Mark Morris, P.E.

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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 58836 JOB: 25-3559-R01 JOB NAME: LOT 0.0002 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *43 Truss Design(s)*

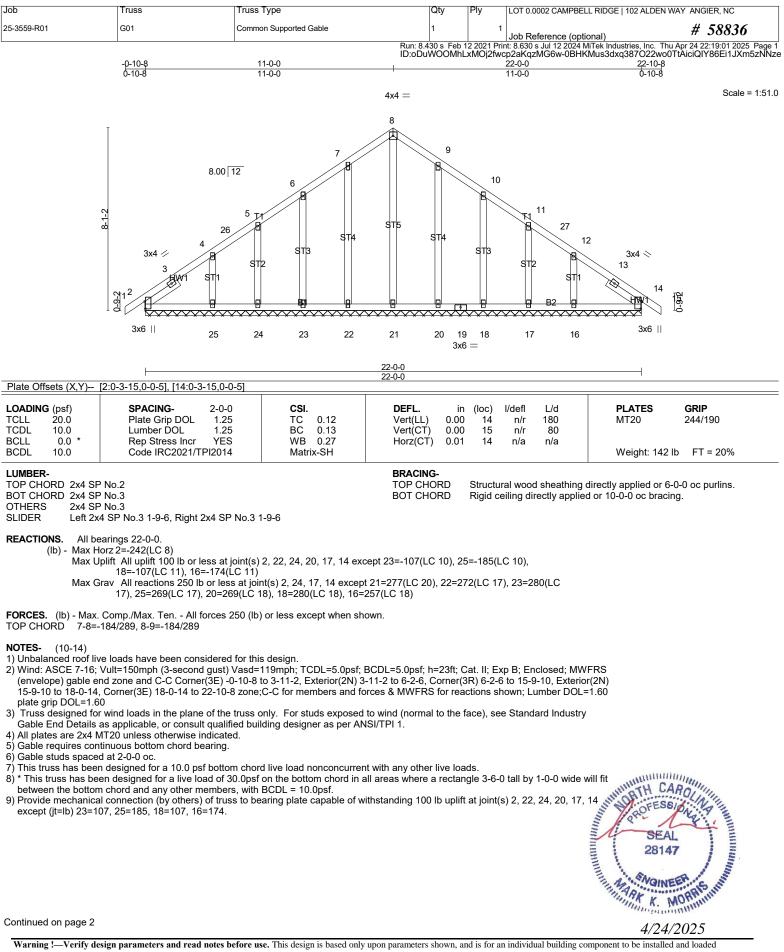
Trusses:

G01, G02, J01, J01A, J02, J04, J05, J06, J08, J09, J10, J11, J12, P02, R01, R02, R05, R06, R06A, R07, R08, R09, R10, R11, R12, R13, R14, R14A, R15, R16, R17, V01, V02, V03, V04, V05, V06, V07, V08, V09, V10, V11, V12



My license renewal date for the state of North Carolina is 12/31/2025

Warning !--- Verify design parameters and read notes before use.



vertically. Applicability of design parameters and read notes before use. This begin is obsed only upon parameters shown, and is for an individual building component to be instanted and toaded vertically. Applicability of design parameters and read notes before use. This essensibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN	WAY ANGIER, NC
25-3559-R01	G01	Common Supported Gable	1	1	Job Reference (optional)	# 58836
		·			nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. The 2aKqzMG6w-UNriZEsiOFywmHzFceJF04	

10) Trusses designed with 2018 IRC also comply with 2015 IRC.

11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

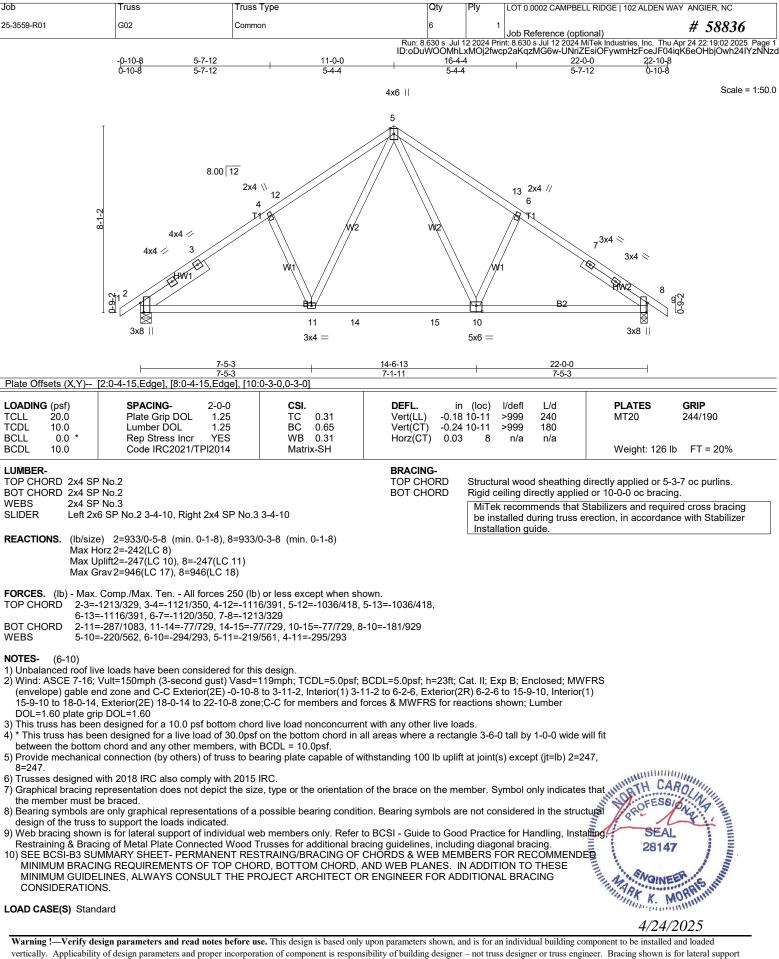
12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

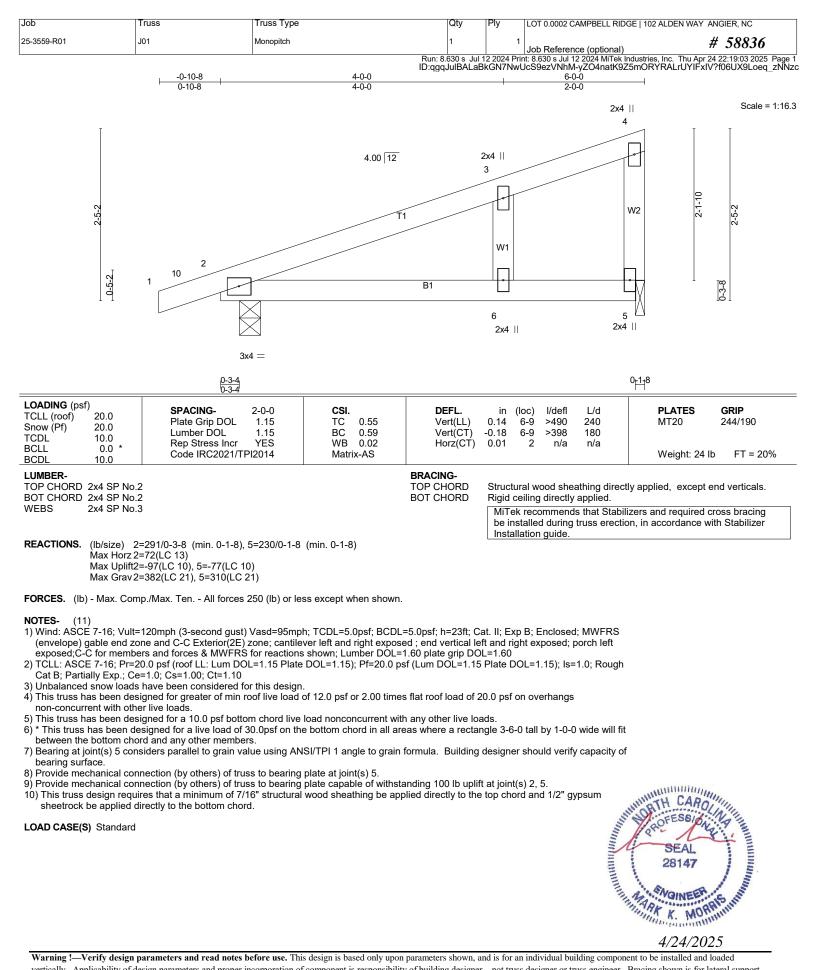
 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

