# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 58814 JOB: 25-3580-R01 JOB NAME: LOT 0.0024 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *37 Truss Design(s)* 

Trusses:

BR01, BR02, GR01, GR02, J01, J02, J03, J04, J05, PB01, PB02, PB03, R01, R02, R02A, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, SP01, SP02, VT01, VT02,



My license renewal date for the state of North Carolina is 12/31/2025

## Warning !--- Verify design parameters and read notes before use.





between the bottom chord and any other members. 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8, 6.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDGE   81 PINON	DRIVE ANGIER, NC
25-3580-R01	GR01	Common Supported Gable	1	1	Job Reference (optional)	# 58814
					8.630 s Jul 12 2024 MiTek Industries, Inc.	Thu Apr 24 14:16:56 2025 Page 2

ID:R4JAOCFxLoAUFpwqRUTCbAz\_Vw6-m9DvWzIUW2Twn0FXrys0YwhdqsNqtJLkPN4S28zNV1b

14) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 15) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 16) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

17) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Tr	uss	Truss Type		Qty	Ply	LOT 0	.0024 CAI	MPBELL RIDG	E   81 PINON DRIVE A	NGIER, NC
25-3580-R01	JO	1	Jack-Open		1		1 Job R	eference	e (optional)	÷	<i># 58814</i>
			-0-1	0-8 	ID:R4JAOCFxLoA 1-6-14 1-6-14	UFpwqR	8.630 UTCbAz	) s Jul 12 	2024 MiTek Ind LnHkIm6HMI	dustries, Inc. Thu Apr 2 onPAqkPfNF57DoyG	4 14:16:57 2025 Page 1 Skwcnste1p?aazNV1a
											Scale = 1:12.3
		1-1-0		7.00 12 2x4    2 Tr W1 5 2x4	1 B1	4		1-6-1			
				Ļ	1-6-14 1-6-14						
LOADING (pst TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TP	2-0-0 1.25 1.25 YES  2014	<b>CSI.</b> TC 0.12 BC 0.04 WB 0.00 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in ( 0.00 -0.00 -0.00	(loc)   5 > 5 > 3	/defl •999 •999 •999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 8 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3		i		BRACING- TOP CHORD BOT CHORD	Structu end ve Rigid c MiTel be ins	rticals. eiling d k recom stalled c	od sheat irectly a mends during tr	hing directly pplied or 10 that Stabiliz uss erectior	/ applied or 1-6-14 1-0-0 oc bracing. ters and required c n, in accordance w	oc purlins, except ross bracing ith Stabilizer
REACTIONS.	(lb/size) 5= Max Horz 5=4 Max Upliff5=	140/0-3-8 (min. 0-1-8), 3 11(LC 14)	=25/Mechanical,	4=10/Mechanical			allon y				

Max Opints=-4(LC 14), 3=-24(LC 14), 4=-4(LC 14)Max Grav 5=193(LC 21), 3=35(LC 21), 4=25(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

LOAD CASE(S) Standard







BOT CHORD

2x4 SP No.3 WFBS

end verticals Rigid ceiling directly applied or 10-0-0 oc bracing

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 7=221/0-3-8 (min. 0-1-8), 4=68/Mechanical, 5=75/Mechanical Max Horz 7=75(LC 12) Max Uplift4=-23(LC 8), 5=-19(LC 12) Max Grav 7=221(LC 1), 4=68(LC 1), 5=88(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 7=221/0-3-8 (min. 0-1-8), 4=33/Mechanical, 5=110/Mechanical Max Horz 7=110(LC 12) Max Uplift4=-11(LC 8), 5=-63(LC 12) Max Grav 7=221(LC 1), 4=33(LC 1), 5=115(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 5.

LOAD CASE(S) Standard





Max Uplift3=-102(LC 12), 4=-15(LC 12) Max Grav 5=221(LC 1), 3=116(LC 20), 4=73(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (it=lb) 3=102.

LOAD CASE(S) Standard





Max Uplift2=-17(LC 12), 4=-21(LC 13) Max Grav 2=93(LC 1), 4=93(LC 1), 6=89(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

## LOAD CASE(S) Standard





Max Uplift2=-17(LC 12), 4=-21(LC 13) Max Grav 2=93(LC 1), 4=93(LC 1), 6=89(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) Gable requires continuous bottom chord bearing.

