust) 2x4 SP No.3 2) 2x4 SP No.3 32=224 (LC 40), 33=228 (LC 40), Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. 34=224 (LC 40), 35=232 (LC 40), II; Exp B; Enclosed; MWFRS (envelope) exterior zone

BRACING 36=190 (LC 56), 37=233 (LC 43), and C-C Corner(3E) -0-10-8 to 2-8-9, Exterior(2N) 2-8-9 TOP CHORD Structural wood sheathing directly applied or 38=227 (LC 43), 39=229 (LC 43), to 7-11-8, Corner(3R) 7-11-8 to 15-4-1, Exterior(2N) 6-0-0 oc purlins, except end verticals, and 40=185 (LC 43), 41=216 (LC 25), 15-4-1 to 20-6-15, Corner(3R) 20-6-15 to 27-11-8, 2-0-0 oc purlins (6-0-0 max.): 8-16. 42=201 (LC 53) Exterior(2N) 27-11-8 to 33-2-7, Corner(3E) 33-2-7 to BOT CHORD Rigid ceiling directly applied or 6-0-0 oc FORCES (Ib) - Maximum Compression/Maximum 36-9-8 zone; cantilever left and right exposed ; end bracing. vertical left and right exposed;C-C for members and Tension WEBS 1 Row at midpt 12-33, 11-34, 10-35, forces & MWFRS for reactions shown; Lumber TOP CHORD 2-42=-162/83, 1-2=0/34, 2-3=-157/153, 9-36, 13-32, 14-31, 15-30 3-4=-118/118, 4-5=-109/129, 5-6=-102/162, DOL=1.60 plate grip DOL=1.60 **REACTIONS** (size) 24=35-11-0. 25=35-11-0. 3) Truss designed for wind loads in the plane of the truss 6-7=-120/215, 7-8=-144/262, 8-9=-122/237. 26=35-11-0. 27=35-11-0. 9-10=-122/237, 10-11=-122/237, only. For studs exposed to wind (normal to the face), 28=35-11-0, 29=35-11-0, see Standard Industry Gable End Details as applicable, 11-12=-123/237, 12-13=-123/237, 30=35-11-0, 31=35-11-0. 13-14=-122/237, 14-15=-122/237, or consult qualified building designer as per ANSI/TPI 1. 32=35-11-0, 33=35-11-0, 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 15-16=-122/237, 16-17=-145/262, 34=35-11-0, 35=35-11-0, Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate 17-18=-121/216, 18-19=-96/157, 36=35-11-0, 37=35-11-0, DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; 19-20=-73/106, 20-21=-73/74, 38=35-11-0, 39=35-11-0, Cs=1.00: Ct=1.10 21-22=-105/98. 22-23=0/34. 22-24=-133/46 111111 40=35-11-0, 41=35-11-0, BOT CHORD 41-42=-96/108, 40-41=-96/108, ORTH 42=35-11-0 CAR 39-40=-96/108, 38-39=-96/108, Max Horiz 42=-225 (LC 12) 37-38=-96/108, 36-37=-96/108, Max Uplift 24=-53 (LC 11), 25=-115 (LC 15), 35-36=-96/108, 33-35=-96/108, 26=-45 (LC 15), 27=-60 (LC 15), 31-33=-96/107, 30-31=-96/107, 28=-62 (LC 15), 29=-47 (LC 15), 29-30=-96/107, 28-29=-96/107, CHARLEN WINDER 31=-31 (LC 11), 32=-26 (LC 10), 27-28=-96/107, 26-27=-96/107, SEAL 33=-24 (LC 11), 34=-26 (LC 10), 25-26=-96/107, 24-25=-96/107 35=-33 (LC 10), 37=-49 (LC 14), 036322 WEBS 12-33=-186/50, 11-34=-184/50, 38=-61 (LC 14), 39=-61 (LC 14), 10-35=-194/55, 9-36=-150/23, 7-37=-193/73, 40=-42 (LC 14), 41=-128 (LC 14), 6-38=-194/86, 5-39=-194/83, 4-40=-158/76, 42=-99 (LC 10) 3-41=-148/118, 13-32=-184/49, 14-31=-194/55, 15-30=-140/3, 17-29=-193/71, 18-28=-194/87, 19-27=-193/82, 20-26=-159/76, C GI

21-25=-141/111

111111111 October 17,2024



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Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE WARNING Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A01	Hip Supported Gable	1	1	Job Reference (optional)	168925780

- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc.
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 99 lb uplift at joint 42, 53 lb uplift at joint 24, 24 lb uplift at joint 33, 26 lb uplift at joint 34, 33 lb uplift at joint 35, 49 lb uplift at joint 37, 61 lb uplift at joint 38, 61 lb uplift at joint 39, 42 lb uplift at joint 40, 128 lb uplift at joint 41, 26 lb uplift at joint 32, 31 lb uplift at joint 31, 47 lb uplift at joint 29, 62 Ib uplift at joint 28, 60 lb uplift at joint 27, 45 lb uplift at joint 26 and 115 lb uplift at joint 25.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S) Standard

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:39 ID:jgeUCYH2wVfnTG7e_tlm6?zuo40-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

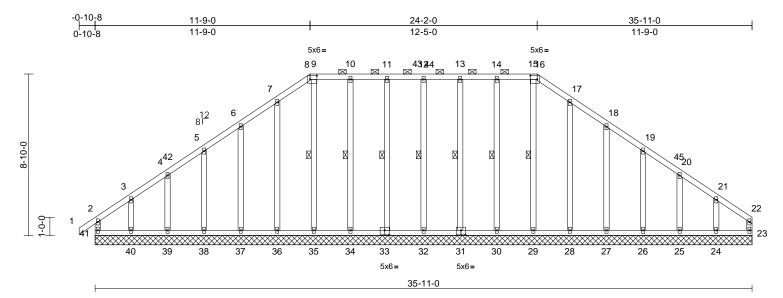
Page: 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A01A	Hip Supported Gable	1	1	Job Reference (optional)	168925781

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Scale = 1:63

Plate Offsets (X, Y): [8:0-4-8,0-2-8], [16:0-4-8,0-2-8], [31:0-3-0,0-3-0], [33:0-3-0,0-3-0] Loading Snow (P) (ps) 20.0 Spacing Plate Grip DOL 1.15 Spacing Plate Grip DOL 1.15 CSI TCDL BC DEFL Vert(LL) in (loc) I/deft L/d PLATES GRIP MT20 Shacing Snow (P) 0.0 Plate Grip DOL 1.00 1.15 TC 0.16 Vert(LL) n/a - n/a 999 CDL 0.0* Rep Stress Incr YES Code IRC2018/TPI2014 Matrix-MR Vert(LC 51), 24-214 (LC 65), 23 - 230 (LC 45), 24-234 (LC 40), 42-324 (LC 40), 22-324 (LC 40), 32-322 (LC 40), 32-322 (LC 41), 38-227 (LC 41), 38-223 (LC 40), 32-328 (LC 40), 32-328 (LC 40), 33-32-32 (LC 41), 38-223 (LC 40), 32-328 (LC 41), 38-227 (LC 41), 38-228 (LC 40), 32-38-310 (LC 40), 34-38-310 (LC 40), 34-38-310 (LC 40), 34-328-310 (LC 40), 34-328	Scale = 1:63																	
TCLL (roof) 20.0 Plate Grip DOL 1.15 TC 0.16 Vert(LL) n/a - n/a 999 MT20 244/190 Snow (Pf) 20.0 Lumber DOL 1.15 EC 0.06 Vert(LL) n/a - n/a 999 MT20 244/190 BCL 0.0* Rep Stress Incr YES Odde IRC2018/TPI2014 Matrix-MR Weight: 269 lb FT = 20% LUMBER 0.0* 2x4 SP No.2 Song (CT) 23 n/a N/a <t< th=""><th>Plate Offsets (</th><th>X, Y): [8:0-4-8,</th><th>0-2-8],</th><th>[16:0-4-8,0-2-8], [31:</th><th>:0-3-0,(</th><th>0-3-0], [33:0-3-0</th><th>0,0-3-0]</th><th></th><th></th><th>-</th><th></th><th></th><th></th><th></th><th></th><th></th><th></th><th></th></t<>	Plate Offsets (X, Y): [8:0-4-8,	0-2-8],	[16:0-4-8,0-2-8], [31:	:0-3-0,(0-3-0], [33:0-3-0	0,0-3-0]			-								
TOP CHORD DOP CHORD $2x4$ SP No.2 $25=180$ (LC 45), $26=230$ (LC 45), $27=227$ (LC 48), $28=232$ (LC 40), $27=227$ (LC 48), $28=232$ (LC 40), $29=176$ (LC 54), $30=234$ (LC 40), $31=224$ (LC 40), $32=228$ (LC 40), $31=224$ (LC 40), $34=222$ (LC 40), $31=224$ (LC 40), $34=227$ (LC 43), $31=224$ (LC 40), $34=272$ (LC 43), $31=224$ (LC 40), $34=210$ (LC 53)this design.2000000000000000000000000000000000000	TCLL (roof) Snow (Pf) TCDL BCLL	2 2 1	20.0 20.0 10.0 0.0*	Plate Grip DOL Lumber DOL Rep Stress Incr	1.15 1.15 YES		TC BC WB	-MR	0.08	Vert(LL) Vert(CT)	n/a n/a	a a	-	n/a n/a	999 999	MT20	244/1	90
33330110, 3433110, 3433110, 3535110, 37355110, 3835110, 39335110, 40-35110, 41=35105, 39340 17-18=-129/209, 18-19=-104/150, 19-20=-76/101, 20-21=-78/67, 19-20=-76/101, 20-21=-78/67, 19-20=-76/101, 20-21=-78/67, 19-20=-76/101, 20-21=-78/67, 19-20=-76/101, 20-21=-78/67, 19-20=-76/101, 20-21=-78/67, 33-35110, 40-35110, 40-431=-85/95, 39-40=-85/95, 38-39=-85/95, 33-39=-85/95, 32-34=-85/95, 30-32=-85/95, 22-24=-85/95, 2	TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 Structural woo 6-0-0 oc purlin 2-0-0 oc purlin Rigid ceiling of bracing. 1 Row at midp (size) 23= 27= 29= 31= 33= 35= 37= 41= Max Horiz 41= Max Uplift 23= 27= 30= 32= 34= Max Horiz 41= Max Uplift 23= 27= 30= 32= 34= 34= 37= 39=	ns, exc ns (6-0 directly pt =35-11: =3	cept end verticals, an -0 max.): 8-16. applied or 6-0-0 oc 12-32, 11-33, 10-34, 9-35, 13-31, 14-30, 1 -0, 24=35-11-0, -0, 28=35-11-0, -0, 32=35-11-0, -0, 32=35-11-0, -0, 34=35-11-0, -0, 40=35-11-0, -0, 40=35-11-0, -0, 40=35-11-0, -0, 40=35-11-0, -0, 40=35-11-0, -0, 11), 24=-116 (LC 16 C 15), 28=-47 (LC 15 C 11), 31=-26 (LC 10 C 11), 33=-26 (LC 10 C 11), 33=-26 (LC 10 C 14), 38=-61 (LC 14 C 14), 40=-131 (LC 14 C 14), 40=-131 (LC 14 C 14), 40=-131 (LC 14)	15-29 (5), (5), (5), (5), (1), (1), (1), (1), (1), (1), (1), (1	FORCES TOP CHORD BOT CHORD WEBS	(lb) - Ma: Tension 2-41=-16 3-4=-125 6-7=-128 9-10=-12 11-12=-1 13-14=-1 17-18=-1 17-18=-1 17-18=-1 19-20=-7 21-22=-1 40-41=-8 37-38=8 34-35=-8 26-27=-8 26-27=-8 23-24=-8 12-32=-1 10-34=-1 6-37=-19 3-40=-14 14-30=-1 17-28=-1	25=180 27=227 29=176 31=224 33=224 35=189 37=225 39=185 41=210 ximum Cc 39/84, 1-2 5/120, 4-5 3/209, 7.8 6/2(231, 10 (28/232, 1 27/232, 1 (27/232, 1 (27/232, 1 (27/232, 1 (29/209, 1 76/101, 20 (00/93, 22 35/95, 39- 35/95, 32- 35/95, 25- 35/95 (36, 5) (36, 5) (36, 5) (37, 10) (36, 5) (37, 10) (37, 10	(LC 45) (LC 45) (LC 45) (LC 45) (LC 40) (LC 40) (LC 40) (LC 56) (LC 41) (LC 56) (LC 41) (LC 53) (LC 41) (LC 53) (LC 53	26=230 (L 28=232 (L 30=234 (L 34=232 (L 34=232 (L 34=232 (L 36=231 (L 38=227 (L 40=214 (L 0n/Maximu -3=-166/15 28, 5-6=-11 5, 8-9=-12 6/231, 28/232, 27/232, 53/256, 04/150, /67, '41 95, 38-39=-1 95, 30-32=- 95, 27-28=- 15, 24-25=- 4/50, /27, 7-36=- 13, 4-39=-1 4/49, 6/6, 4/87,	C 45), C 45), C 40), C 40), C 40), C 43), C 43), C 43), C 43), C 43), C 25), m 3, 1/161, 6/231, 85/95, 85/95, 85/95, 85/95, 85/95, 85/95, 85/95,	2)	this of Wind Vasc II; E2 and to 7- 15-4 Exte 35-9 Vertif force DOL Trus only. see 2 or cc TCLI Plate DOL	design. d: ASCE d=103m xp B; Er C-C Co -11-8, C -1 to 20 erior(2N) -4 zone cal left a es & MW =1.60 p so desig . For st Standau onsult q L: ASCI e DOL= =1.15); 1.00; C1	E 7-166 hph; TC nrner(3 orner(-6-15, 27-11 ; canti and rig VFRS blate g ned fo VFRS blate g ned fo vlatifie E 7-16 late g ned fo vlatifie e 1.15 vlatifie e 1.15 vlatifie vla	; Vult=130mph CDL=6.0psf; Bi d; MWFRS (er E) -0-10-8 to 2 3R) 7-11-8 to 7 Corner(3R) 20 1-8 to 31-11-8, ilever left and r sht exposed;C- for reactions s rip DOL=1.60 r wind loads in coposed to wind ustry Gable En d building desi ; Pr=20.0 psf (L Pf=20.0 psf (L C); Rough Cat E	(3-second CDL=6.0p ivelope) e: -8-9, Exte 15-4-1, Ex 0-6-15 to 2 Corner(3i ight expose C for men hown; Lur the plane ti d Details a gner as per roof LL: Li um DOL= 3; Fully Ex	d gust) sf; h=25f; Cat. xterior zone rior(2N) 2-8-9 tterior(2N) 27-11-8, E) 31-11-8 to sed ; end nbers and nber e of the truss o the face), as applicable, er ANSI/TPI 1. um DOL=1.15 1.15 Plate p.; Ce=0.9;

October 17,2024

Page: 1

Continued on page 2 WARNING - Verify

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	100005704
25040191	A01A	Hip Supported Gable	1	1	Job Reference (optional)	168925781

- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 2x4 MT20 unless otherwise indicated.
- 9) Gable requires continuous bottom chord bearing.
- 10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 11) Gable studs spaced at 2-0-0 oc. 12) This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 101 lb uplift at joint 41, 51 lb uplift at joint 23, 24 lb uplift at joint 32, 26 lb uplift at joint 33, 33 lb uplift at joint 34, 49 lb uplift at joint 36, 61 lb uplift at joint 37, 61 lb uplift at joint 38, 42 lb uplift at joint 39, 131 lb uplift at joint 40, 26 lb uplift at joint 31, 31 lb uplift at joint 30, 47 lb uplift at joint 28, 62 Ib uplift at joint 27, 60 lb uplift at joint 26, 45 lb uplift at joint 25 and 116 lb uplift at joint 24.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S) Standard

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Page: 2

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A02	Piggyback Base	3	1	Job Reference (optional)	168925782