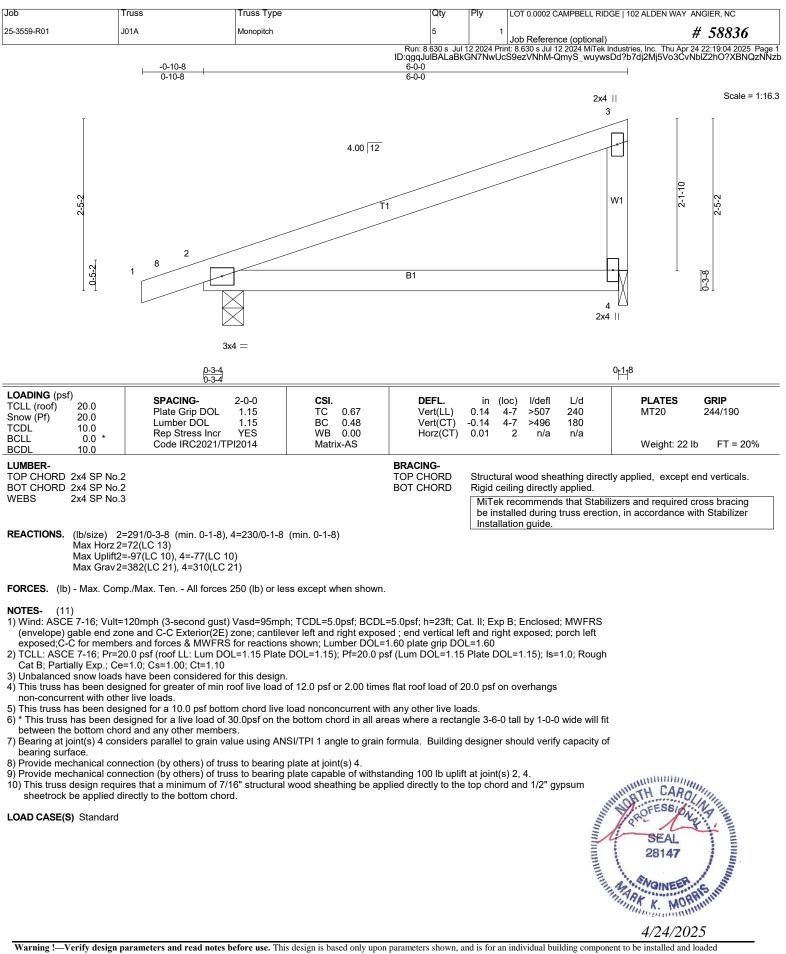
4) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

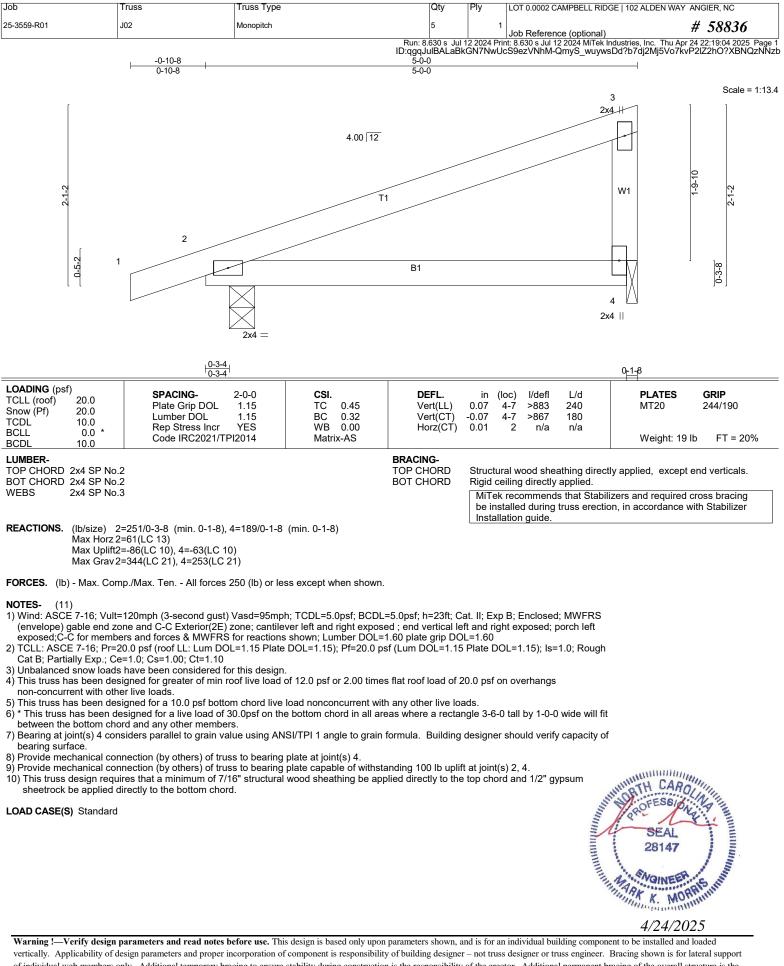
LOAD CASE(S) Standard



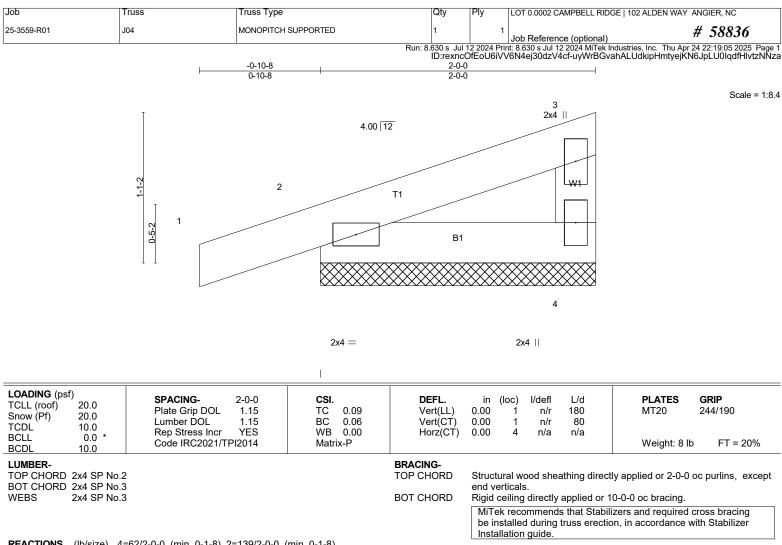








vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



REACTIONS. (lb/size) 4=62/2-0-0 (min. 0-1-8), 2=139/2-0-0 (min. 0-1-8) Max Horz 2=27(LC 11) Max Uplift4=-9(LC 14), 2=-41(LC 10) Max Grav 4=76(LC 21), 2=180(LC 21)

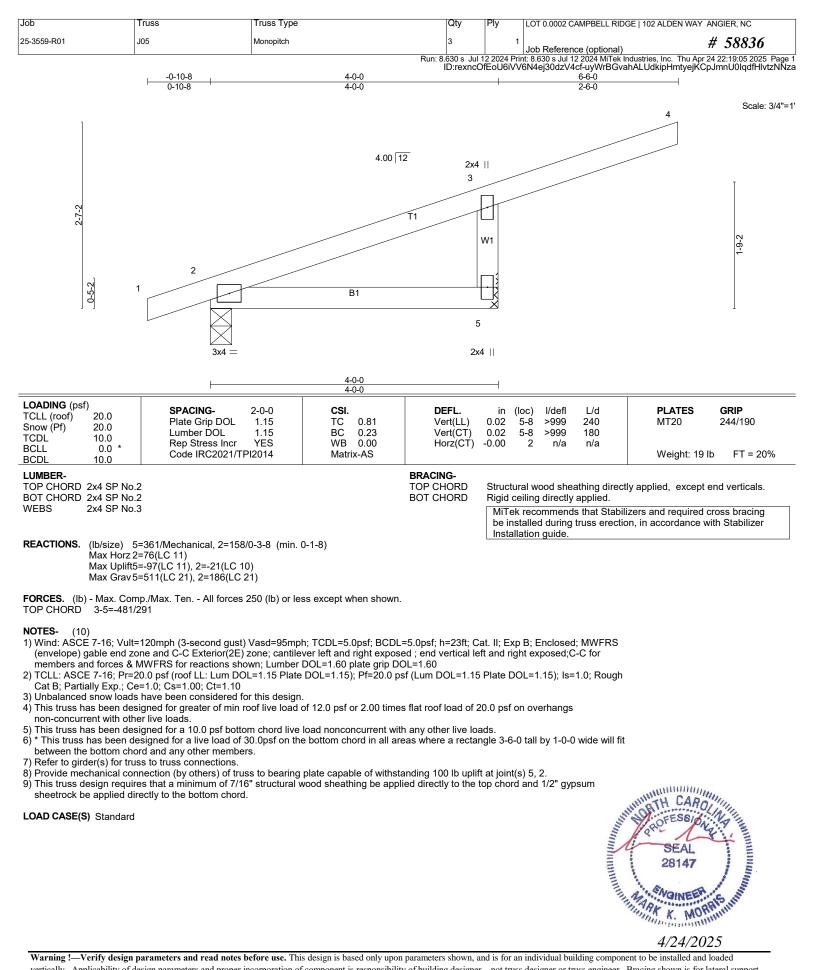
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