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4. 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

## LOAD CASE(S) Standard







vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDO	GE   81 PINON DRIVE ANGIER, NC
25-3580-R01	R04	Common	5	1		# 58814
					Job Reference (optional) 8.630 s Jul 12 2024 MiTek Ir	ndustries, Inc. Thu Apr 24 14:17:03 2025 Page 1
-0-	10-8 8-0-4	15-9-0	D:R4JAOCFxLoAU	FpwqRUT(	CbAz_Vw6-3V8Y_MrttCLx7	75HumwUfKOTcagaO0Oam0yGJoEzNV1L
0-	10-8 8-0-4	7-8-12	7-8-1	2	8-0-4	
		5	×8 —			Scale = 1:68.1
			×0 —			
т		7.00 12	5			
			A A			
			´ \\			
		5x6 18 12	11	19		
				$\nearrow$	5x6 🖄	
4		4			6	
10-2		W2	wz			
	5x5 /			1	75	x5 <>
	5X5 - 1			ha		5x5 ≈
	EHW1					8
9 <sup>1</sup>				₩/	D4	
110				·····	B1	
	8x8    20	21 17 10 22 4×4 —	13 10 <sup>23 27</sup>	9 11 ×4 —	24 25	8x8
		4x4 — 4x12 M 2x4 =	MT20HS= 4 2x4	4 — 1 —		
		2x4    2x4	2x4    2x4			
		247 11	2,4 11			
	11-3-0	13-9-0 15-9-0	17-9-0 20-3-0	+	31-6-0	
Plate Offsets (X,Y) [4:0	-3-0,0-3-4], [6:0-3-0,0-3-4]	2-0-0 2-0-0	2-0-0 2-0-0		11-5-0	
LOADING (psf)	SPACING-	2-0-0 <b>CSI</b>	DEEI	in (lo	c) l/defl l/d	PLATES GRIP
TCLL (roof) 20.0 Snow (Pf) 20.0	Plate Grip DOL	1.25 TC 0.94	Vert(LL)	-0.41 10-	15 >923 240	MT20 244/190
TCDL 10.0	Lumber DOL Rep Stress Incr	1.25 BC 0.86	Vert(CT)	-0.60 10-	15 >631 180 8 p/a p/a	MT20HS 187/143
BCLL 0.0 * BCDI 10.0	Code IRC2021/TPI	2014 Matrix-SH	1012(01)	0.07	0 11/4 11/4	Weight: 195 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x4 SP SS	*Except*		TOP CHORD	Structura	I wood sheathing direct	ly applied.
I 1, I 3: 2x4 BOT CHORD 2x4 SP SS	SP No.2 S*Except*		BOT CHORD	Rigid cei 6-0-0 oc	ling directly applied or 1 bracing: 11-16	0-0-0 oc bracing. Except:
B2: 2x4 SF	P No.2			MiTek	ecommends that Stabili	zers and required cross bracing
WEBS 2x4 SP No SLIDER Left 2x8 SE	9.3 P No 2 4-9-1 Right 2x8 SP	No 2 4-9-1		be insta	alled during truss erection	on, in accordance with Stabilizer
				Installa	lion guide.	
REACTIONS. (Ib/size) 2 Max Horz 2	2=1398/0-3-8 (min. 0-1-15) 2=-194(I C 10)	, 8=1344/Mechanical				
Max Uplift	2=-65(LC 14), 8=-51(LC 15)	)				
Max Grav	2=1654(LC 24), 8=1605(LC	25)				
FORCES. (Ib) - Max. Cor	mp./Max. Ten All forces 2	50 (lb) or less except when shown.				
TOP CHORD 2-3=-238 6-19=-21	6/68, 3-4=-2278/107, 4-18= 68/119, 6-7=-2280/108, 7-8	-2166/118, 5-18=-2067/142, 5-19=-: =-2389/85	2068/143,			
BOT CHORD 2-20=-10	0/2043, 20-21=-100/2043, 1	7-21=-100/2043, 17-22=0/1532, 15	-22=0/1532,			
13-15=0/ 8-25=-5/1	1532, 10-13=0/1532, 10-23 1902	=0/1532, 9-23=0/1532, 9-24=-5/190	2, 24-25=-5/1902			
WEBS 5-11=-62	/991, 9-11=-82/892, 6-9=-40	05/231, 16-17=-81/889, 5-16=-61/98	88, 4-17=-403/230			
<b>NOTES-</b> (11)						
1) Unbalanced roof live lo	ads have been considered	for this design.				
(envelope) dable end z	ene and C-C Exterior(2E) -(	/asd=95mpn; TCDL=5.0psf; BCDL= )-10-8 to 3-11-2. Interior(1) 3-11-2 to	5.0psf; n=23π; Ca	at. II; Exp⊺ r(2R) 10-1	1-6 to 20-6-10.	
Interior(1) 20-6-10 to 20	6-8-6, Exterior(2E) 26-8-6 to	31-6-0 zone;C-C for members and	forces & MWFRS	for reacti	ons shown; Lumber	
3) TCLL: ASCE 7-16: Pr=	OL=1.60 20.0 psf (roof LL: Lum DOL	=1.25 Plate DOL=1.25): Pf=20.0 ps	f (Lum DOL=1.15	Plate DO	L=1.15): ls=1.0: Rough	AND
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10		(			WINGTH CARO
<ul><li>4) Unbalanced snow load</li><li>5) This truss has been de</li></ul>	s nave been considered for signed for greater of min ro	this design. of live load of 12.0 psf or 2.00 times	flat roof load of 2	0.0 psf on	overhands	OFESSION
non-concurrent with oth	ner live loads.				in the second second	and the second
<ul> <li>6) All plates are M120 pla</li> <li>7) This truss has been de</li> </ul>	ates unless otherwise indica	ted. h chord live load nonconcurrent with	any other live loa	ds	H III	SEAL
8) * This truss has been d	lesigned for a live load of 30	0.0psf on the bottom chord in all are	as where a rectar	gle 3-6-0	tall by 1-0-0 wide will fit	28147
9) Refer to girder(s) for true	ord and any other members	, with BCDL = 10.0psf.			Inn	No. al I
10) Provide mechanical c	onnection (by others) of trus	ss to bearing plate capable of withst	anding 100 lb upli	ft at joint(	s) 2, 8.	A POINEE AS IN
LOAD CASE(S) Standard	1					Min K. MOhmmin
						1/21/2025
						4/24/2023