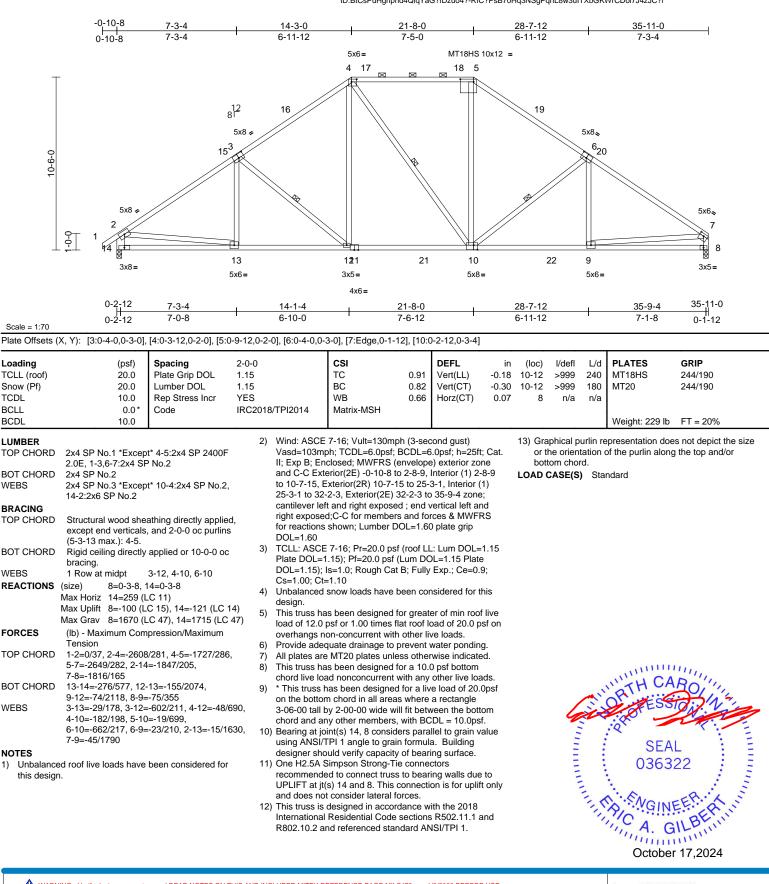
BCDL

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:40 ID:BtCsPuHghpnd4QiqYaG?fDzuo4?-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

818 Soundside Road

Edenton, NC 27932



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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A02A	Piggyback Base	1	1	Job Reference (optional)	168925783

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:40 Page: 1 ID:BtCsPuHghpnd4QiqYaG?fDzuo4?-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f -0-10-8 36-9-8 7-3-4 28-7-12 14-3-0 21-8-0 35-11-0 0-10-8 6-11-12 7-3-4 7-5-0 6-11-12 7-3-4 0-10-8 5x6= MT18HS 10x12 = 4 18 19 5 8¹² 17 5x8 🖌 5x8。 16³ ⁶21 5x8 -5x8、 2 7 8 Å 45 ł 14 22 11 23 10 132 3x8= 3x8= 5x6= 3x5 = 5x8= 5x6= 4x6 =35-11-0 || 0-2-12 0-2-12 7-3-4 14-1-4 21-6-4 28-7-12 35-8-4 0-2-12 7-0-8 6-10-0 7-5-0 7-1-8 7-0-8

Plate Offsets (X, Y): [3:0-4-0,0-3-0], [4:0-3-12,0-2-0], [5:0-9-12,0-2-0], [6:0-4-0,0-3-0], [11:0-4-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.87	Vert(LL)	-0.17	11-13	>999	240	MT18HS	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.82	Vert(CT)	-0.28	11-13	>999	180	MT20	244/190
TCDL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(CT)	0.07	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 231 lb	FT = 20%

LUMBER

Scale = 1:70

10-6-0

0-0-

- TOP CHORD 2x4 SP No.1 *Except* 4-5:2x4 SP 2400F 2.0E, 1-3,6-8:2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 *Except* 11-4:2x4 SP No.2, 15-2,9-7:2x6 SP No.2 BRACING
- TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (5-4-1 max.): 4-5. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
- WEBS 1 Row at midpt 3-13, 4-11, 6-11 **REACTIONS** (size) 9=0-3-8. 15=0-3-8 Max Horiz 15=-266 (LC 12) Max Uplift 9=-121 (LC 15), 15=-121 (LC 14) Max Grav 9=1722 (LC 47), 15=1710 (LC 47) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/37, 2-4=-2600/280, 4-5=-1719/285, 5-7=-2621/279, 7-8=0/37, 2-15=-1842/205, 7-9=-1859/203 BOT CHORD 14-15=-270/584, 13-14=-144/2077, 10-13=-36/2094, 9-10=-124/473
- WEBS 3-14=-27/180, 3-13=-604/211, 4-13=-49/686, 4-11=-184/195, 5-11=-17/678, 6-11=-637/211, 6-10=-25/209, 2-14=-13/1625, 7-10=0/1665 NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-8-9, Interior (1) 2-8-9 to 10-7-15, Exterior(2R) 10-7-15 to 25-3-1, Interior (1) 25-3-1 to 33-2-7, Exterior(2E) 33-2-7 to 36-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed C-C for members and forces & MWERS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this desian.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding. 6)
- All plates are MT20 plates unless otherwise indicated. 7) This truss has been designed for a 10.0 psf bottom 8)
- chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf 9)
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Bearing at joint(s) 15, 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 15 and 9. This connection is for uplift only and does not consider lateral forces.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



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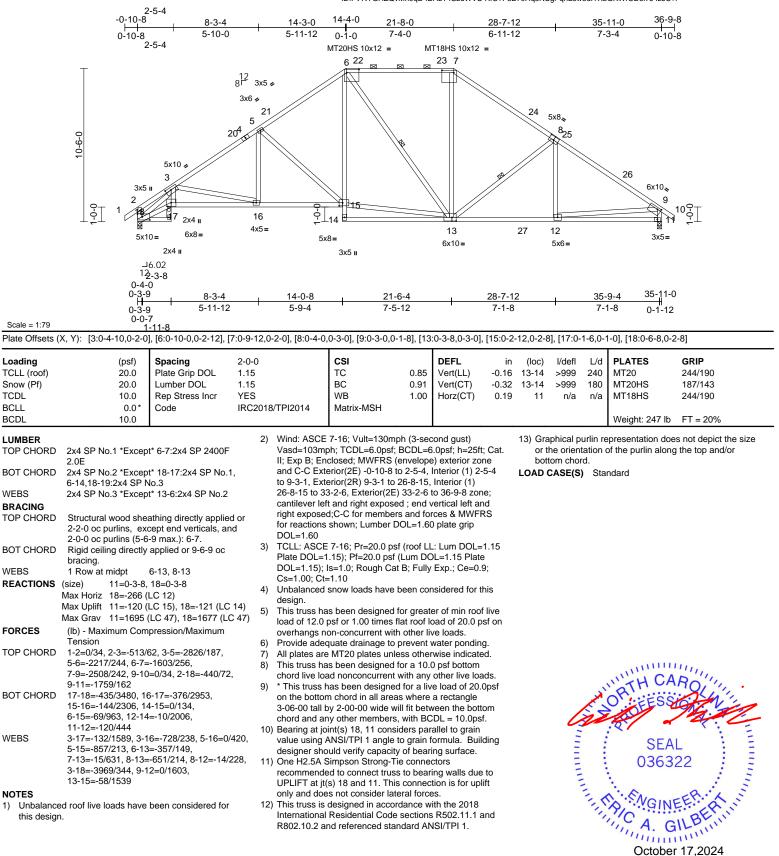
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A03	Piggyback Base	7	1	Job Reference (optional)	168925784

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818 Soundside Road

Edenton, NC 27932



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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A04	Piggyback Base	4	1	Job Reference (optional)	168925785

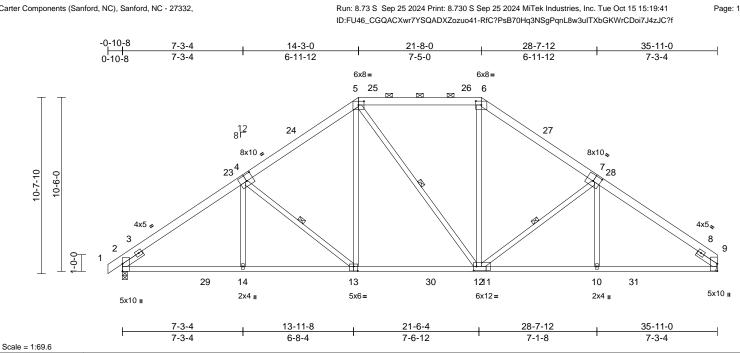


Plate Offsets (X, Y): [4:0-5-0,0-4-8], [5:0-4-0,0-2-12], [6:0-4-0,0-2-13], [7:0-5-0,0-4-8], [12:0-2-0,0-3-0], [13:0-3-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.67	Vert(LL)	-0.19	12-13	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.85	Vert(CT)	-0.31	12-13	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.26	Horz(CT)	0.12	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 246 lb	FT = 20%
LUMBER 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) 13) Graphical purlin representatio TOP CHORD 2x6 SP No.2 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; Cat. or the orientation of the purlin								1				

2x4 SP No.1 *Except* 13-11:2x4 SP No.2 BOT CHORD WEBS 2x4 SP No.3 *Except* 12-5:2x4 SP No.2 SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0 BRACING TOP CHORD Structural wood sheathing directly applied or 3-4-0 oc purlins, except 2-0-0 oc purlins (5-0-12 max.): 5-6. 3) BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. WEBS 4-13, 5-12, 7-12 1 Row at midpt REACTIONS (size) 2=0-3-8, 9= Mechanical 4) Max Horiz 2=233 (LC 13) Max Uplift 2=-119 (LC 14), 9=-102 (LC 15) 5) Max Grav 2=1746 (LC 47), 9=1695 (LC 47) FORCES (Ib) - Maximum Compression/Maximum Tension 6) TOP CHORD 1-2=0/29, 2-5=-2763/285, 5-6=-1786/287, 7) 6-9=-2758/283 BOT CHORD 2-14=-239/2153, 12-14=-178/2149, 8) 10-12=-104/2145, 9-10=-109/2149 WEBS 4-14=0/269, 4-13=-632/226, 5-13=-56/723, 5-12=-181/200, 6-12=-17/698, 7-12=-651/226, 7-10=0/285 ۹)

NOTES

1) Unbalanced roof live loads have been considered for this design.

- sf: BCDL=6.0psf: h=25ft: Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-8-9, Interior (1) 2-8-9 to 10-7-15, Exterior(2R) 10-7-15 to 25-3-1, Interior (1) 25-3-1 to 32-3-15, Exterior(2E) 32-3-15 to 35-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed C-C for members and forces & MWERS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this desian.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom
- chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 102 lb uplift at joint
- 11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at it(s) 2. This connection is for uplift only and does not consider lateral forces.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard



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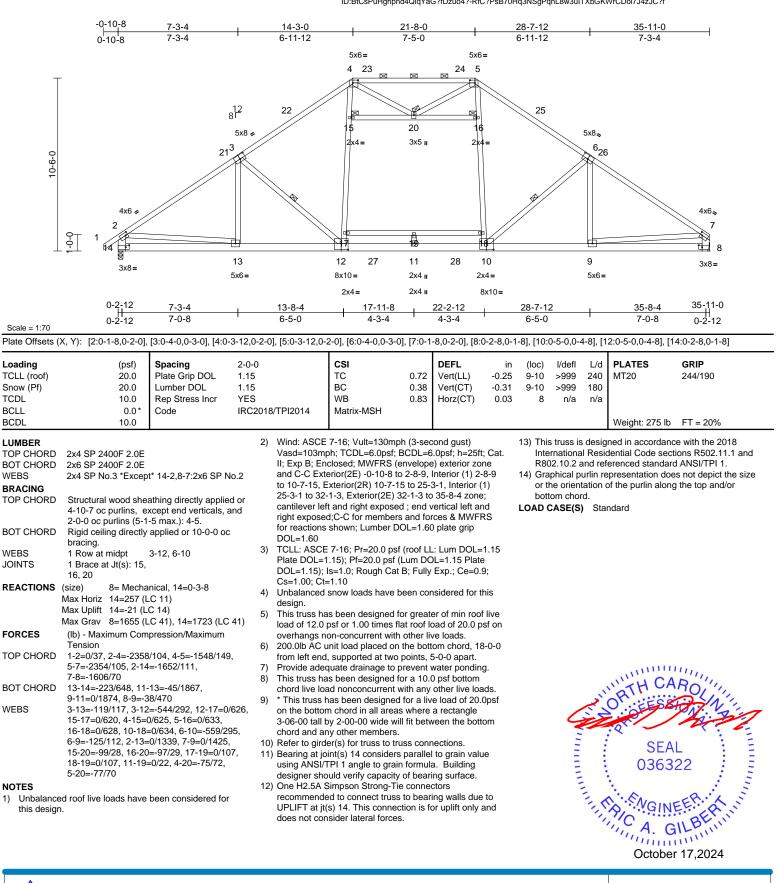
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A05	Piggyback Base	7	1	Job Reference (optional)	168925786

1)

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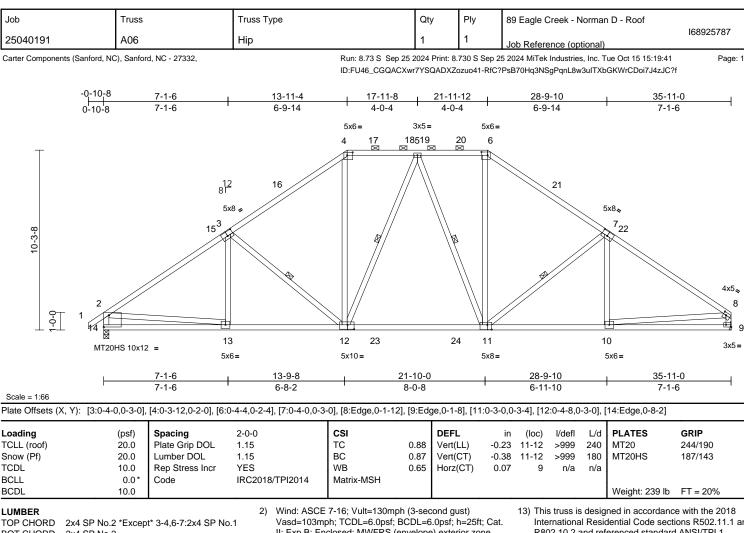
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof		
25040191	A06	Нір	1	1	Job Reference (optional)	168925787	



BOT CHORD	2X4 SP N0.2
WEBS	2x4 SP No.3 *Except* 12-5,11-5:2x4 SP No.2
BRACING	
TOP CHORD	Structural wood sheathing directly applied,
	except end verticals, and 2-0-0 oc purlins

(4-3-4 max.): 4-6.

BOT CHORD	Rigid ceili	ing directly	/ applied or 10-0-0 oc
	bracing.		
WEBS	1 Row at	midpt	3-12, 5-12, 5-11, 7-11
REACTIONS	(size)	9= Mech	anical, 14=0-3-8
	Max Horiz	14=254 (LC 11)
	Max Uplift	9=-98 (L	C 15), 14=-118 (LC 14)
	Max Grav	9=1645 (LC 47), 14=1697 (LC 47)
FORCES	(lb) - Max	imum Cor	npression/Maximum
	Tension		
TOP CHORD	1-2=0/34,	2-4=-261	5/277, 4-5=-1727/281,
	5-6=-173°	1/281, 6-8	=-2613/277,
	2-14=-183	30/201, 8-	9=-1790/164
BOT CHORD	13-14=-25	57/550, 10)-13=-157/2095,
	9-10=-64/	334	

WEBS 3-13=-30/171, 3-12=-603/210, 4-12=-21/750, 5-12=-269/173, 5-11=-263/174, 6-11=-23/751, 7-11=-616/213, 7-10=-35/171, 2-13=-34/1669, 8-10=-59/1786

NOTES

Unbalanced roof live loads have been considered for 1) this design.

- II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-8-9, Interior (1) 2-8-9 to 10-4-3, Exterior(2R) 10-4-3 to 17-6-5, Interior (1) 17-6-5 to 18-4-11, Exterior(2R) 18-4-11 to 25-6-13, Interior (1) 25-6-13 to 32-2-3, Exterior(2E) 32-2-3 to 35-9-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this 4) desian.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding. 6) All plates are MT20 plates unless otherwise indicated. 7)
- This truss has been designed for a 10.0 psf bottom 8)
- chord live load nonconcurrent with any other live loads. 9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom
- chord and any other members, with BCDL = 10.0psf. 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint
- 12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 14. This connection is for uplift only and does not consider lateral forces.

- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 14) Graphical purlin representation does not depict the size
- or the orientation of the purlin along the top and/or bottom chord.