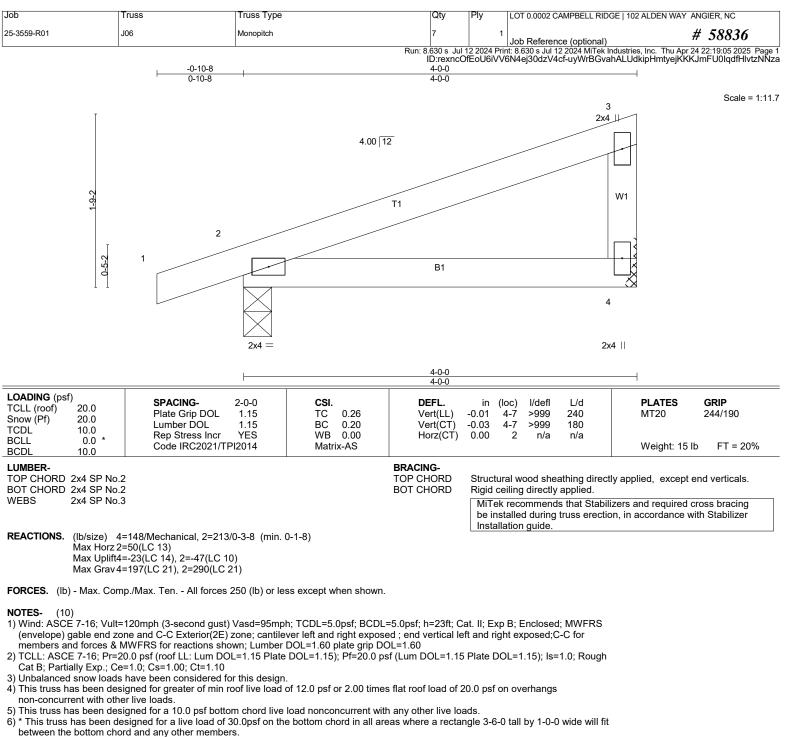
NOTES- (11)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry
- Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard



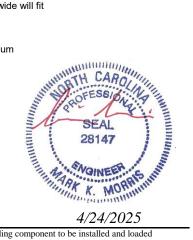




- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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LOAD CASE(S) Standard
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Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE	E 102 ALDEN WAY ANGIER, NC
25-3559-R01	J08	Jack-Open	10		1 Job Reference (optional)	# 58836
			Run: 8.630 s Jul ID:qgqJulBALaE 0-10-0 0-10-0	 12 2024 F 3kGN7N\ -	rint: 8.630 s Jul 12 2024 MiTek Ind wUcS9ezVNhM-M84DPbvCSU	lustries, Inc. Thu Apr 24 22:19:06 2025 Page 1 JTLFuH0rTOBAwtXoj9jDTY_rJ0IRJzNNzZ
						Scale = 1:14.2
		8.00 12 2x	3 4 2 T1□			
		2-2-2	W1			
			B1			
			5 2x4 4	-1		
LOADING (psf) TCLL (roof) 20. Snow (Pf) 20. TCDL 10. BCLL 0. BCDL 10.	0 Lumber DOL 0 Rep Stress Incr 0 * Code IBC2021/	2-0-0 CSI. 1.15 TC 0.15 1.15 BC 0.05 YES WB 0.00 'PI2014 Matrix-MR	DEFL. Vert(LL) Vert(CT) Horz(CT)	in -0.00 -0.00 -0.00	(loc) l/defi L/d 5 >999 240 5 >999 180 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 6 lb FT = 20%
LUMBER- TOP CHORD 2x4 BOT CHORD 2x4	SP No.2	, ,	BRACING- TOP CHORD BOT CHORD	end ve Rigid o MiTe be in	erticals. ceiling directly applied or 10 k recommends that Stabiliz stalled during truss erection	applied or 0-10-0 oc purlins, except -0-0 oc bracing. ers and required cross bracing n, in accordance with Stabilizer
	ize) 5=141/0-3-0 (min. 0-1-8 Horz 5=39(LC 9)	, 3=-22/Mechanical, 4=-1/Mechanica	al	Insta	llation guide.	

REACTIONS. (lb/size) 5=141/0-3-0 (min. 0-1-8), 3=-22/Mechanical, 4=-1/Mechanical Max Horz 5=39(LC 9) Max Uplift3=-63(LC 18), 4=-29(LC 9) Max Grav 5=203(LC 18), 3=12(LC 10), 4=33(LC 10)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

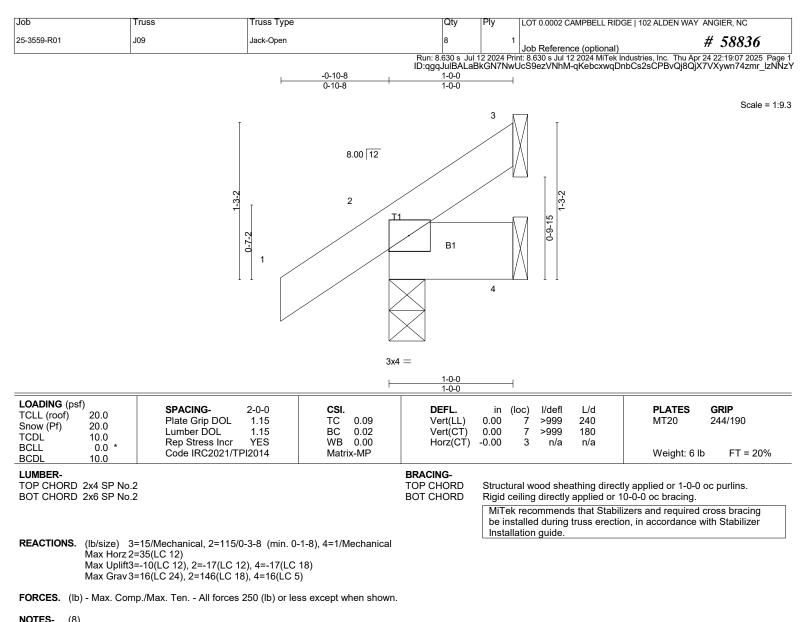
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