D'Onofrio Drive, Madison, WI 53719.

& Bracing of Metal Plate Connected Wood Trusses from Tr

Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDGE   81 PINON DRIVE	ANGIER, NC
25-3580-R01	R05	Half Hip	1	1	Job Reference (optional)	# 58814

8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Apr 24 14:17:04 2025 Page 2 ID:R4JAOCFxLoAUFpwqRUTCbAz\_Vw6-XiiwCisVeVTokFs4Je?utc0oR4wclkRvFc0tKgzNV1T

LOAD CASE(S) Standard





Max Uplift12=-111(LC 11), 2=-23(LC 14) Max Grav 12=1998(LC 37), 2=1667(LC 38)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-2380/44, 3-22=-2257/60, 4-22=-2238/71, 4-5=-2248/47, 5-6=-1945/66, 6-7=-2187/91,

- 7-23=-2192/69, 8-23=-2192/69, 8-9=-2192/69, 9-10=-2192/69, 11-12=-250/48 BOT CHORD 2-21=-179/1850, 21-25=-84/2219, 25-26=-84/2219, 19-26=-84/2219, 17-19=-84/2219, 14-17=-84/2219, 14-27=-84/2219, 13-27=-84/2219, 13-28=-85/1337, 28-29=-85/1337,
- 12-29=-85/1337 WEBS 4-21=-396/194, 5-21=0/912, 8-13=-516/117, 10-13=0/1411, 10-12=-2088/139,
  - 7-15=-297/139, 13-15=-327/120, 20-21=-671/143, 6-20=-560/165

NOTES-(12)