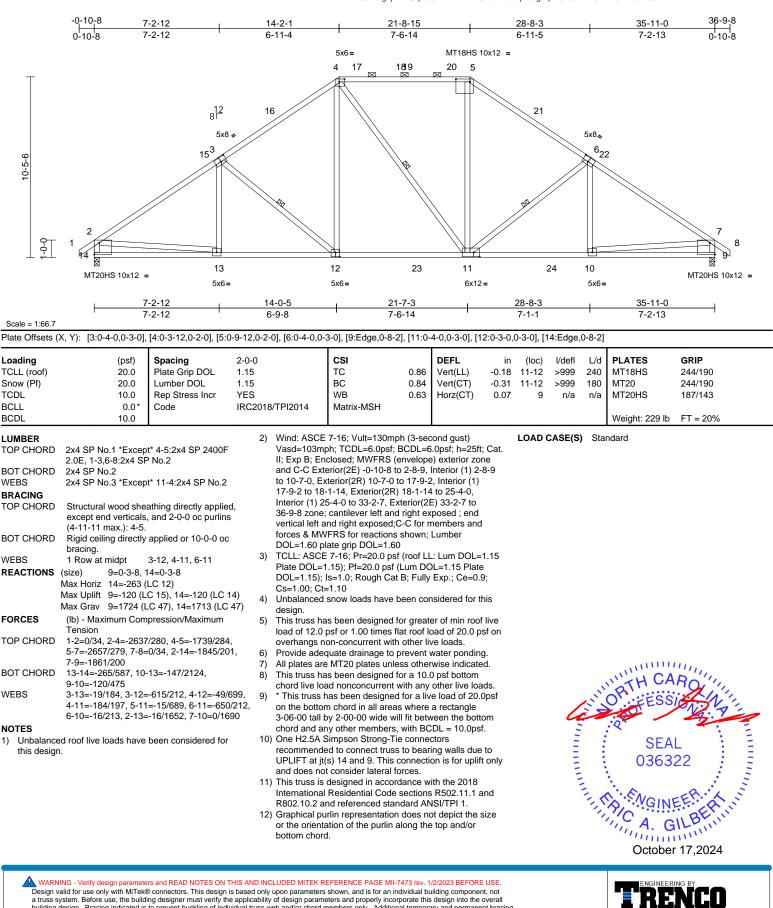
LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	A06A	Нір	1	1	Job Reference (optional)	168925788

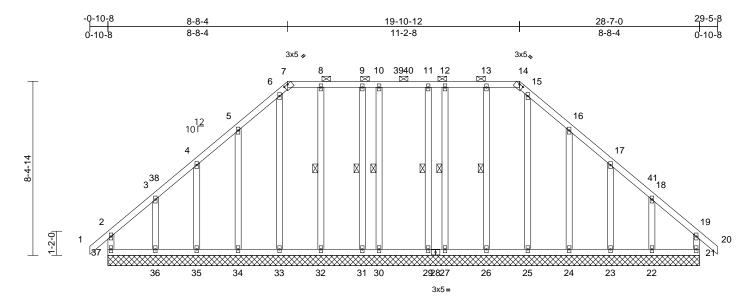
Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:42 ID:BtCsPuHghpnd4QiqYaG?fDzuo4?-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	B01	Hip Supported Gable	1	1	Job Reference (optional)	168925789

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:42 ID:0l2i579nHROBGkMjOnZQiuzuo4A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



0	n -	~ /	2	

Scale = 1:55.7 Plate Offsets (X, Y): [7:0-2-8,0-0-3], [14:0-2-8,0-0-3]

	(X, T): [1:0 Z 0,0	0 0], [1	1.0 2 0,0 0 0]											
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(ps 20. 20. 10. 0. 10.	0 F 0 L 0 F 0* C	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11 1.15 1.15 YES IRC2		CSI TC BC WB Matrix-MR	0.16 0.09 0.18	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 21	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 229 lb	GRIP 244/190 FT = 20%
	2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 Structural wood 6-0-0 oc purlins 2-0-0 oc purlins Rigid ceiling dire bracing. 1 Row at midpt (size) 21=22 24=22 31=22 34=22 34=22 37=22 Max Horiz 37=-2 Max Uplift 21=-6 23=-5 26=-1 31=-2 34=-5 23=-1 34=-5 34=-5 34=-5 36=-1 Max Grav 21=19 23=2 25=2 27=11. 30=2	, except (6-0-0 122 8-7-0, 2 8-7-0, 2 8-7-0, 2 8-7-0, 2 8-7-0, 2 8-7-0, 2 8-7-0, 2 9 (LC 99 (LC 22 (LC 99 (LC 22 (LC 99 (LC 25 (LC 25 (LC 255 (LC 255 (LC 255 (LC 255 (LC 219 (LC 255 (LC 219 (LC 255 (LC 255 (LC 255 (LC 219 (LC 255	plied or 6-0-0 oc -30, 11-29, 9-31, -27, 13-26 22=28-7-0, 23=28 55=28-7-0, 30=28 29=28-7-0, 30=28 22=28-7-0, 33=28 55=28-7-0, 36=28 12) 11), 22=-139 (LC 15), 24=-98 (LC 1 11), 27=-22 (LC 1 11), 30=-19 (LC 1 14), 35=-52 (LC 1 14), 37=-97 (LC	nd 8-32, -7-0, -7-0, -7-0, -7-0, -7-0, 5), 0), 1), 1), 1), 4), 10) 53), 53), 53), 40), 46), 40),	this design 2) Wind: ASC Vasd=103r II; Exp B; E and C-C C to 5-8-4, C to 16-10-12	E 7-16; Vult=130m nph; TCDL=6.0psf; inclosed; MWFRS (orner(3E) -0-10-8 to orner(3R) 5-8-4 to 2, Corner(3R) 16-10	5=-108/17 5=-108/17 5=-117/22 0-11=-11 12-13=-11 12-13=-11 12-13=-11 13-13=-11 5-36=-99 3-34=-99 1-32=-99 9-30=-99 9-23=-99 1-29=-15 32=-19 1/29=-15 32=-19 1/29=-15 32=-19 1/29=-15 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-99 9-20=-98 9-20=-98 9-20=-99 9-20=-99 9-20=-98 9-20=-98 9-20=-99 9-20=-98 9-20=-99 9-20=-98 9-20=-98 9-20=-99 9-20=	77, 5-6=-149/26 19, 8-9=-117/22 7/229, 17/229, 17/229, 22/201, 08/177, 4/108, 19-20=0 /106, /106, /106, /106, /106, /106, 8/40, 19, 6-33=-158/1 /85, 0/42, 3/1, 97/84, considered for cond gust) 6.0psf; h=25ff; (6) exterior(2N) 21- 2-10-12, Exterior	29,)/38, 17, 17, 24. 3-8 8-4 or	on sec 74) TC Pla DC CS 5) Un de 6) Th loz 55 5) Ga 10) Tn 8) All 9) Ga 10) Tn br 6 11) Ga 11) Ga	y. For si a Standa consult q LL: ASC balancec sign. is truss h d of 12.0 erhangs i boide add plates ar ble requi uss to be acced agai ble studs is truss h ord live lo	uds ex rd Indu ualifiee E 7-16 (1.15); (Is=1.0 d snow as bee psf or non-cco quate re 2x4 tres co fully si inst latt s space as bee ad non- co	kposed to wind (n istry Gable End D d building designer ; Pr=20.0 psf (roc Pf=20.0 psf (Lur 0; Rough Cat B; F loads have been en designed for g 1.00 times flat rc incurrent with oth drainage to prevy MT20 unless othen thinuous bottom of heathed from one eral movement (i, ed at 2-0-0 c. en designed for a inconcurrent with heathed from one (i) the concurrent with inconcurrent with inconcure	ent water ponding. erwise indicated. shord bearing. of face or securely e. diagonal web). 10.0 psf bottom
FORCES	36=2	94 (LC	51), 35=232 (LC 51), 37=217 (LC ession/Maximum		zone; canti and right e	-12 to 26-3-8, Corn lever left and right of xposed;C-C for mean r reactions shown; 1.60	exposed mbers ar	; end vertical le	eft				A. G	

October 17,2024

Page: 1



Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, recetion and bracing of trusses and truss systems, see **ANSI/TP1 Quility Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof		
25040191	B01	Hip Supported Gable	1	1	Job Reference (optional)	168925789	

- 13) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 97 lb uplift at joint 37, 69 lb uplift at joint 21, 19 lb uplift at joint 30, 19 lb uplift at joint 29, 22 lb uplift at joint 31, 14 lb uplift at joint 32, 97 lb uplift at joint 34, 52 lb uplift at joint 35, 146 lb uplift at joint 36, 22 lb uplift at joint 27, 15 lb uplift at joint 26, 98 lb uplift at joint 24, 53 lb uplift at joint 23 and 139 lb uplift at joint 22.
- 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
- 16) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:42 ID:0l2i579nHROBGkMjOnZQiuzuo4A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 2

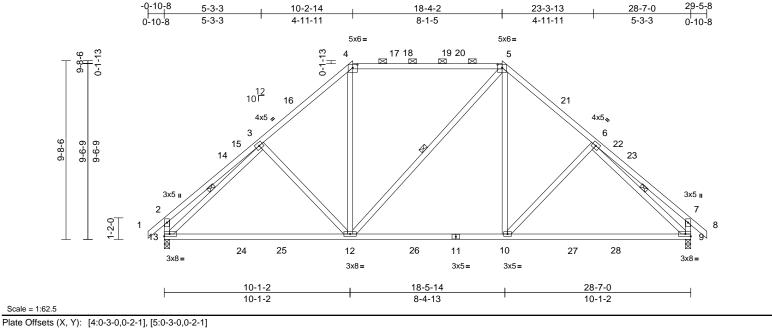
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	B02	Нір	1	1	Job Reference (optional)	168925790

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:42 ID:Uyb4JTAP2kW2uuxvyU4fF6zuo49-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1 28-7-0 29-5-8



Loading	(psf)	Spacing	1-11-4		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.83	Vert(LL)		12-13	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.45	Vert(CT)		12-13	>853	180		
CDL	10.0	Rep Stress Incr	YES		WB	0.36	Horz(CT)	0.03	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018	B/TPI2014	Matrix-MSH							-	
BCDL	10.0											Weight: 186 lb	FT = 20%
UMBER			2)	Wind: ASCE	7-16; Vult=130mp	oh (3-seo	cond gust)						
OP CHORD	2x4 SP 2400F 2.0E				oh; TCDL=6.0psf;								
OT CHORD	2x4 SP 2400F 2.0E				closed; MWFRS (
/EBS	2x4 SP No.3 *Excep	ot* 5-12:2x4 SP No.2			erior(2E) -0-10-8 to								
RACING					terior(2R) 7-2-14								
OP CHORD	Structural wood she				5-4-2, Exterior(2R) 26-5-8, Exterior(2								
	6-0-0 oc purlins, ex		nd		t and right expose								
	2-0-0 oc purlins (4-1				d;C-C for member								
OT CHORD	Rigid ceiling directly	applied or 10-0-0 oc)		shown; Lumber D								
EBS	bracing. 1 Row at midpt	3-13, 6-9, 5-12		DOL=1.60	,		1						
			3)	TCLL: ASCE	7-16; Pr=20.0 ps	f (roof Ll	: Lum DOL=	1.15					
EACTIONS	(size) 9=0-3-8, 1 Max Horiz 13=-241 (.15); Pf=20.0 psf								
	Max Uplift 9=-80 (LC	,	`		ls=1.0; Rough Cat	B; Fully	Exp.; Ce=0.9	9;					
	Max Grav 9=1381 (L		47)	Cs=1.00; Ct=									
ORCES			47) 4)		snow loads have l	been cor	nsidered for the	his					
JRCES	(lb) - Maximum Com Tension	pression/waximum	C)	design.	a haan daalamad i			1.					
OP CHORD	1-2=0/38, 2-3=-568/	120 3-1-1506/215	5)		is been designed f psf or 1.00 times f								
	4-5=-1158/218, 5-6=				on-concurrent with			51 011					
	6-7=-544/120, 7-8=0		6)		quate drainage to			a					
	7-9=-498/136	,,	7)		is been designed f			9.					
OT CHORD	12-13=-140/1237, 10	0-12=-6/1075,	• • • •		ad nonconcurrent			ads.				1111 00	
	9-10=-1/1244		8)		nas been designed							TH UA	ROUT
EBS	3-13=-1245/103, 6-9		′538, [′]		n chord in all area			•			N	04.000	C. LIN'S
	3-12=-247/210, 5-10			3-06-00 tall b	y 2-00-00 wide wi	ill fit betv	veen the bott	om		/	52	OPTEE89	The an
	6-10=-246/210, 5-12	2=-132/133			ny other members,			f.		4	ÚD		KA U
DTES			9)		Simpson Strong-Ti						0	i K	1 1 1 1
	ed roof live loads have	been considered for			ed to connect truss					TTTTTTTTTTTT		SEA	
this desigr	۱.				s) 13 and 9. This		on is for uplif	t only		=			• •
			40		t consider lateral fo		ith the 2010			1		0363	ZZ : :
			10		designed in accor Residential Code			and			0	N	1 2
					nd referenced star			anu			5	·	all S
				1.002.10.2 d	ina referenced Stal	iuaiu Al					10	C. ENOIN	-FULX

- 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



A. GILB

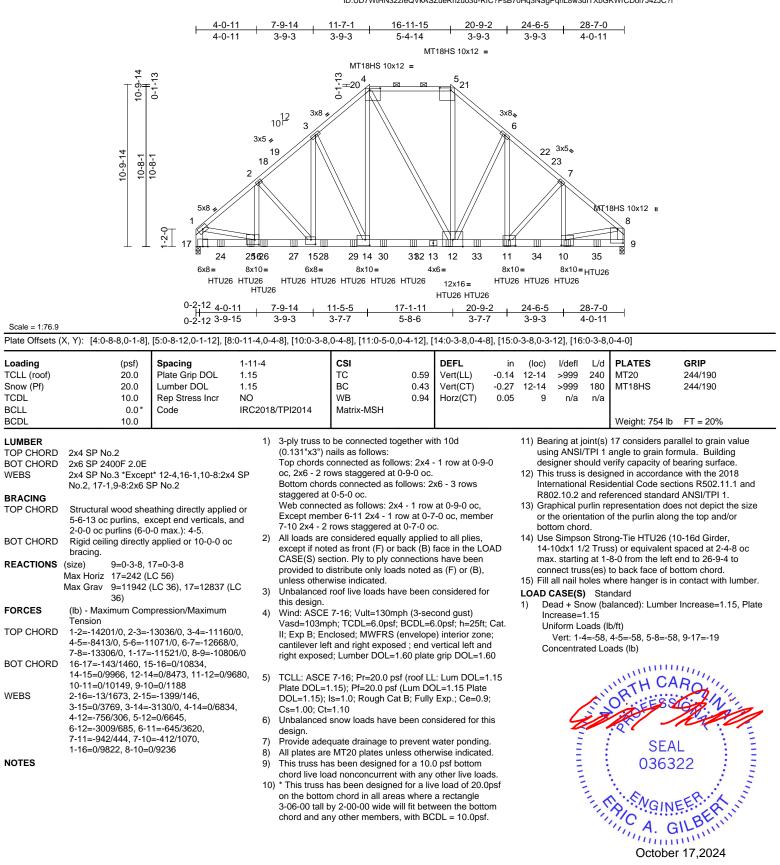
A. GILD

October 17,2024

C

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	B03	Hip Girder	1	3	Job Reference (optional)	168925791

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:42 ID:UD7WtHN32zfeQVkASZueRhzuo3u-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Continued on page 2

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A MiTek Affi 818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	B03	Hip Girder	1	3	Job Reference (optional)	168925791

Vert: 12=-1636 (B), 11=-1554 (B), 10=-1554 (B), 24=-1539 (B), 25=-1539 (B), 26=-1636 (B), 27=-1636 (B), 28=-1636 (B), 29=-1636 (B), 30=-1636 (B), 31=-1636 (B), 33=-1554 (B), 34=-1554 (B), 35=-1534 (B) Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:42 ID:UD7WtHN32zfeQVkASZueRhzuo3u-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 2

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	C01	Monopitch	8	1	Job Reference (optional)	168925792

5-7-0

5-7-0

12 3 Г

-0-10-8

0-10-8

Carter Components (Sanford, NC), Sanford, NC - 27332,

1-11-0

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:43 ID:B?wkH7GN3P?6eZhE6U?4VxzuWZL-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

2x4 🛛

3



0 8 9 1-7-3 2)-5-15 0 4 \bigotimes 2x4 🛚 3x5 = 0-3-4 5-7-0 5-5-8 0-3-4 0-1-8 5-2-4 T