LOAD CASE(S) Standard



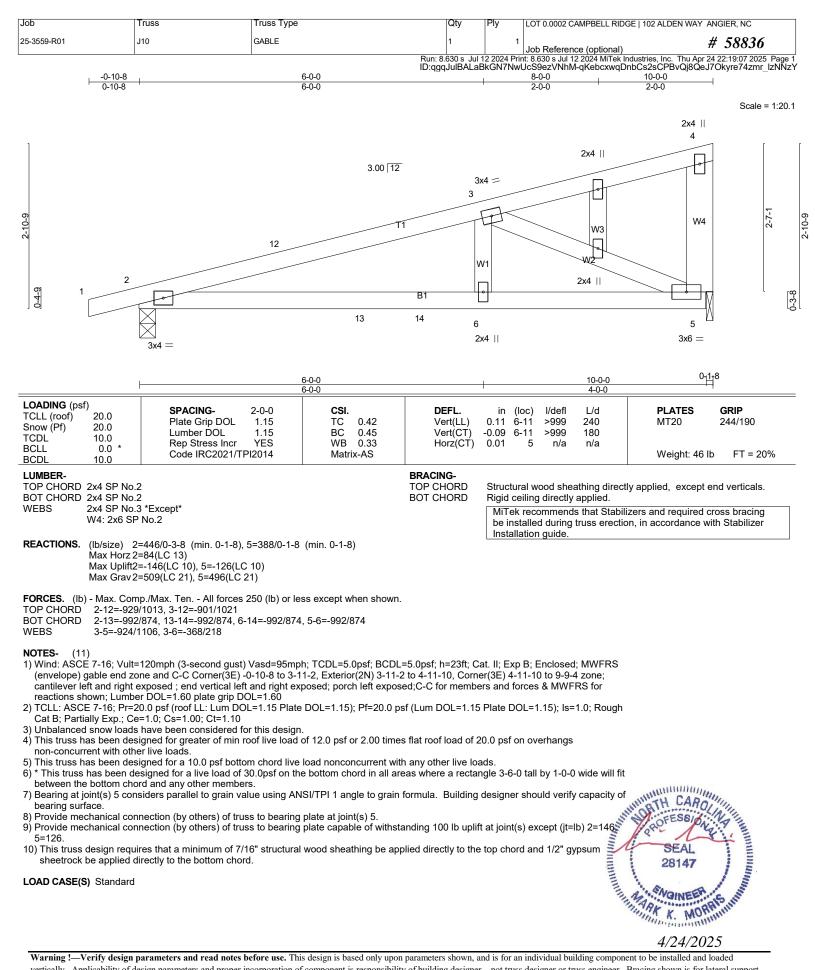


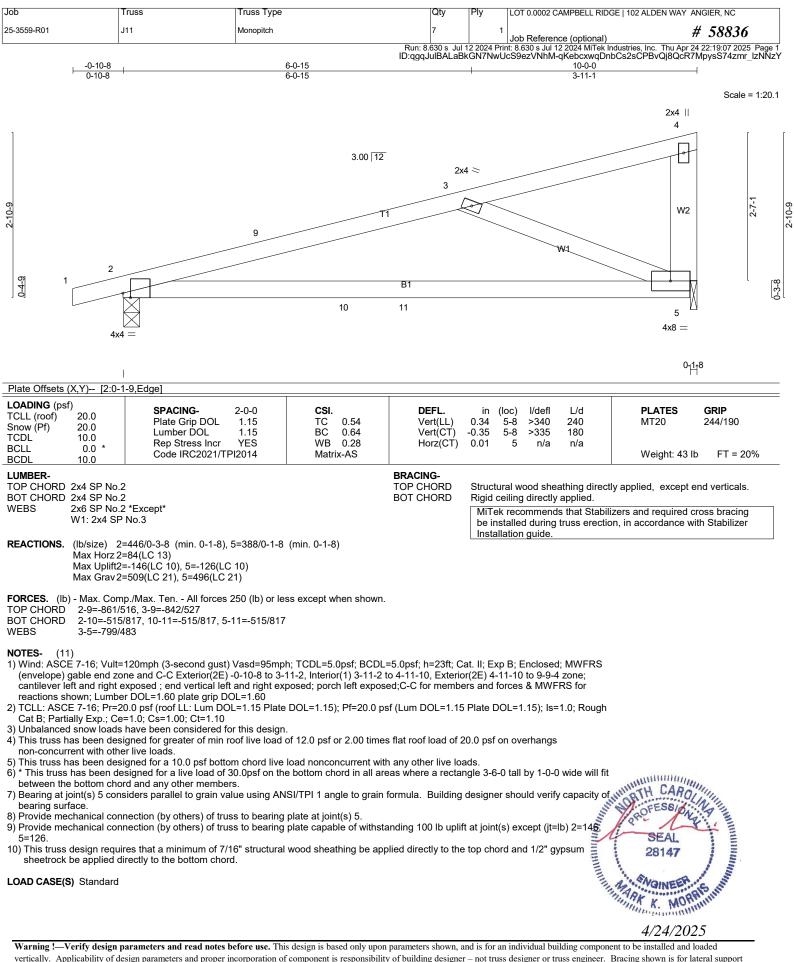
NOTES-

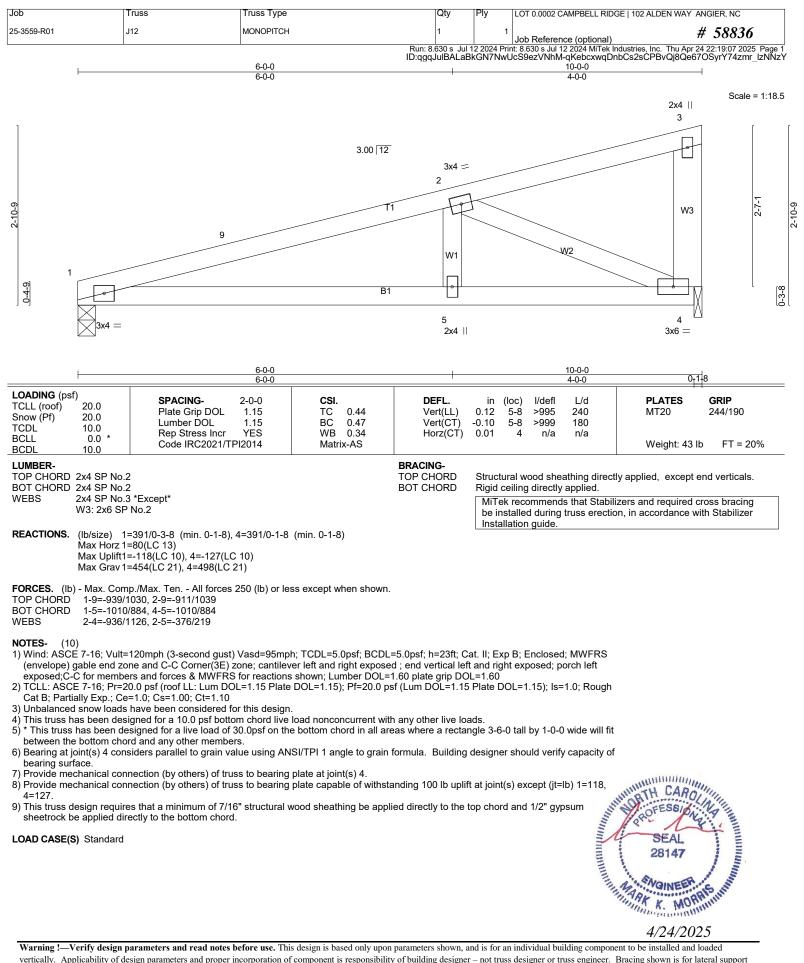
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