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-10-8 to 3-11-2, Exterior(2R) 3-11-2 to 17-6-14, Interior(1) 17-6-14 to 26-6-10,

- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9)
- 10) Refer to girder(s) for truss to truss connections.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb)

In the DDL=1.15); Is=1.0; Rough and the provide the PALZO2 C Vand LOADIDASE(S): By and by an and the second se vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL	RIDGE   81 PINON DRIVE	ANGIER, NC
25-3580-R01	R07	Half Hip Girder	1	1	Job Reference (option	al)	# 58814
			ID:R4JAOCFxLoA	UFpwqRL	8.630 s Jul 12 2024 MiT JTCbAz_Vw6-T4qhdNt	ek Industries, Inc. Thu Apr I97jW_Z0SR21My156?u	24 14:17:06 2025 Page 1 Ic9DiHCiwVzPZzNV1R
-Q-10-8 3-11 0-10-8 3-11	I-3 7-6-14	13-7-8 6-0-11	19-6-7 5-10-15		25-5-5 5-10-15	<u>31-6-0</u> 6-0-11	
							Scale - 1:58 1
							Scale - 1.56.1
			NAI	ED		<b>)</b>	
	5x8 =		-D NAILED 4x8				x6 =
	5	19 20 <sup>6</sup> 21 <u>-22</u>	23 7	NAIL 24	4 8 9	-25 26	10
7.00 N	ALED NAILED						र्म
	4x6 18						
NAILED	4 T10542 W/D				W4	W4	WB
u 4×0 2		VV4 VV5	VV4 VV5				
HW	W1 W2						
					<u>B2</u>		F
	47 28 40	29 30 31 45 1	14 <sup>32</sup> 33 34 12	35 36	6 37 40	38 39 40	
5x8	- 2x4    NAILED 4x4	= NAILED NAILED  4x8 = 5	$^{13}_{8} = \text{NAILED}_{2x4}$	NAIL	ED NAILED 6x8 =		11 x10
NAILE	NAILED NAILE	D NAILED NAILE	ED NAIL	ED	NAILED	)	
		10 7 0	40.07		05 5 5		
3-11	I-3 7-6-14 I-3 3-7-11	<u> </u>	19-6-7 5-10-15		25-5-5 5-10-15	<u>31-6-0</u> 6-0-11	
Plate Offsets (X,Y) [5:0	-6-0,0-2-4], [11:Edge,0-3-8]	, [12:0-1-8,0-3-4]					
LOADING (psf) TCLL (roof) 20.0	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in (lo	oc) I/defl L/d	PLATES	GRIP
Snow (Pf) 20.0	Plate Grip DOL Lumber DOL	1.25 TC 1.00 1.25 BC 0.86	Vert(LL) Vert(CT)	0.18 13- -0.28 13-	15 >999 240 15 >999 180	MT20	244/190
ICDL 10.0 BCLL 0.0 *	Rep Stress Incr	NO WB 0.69	Horz(CT)	0.07	11 n/a n/a	Wainht 04	0.1h FT - 200/
BCDL 10.0		2014 Matrix-SH				vveight: 218	3 ID FT = 20%
LUMBER-	2 *Excent*		BRACING-	Structure	al wood sheathing dir	ectly applied except	end verticals
T2: 2x4 SF	PSS		BOT CHORD	Rigid cei	ling directly applied of	or 6-2-0 oc bracing.	end venticals.
BOT CHORD 2x6 SP No WEBS 2x4 SP No	.2 .3 *Except*		WEBS	1 Row at	t midpt 10-	11, 7-12, 10-12	oroco brooing
W4: 2x4 S	P No.2			be insta	alled during truss ere	ection, in accordance v	vith Stabilizer
SLIDER Leil 2x6 SF	<sup>2</sup> NO.2 2-3-8			Installa	tion guide.		
REACTIONS. (lb/size)	11=1733/Mechanical, 2=18 2=151(LC 12)	11/0-3-8 (min. 0-2-5)					
Max Uplift	11=-943(LC 9), 2=-721(LC	12)					
Max Grav	11=2247(LC 72), 2=1978(L	C 34)					
FORCES. (lb) - Max. Cor	mp./Max. Ten All forces 2	50 (lb) or less except when shown.	10- 2410/1409				