Scale = 1:29.6

Loading (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0* BCDL 10.0	Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC 0.6 BC 0.4 WB 0.0 Matrix-MP	2 Vert(CT)	in -0.06 -0.11 0.01	(loc) 4-7 4-7 2	l/defl >999 >606 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 20 lb	GRIP 244/190 FT = 20%
 5-7-0 oc purlins, exc Rigid ceiling directly bracing. REACTIONS (size) 2=0-3-8, 4 Max Horiz 2=62 (LC Max Uplift 2=-65 (LC Max Grav 2=366 (LC FORCES (lb) - Maximum Comp Tension TOP CHORD 1-2=0/18, 2-3=-107/5 BOT CHORD 2-4=-77/114 NOTES 1) Wind: ASCE 7-16; Vult=130mph Vasd=103mph; TCDL=6.0psf; BC II; Exp B; Enclosed; MWFRS (en and C-C Exterior(2E) 2-5-4 to 5-5 and right exposed ; end vertical lt C for members and forces & MW shown; Lumber DOL=1.60 plate 9 2) TCLL: ASCE 7-16; Pr=20.0 psf (L DOL=1.15); Is=1.0; Rough Cat B Cs=1.00; Ct=1.10 3) Unbalanced snow loads have be design. 4) This truss has been designed for 	applied or 10-0-0 oc I=0-1-8 13) 10), 4=-37 (LC 14) 2 21), 4=280 (LC 21) pression/Maximum 51, 3-4=-201/154 (3-second gust) CDL=6.0psf; h=25ft; Ci velope) exterior zone 2-1-8, Interior (1) 2-1-8 5-4 zone; cantilever left and right exposed; (FRS for reactions grip DOL=1.60 roof LL: Lum DOL=1.1 um DOL=1.15 Plate ; Fully Exp.; Ce=0.9; en considered for this greater of min roof liv	on the bottor 3-06-00 tall b chord and ar 7) Bearing at jo using ANSI/1 designer sho 8) Provide mec bearing platte 9) H10A Simps connect truss This connect lateral forces 10) One H2.5A S recommende UPLIFT at jt(does not con 11) This truss is International R802.10.2 ar at. LOAD CASE(S) 5 5	Simpson Strong-Tie con ad to connect truss to be s) 4. This connection is isider lateral forces. designed in accordance Residential Code section nd referenced standard	re a rectangle etween the bott etween the bott et to grain value ula. Building aring surface. thers) of truss s recommende UPLIFT at jt(s) does not conside the cons aring walls due for uplift only ar with the 2018 ns R502.11.1 a	d to 2. der to nd			i	SEA 0363	L
load of 12.0 psf or 1.00 times flat overhangs non-concurrent with o5) This truss has been designed for chord live load nonconcurrent wit	ther live loads.									DE N

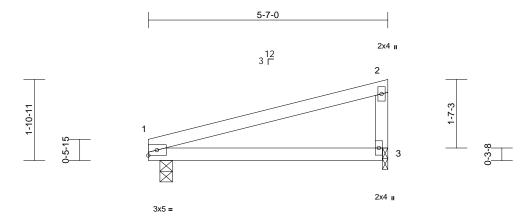
October 17,2024

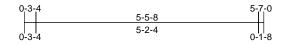
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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	C02	Monopitch	5	1	Job Reference (optional)	168925793

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:43 ID:NRvWGv3OCKKEiPwpSKZ4mYzuknr-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





Scale = 1:26.9

Loading (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0* BCDL 10.0	Plate Grip DOL1Lumber DOL1Rep Stress IncrY	I-11-4 I.15 I.15 (ES RC2018/TPI2014	CSI TC BC WB Matrix-MP	0.62 0.44 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.06 -0.11 0.02	(loc) 3-6 3-6 1	l/defl >999 >587 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 19 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 BRACING TOP CHORD Structural wood she 5-7-0 oc purlins, ex	applied or 10-0-0 oc 3=0-1-8 13) 2 (10), 3=-37 (LC 14) C 21), 3=277 (LC 21) pression/Maximum 8/154 (3-second gust) CDL=6.0psf; h=25ft; Cat ivelope) exterior zone ilever left and right ght exposed;C-C for for reactions shown; u=1.60 roof LL: Lum DOL=1.15 um DOL=1.15 Plate 8; Fully Exp.; Ce=0.9; sen considered for this r a 10.0 psf bottom th any other live loads. or a live load of 20.0psf where a rectangle	using ÅNSİ/T designer sho 7) Provide med bearing plate string som the som the source of the bearing plate som the source of the	on Strong-Tie conr s to bearing walls c ion is for uplift only simpson Strong-Tie d to connect truss s) 3. This connect sider lateral forces designed in accord Residential Code and referenced stan	n formula of beari (by oth nectors i due to U and do e connec to beari on is for ance w sections	a. Building ng surface. ers) of truss f recommende PLIFT at jt(s) es not consid ctors ing walls due uplift only ar ith the 2018 s R502.11.1 a	to d to d 1. der to nd		4		SEA 0363	ROLU 22

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

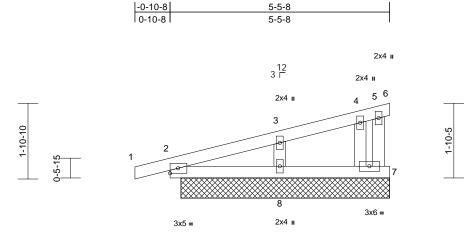
A MITek Affilia 818 Soundside Road Edenton, NC 27932

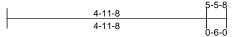
October 17,2024

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	C03	Monopitch	2	1	Job Reference (optional)	168925794

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:43 ID:YZUKun89W7GKeanXr32B9hzuo4B-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f







Scale = 1:28.7

Scale = 1.20.7													
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 YES IRC2018	3/TPI2014	CSI TC BC WB Matrix-MP	0.09 0.06 0.06	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 6	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 22 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD REACTIONS FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Wind: AS Vasd=10: II; Exp 8; and C-C (to 5-5-8 z	 2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3 Structural wood she 5-5-8 oc purling, ex Rigid ceiling directly bracing. (size) 2=5-2-4, 9=5-2-4 Max Horiz 2=58 (LC Max Uplift 2=-46 (LC (LC 10), 8 10) Max Grav 2=210 (Ld (LC 21), 8 21) (lb) - Maximum Con Tension 1-2=0/17, 2-3=-69/4 4-5=-22/31, 5-6=-5/3 	cept end verticals. ² applied or 10-0-0 or 6=5-2-4, 7=5-2-4, 8= 13), 9=58 (LC 13) C 10), 6=-3 (LC 11), 3=-35 (LC 14), 9=-46 C 21), 6=6 (LC 7), 7= 3=278 (LC 21), 9=21 hpression/Maximum 0, 3-4=-59/38, 3, 5-7=-14/4 /49 108/116 n (3-second gust) CDL=6.0psf; h=25fr; hvelope) exterior zor 2-1-8, Exterior(2N) 2- I right exposed ; end	4) ed or 5) c 6) 5-2-4, 7) 5 (LC 9) 0 (LC 10 11 10 11 LC Cat. ne -1-8	Plate DOL= ² DOL=1.15); Cs=1.00; Ct: Unbalanced design. This truss ha load of 12.0 overhangs n Gable studs This truss ha chord live loa * This truss ha chord live loa * This truss ha chord live loa * This truss ha chord and an Provide mee bearing plate 24 lb uplift a joint 8 and 4) Non Standari 1 This truss is International	snow loads have as been designed psf or 1.00 times on-concurrent wii spaced at 2-0-0 d as been designed in chord in all are by 2-00-00 wide w ny other members hanical connectid c capable of withs t joint 7, 46 lb upl 6 lb uplift at joint rd bearing conditi designed in accor Residential Code nd referenced sta	f (Lum DC at B; Fully been cor flat roof l ffat roof l th other liv oc. for a 10.0 t with any ed for a liv as where will fit betv s. on (by oth standing 3 ift at joint 2. on. Revie erdance w e sections	DL=1.15 Plate Exp.; Ce=0.9 Insidered for the er of min roof bad of 20.0 p: re loads. D psf bottom other live loa e load of 20.1 a rectangle veen the bottw ers) of truss t i b uplift at jo 2, 35 lb uplift ew required. ith the 2018 ; R502.11.1 a	e); live sf on ds. Dpsf om o int 6, at				NITH CA	
DOL=1.6 2) Truss des only. For see Stand	MWFRS for reactions s 0 plate grip DOL=1.60 signed for wind loads ir r studs exposed to winc dard Industry Gable En t qualified building desi	h the plane of the true I (normal to the face) d Details as applical), ole,								in the second se	SEA 0363	EER. KINN

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCEL Building Component Science Use Component Categories (http://www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

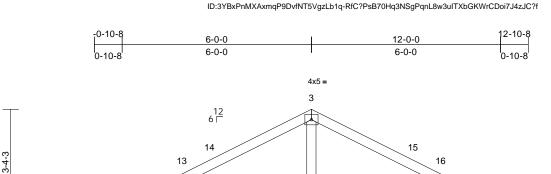


October 17,2024

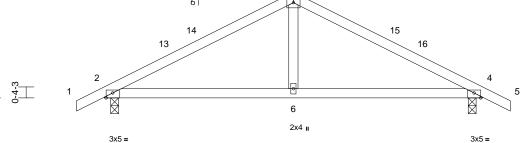
A. GI Children and Child

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	D01	Common	2	1	Job Reference (optional)	168925795

3-9-3



Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:43





Scale = 1:36.5 Plate Offsets (X, Y): [2:0-2-8,Edge], [4:0-2-8,Edge]

Plate Offsets ()	K, Y): [2:0-2-8,Edge],	[4:0-2-8,Edge]										-	
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MSH	0.71 0.68 0.11	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.08 -0.12 0.01	(loc) 6-9 6-9 4	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 45 lb	GRIP 244/190 FT = 20%
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalance this design	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 Structural wood she: 4-8-2 oc purlins. Rigid ceiling directly bracing. (size) 2=0-3-0, 4 Max Horiz 2=54 (LC Max Uplift 2=-78 (LC Max Grav 2=632 (LC (lb) - Maximum Com Tension 1-2=0/23, 2-3=-757/- 4-5=0/23 2-6=-286/575, 4-6=- 3-6=-148/283 d roof live loads have E 7-16; Vult=130mph	applied or 10-0-0 oc 4=0-3-0 14) 2 11), 4=-78 (LC 10) C 21), 4=632 (LC 22) apression/Maximum 453, 3-4=-757/453, 286/575 been considered for	; 7) 8) 9) LC	design. This truss ha load of 12.0 overhangs n This truss ha chord live loa * This truss ha on the bottor 3-06-00 tall t chord and ar One H2.5A S recommende UPLIFT at jtt and does no This truss is International	snow loads have I as been designed 1 psf or 1.00 times f on-concurrent with as been designed n chord in all area by 2-00-00 wide wi y other members. Simpson Strong-Ti ed to connect truss (s) 2 and 4. This ca t consider lateral f designed in accor Residential Code nd referenced star Standard	for great lat roof lo o other li for a 10. with any d for a liv s where e conne s to bear connectio orces. dance w sections	er of min roo bad of 20.0 p ve loads. 0 psf bottom other live loa re load of 20. a rectangle veen the bott ctors ing walls due n is for uplift ith the 2018 s R502.11.1 a	f live ads. Opsf om e to only				WITH CA	NRC
Vasd=103r	mph; TCDL=6.0psf; B(Enclosed; MWFRS (en	CDL=6.0psf; h=25ft;								1	J.	OR	China .

- II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 3-0-0, Exterior(2R) 3-0-0 to 9-0-0, Interior (1) 9-0-0 to 9-10-8, Exterior(2E) 9-10-8 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

SEAL 036322

October 17,2024

Page: 1



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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	D02	Hip Girder	1	1	Job Reference (optional)	168925796

NAILED

Carter Components (Sanford, NC), Sanford, NC - 27332,

2-4-3

(psf)

20.0

20.0

10.0

0.0

10.0

2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

bracing

Tension

DOL=1.60 plate grip DOL=1.60

5-7=-318/1275

4-6-1 oc purlins, except

Max Horiz 2=39 (LC 12)

4-5=-1471/392. 5-6=0/35

Wind: ASCE 7-16; Vult=130mph (3-second gust)

right exposed; porch left and right exposed; Lumber

2-8=-334/1272, 7-8=-330/1257,

2-0-0 oc purlins (4-4-14 max.): 3-4.

2=0-3-0. 5=0-3-0

Max Uplift 2=-223 (LC 9), 5=-223 (LC 8)

(lb) - Maximum Compression/Maximum

3-8=-56/250, 3-7=-47/54, 4-7=-57/251

Spacing

Code

Plate Grip DOL

Rep Stress Incr

Lumber DOL

2-9-3

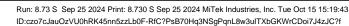
0-10-8

0-10-8

4-0-0

4-0-0

12 6 Г



NAILED

8-0-0 4-0-0

NAILED

Page: 1

VIIIIIIIIIIII

2-10-8

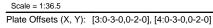
0-10-8

12-0-0

4-0-0

NAILED 5x6 = 5x6 = NAILED 3 16 4 \square \square Ш 15 17 T) [. h 6 ΠΓ Спг ΠΓ Π 18 8 19 7 20 2x4 u 3x5 = 3x5 = 3x5 : NAILED NAILED NAILED NAILED NAILED 12-0-0 3 - 10 - 48-1-12 11-9-8 0-2-8 0-2-8 3-7-12 4-3-8 3-7-12 2-0-0 CSI DEFL in l/defl L/d PLATES GRIP (loc) 1.15 TC 0.57 Vert(LL) -0.03 8-11 >999 240 MT20 244/190 1.15 BC 0.59 Vert(CT) -0.06 7-8 >999 180 WB NO 0.10 Horz(CT) 0.02 5 n/a n/a IRC2018/TPI2014 Matrix-MSH Weight: 52 lb FT = 20% 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 6) Provide adequate drainage to prevent water ponding. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. Structural wood sheathing directly applied or * This truss has been designed for a live load of 20.0psf 8) on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom Rigid ceiling directly applied or 10-0-0 oc chord and any other members. 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 5. This connection is for uplift only and does not consider lateral forces. Max Grav 2=930 (LC 37), 5=932 (LC 37) 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1. 1-2=0/35, 2-3=-1467/391, 3-4=-1261/363, 11) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord. 12) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines. 13) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B). Unbalanced roof live loads have been considered for LOAD CASE(S) Standard O Dead + Snow (balanced): Lumber Increase=1.15, Plate 1) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. Increase=1.15 II; Exp B; Enclosed; MWFRS (envelope) exterior zone; Uniform Loads (lb/ft) cantilever left and right exposed : end vertical left and Vert: 1-3=-60, 3-4=-60, 4-6=-60, 9-12=-20 SEAL Concentrated Loads (lb) 036322 Vert: 4=-95 (B), 8=-42 (B), 7=-42 (B), 3=-95 (B), 15=-26 (B), 16=-95 (B), 17=-26 (B), 18=-95 (B), 19=-42 (B), 20=-95 (B)

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00: Ct=1.10 Unbalanced snow loads have been considered for this G design. mmm October 17,2024 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) 818 Soundside Road and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com) Edenton, NC 27932



Loading

TCLL (roof)

Snow (Pf)

LUMBER

TOP CHORD BOT CHORD

TCDL

BCLL

BCDL

WEBS

BRACING

TOP CHORD

BOT CHORD

FORCES

TOP CHORD

BOT CHORD

this design.