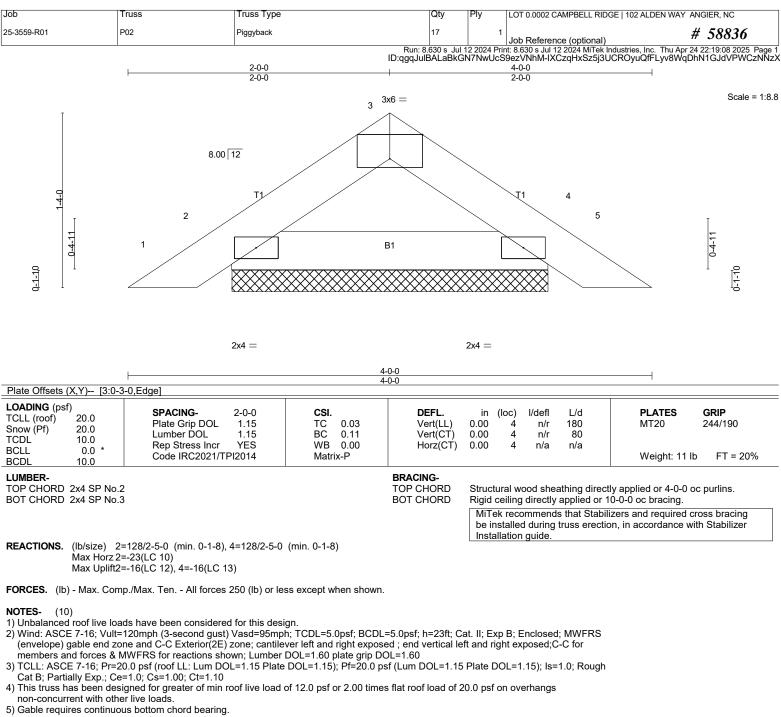
LOAD CASE(S) Standard









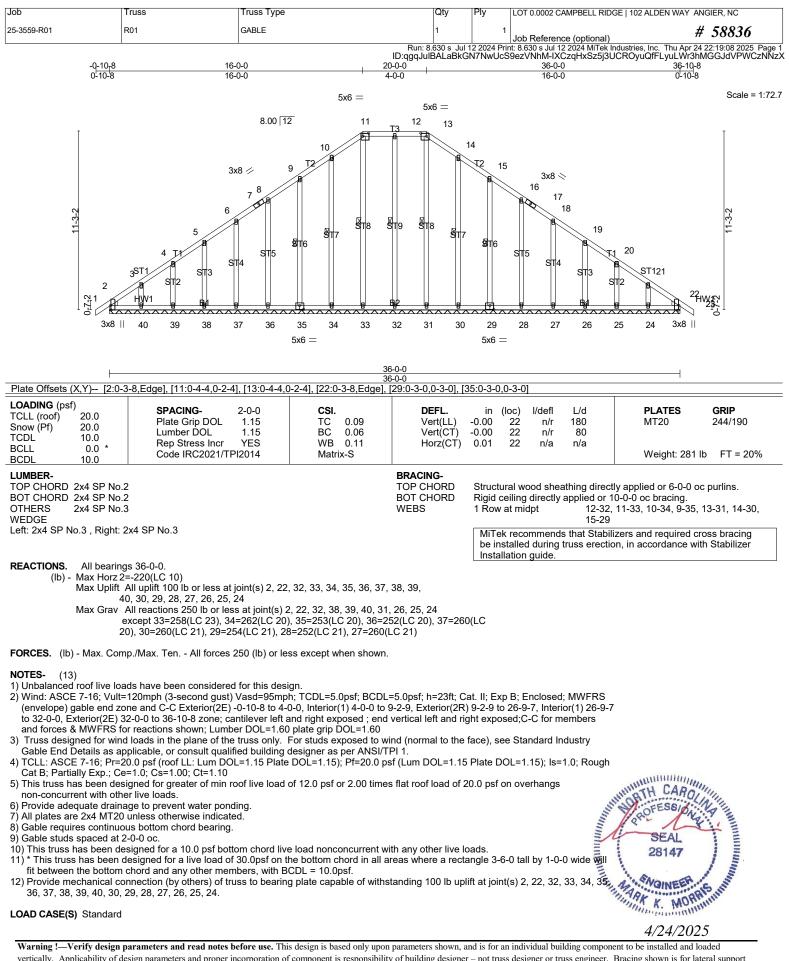


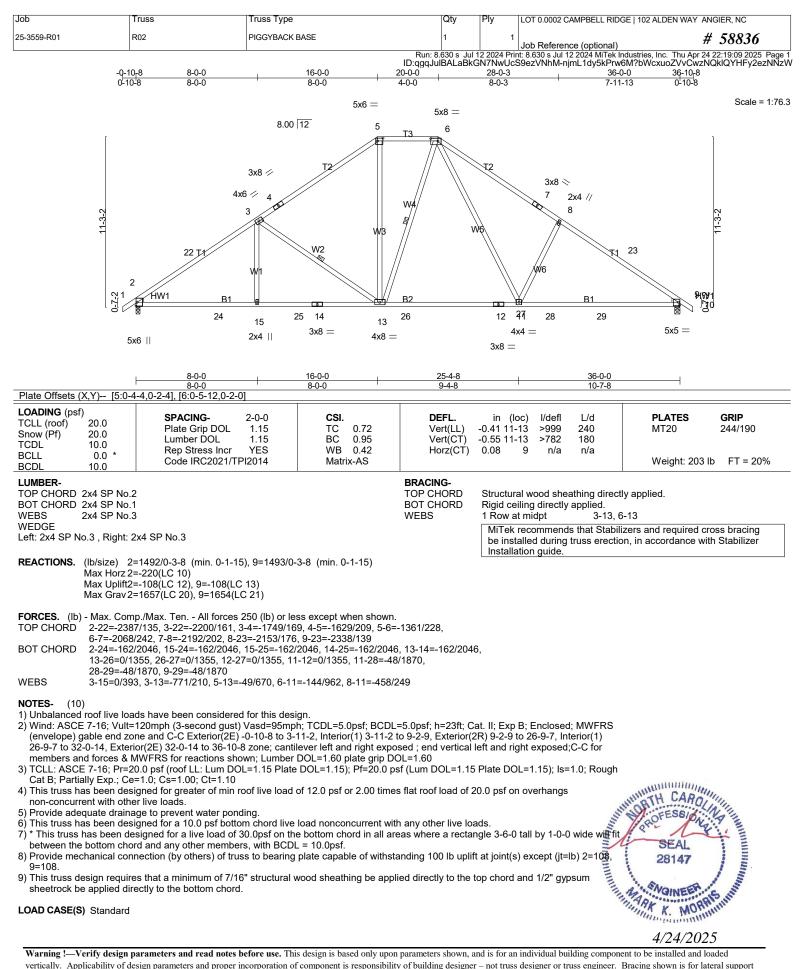
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

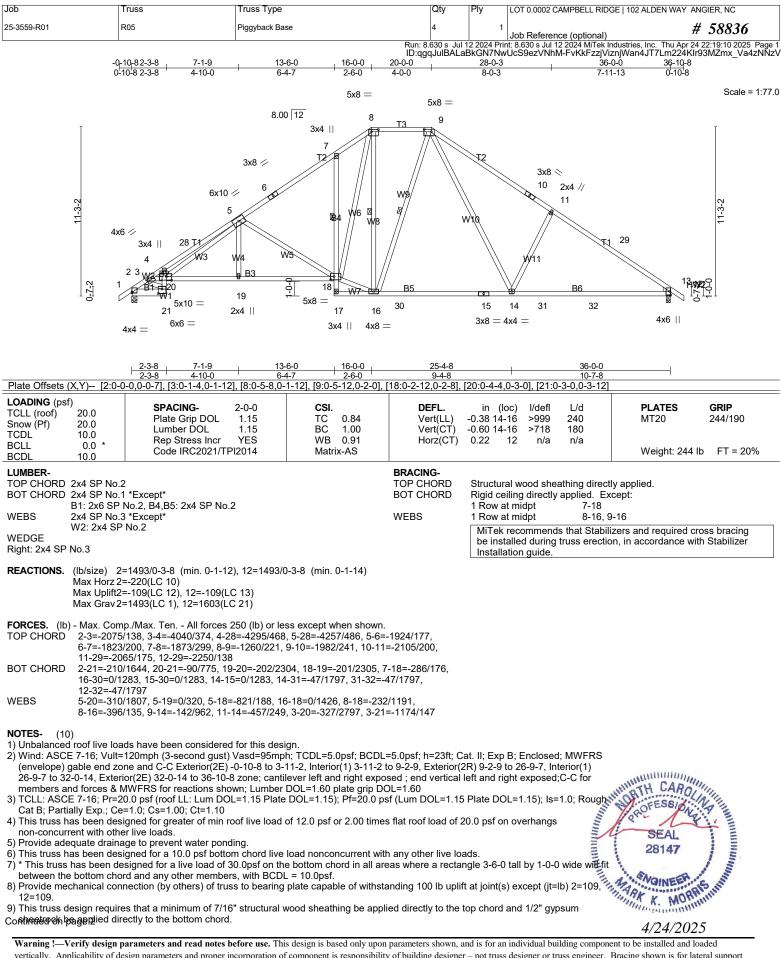
7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- 9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- LOAD CASE(S) Standard





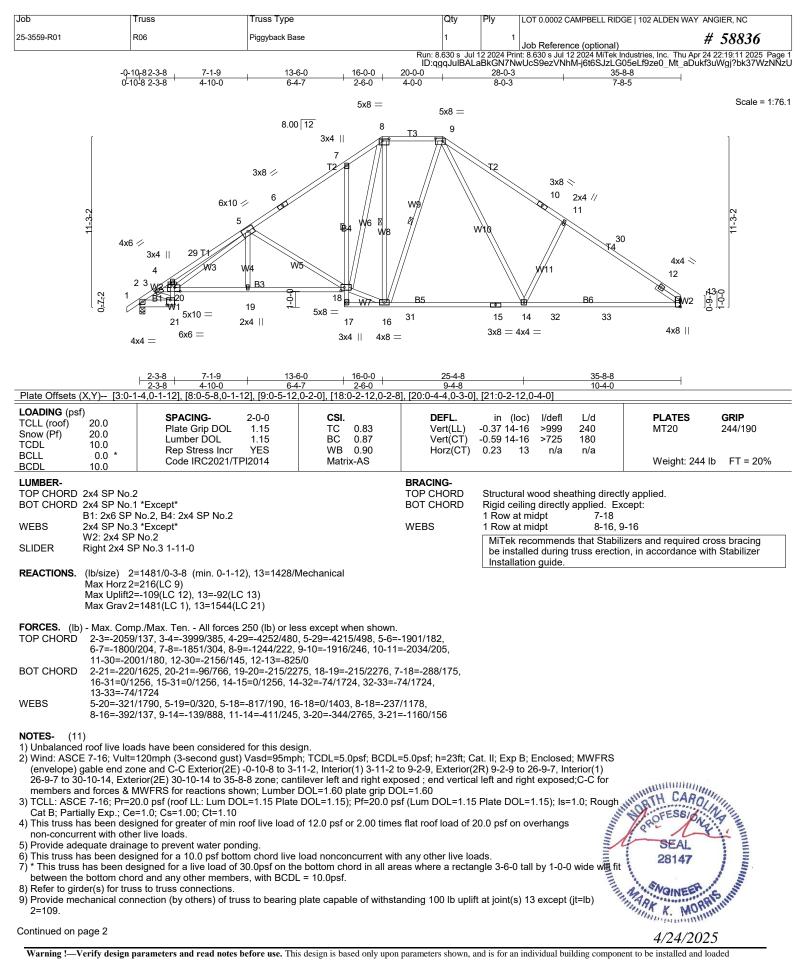




Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN W	AY ANGIER, NC
25-3559-R01	R05	Piggyback Base	4	1	Job Reference (optional)	# 58836
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu /UcS9ezVNhM-FvKkFzzjViznjWan4JT7Lm2	