19-20=-3	410/1498, 6-20=-3410/1498	3, 6-21=-3410/1498, 21-22=-3410/14	98,				
22-23=-3 9-25=-22	410/1498, 7-23=-3410/1498	3, 7-24=-2207/965, 8-24=-2207/965, 10-26=-2207/965, 10-11=-2114/937	8-9=-2207/965,				
BOT CHORD 2-27=-99	7/2289, 17-27=-997/2289,	17-28=-997/2289, 16-28=-997/2289,	16-29=-1072/243	8,			
29-30=-1 14-32=-1	072/2438, 30-31=-1072/243 452/3346, 14-33=-1452/334	38, 15-31=-1072/2438, 15-32=-1452/ 46, 33-34=-1452/3346, 13-34=-1452/	/3346, /3346,				
13-35=-1	452/3346, 35-36=-1452/334	46, 36-37=-1452/3346, 12-37=-1452/	/3346				
7-12=-14	98/640, 9-12=-720/391, 10-	-361/1263, 6-15716/393, 7-1325 12=-1249/2861	0/320,				
NOTES_ (12)							
1) Wind: ASCE 7-16; Vult	=120mph (3-second gust)	/asd=95mph; TCDL=5.0psf; BCDL=	5.0psf; h=23ft; Ca	t. II; Exp	B; Enclosed; MWFR	S	
(envelope) gable end z 2) TCLL: ASCE 7-16: Pr=	one; Lumber DOL=1.60 pla 20.0 psf (roof LL: Lum DOL	te grip DOL=1.60 =1.25 Plate DOL=1.25): Pf=20.0 psf	(Lum DOL=1.15)	Plate DO	L=1.15): Is=1.0: Rou	ah WHUTH CAR	tille.
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10		(			IN SPESSIA	IN SUL
<ul><li>3) Unbalanced snow load</li><li>4) This truss has been de</li></ul>	s have been considered for signed for greater of min ro	this design. of live load of 12.0 psf or 2.00 times	flat roof load of 20	).0 psf on	overhangs	THE PART	R. M.
non-concurrent with oth	ner live loads.			•		SEAL	
6) This truss has been de	signed for a 10.0 psf botton	n chord live load nonconcurrent with	any other live load	ds.		28147	
7) * This truss has been d	lesigned for a live load of 3	0.0psf on the bottom chord in all area	as where a rectang	gle 3-6-0	tall by 1-0-0 wide wit	Efit	
8) Refer to girder(s) for tru	uss to truss connections.	, with DODE - 10.0p31.				NOINEE	S.S. INT
<ol> <li>Provide mechanical co 2=721.</li> </ol>	nnection (by others) of trus	s to bearing plate capable of withstar	nding 100 lb uplift	at joint(s)	) except (jt=lb) 11=94	13, MARK K. MOR	in the second
10) "NAILED" indicates 3-	-10d (0.148"x3") or 3-12d (0	0.148"x3.25") toe-nails per NDS guid	lines.			1/01/00	25
Warning !	) section, loads applied to the parameters and read notes by	the face of the truss are noted as from	u (F) or back (B).	d is for an	individual building com	4/24/20	LJ loaded
COADLCASE(Sage and ard Vertically. Applicability of	design parameters and proper in	corporation of component is responsibility	of building designer	– not truss	designer or truss engine	er. Bracing shown is for	lateral support
of individual web members	only. Additional temporary brac	cing to ensure stability during construction	is the responsibility of	of the erect	or. Additional permanent	nt bracing of the overall s	tructure is the

responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDGE   81 PINO	N DRIVE ANGIER, NC
25-3580-R01	R07	Half Hip Girder	1	1	Job Reference (optional)	# 58814
					8.630 c. Jul 12.2024 MiTok Industrios, Inc.	Thu Apr 24 14:17:06 2025 Page 2

8.630 s Jul 12 2024 Millek Industries, Inc. Thu Apr 24 14:17:06 2025 Page 2 ID:R4JAOCFxLoAUFpwqRUTCbAz\_Vw6-T4qhdNtl97jW\_Z0SR21My156?uc9DiHCiwVzPZzNV1R

## LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-60, 5-10=-60, 2-11=-20

Concentrated Loads (lb)

Vert: 5=-40(B) 8=-40(B) 4=-8(B) 17=-55(B) 16=-23(B) 15=-23(B) 6=-40(B) 13=-23(B) 7=-40(B) 9=-40(B) 12=-23(B) 3=-41(B) 19=-40(B) 20=-40(B) 22=-40(B) 23=-40(B) 24=-40(B) 25=-40(B) 26=-40(B) 26=-40(B) 27=-27(B) 28=-90(B) 30=-23(B) 31=-23(B) 32=-23(B) 33=-23(B) 33=-23(B











Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDGE   81 PINON DR	IVE ANGIER, NC
25-3580-R01	R11	PIGGYBACK BASE GIRDE	1	3	Job Reference (optional)	# 58814
		ID:R4	JAOCFxL	oAUFpwql	8.630 s Jul 12 2024 MiTek Industries, Inc. Thu RUTCbAz_Vw6-ufVpFPvdS264r0I16Ba3af	Apr 24 14:17:09 2025 Page 2 oe5b8Q0DeOuje?uzNV10

NOTES- (15)

12) Use Simpson Strong-Tie HTU26 (20-10d Girder, 14-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 16-0-12 from the left end to 19-11-4 to connect truss(es) R05 (1 ply 2x4 SP), R06 (1 ply 2x4 SP), R07 (1 ply 2x6 SP) to back face of bottom chord.

13) Fill all nail holes where hanger is in contact with lumber.

14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1359 lb down and 113 lb up at 2-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-7=-60, 6-14=-20

Concentrated Loads (lb)

Vert: 10=-1785(B) 13=-1239(B) 12=-1324(B) 11=-1324(B) 9=-2181(B) 15=-1239(B) 16=-1324(B) 17=-1324(B) 18=-1324(B) 20=-1885(B)





LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 20.0 20.0 10.0 0.0 * 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.26 BC 0.35 WB 0.04 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.02 6-7 -0.05 6-7 0.00 5	l/defl >999 >999 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 33 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-	0.40DN			BRACING-	o				
BOT CHORD	2x4 SP No.2 2x4 SP No.3	2		TOP CHORD	end vertica	vood she ls.	athing dired	ctly applied or 5-3-0 oc	c purlins, except
WEBS	2x4 SP No.3	3		BOT CHORD	Rigid ceilin	g directly	applied or	10-0-0 oc bracing.	
UTHERS	2X4 SP NO.3				MiTok ror	ommond	e that Stah	ilizers and required cr	nee hracing

REACTIONS. (lb/size) 7=266/0-3-8 (min. 0-1-8), 5=192/0-1-8 (min. 0-1-8) Max Horz 7=89(LC 14) Max Uplift7=-17(LC 14), 5=-52(LC 14) Max Grav 7=349(LC 21), 5=275(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-7=-284/107

NOTES-(12)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. Gable studs spaced at 2-0-0 oc.
- a) \* This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
   8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit with CAROY
   9) Bearing at joint(s) 5 considers parallel to review in the parallel to
- 9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.

LOAD CASE(S) Standard

ROFESS SEAL 28147 NOINEE K. MORR 4/24/2025

be installed during truss erection, in accordance with Stabilizer

Installation guide.



Max Uplift5=-17(LC 14), 4=-52(LC 14) Max Grav 5=349(LC 21), 4=275(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. TOP CHORD 2-5=-318/134

**NOTES-** (10)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.







REACTIONS. (lb/size) 4=65/Mechanical, 6=163/0-3-8 (min. 0-1-8), 5=20/0-3-8 (min. 0-1-8) Max Horz 6=49(LC 14) Max Uplift4=-38(LC 14), 6=-18(LC 14)

Max Grav 4=95(LC 21), 6=231(LC 21), 5=62(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Refer to girder(s) for truss to truss connections.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 6.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 4=-30/Mechanical, 5=138/0-3-8 (min. 0-1-8)

Max Horz 5=22(LC 14) Max Uplift4=-78(LC 20) Max Grav 5=203(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

- between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4.