WEBS

NOTES

1)

2)

3)

4)

REACTIONS (size)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	100005707
25040191	E01	Jack-Open	3	1	I68 Job Reference (optional)	8925797

-0-10-8

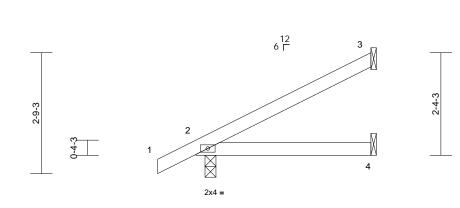
0-10-8

Carter Components (Sanford, NC), Sanford, NC - 27332,

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•





4-0-0

4-0-0

Scale = 1:26.3

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-MP	0.31 0.24 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.02 -0.03 0.00	(loc) 4-7 4-7 2	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD BOT CHORD REACTIONS	4-0-0 oc purlins. Rigid ceiling directly bracing. (size) 2=0-3-0, Max Horiz 2=86 (LC Max Uplift 2=-26 (LC Max Grav 2=319 (LC 4=-72 (LC	: 14) C 14), 3=-50 (LC 14), C 11) C 21), 3=155 (LC 21	chord live I 6) * This truss on the bott ad or 3-06-00 tal chord and 7) Bearings a 8) Refer to gi 9) Provide me bearing pla 3, 15 lb up 10) This truss i Internation 8802 10 2	has been designed oad nonconcurrent is has been designed om chord in all area I by 2-00-00 wide w any other members re assumed to be: reder(s) for truss to idechanical connectio the capable of withs lift at joint 4 and 26 is designed in accola al Residential Code and referenced sta S) Standard	with any d for a liv as where fill fit betw Joint 2 U truss con n (by oth tanding 5 Ib uplift a rdance w e sections	other live loa e load of 20. a rectangle ween the bott Jser Defined nections. ers) of truss i0 lb uplift at it joint 2. ith the 2018 i R502.11.1 a	0psf .om to joint					
TOP CHORD BOT CHORD	(IB) - Maximum Con Tension 1-2=0/37, 2-3=-67/5 2-4=-43/49											

NOTES

 Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

SEAL 036322 October 17,2024

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	E02	Jack-Open Girder	2	1	Job Reference (optional)	168925798

2-0-0

2-0-0

-0-10-8

0-10-8

1-9-3

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:44 ID:4uq4FrOs92Um5DDF8kU1SbzLb0V-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

5

4-0-0

2-0-0

Page: 1

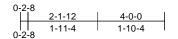
Special 6 Г 4x5 = 4 3 . 1-4-3 2 0-4-3 X 0

3x5 =



6

2x4 II



Scale = 1:32

Scale = 1:32												
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 NO IRC2018/TPI20		0.16 0.44 0.03	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.03 -0.05 0.02	(loc) 6 6 4	l/defl >999 >990 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 15 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD 2x4 SF BOT CHORD 2x4 SF BRACING TOP CHORD Structu 4-0-0 0 2-0-0 0 BOT CHORD Rigid 0 BOT CHORD Rigid 0 bracin REACTIONS (size) Max Ho Max Up Max Gra FORCES (lb) - M Tensic TOP CHORD 1-2=0/ BOT CHORD 2-6=-1 WEBS 3-6=-1 NOTES 1) Unbalanced roof liv this design. 2) Wind: ASCE 7-16; Vasd=103mph; TC II; Exp B; Enclosed cantilever left and right exposed; poro DOL=1.60 plate gr	 No.2 No.2 No.3 ural wood she oc purlins, exc oc purlins; 3-4 zeiling directly 2=0-3-0, - Mechanic mechanic 	4 Mechanical, 5= sal 12) C 12), 4=-24 (LC 8), 4 C 34), 4=86 (LC 33), C 30) npression/Maximum 0, 3-4=0/0 been considered for (3-second gust) CDL=6.0psf; h=25ft; nvelope) exterior zon ; end vertical left ani- ht exposed; Lumber	board of overh. 6) Provio 7) This ti chord 8) * This on the 3 - 3 - 6 - 0 chord 9) Bearin 10) Refer 11) Provio 6 - 3 - 6 - 0 chord 9) Bearin 10) Refer 11) Provio 6 - 3 - 6 - 0 chord 9) Bearin 12) This ti Intern R802. 13) Graph or the bottor 14) Hange provio down r down r cat. 15) In the let; of the 10) Deaa (15) In the let; 10) Refer 11) Provio 12) This ti 12) This ti 12) This ti 13) Graph or the bottor 14) Hange provio down r spo	uss has been designed f 12.0 psf or 1.00 times angs non-concurrent wi le adequate drainage to russ has been designed live load nonconcurren truss has been designed bottom chord in all are 0 tall by 2-00-00 wide v and any other member 10 tall by 2-00 to 10.2 and referenced st tical purlin representatio orientation of the purlin n chord. and 42 lb up at 2-0-0 to and 42 lb up at 2-0-0 to nyselection of such com- nisbility of others. LOAD CASE(S) section truss are noted as from SE(S) Standard d + Snow (balanced): Li	I flat roof I thother lip oprevent of thother lip oprevent of thota and the lip oprevent of the lip oprevent of the lip oprevent of the lip oprevent operation operati	bad of 20.0 p ve loads. water pondin 0 psf bottom other live loa re load of 20. a rectangle veen the bott Jser Defined nections. ers) of truss the 2018 s R502.11.1 a Jsl/TPI 1. ot depict the b top and/or shall be ated load(s) st rd, and 18 lb chord. The vice(s) is the pplied to the ck (B).	osf on ig. ads. Opsf tom to joint and size 55 lb face				ORTH CA	
	Pf=20.0 psf (L ; Rough Cat E	um DOL=1.15 Plate 3; Fully Exp.; Ce=0.9	Ve I; Cono Ve	orm Loads (lb/ft) ort: 1-3=-60, 3-4=-60, 5- centrated Loads (lb) ort: 3=-8 (B), 6=-18 (B)	-7=-20				9	A DIMENTICE	111111	EER. AL AL

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	E03	Jack-Open	2	1	Job Reference (optional)	168925799

-0-10-8

0-10-8

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:44 ID:mkJ840hTpEA_8Jn6IYSQ5MzLb1P-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1

$6\frac{12}{1}$



2-0-0

2-0-0

Scale = 1:24.2

					1		· · · · ·						
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MP	0.08 0.04 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4-7 4-7 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 8 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 2x4 SP No.2 Structural wood shea 2-0-0 oc purlins. Rigid ceiling directly bracing. (size) 2=0-3-0, 3 Mechanica Max Horiz 2=50 (LC Max Uplift 2=-25 (LC (LC 11) Max Grav 2=199 (LC (LC 7)	applied or 10-0-0 oc = Mechanical, 4= al 14) 14), 3=-21 (LC 14),	7) 8) 9) 4=-7 ¹⁽	 chord live loa * This truss I on the bottoo 3-06-00 tall I chord and at Bearings are Refer to gird Provide mec bearing plate 21 lb uplift at This truss is International 	is been designed ad nonconcurrent has been designe in chord in all area by 2-00-00 wide w hy other members er(s) for truss to hanical connectic e capable of withs t joint 3 and 25 b designed in acco Residential Code ind referenced sta Standard	with any d for a liv as where fill fit betw Joint 2 U truss con n (by oth tanding 7 uplift at ju rdance w s sections	other live load e load of 20.0 a rectangle veen the bottu Jser Defined nections. ers) of truss t 7 lb uplift at jo oint 2. ith the 2018 \$ R502.11.1 a	Opsf om o int 4,					
Vasd=103 II; Exp B;	(lb) - Maximum Comp Tension 1-2=0/34, 2-3=-29/23 2-4=-14/22 CE 7-16; Vult=130mph mph; TCDL=6.0psf; BC Enclosed; MWFRS (en xterior(2E) zone; cantil	(3-second gust) (DL=6.0psf; h=25ft; (velope) exterior zone										WH CA	

and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	J01	Jack-Open	2	1	Job Reference (optional)	168925800

-1-2-14 1-2-14

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:44 ID:quFFny4YH3CS_DEFdG1wO6zLb0v-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

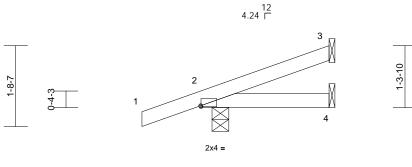
2-8-7

2-8-7



4 2x4 = 2-14 n 2-8-7 2-5-9 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





Scale = 1:24.3

Plate Offsets (X, Y): [2:0-0-1.0-0-3]

Plate Offsets ((X, Y): [2:0-0-1,0-0-3]													
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MP	0.15 0.06 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 4-7 4-7 3	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 10 lb	GRIP 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.2 Structural wood she 2-8-7 oc purlins. Rigid ceiling directly bracing. (size) 2=0-4-4, 3 Mechanic Max Horiz 2=56 (LC Max Uplift 2=-91 (LC (LC 11) Max Grav 2=273 (LC (LC 7) (lb) - Maximum Com	applied or 10-0-0 oc 3= Mechanical, 4= al 10) 2 10), 3=-26 (LC 10), C 21), 3=82 (LC 21),	6) d or 7) 8) 9) 4=-9	chord live loa * This truss h on the bottor 3-06-00 tall b chord and ar Bearings are Refer to gird Provide mec bearing plate 3, 9 lb uplift :) This truss is International	is been designed i ad nonconcurrent has been designed in chord in all area by 2-00-00 wide w by other members assumed to be: , er(s) for truss to t hanical connection e capable of withst at joint 4 and 91 lb designed in accor Residential Code ind referenced star Standard	with any d for a liv as where ill fit betw Joint 2 U russ con n (by oth tanding 2 o uplift at dance w sections	other live loa e load of 20.0 a rectangle ween the botto Jser Defined nections. ers) of truss t 6 lb uplift at j joint 2. ith the 2018 s R502.11.1 a	Opsf om o oint						
 Vasd=103 II; Exp B; and C-C C exposed; and right e MWFRS fr grip DOL= 2) TCLL: AS Plate DOL DOL=1.15 Cs=1.00; 3) Unbalance design. 4) This truss load of 12 	2-4=-33/35 CE 7-16; Vult=130mph 3mph; TCDL=6.0psf; Br Enclosed; MWFRS (er corner (3) zone; cantile end vertical left and rig exposed;C-C for memb for reactions shown; Lu =1.60 CE 7-16; Pr=20.0 psf (L =1.15); Pf=20.0 psf (L 5); Is=1.0; Rough Cat E	(3-second gust) CDL=6.0psf; h=25ft; ivelope) exterior zon wer left and right ght exposed; porch le bers and forces & imber DOL=1.60 plat roof LL: Lum DOL=1 um DOL=1.15 Plate 3; Fully Exp.; Ce=0.9 even considered for th r greater of min roof I t roof load of 20.0 ps	e eft .15 ; is									SEA 0363	L 22 EEER. ILBERTITI T 17,2024	Manunum

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	PB01	Piggyback	7	1	Job Reference (optional)	168925801

2-11-6

2-11-6

6 2x4 **"**

5-10-12

-0-8-12

0-8-12

2

ø

2x4 =

Carter Components (Sanford, NC), Sanford, NC - 27332,

2-5-11

2-4-1

7-4-7

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:44 ID:wbrRmEFlyLSjMQiJcAgh2?zDvy?-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



4x5 =

5-10-12

2-11-6

6-7-8

0-8-12

2x4 =

Scale = 1:26.4 Ploto Offecto (X, Y): [2:0.2.5.0.1.0] [4:0.2.

Plate Offsets	s (X, Y): [2:0-2-5,0-1-0],	[4:0-2-5,0-1-0]										-	
Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MP	0.16 0.17 0.02	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 2	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 24 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORE BOT CHORE OTHERS BRACING TOP CHORE BOT CHORE REACTIONS	D 2x4 SP No.2 2x4 SP No.3 D Structural wood she 6-0-0 oc purlins. D Rigid ceiling directly bracing. S (size) 2=5-10-12 7=5-10-12 Max Horiz 2=54 (LC Max Uplift 2=-31 (LC 7=-31 (LC Max Grav 2=232 (LC	2, 4=5-10-12, 6=5-10 2, 11=5-10-12 13), 7=54 (LC 13) 2 14), 4=-38 (LC 15), 2 14), 11=-38 (LC 15) 2 21), 4=232 (LC 22) 2 22), 7=232 (LC 21)	4) d or ; 5) I-12, 6) I-12, 6) I, 9)	only. For sti see Standar or consult qu TCLL: ASCE Plate DOL= DOL=1.15); Cs=1.00; Ct: Unbalanced design. This truss ha load of 12.0 overhangs n Gable requir Gable studs This truss ha chord live loo	ted for wind load ids exposed to w d Industry Gable talified building d 7-16; Pr=20.0 ps s=1.0; Rough C: 1.10 snow loads have psf or 1.00 times on-concurrent wi es continuous bo spaced at 4-0-0 is been designed ad nonconcurrent	vind (norm End Detai esigner as sf (roof LL f (Lum DC at B; Fully e been cor d for greate flat roof lc th other liv toton chor oc. f for a 10.0 t with any	al to the face ils as applical s per ANSI/TF :Lum DOL=: L=1.15 Plate Exp.; Ce=0.9 asidered for the er of min roof pad of 20.0 ps re loads. d bearing.), ble, 1 1. 1.15 ; his live sf on ds.					
FORCES TOP CHORE BOT CHORE WEBS	4-5=0/25	78, 3-4=-123/78,	11	on the bottor 3-06-00 tall I chord and ar) N/A ?) This truss is	n chord in all are by 2-00-00 wide by other member designed in acco	as where will fit betw s. ordance wi	a rectangle veen the botto ith the 2018	om				OR EESS	ROUN
NOTES 1) Unbaland this design 2) Wind: AS Vasd=10 II; Exp B and C-C to 4-1-11 left and r exposed	inced roof live loads have ign. SCE 7-16; Vult=130mph O3mph; TCDL=6.0psf; B ; Enclosed; MWFRS (er Exterior(2E) 0-3-5 to 3- 1, Exterior(2E) 4-1-11 to right exposed ; end verti t;C-C for members and f s shown; Lumber DOL=	(3-second gust) CDL=6.0psf; h=25ft; hvelope) exterior zon 3-5, Exterior(2R) 3-3 7-1-11 zone; cantile cal left and right orces & MWFRS for	Cat. LC e -5	R802.10.2 a S) See Standar Detail for Co	Residential Cod nd referenced st d Industry Piggy) nnection to base fied building des Standard	andard AN back Truss truss as a	ISI/TPI 1. s Connection			7		SEA 0363	L 22 EER R

October 17,2024

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	PB02	Piggyback	1	1	I6892580 Job Reference (optional))2

1-5-2

1-5-2

12 8 Г

3x5 =

-0-8-12

0-8-12

Carter Components (Sanford, NC), Sanford, NC - 27332,

Loading

TCLL (roof)

Snow (Pf)

LUMBER

OTHERS

BRACING

FORCES

WEBS

NOTES

1)

2)

TCDL

BCLL

BCDL

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:44 ID:wbrRmEFlyLSjMQiJcAgh2?zDvy?-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

5-10-12

1-5-2

3x5 =

0-8-12

818 Soundside Road

Edenton, NC 27932

4-5-10

3-0-8

2x4 II

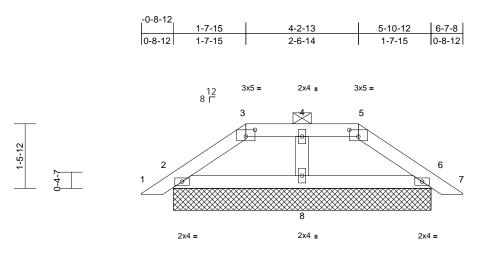


* 3 5 0 1-3-14 1-5-8 6 8 2x4 =2x4 u 2x4 =5-10-12 Scale = 1:28.8 Plate Offsets (X, Y): [3:0-2-8,0-1-13], [5:0-2-8,0-1-13] Spacing 2-0-0 CSI DEFL l/defl L/d PLATES GRIP (psf) in (loc) 20.0 Plate Grip DOL 1.15 тс 0.07 Vert(LL) 999 MT20 244/190 n/a n/a 20.0 Lumber DOL 1.15 BC 0.08 Vert(CT) n/a n/a 999 10.0 Rep Stress Incr WB 0.04 Horz(CT) 6 YES 0.00 n/a n/a 0.0 Code IRC2018/TPI2014 Matrix-MP 10.0 Weight: 22 lb FT = 20% 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), 2x4 SP No 2 TOP CHORD see Standard Industry Gable End Details as applicable, BOT CHORD 2x4 SP No 2 or consult qualified building designer as per ANSI/TPI 1. 2x4 SP No.3 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 4) Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate TOP CHORD Structural wood sheathing directly applied or DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; 6-0-0 oc purlins, except Cs=1 00. Ct=1 10 2-0-0 oc purlins (6-0-0 max.): 3-5. 5) Unbalanced snow loads have been considered for this BOT CHORD Rigid ceiling directly applied or 10-0-0 oc design. bracing. 6) This truss has been designed for greater of min roof live **REACTIONS** (size) 2=5-10-12, 6=5-10-12, 8=5-10-12, load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on 9=5-10-12, 13=5-10-12 overhangs non-concurrent with other live loads. Max Horiz 2=-31 (LC 12), 9=-31 (LC 12) Provide adequate drainage to prevent water ponding. Max Uplift 2=-33 (LC 14), 6=-34 (LC 15), Gable requires continuous bottom chord bearing. 8) 8=-13 (LC 11), 9=-33 (LC 14), 9) Gable studs spaced at 4-0-0 oc. 13=-34 (LC 15) 10) This truss has been designed for a 10.0 psf bottom Max Grav 2=212 (LC 41), 6=212 (LC 41), chord live load nonconcurrent with any other live loads. 8=252 (LC 40), 9=212 (LC 41), 11) * This truss has been designed for a live load of 20.0psf 13=212 (LC 41) on the bottom chord in all areas where a rectangle (lb) - Maximum Compression/Maximum 3-06-00 tall by 2-00-00 wide will fit between the bottom Tension chord and any other members. 1-2=0/25, 2-3=-156/66, 3-4=-121/67, TOP CHORD 12) N/A ORTH 4-5=-121/67, 5-6=-156/66, 6-7=0/25 13) This truss is designed in accordance with the 2018 BOT CHORD 2-8=-11/108, 6-8=-11/108 International Residential Code sections R502.11.1 and 4-8=-185/76 R802 10 2 and referenced standard ANSI/TPL1 14) See Standard Industry Piggyback Truss Connection VIIIIIII Unbalanced roof live loads have been considered for Detail for Connection to base truss as applicable, or this design. SEAL consult qualified building designer. Wind: ASCE 7-16; Vult=130mph (3-second gust) 15) Graphical purlin representation does not depict the size 036322 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. or the orientation of the purlin along the top and/or II; Exp B; Enclosed; MWFRS (envelope) exterior zone bottom chord. and C-C Exterior(2E) 0-3-5 to 2-2-4, Exterior(2R) 2-2-4 LOAD CASE(S) Standard to 5-2-12, Exterior(2E) 5-2-12 to 7-1-11 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip GI DOL=1.60 minim October 17,2024 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	PB03	Piggyback	1	1	Job Reference (optional)	168925803