LOAD CASE(S) Standard



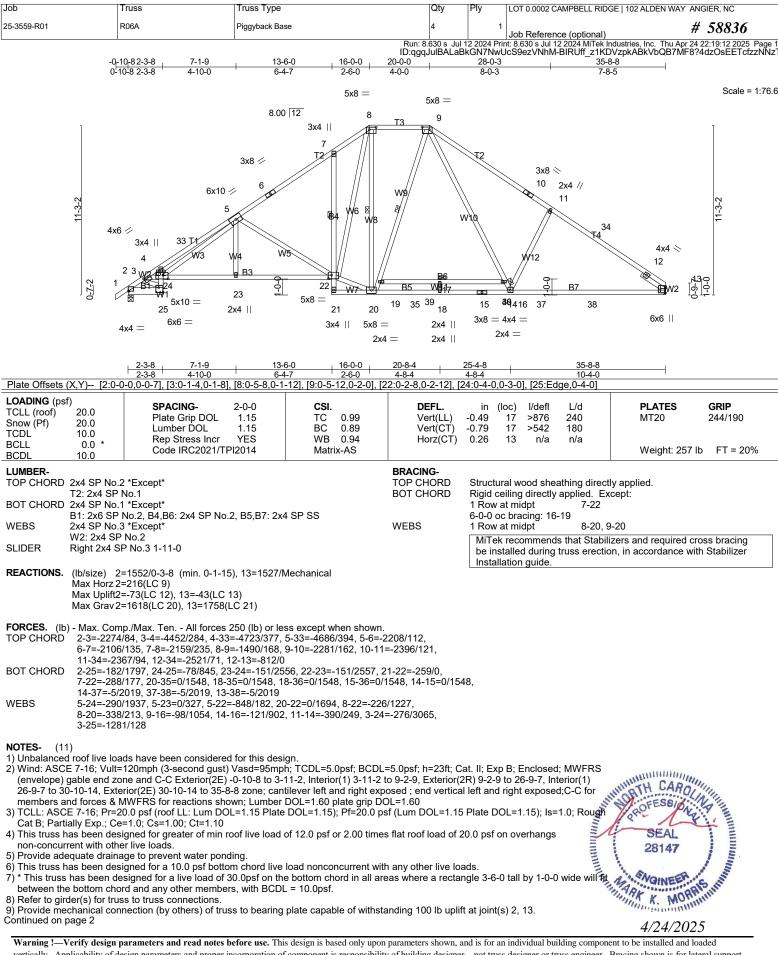


[Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN WAY ANGIER, NC
	25-3559-R01	R06	Piggyback Base	1	1	Job Reference (optional) # 58836
						nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Apr 24 22:19:11 2025 Page 2vucS9ezVNhM-j6t6SJzLG05eLf9ze0 Mt aDukf3uWgj?bk37WzNNzU

NOTES- (11)
 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



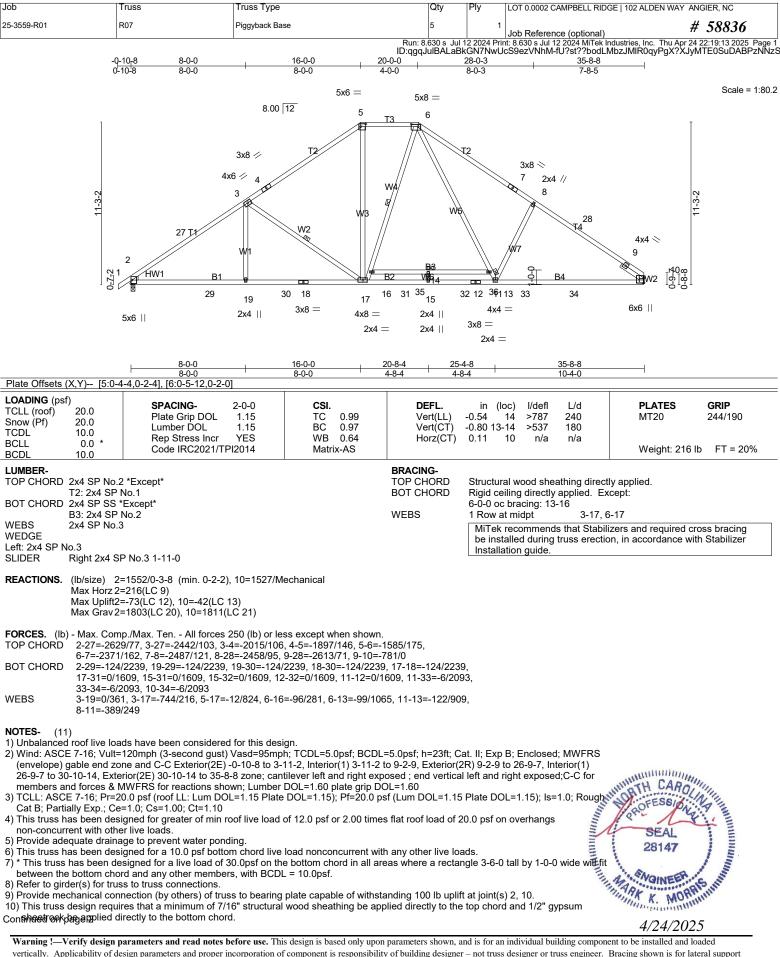


Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN WAY	ANGIER, NC
25-3559-R01	R06A	Piggyback Base	4	1	Job Reference (optional)	# 58836
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Apr : JcS9ezVNhM-BIRUff_z1KDVzpkABkVbQB7MF	

NOTES- (11) 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

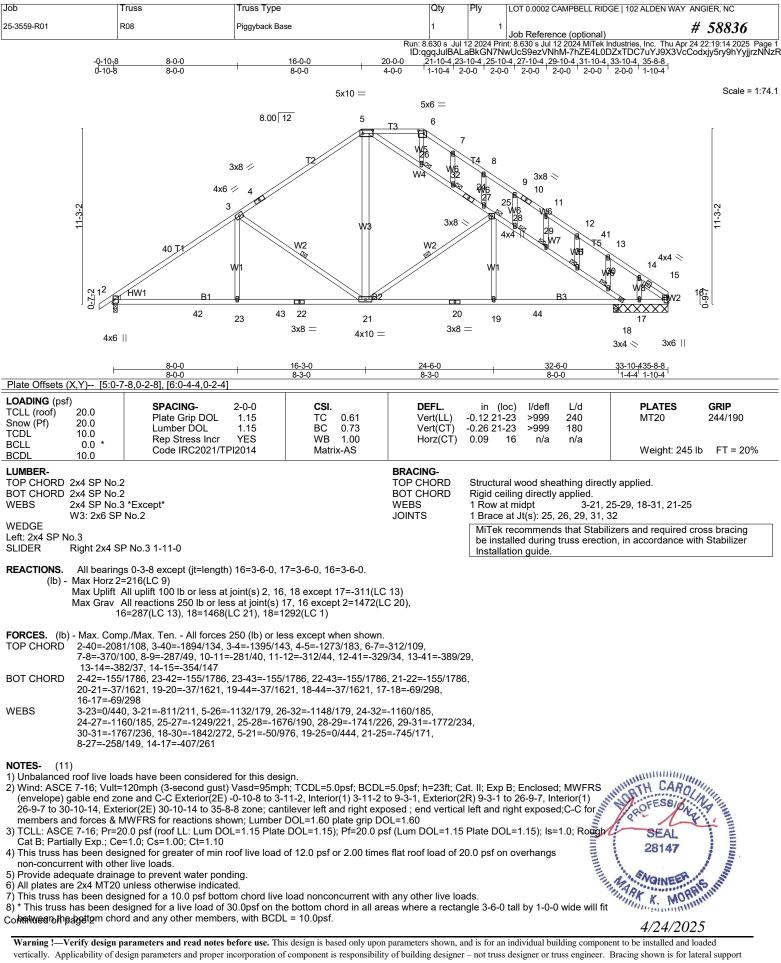




Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN WAY	ANGIER, NC
25-3559-R01	R07	Piggyback Base	5	1	Job Reference (optional)	# 58836
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Ap cS9ezVNhM-fU?st??bodLMbzJMIR0qyPgX??	