LOAD CASE(S) Standard





Max Uplift4=-117(LC 10), 2=-150(LC 6)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-223/291

**NOTES-** (8)

- 1) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

3) Gable studs spaced at 2-0-0 oc.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=117, 2=150.

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LOAD CASE(S) Standard
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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-223/291

NOTES- (6)

- 1) Wind: ASCE 7-16; Vult=150mph (3-second gust) Vasd=119mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 4=117, 2=150.

LOAD CASE(S) Standard





Installation guide

# REACTIONS. All bearings 16-0-10.

(lb) - Max Horz 1=290(LC 12)

Max Uplift All uplift 100 lb or less at joint(s) 7, 1 except 9=-176(LC 12), 10=-174(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 7, 1 except 8=516(LC 19), 9=545(LC 19), 10=459(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 1-2=-348/232

WEBS 4-9=-284/218. 2-10=-265/201

NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Interior(1) 5-1-13 to 7-3-0, Exterior(2R) 7-3-0 to 12-0-10, Exterior(2E) 12-0-10 to 15-10-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 1 except (jt=lb) 9=176, 10=174.

LOAD CASE(S) Standard



Job	Truss	Tr	russ Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDG	E   81 PINON DRIVE AN	IGIER, NC
25-3580-R01	VT02	Va	alley	1	1	Job Reference (optional)	#	58814
			<u> </u>	ID:R4JAOCFxLoAl	JFpwqRU 14-10-1( 4-0-0	8.630 s Jul 12 2024 MiTek Ind TCbAz_Vw6-q2dag5xu_fMc )	dustries, Inc. Thu Apr 24 94KuQEcdXf4o91vQG	14:17:11 2025 Page 1 u4fxsCCk4mzNV1M
				4x4 =	=			Scale = 1:65.6
			$\begin{array}{c} 12.00 \overline{12}  2x4 \\ 12.00 \overline{12}  2x4 \\ 1 \\ 2x4 \\ 1 \\ 2x4 \\ 3x4 \\ 9 \\ 2x4 \\ 9 \\ 2x4 \\ 1 \\ 2x4 \\ 1 \\ 2x4 \\ 1 \\ 2x4 \\ 2x4 \\ 1 \\ 1 \\ 2x4 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ $	4 5 5 5 5 5 5 5 5 5 5 5 5 5		3x4    5 W1 01-01-0 W1 6 3x4		
			14	-10-10				
TCLL (roof) Snow (Pf) TCDL BCLL	20.0 20.0 10.0	SPACING-2-0Plate Grip DOL1Lumber DOL1Rep Stress IncrY	0-0 <b>CSI.</b> .25 TC 0.21 .25 BC 0.42 ES WB 0.24	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (lo n/a n/a 0.00	oc) l/defl L/d - n/a 999 - n/a 999 6 n/a n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL	10.0	Code IRC2021/TPI20	014 Matrix-SH				Weight: 91 lb	FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS	2x4 SP No.2 2x4 SP No.3 2x4 SP No.3 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD WEBS	Structura end vert Rigid ce 1 Row a	al wood sheathing directly icals. iling directly applied or 10 t midpt 4-7	y applied or 6-0-0 oc )-0-0 oc bracing.	purlins, except
					MiTek be inst Installa	recommends that Stabiliz alled during truss erection tion guide.	zers and required cro n, in accordance with	oss bracing h Stabilizer
REACTIONS. (lb) -	All bearings 14- Max Horz 1=255( Max Uplift All up Max Grav All rea	.10-10. (LC 12) lift 100 lb or less at join actions 250 lb or less a	nt(s) 1, 6 except 8=-183(LC 12), 9 t joint(s) 1, 6 except 7=512(LC 19	=-145(LC 12) ), 8=548(LC 19), 9	9=348(LC	19)		
FORCES. (Ib)	- Max. Comp./Ma	ax. Ten All forces 250	) (Ib) or less except when shown.					