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:44 ID:eN_rsXBYJSMw3bcYjbqbmmzuWVa-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



5-10-12

Scale = 1:26.4 Plate Offsets (X, Y): [3:0-2-8,0-1-13], [5:0-2-8,0-1-13]

1-7-6

Loading (psf) ICLL (roof) 20.0 Snow (Pf) 20.0 ICDL 10.0 3CLL 0.0* 3CDL 10.0	Spacing2-0Plate Grip DOL1.1Lumber DOL1.1Rep Stress IncrYECodeIR0	5 5	CSI TC BC WB Matrix-MP	0.06 0.11 0.03	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 6	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190 ET - 20%
UMBER OP CHORD 2x4 SP No.2 OT CHORD 2x4 SP No.2 OT CHORD 2x4 SP No.3 BRACING OP CHORD Structural wood she 6-0-0 oc purlins, exc 2-0-0 oc purlins, exc 2-	-0 max.): 3-5. applied or 10-0-0 oc 2, 6=5-10-12, 8=5-10-12, 2, 13=5-10-12 13), 9=34 (LC 13) 14), 6=-29 (LC 15), 8=-5 1=-28 (LC 14), 13=-29 (LC 2 41), 6=-224 (LC 41), 2 40), 9=224 (LC 41), 10 40), 9=224 (LC 41), 10 40), 9=224 (LC 41), 10 40), 9=224 (LC 41), 10 5 10 5 10 5 10 5 10 5 10 5 10 5 10 5	 only. For s see Standa or consult of the see Standa or consult of the DOL=1.15) or the Second Seco	I snow loads have to as been designed for post or 1.00 times for on-concurrent with equate drainage to p res continuous bott as been designed for as been designed for ad nonconcurrent to has been designed for ad nonconcurrent to has been designed in accord by 2-00-00 wide with and referenced star rd Industry Piggyba onnection to base to lified building desig urlin representation tation of the purlin a rd.	Id (norm nd Deta signer as i (roof LL Lum DC B; Fully been cor or greate at roof k other lin brorevent N or a 10.0 with any for a liv s where Il fit betw dance w sections idard AN ack Trus: russ as a ner.	al to the face) Is as applicat as per ANSI/TF L=1.15 Plate Exp.; Ce=0.9 asidered for the er of min roof bad of 20.0 ps re loads. water ponding d bearing. D psf bottom other live load e load of 20.02 a rectangle veen the bottod ith the 2018 R502.11.1 a ISI/TPI 1. s Connection applicable, or bt depict the s), ble, Pl 1. 1.15 j; his live sf on g. ds. opsf om		C 111111		Weight: 23 lb ORTH CA ORTEESS SEA 0363	EER. KUU

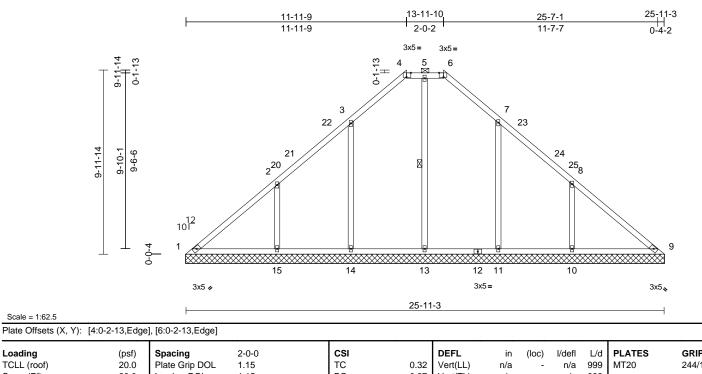
WARNING - Verity design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE MIT-4/3 fev. 1/2/2023 BEFORE USE. Design valid for use only with MiTeR's connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V01	Valley	1	1	Job Reference (optional)	168925804

Scale = 1:62.5

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:44 ID:J6zMZWEAeaGCcpO3JIB3UNzuo43-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-MSH	0.32 0.27 0.37	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.01	(loc) - - 9	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 132 lb	GRIP 244/190 P FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD WEBS REACTIONS	2x4 SP No.3 Structural wood she 6-0-0 oc purlins, exc 2-0-0 oc purlins, exc 2-0-0 oc purlins (6-0 Rigid ceiling directly bracing. 1 Row at midpt (size) 1=25-11- 10=25-11 15=25-11 Max Horiz 1=-228 (L Max Uplift 1=-29 (LC 11=-128 (15=-182 (L Max Grav 1=202 (LC 10=600 (L	 -0 max.): 4-6. applied or 6-0-0 oc 5-13 13, 9=25-11-13, -13, 11=25-11-13, -13, 14=25-11-13, -13 C 10) (10), 10=-181 (LC 15 LC 15), 14=-133 (LC LC 14) C 54), 9=181 (LC 56), LC 52), 11=545 (LC 56) LC 27), 14=550 (LC 55) 	Vasd=10 II; Exp E and C-C 8-11-10 16-11-1 zone; ca and righ MWFRS grip DO 3) Truss du or consu 4) TCLL: A Plate DC DOL=1. 1, Cs=1.00 14), 5) Unbalar design. 2), 7) All plate 0, 8) Gable re	SCE 7-16; Vult=130m 3mph; TCDL=6.0psf ; Enclosed; MWFRS Exterior(2E) 0-0-0 to Exterior(2R) 8-11-10 to 22-11-13, Exterior ntilever left and right exposed;C-C for me for reactions shown; =1.60 signed for wind loads r studs exposed to w dard Industry Gable It qualified building d SCE 7-16; Pr=20.0 psi 5); Is=1.0; Rough Ca ; Ct=1.10 ced snow loads have adequate drainage to s are 2x4 MT20 unles quires continuous bo uds spaced at 4-0-0	BCDL=((envelope 3-0-0, In) to 16-11 rr(2E) 22- exposed mbers ar Lumber I s in the pl ind (norm End Deta esigner a sf (roof Ll f (Lum DC at B; Fully been con p prevent s otherwittom choi	6.0psf; h=25ft a) exterior zo terior (1) 3-0- 10, Interior (11-13 to 25-1 ; end vertical d forces & DOL=1.60 pla ane of the tru al to the face ils as applica s per ANSI/T :: Lum DOL= DL=1.15 Plate Exp.; Ce=0.1 msidered for t water pondin se indicated.	;; Cat. ne 0 to 1) 1 - 13 left ate ss s), bble, PI 1. 1.15 s 9; 9; his g.	Inte R80 15) Gra or ti	rnationa)2.10.2 a phical p he orien tom choi	Il Resid and ref urlin re tation o rd.	dential Code sed erenced standar presentation do of the purlin alon	to e with the 2018 ctions R502.11.1 and rd ANSI/TPI 1. ses not depict the size ing the top and/or
FORCES	4-5=-64/142, 5-6=-6	149/183, 3-4=-141/15 4/142, 6-7=-141/146,	, chord liv 5, 11) * This tr	s has been designed e load nonconcurrent iss has been designe ottom chord in all are	t with any ed for a liv	other live loa e load of 20.			4	à	ON	The
BOT CHORD	13-14=-131/218, 11 10-11=-131/218, 9-1	5=-131/218, -13=-131/218, 0=-131/218	chord aı 12) Provide bearing	tall by 2-00-00 wide wide any other members mechanical connection plate capable of withs	s, with BC on (by oth standing 2	CDL = 10.0ps ers) of truss 29 lb uplift at j	f. to joint				SEA 0363	• –
NOTES	7-11=-383/179, 8-10		uplift at 13) Beveled	uplift at joint 14, 182 oint 11 and 181 lb up plate or shim require vith truss chord at joi	lift at join d to provi	t 10. de full bearin				in the second se	SEA 0363	EER HALIN

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October 17,2024

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V02	Valley	1	1	Job Reference (optional)	168925805

TCDL

BCLL

BCDL

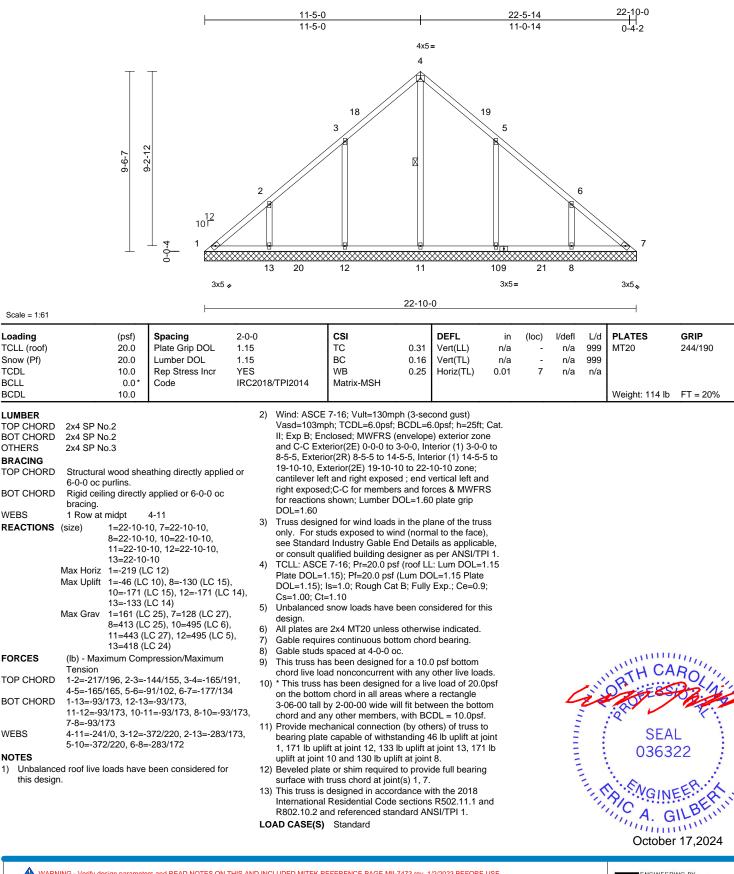
Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:44 ID:qvPzMAEYtH8L_fptl1gqy9zuo44-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

VIIIIIIIIIIII

818 Soundside Road

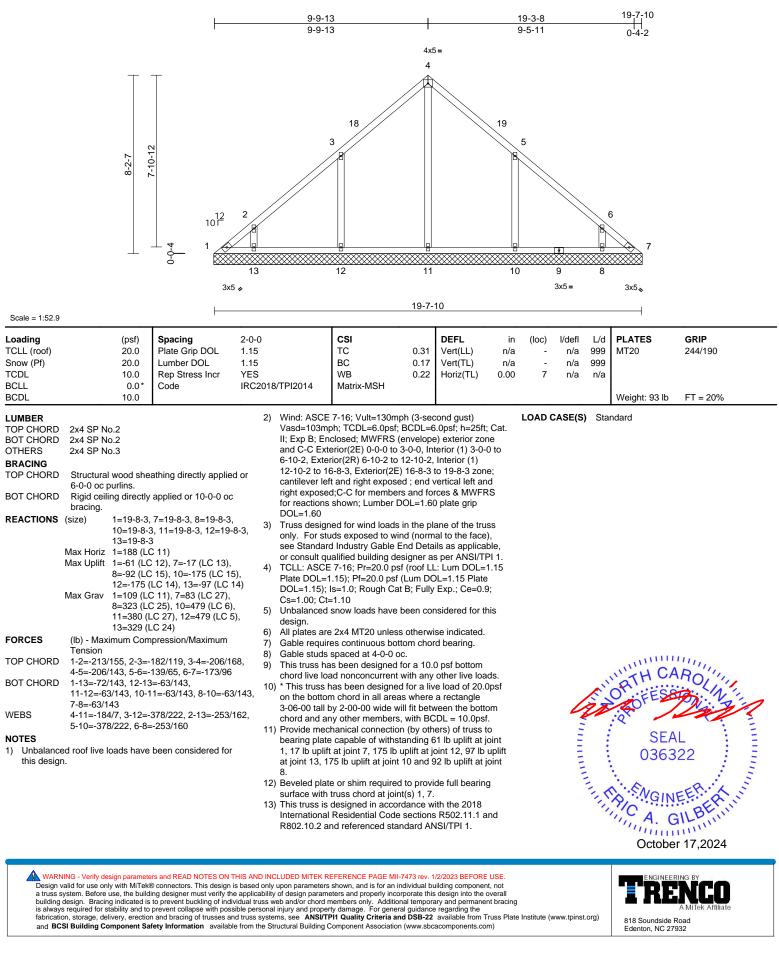
Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V03	Valley	1	1	Job Reference (optional)	168925806

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:45 ID:Mjrb9rDw6z0UMVEgBK9bPyzuo45-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V04	Valley	1	1	Job Reference (optional)	168925807

Scale = 1:49.8 Loading

TCLL (roof)

Snow (Pf)

LUMBER

OTHERS

BRACING

TOP CHORD

BOT CHORD

FORCES

WEBS

NOTES

TOP CHORD

BOT CHORD

TOP CHORD BOT CHORD

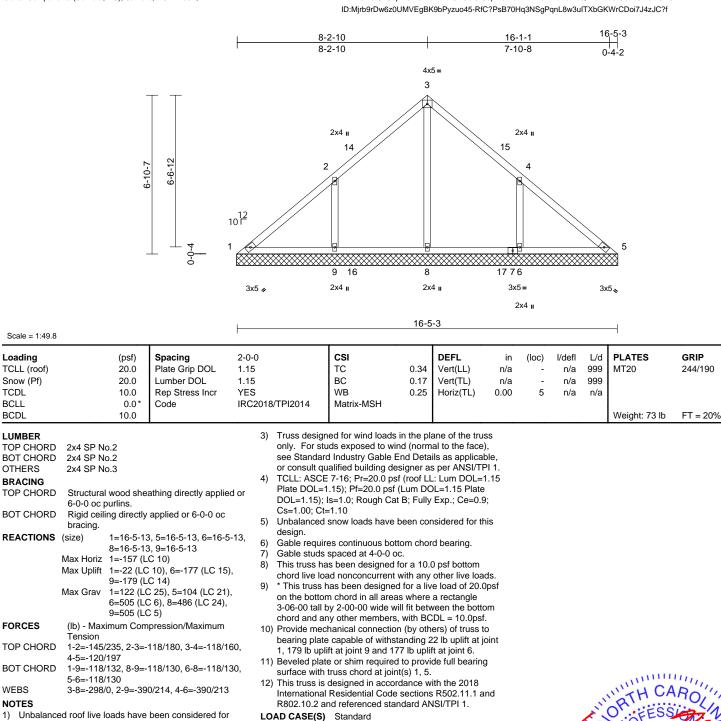
TCDL

BCLL

BCDL

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:45

Page: 1



- 1) this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) 2)
- Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 5-2-14, Exterior(2R) 5-2-14 to 11-2-14, Interior (1) 11-2-14 to 13-5-13, Exterior(2E) 13-5-13 to 16-5-13 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - WARNING Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V05	Valley	1	1	Job Reference (optional)	168925808

Scale = 1:41.3 Loading

TCLL (roof)

Snow (Pf)

LUMBER

OTHERS

BRACING

TOP CHORD

BOT CHORD

TOP CHORD

BOT CHORD

FORCES

WEBS

NOTES

1)

2)

TOP CHORD

BOT CHORD

this design.