LOAD CASE(S) Standard





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEI	N WAY ANGIER, NC
25-3559-R01	R08	Piggyback Base	1	1	Job Reference (optional)	# 58836
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. wUcS9ezVNhM-7hZE4L0DZxTDC7uYJ	

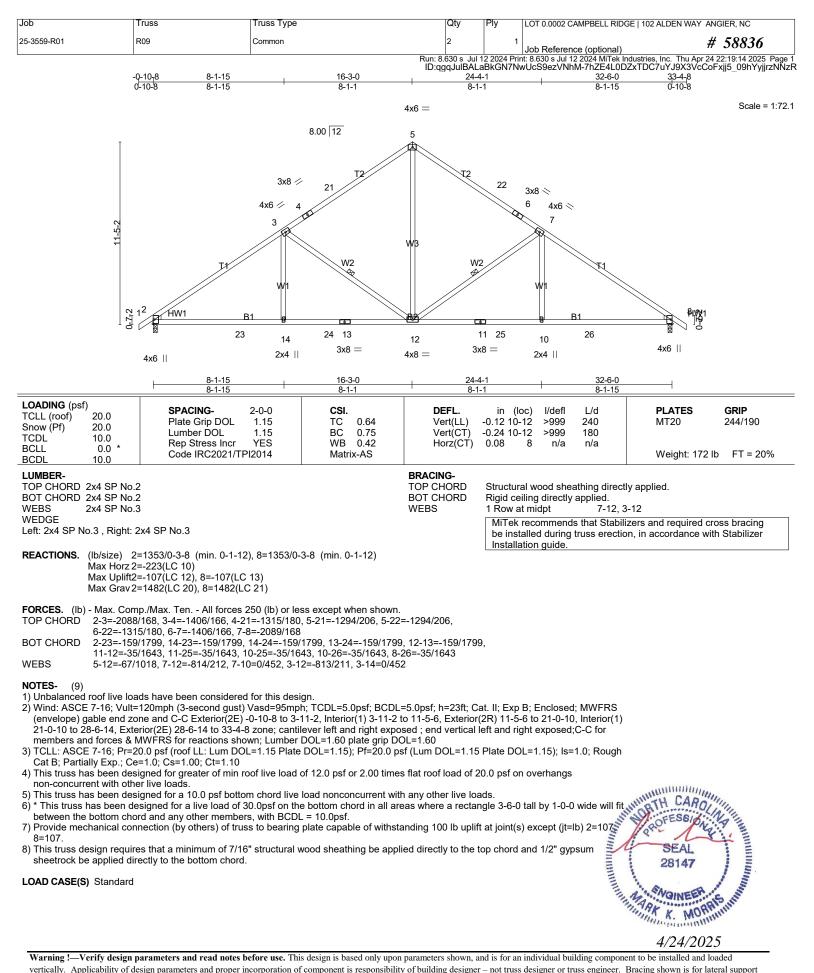
NOTES- (11)

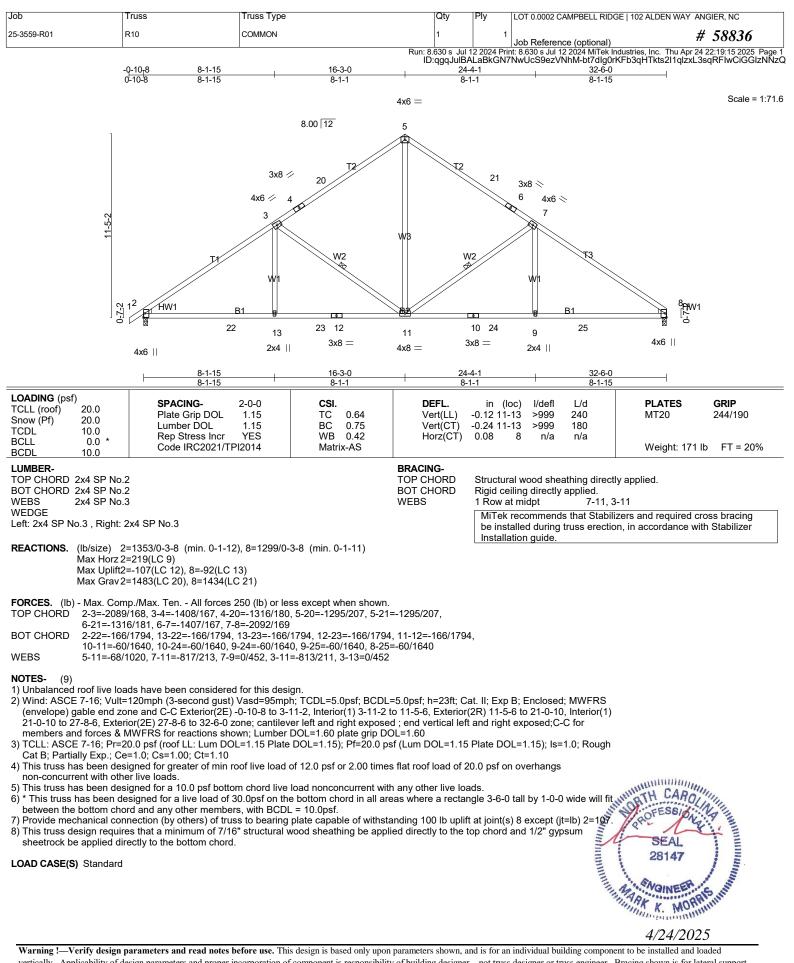
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 18, 16 except (it=lb) 17=311.

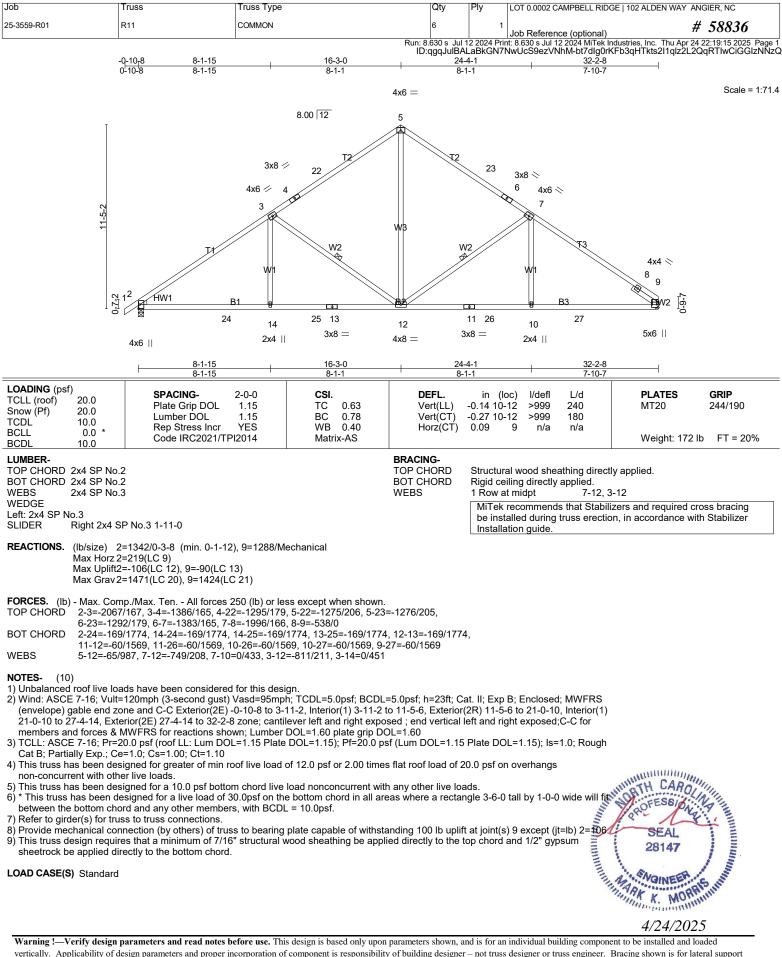
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

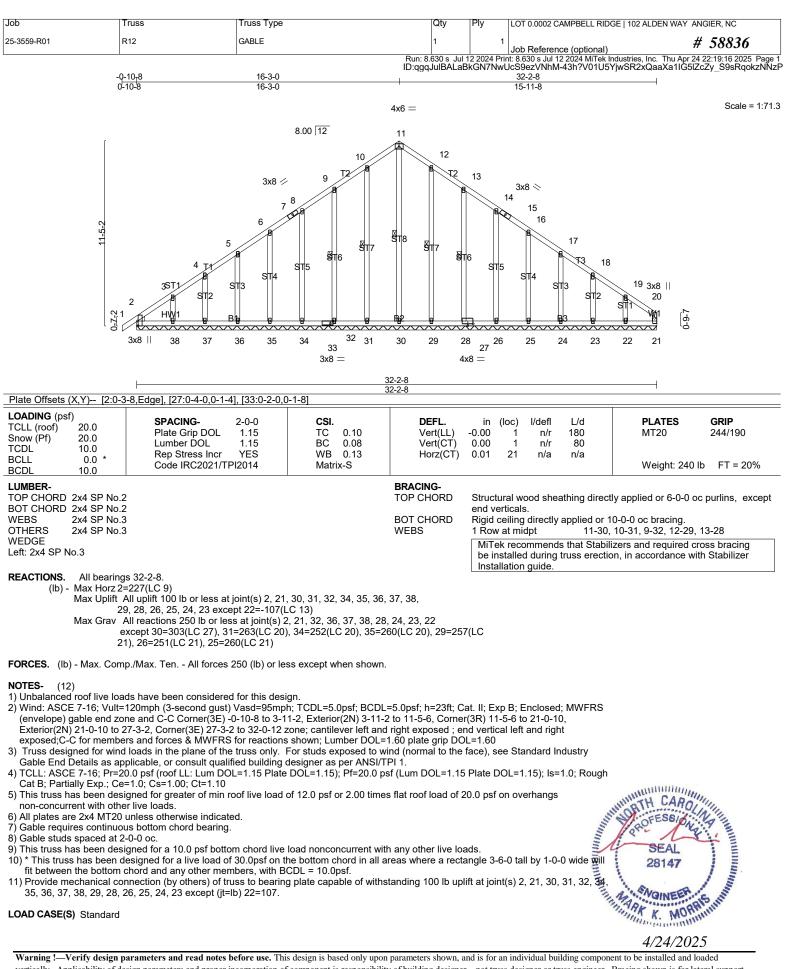
LOAD CASE(S) Standard











35, 36, 37, 38, 29, 28, 26, 25, 24, 23 except (jt=lb) 22=107.