#### WEBS 3-8=-292/229

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Interior(1) 5-1-13 to 6-1-0, Exterior(2R) 6-1-0 to 10-10-10, Exterior(2E) 10-10-10 to 14-8-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6 except (jt=lb) 8=183, 9=145.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LOT 0.0024 CAMPBELL RIDO	GE   81 PINON DRIVE ANGIER, NC
25-3580-R01	VT03	Valley	1	1	lob Reference (ontional)	# 58814
			ID:R4JAOCFxLoA	UFpwaRU	8.630 s Jul 12 2024 MiTek In JTCbAz Vw6-g2dag5xu fM	ndustries, Inc. Thu Apr 24 14:17:11 2025 Page 1 104KuQEcdXf4o9?vQ1u5rxsCCk4mzNV1N
		9-8-10		13-8-10		
				100		Scale = 1:58 7
			4x4 =			00010 - 1.00.7
		$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4 8 8 8 8 7 8 7 8 8 8 7 8 8 8 7 8 8 8 7 8 8 8 7 8 7 8 8 7 8 8 7 8	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	3x4    5 W1 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	
LOADING (psf)	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in (le	oc) l/defl l/d	PLATES GRIP
TCLL (roof) 20.0 Snow (Pf) 20.0	Plate Grip DOL	1.25 TC 0.21	Vert(LL)	n/a	- n/a 999	MT20 244/190
TCDL 10.0 BCLL 0.0 *	Rep Stress Incr	YES WB 0.16	Horz(CT)	0.00	6 n/a n/a	Weight: 90 lb $ET = 20\%$
BCDL10.0LUMBER- TOP CHORD2x4 SP No SP No WEBS2x4 SP No SP No OTHERSOTHERS2x4 SP No SP No S	0.2 0.3 0.3	Т2014   Maunx-SП	BRACING- TOP CHORD BOT CHORD WEBS	Structur end vert Rigid ce 1 Row a	al wood sheathing direct icals. iling directly applied or 1 t midpt 4-7	ly applied or 6-0-0 oc purlins, except 0-0-0 oc bracing.
				MiTek be inst Installa	recommends that Stabili alled during truss erection ation guide.	zers and required cross bracing on, in accordance with Stabilizer
REACTIONS. All beari (lb) - Max Horz Max Uplif Max Grav	ngs 13-8-10. 1=219(LC 12) t All uplift 100 lb or less at All reactions 250 lb or les	joint(s) 6 except 1=-136(LC 10), 8=- ss at joint(s) 1, 6 except 7=513(LC 1	185(LC 12), 9=-13 9), 8=523(LC 19), 9	1(LC 12) =285(LC	: 19)	
FORCES. (lb) - Max. Co	mp./Max. Ten All forces	250 (lb) or less except when shown.				

#### WEBS 3-8=-293/244

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 9-8-10, Exterior(2E) 9-8-10 to 13-6-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 1=136, 8=185, 9=131.

LOAD CASE(S) Standard





- Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=500(LC 19), 7=504(LC 19)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown. 2-7=-310/255

WEBS

NOTES-(8)

- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 8-6-10, Exterior(2E) 8-6-10 to 12-4-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 6) between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 7=206.

LOAD CASE(S) Standard





Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 7=-177(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=497(LC 19), 7=410(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. 2-7=-271/232 WFBS

NOTES-(8)

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 7-4-10, Exterior(2E) 7-4-10 to 11-2-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 7=177

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LOAD CASE(S) Standard
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Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 6 except 7=-162(LC 12) Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 6=461(LC 19), 7=349(LC 19)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

WEBS 2-7=-254/225

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-5-8, Exterior(2E) 6-5-8 to 10-3-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 6 except (jt=lb) 7=162.

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LOAD CASE(S) Standard
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REACTIONS. All bearings 9-3-8.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

(10) NOTES-

LOAD CASE(S) Standard

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 4-1-8, Exterior(2R) 4-1-8 to 6-5-8, Exterior(2E) 6-5-8 to 9-1-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Provide adequate drainage to prevent water ponding
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 10, 9 except (jt=lb) 8=107.



<sup>(</sup>lb) - Max Horz 1=72(LC 9) Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 10, 9 except 8=-107(LC 13) Max Grav All reactions 250 lb or less at joint(s) 1, 7, 8, 9 except 10=330(LC 19)