REACTIONS (size)

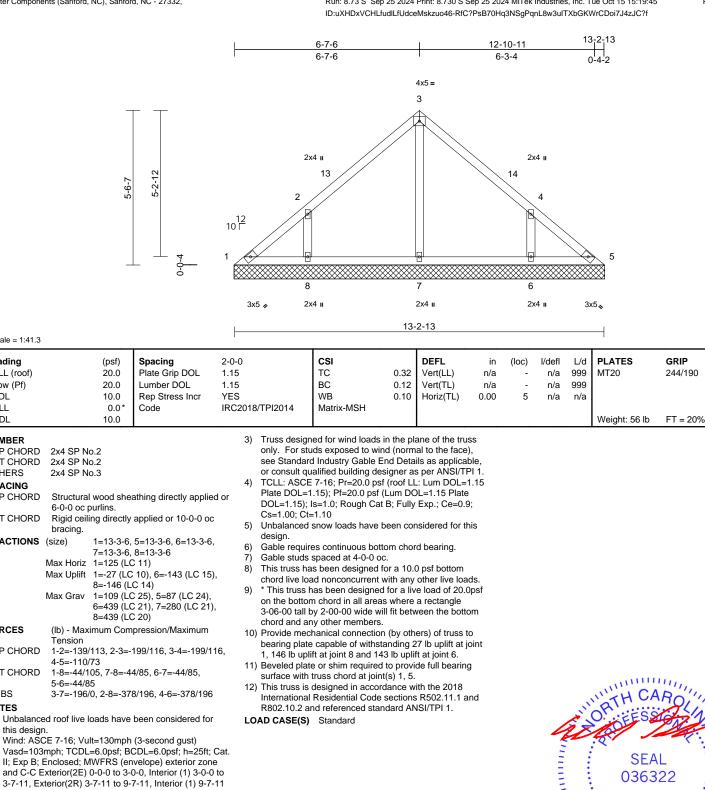
TCDL

BCLL

BCDL

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:45

Page: 1



3-7-11, Exterior(2R) 3-7-11 to 9-7-11, Interior (1) 9-7-11 to 10-3-6, Exterior(2E) 10-3-6 to 13-3-6 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

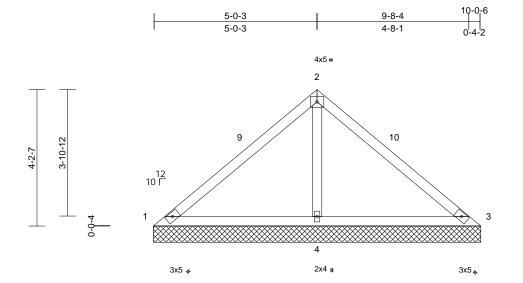


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V06	Valley	1	1	Job Reference (optional)	168925809

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:45 ID:QKjrk9BfaMmm7C4I4v77KXzuo47-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



10-0-6

Scale = 1:35.5

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MSH	0.50 0.46 0.21	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.01	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 38 lb	GRIP 244/190 FT = 20%
	10-0-0 oc purlins. Rigid ceiling directly bracing.	, 3=10-1-0, 4=10-1-0 11) C 21), 3=-66 (LC 20), C 14)	5) d or 6) 7) 8) 9)	Plate DOL=1 DOL=1.15); Cs=1.00; Ct Unbalanced design. Gable require Gable studs This truss ha chord live loa * This truss h on the bottor 3-06-00 tall b chord and ar 0) Provide mec	snow loads have es continuous bot spaced at 4-0-0 o s been designed ad nonconcurrent nas been designed n chord in all area by 2-00-00 wide w y other members hanical connectio	(Lum DC t B; Fully been cor tom chor c. for a 10.0 with any d for a liv s where ill fit betw n (by oth	DL=1.15 Plate Exp.; Ce=0.9 asidered for the d bearing. D psf bottom other live load e load of 20.0 a rectangle veen the bottwers) of truss to	e); ds. Dpsf om o					
FORCES TOP CHORD BOT CHORD WEBS NOTES 1) Unbalance	(lb) - Maximum Com Tension 1-2=-130/417, 2-3=- 1-4=-275/186, 3-4=- 2-4=-700/293	130/417 275/186	12	 66 lb uplift Beveled plate surface with This truss is International 	e capable of withs: at joint 3 and 120 e or shim required truss chord at join designed in accor Residential Code nd referenced star Standard) Ib uplift I to provi it(s) 1, 3. dance w sections	at joint 4. de full bearing ith the 2018 R502.11.1 a	g				ammu	10

this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Exterior(2R) 3-0-0 to 7-1-0, Exterior(2E) 7-1-0 to 10-1-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and PCB Building Component Science Michael Component Advancing Component Advancing Component Advancing and PCB and Component Advancing Component Compone and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

THILL WARNING With Hilling SEAL 036322 GI 11111111 October 17,2024



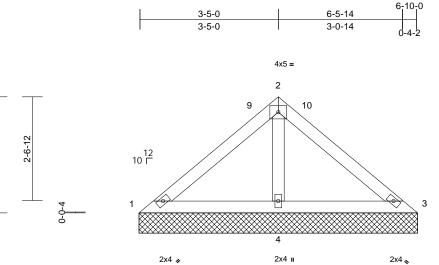
Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V07	Valley	1	1	Job Reference (optional)	168925810

2-10-7

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:45 ID:y89TWpA1p2evV2W6WCbunJzuo48-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Page: 1



6-10-0



Scale = 1:28.5

00010 - 112010													
Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15		CSI TC	0.22	DEFL Vert(LL)	in n/a	(loc)	l/defl n/a	L/d 999	PLATES MT20	GRIP 244/190
Snow (Pf)	20.0	Lumber DOL	1.15		BC	0.22	Vert(TL)	n/a	_	n/a	999	101120	244/130
TCDL	10.0	Rep Stress Incr	YES		WB	0.24	· · ·	0.00	4	n/a	n/a		
BCLL	0.0*	Code		8/TPI2014	Matrix-MP	0.00	110112(12)	0.00	-	n/a	n/a		
BCDL	10.0		110201	0,1112011								Weight: 25 lb	FT = 20%
BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS (S M M M	6-10-0 oc purlins. Rigid ceiling directly bracing. size) 1=6-10-11 Aax Horiz 1=-63 (LC Aax Uplift 1=-13 (LC 4=-67 (LC Aax Grav 1=101 (LC 4=498 (LC	C 21), 3=-13 (LC 20), C 14) C 20), 3=101 (LC 21) C 20)	6) 7) 8) D-10 9)	Plate DOL= DOL=1.15); Cs=1.00; Ct Unbalanced design. Gable requir Gable studs This truss ha chord live lo. * This truss I on the bottoo 3-06-00 tall I chord and at	E 7-16; Pr=20.0 p 1.15); Pf=20.0 ps Is=1.0; Rough Ca =1.10 snow loads have res continuous bc spaced at 4-0-0 as been designed m chord in all are by 2-00-00 wide to hanical connecting to capable of withstantic second second to the second second second second to the second second second second second to the second second second second second second to the second se	f (Lum DC at B; Fully be been con oc. d for a 10. t with any ed for a liv eas where will fit bety s. on (by oth	DL=1.15 Plate Exp.; Ce=0.9 nsidered for the d bearing. 0 psf bottom other live loa re load of 20.1 a rectangle veen the botther ers) of truss f	e 9; his dds. 0psf om to					
	(lb) - Maximum Com Tension	npression/Maximum	11	1, 13 lb uplif	t at joint 3 and 67 e or shim require	7 lb uplift a	at joint 4.						
	1-2=-85/213, 2-3=-8				truss chord at joi			э					
	1-4=-168/145, 3-4=-	-168/145	12	?) This truss is	designed in acco	ordance w							
	2-4=-394/188				Residential Cod			and					
NOTES					nd referenced sta	andard Al	NSI/TPI 1.						
 Unbalanced this design. 	I roof live loads have	been considered for	r LC	DAD CASE(S)	Standard							UNITH CA	11111
	7 40. 1/1.14 400 mm m	(2 accord such)										N'L CA	Rollin

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-0 to 3-0-0, Exterior(2R) 3-0-0 to 3-10-10, Exterior(2E) 3-10-10 to 6-10-10 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

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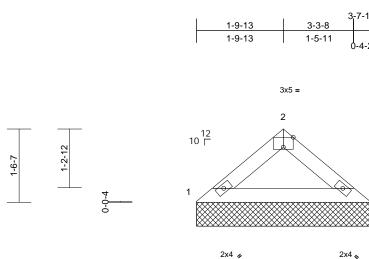
818 Soundside Road Edenton, NC 27932

A THE COMPANY

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V08	Valley	1	1	I6 Job Reference (optional)	8925811

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:45 ID:y89TWpA1p2evV2W6WCbunJzuo48-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



2x4 💊

3

3-7-10

Scale = 1:24.1

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0* BCDL 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2018/TPI2014	CSI TC BC WB Matrix-MP	0.10 0.08 0.00	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 11 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING TOP CHORD Structural wood she 3-7-10 oc purlins. BOT CHORD Rigid ceiling directly bracing. REACTIONS (size) 1=3-7-10, Max Horiz 1=32 (LC Max Uplift 1=-12 (LC Max Uplift 1=-12 (LC Max Grav 1=171 (LC FORCES (b) - Maximum Com Tension TOP CHORD 1-2=-222/84, 2-3=-2 BOT CHORD 1-3=-51/161 NOTES 1) Unbalanced roof live loads have this design. 2) Wind: ASCE 7-16; Vult=130mph Vasd=103mph; TCDL=6.0psf; Bd II; Exp B; Enclosed; MWFRS (er and C-C Exterior(2E) zone; canti exposed ; end vertical left and rig members and forces & MWFRS Lumber DOL=1.60 plate grip DO 3) Truss designed for wind loads in only. For studs exposed to wind see Standard Industry Gable En- or consult qualified building desig 4) TCLL: ASCE 7-16; PT=20.0 psf (L DOL=1.15); Is=1.0; Rough Cat E Cs=1.00; Ct=1.10 5) Unbalanced snow loads have be design. 6) Gable requires continuous botton	applied or 10-0-0 oc 3=3-7-10 11) 14), 3=-12 (LC 15) 2 0), 3=171 (LC 21) pression/Maximum 22/84 been considered for (3-second gust) CDL=6.0psf; h=25ft; ivelope) exterior zone lever left and right pht exposed; C-C for for reactions shown; L=1.60 the plane of the trus (normal to the face), d Details as applicab gner as per ANSI/TP roof LL: Lum DOL=1.15 Plate ; Fully Exp.; Ce=0.9; en considered for thi	8) This truss chord live 9) * This trus on the bot 3-06-00 ta chord and 10) Provide m bearing pl 1 and 12 I 11) This truss Internation R802.10.2 LOAD CASE(Cat. e	Is spaced at 4-0-0 has been designed load nonconcurren s has been designed om chord in all are ll by 2-00-00 wide v any other member echanical connectii ate capable of withs o uplift at joint 3. is designed in acco al Residential Cod and referenced sta 5) Standard	I for a 10.0 t with any ed for a liv as where will fit betw s. on (by oth standing 1 ordance w e sections	other live loa re load of 20.0 a rectangle veen the botto l2 lb uplift at j ith the 2018 \$ R502.11.1 a	Opsf om o oint			Ó	SEA 0363	L 22 EER AL



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818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V09	Valley	1	1	Job Reference (optional)	168925812

3-6-12

Scale = 1:32.8

Loading

TCLL (roof)

Snow (Pf)

LUMBER

TOP CHORD

BOT CHORD

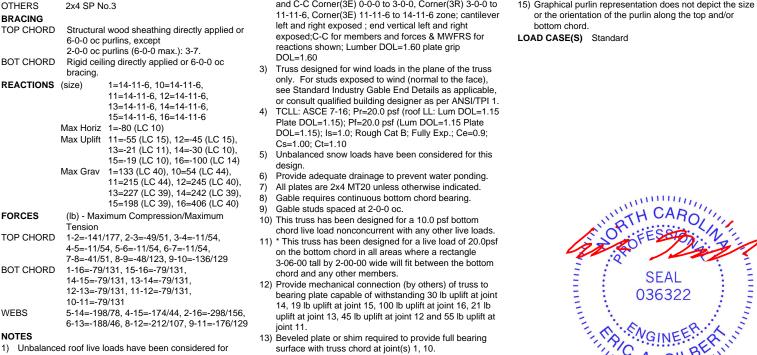
TCDL

BCLL

BCDL

Page: 1

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:45 ID:qvPzMAEYtH8L_fptl1gqy9zuo44-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 14-10-13 4-3-0 10-7-13 14-6-11 4-3-0 6-4-13 3-10-14 3x5 🖌 3x5 。 3 4 6 7 \boxtimes 8 2 21 22 έ κ κ 12 10 Г 9 10 16 15 14 13 12 11 3x5 🍫 3x5 💊 14-10-13 Plate Offsets (X, Y): [3:0-2-8,0-0-3], [7:0-2-8,0-0-3] 2-0-0 CSI DEFL l/defl L/d PLATES GRIP (psf) Spacing in (loc) 20.0 Plate Grip DOL 1.15 TC 0.19 Vert(LL) 999 MT20 244/190 n/a n/a 20.0 Lumber DOL 1.15 BC 0.19 Vert(TL) n/a n/a 999 Rep Stress Incr WB Horiz(TL) 10 10.0 YES 0.06 0.00 n/a n/a 0.0 IRC2018/TPI2014 Matrix-MSH Code 10.0 Weight: 68 lb FT = 20% 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) 14) This truss is designed in accordance with the 2018 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. International Residential Code sections R502.11.1 and 2x4 SP No.2 II; Exp B; Enclosed; MWFRS (envelope) exterior zone 2x4 SP No 2 R802.10.2 and referenced standard ANSI/TPI 1. and C-C Corner(3E) 0-0-0 to 3-0-0, Corner(3R) 3-0-0 to 2x4 SP No.3 15) Graphical purlin representation does not depict the size 11-11-6. Corner(3E) 11-11-6 to 14-11-6 zone: cantilever or the orientation of the purlin along the top and/or left and right exposed ; end vertical left and right bottom chord. exposed;C-C for members and forces & MWFRS for LOAD CASE(S) Standard reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss



this design.

surface with truss chord at joint(s) 1, 10.

400000 October 17,2024

GI

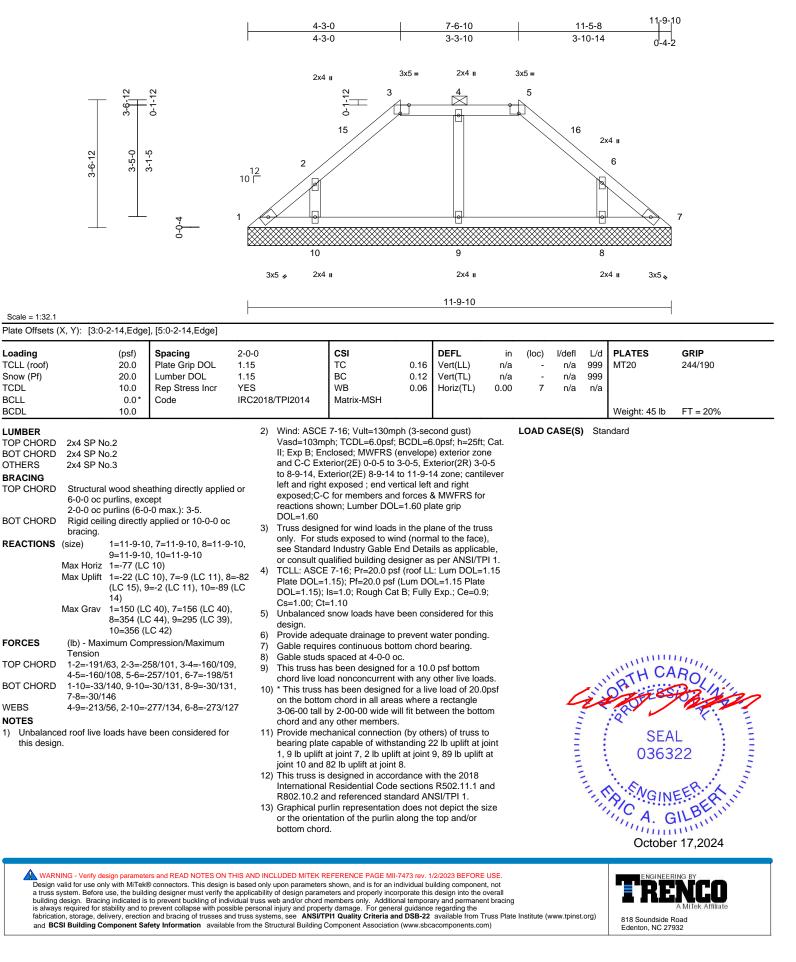
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overal bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road

Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V10	Valley	1	1	Job Reference (optional)	168925813

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:46 ID:Mjrb9rDw6z0UMVEgBK9bPyzuo45-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V11	Valley	1	1	Job Reference (optional)	168925814

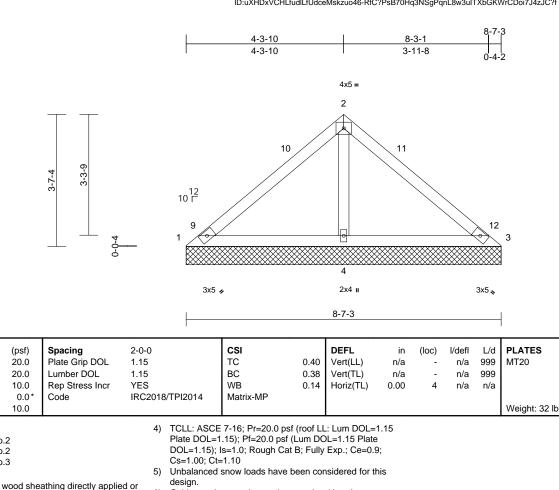
Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:46 ID:uXHDxVCHLfudlLfUdceMskzuo46-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1

GRIP

244/190

FT = 20%



Scale = 1:31.5

Loading

TCLL (roof)

Snow (Pf)

TCDL

BCLL

BCDL		10.0		
LUMBER				4)
TOP CHORD	2x4 SP N	0.2		
BOT CHORD	2x4 SP N	0.2		
OTHERS	2x4 SP N	0.3		
BRACING				5)
TOP CHORD	Structural 8-7-3 oc p		athing directly applied or	6
BOT CHORD	Rigid ceili bracing.	ing directly	applied or 6-0-0 oc	7) 8)
REACTIONS	(size)	1=8-7-3, 3	8=8-7-3, 4=8-7-3	0
	Max Horiz	1=80 (LC	11)	9)
	Max Uplift	1=-48 (LC 4=-105 (L0	21), 3=-48 (LC 20), C 14)	
	Max Grav	1=87 (LC (LC 20)	20), 3=87 (LC 21), 4=694	1
FORCES	(lb) - Max	imum Com	pression/Maximum	
	Tension			1
TOP CHORD	1-2=-127/	/320, 2-3=-	127/320	•
BOT CHORD	1-4=-247/	188, 3-4=-2	247/188	
WEBS	2-4=-563/	/258		L
NOTES				_

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-5 to 3-0-5, Exterior(2R) 3-0-5 to 5-7-8, Exterior(2E) 5-7-8 to 8-7-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- Gable requires continuous bottom chord bearing. Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 0) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 1, 48 lb uplift at joint 3 and 105 lb uplift at joint 4.
- 1) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

OAD CASE(S) Standard



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818 Soundside Road Edenton, NC 27932

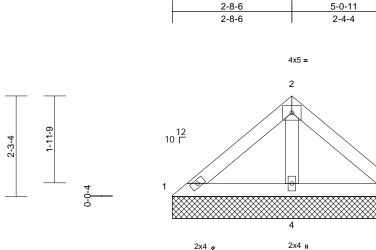
Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V12	Valley	1	1	Job Reference (optional)	168925815

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:46 ID: uXHDxVCHLfudlLfUdceMskzuo46-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?ff

3

2x4 💊





2x4 🍬

5-4-13

Scale = 1:26

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL LUMBER TOP CHORD BOT CHORD OTHERS		Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	5)	design. Gable requir	CSI TC BC WB Matrix-MP snow loads have es continuous bot spaced at 4-0-0 o	tom chor	Vert(TL) Horiz(TL)	in n/a 0.00 his	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 20 lb	GRIP 244/190 FT = 20%	
BRACING TOP CHORD BOT CHORD REACTIONS	 Structural wood she 5-4-13 oc purlins. Rigid ceiling directly bracing. 	applied or 6-0-0 oc 3=5-4-13, 4=5-4-13 : 12) 15), 4=-41 (LC 14)	8) ed or 9)	This truss ha chord live loa * This truss h on the bottor 3-06-00 tall h chord and ar) Provide mec bearing plate and 41 lb up) This truss is	is been designed ad nonconcurrent has been designed in chord in all area by 2-00-00 wide w hy other members hanical connectio e capable of withsi	for a 10. with any d for a liv as where vill fit betw a n (by oth tanding 6 rdance w	other live loa e load of 20.0 a rectangle veen the botto ers) of truss t b uplift at jo ith the 2018	Opsf om to int 3						
, this desig	 1-4=-103/103, 3-4=- 2-4=-254/121 ced roof live loads have gn. 	5/128 103/103 been considered fo		R802.10.2 a DAD CASE(S)	nd referenced sta Standard	ndard AN	ISI/TPI 1.						1117.	
Vasd=103 II; Exp B; and C-C I exposed members Lumber D 3) Truss des only. For see Stand or consult 4) TCLL: AS Plate DOI	CE 7-16; Vult=130mph 3mph; TCDL=6.0psf; Br Enclosed; MWFRS (er Exterior(2E) zone; cant ; end vertical left and rig and forces & MWFRS DOL=1.60 plate grip DC signed for wind loads in r studs exposed to wind dard Industry Gable En t qualified building desi; SCE 7-16; Pr=20.0 psf (L L=1.15); Pf=20.0 psf (L 5); Is=1.0; Rough Cat E Ct=1.10	CDL=6.0psf; h=25ft; velope) exterior zor lever left and right ght exposed;C-C for for reactions shown L=1.60 the plane of the true (normal to the face) d Details as applicat gner as per ANSI/TF roof LL: Lum DOL= um DOL=1.15 Plate	ne ; ss), ple, Pl 1. I.15							M. HILLING		SEA 0363	EER ER	Nummer.

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October 17,2024

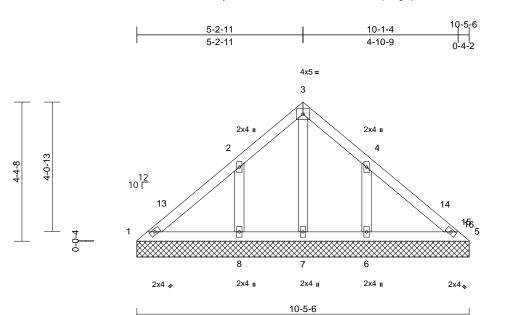
A. GI A. GIL



Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V13	Valley	1	1	Job Reference (optional)	168925816

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Page: 1



Scale = 1:36.2

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL		(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	1-11-4 1.15 1.15 YES IRC201	8/TPI2014	CSI TC BC WB Matrix-MSH	0.18 0.14 0.06	Vert(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 46 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP N Structural 10-0-0 oc Rigid ceili bracing. (size) Max Horiz	0.2 0.3 wood she purlins. ng directly 1=10-5-6, 7=10-5-6, 1=95 (LC 8=-112 (L 1=-59 (LC	11) 10), 6=-108 (LC 15), C 14) 20), 5=70 (LC 21), 6	5) (6) (7) (8) (=390) (5)	only. For stu see Standar, or consult qu TCLL: ASCE Plate DOL= ⁻¹ DOL=1.15); Cs=1.00; Ct: Unbalanced design. Gable requir Gable studs This truss ha chord live lo: * This truss l	ed for wind loads ids exposed to wi d Industry Gable I lailified building de ; 7-16; Pr=20.0 ps .15); Pf=20.0 ps Is=1.0; Rough Ca =1.10 snow loads have es continuous bol spaced at 2-0-0 c is been designed ad nonconcurrent nas been designed n chord in all area	nd (norm End Deta ssigner as if (roof LL (Lum DC t B; Fully been cor tom chor cc. for a 10.0 with any d for a liv	al to the face ils as applica s per ANSI/T : Lum DOL= 5L=1.15 Plate Exp.; Ce=0. hsidered for t d bearing. 0 psf bottom other live loa e load of 20.	e), hble, PI 1. 1.15 e 9; his ads.					
FORCES TOP CHORD BOT CHORD WEBS NOTES	Tension 1-2=-101/ 4-5=-101/ 1-8=-89/1 5-6=-89/1	20) imum Com 156, 2-3=- 156 41, 7-8=-8 41	'=171 (LC 20), 8=391 pression/Maximum 28/92, 3-4=-28/90, 9/141, 6-7=-89/141, 7/213, 4-6=-288/212	10 11	3-06-00 tall I chord and an Provide mec bearing plate 112 lb uplift) This truss is International	by 2-00-00 wide w by other members hanical connection e capable of withs at joint 8 and 108 designed in accoor Residential Code and referenced sta	vill fit betv s. n (by oth tanding 5 lb uplift a rdance w sections	veen the bott ers) of truss 5 lb uplift at jo at joint 6. ith the 2018 5 R502.11.1 a	to vint 1,				WITH CA	RODI

- Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=130mph (3-second gust)
- Vinit: ASCL 7-10, Vult=150m[GSEc00psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) 0-0-5 to 3-3-0, Corner(3R) 3-3-0 to 7-3-0, Corner(3E) 7-3-0 to 10-1-2 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



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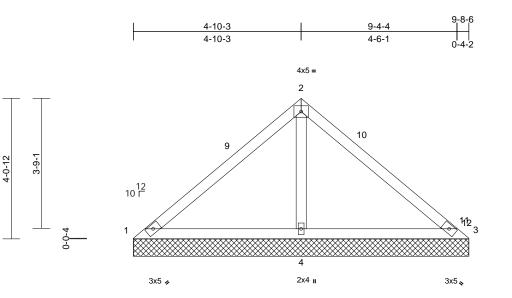


Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V14	Valley	1	1	Job Reference (optional)	168925817

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries, Inc. Tue Oct 15 15:19:46 ID:QKjrk9BfaMmm7C4I4v77KXzuo47-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Page: 1



9-8-6

Scal	1:33.3

Loading TCLL (roof) Snow (Pf) TCDL BCLL BCDL	(psf) 20.0 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC20 ⁴	18/TPI2014	CSI TC BC WB Matrix-MSH	0.45 0.43 0.19	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.01	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 37 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD WEBS NOTES	2x4 SP No.2 2x4 SP No.3 Structural wood sh 9-8-6 oc purlins. Rigid ceiling directl bracing. (size) 1=9-8-6, Max Horiz 1=90 (LC Max Uplift 1=-49 (L 4=-111 (Max Grav 1=94 (LC (LC 21)) (lb) - Maximum Con Tension 1-2=-118/372, 2-3=	C 21), 3=-60 (LC 20) LC 14) C 20), 3=74 (LC 21), - mpression/Maximum 117/376	6 7 8 9 4=777 1 1	 Plate DOL=² DOL=1.15); Cs=1.00; Ct: Unbalanced design. Gable requir Gable studs This truss ha chord live loo; * This truss la chord live loo; * This truss la chord and ai Provide mec bearing plate 1, 60 lb uplif This truss is International 	snow loads have l res continuous bott spaced at 4-0-0 o as been designed ad nonconcurrent has been designed m chord in all area by 2-00-00 wide w ny other members; chanical connection e capable of withst t at joint 3 and 111 designed in accor Residential Code nd referenced star	(Lum DC B; Fully been cor tom chor c. for a 10.0 with any d for a liv s where ill fit betv - h (by oth anding 4 lb uplift dance w sections	DL=1.15 Plate Exp.; Ce=0.9 nsidered for the d bearing. 0 psf bottom other live loa re load of 20.0 a rectangle veen the bottk ers) of truss t 19 lb uplift at j at joint 4. ith the 2018 \$ R502.11.1 a	ds. Dpsf om o oint					
	ed roof live loads hav	e been considered fo	r										

- this design. 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-5 to 3-0-5, Exterior(2R) 3-0-5 to 6-4-2, Exterior(2E) 6-4-2 to 9-4-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

shown; Lumber DOL=1.60 plate grip DOL=1.60

Vanannon GI 11111111 October 17,2024

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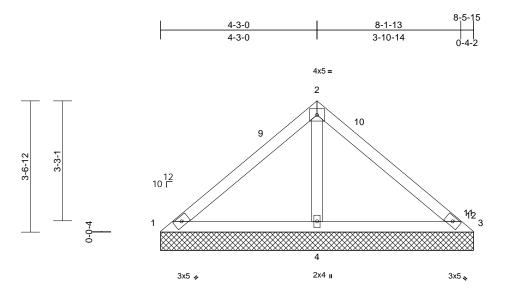
SEAL

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VIIIIIIIIIIII

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V15	Valley	1	1	Job Reference (optional)	168925818

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8-5-15

Scale = 1:31.3

Loading TCLL (roof) Snow (Pf) TCDL	(psf) 20.0 20.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES		CSI TC BC WB	0.39 0.37 0.13	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 4	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	GRIP 244/190
BCLL BCDL	0.0* 10.0	Code	IRC201	8/TPI2014	Matrix-MP							Weight: 32 lb	FT = 20%
	2x4 SP No.2 2x4 SP No.3 Structural wood she 8-5-15 oc purlins. Rigid ceiling directly bracing. (size) 1=8-5-15 Max Horiz 1=79 (LC Max Uplift 1=-44 (LC 4=-103 (I Max Grav 1=77 (LC (LC 21) (lb) - Maximum Con Tension 1-2=-126/302, 2-3=: 1-4=-236/174, 3-4=:	C 21), 3=-51 (LC 20), C 14) 20), 3=69 (LC 21), 4 npression/Maximum -125/311	5) ed or 6) 7) 8) 9) 4=678 1(Plate DOL=1 DOL=1.15); Cs=1.00; Ct= Unbalancet design. Gable requir Gable studs This truss ha chord live loa * This truss h on the bottor 3-06-00 tall b chord and ar D) Provide mec bearing plate 1, 51 lb upliff 1) This truss is International R802.10.2 at	snow loads have es continuous bo spaced at 4-0-0 d s been designed ad nonconcurrent nas been designe n chord in all aree yy 2-00-00 wide w yy other members hanical connectic e capable of withs at joint 3 and 10 designed in acco Residential Code nd referenced sta	(Lum DC t B; Fully been cor ttom chor oc. for a 10.0 with any d for a liv as where vill fit betv 5. in (by oth tanding 4 3 lb uplifit rdance w e sections	DL=1.15 Plate Exp.; Ce=0.9 nsidered for the d bearing. D psf bottom other live loa e load of 20.4 a rectangle ween the botther ers) of truss the t4 lb uplift at j at joint 4. the the 2018 s R502.11.1 a	e 9; his Opsf om to oint					
WEBS NOTES	2-4=-550/254		L	DAD CASE(S)	Standard								
 Unbalance this design Wind: ASO Vasd=103 II; Exp B; 	ed roof live loads have n. CE 7-16; Vult=130mpt 3mph; TCDL=6.0psf; B Enclosed; MWFRS (er 5xterior(2E) 0-0-5 to 3	n (3-second gust) CDL=6.0psf; h=25ft; nvelope) exterior zon	Cat. e							2	Z	ORTH CA	ROIN

Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-5 to 3-0-5, Exterior(2R) 3-0-5 to 5-1-11, Exterior(2E) 5-1-11 to 8-1-11 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

 Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. SEAL 036322 October 17,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY A Mi Tek Atfiliate 818 Soundside Road Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	89 Eagle Creek - Norman D - Roof	
25040191	V16	Valley	1	1	Job Reference (optional)	168925819

3-9-0

3-9-0

Carter Components (Sanford, NC), Sanford, NC - 27332,

Scale = 1:29.6 Loading

TCLL (roof)

Snow (Pf)

LUMBER

OTHERS

BRACING

TOP CHORD

BOT CHORD

TOP CHORD

BOT CHORD

FORCES

WFBS

NOTES

1)

2)

TOP CHORD

BOT CHORD

this design

DOL=1.60

REACTIONS (size)

TCDL

BCLL

BCDL

Run: 8.73 S Sep 25 2024 Print: 8.730 S Sep 25 2024 MiTek Industries. Inc. Tue Oct 15 15:19:46 ID:y89TWpA1p2evV2W6WCbunJzuo48-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

4x5 =

7-1-13

3-4-14



2 11 10 2-10-1 3-1-12 12 10 Г 9 1f3 3 4 3x5 🍫 2x4 ı 3x5 💊 7-5-15 2-0-0 CSI DEFL l/defl L/d PLATES GRIP Spacing in (loc) Plate Grip DOL 1.15 TC 0.28 Vert(LL) n/a n/a 999 MT20 244/190 1 15 BC 0.28 Lumber DOL Vert(TL) n/a n/a 999 Rep Stress Incr YES WB 0.09 Horiz(TL) 0.00 4 n/a n/a Code IRC2018/TPI2014 Matrix-MP Weight: 28 lb FT = 20%TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 4) Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10 Unbalanced snow loads have been considered for this 5) desian. Structural wood sheathing directly applied or Gable requires continuous bottom chord bearing. 6) 7) Gable studs spaced at 4-0-0 oc. Rigid ceiling directly applied or 6-0-0 oc 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf 1=7-5-15, 3=7-5-15, 4=7-5-15 9) Max Horiz 1=69 (LC 11) on the bottom chord in all areas where a rectangle 1=-20 (LC 21), 3=-31 (LC 20), 3-06-00 tall by 2-00-00 wide will fit between the bottom 4=-80 (LC 14) chord and any other members. 1=92 (LC 20), 3=72 (LC 21), 4=555 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint (lb) - Maximum Compression/Maximum 1, 31 lb uplift at joint 3 and 80 lb uplift at joint 4. 11) This truss is designed in accordance with the 2018 1-2=-99/239, 2-3=-97/241 International Residential Code sections R502.11.1 and 1-4=-185/147, 3-4=-185/147 R802.10.2 and referenced standard ANSI/TPI 1. LOAD CASE(S) Standard Unbalanced roof live loads have been considered for Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-5 to 3-0-5, Exterior(2R) 3-0-5 VIIIIIIIIIIIIIIII to 4-1-11, Exterior(2E) 4-1-11 to 7-1-11 zone; cantilever left and right exposed ; end vertical left and right SEAL exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip 036322

Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable or consult qualified building designer as per ANSI/TPI 1.

(psf)

20.0

20.0

10.0

10.0

(LC 21)

2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

bracing.

Max Uplift

Max Grav

Tension

2-4=-445/212

7-5-15 oc purlins.

0.0

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818 Soundside Road

Edenton, NC 27932

G mmm October 17,2024