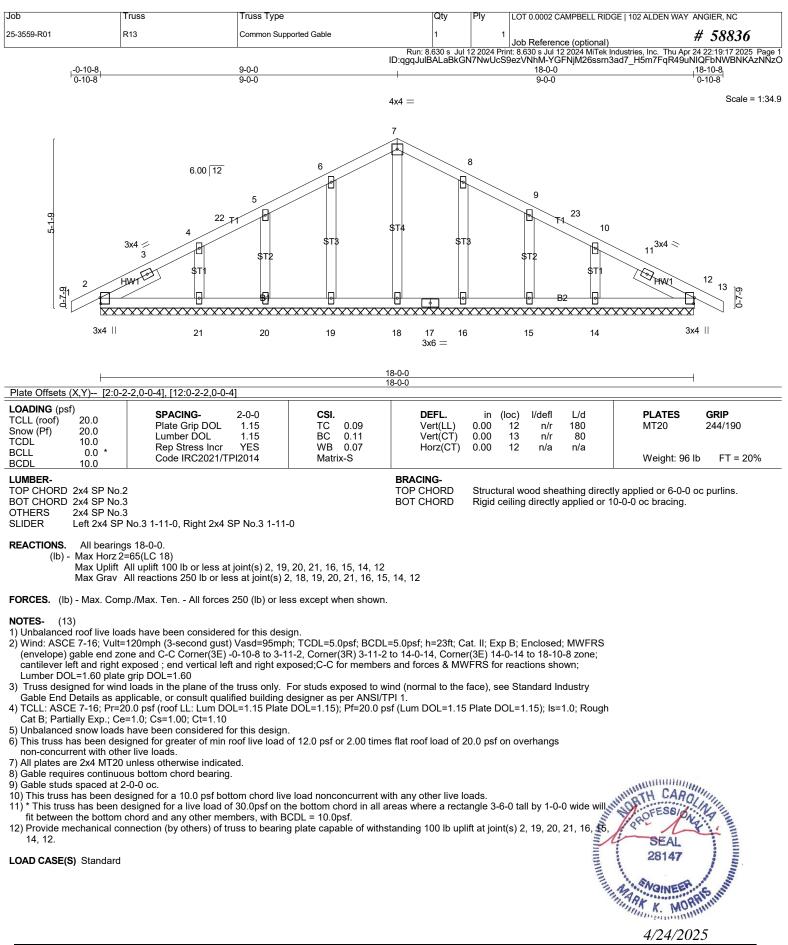
LOAD CASE(S) Standard

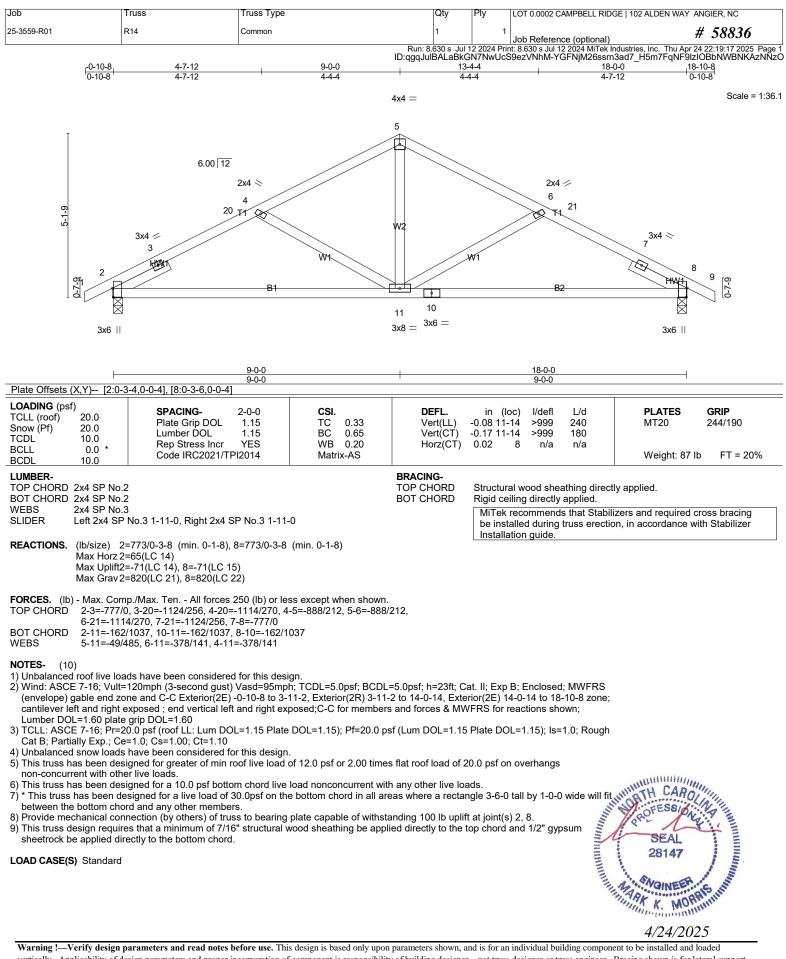
Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

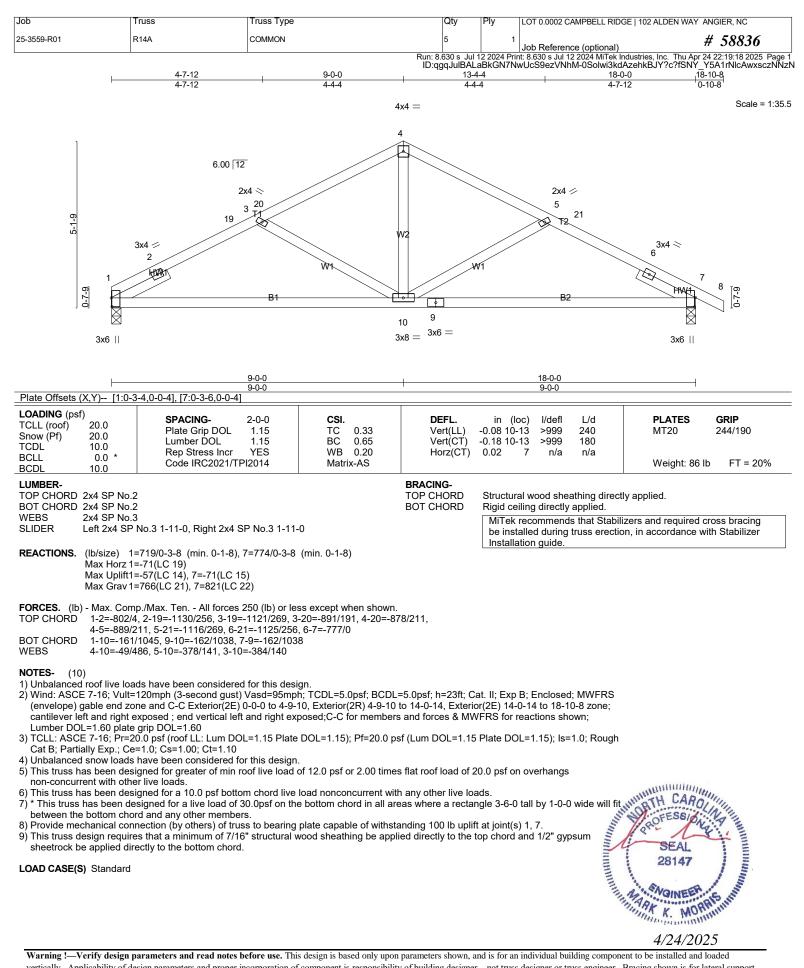
NOINEE

K. MORR

4/24/2025







25-3559-R01	R15	COMMON GIRDER	Qty 1	3 Job Referen		2 ALDEN WAY ANGIER, NC # 58836
	1	1	Bup: 9.620 a Jul 1		ice (opilonal)	
			ID:gggJulBALaBkGN	2 2024 Print: 8.630 s Jul 1 7NwUcS9ezVNhM-Ue	2 2024 MiTek Industri M7824MOT6VJum	es, Inc. Thu Apr 24 22:19:19 2025 Pag W6i7ECgvd5yUAm7qurqgUP3zNN
		0-2-8 3-3-0 5-3-0 7-3-0 9	9-3-0 11-3-0 13-3-0 2-0-0 2-0-0 2-0-0	15-3-0 18-3-8 18 ₁ 6-	0	
			4x6			Scale = 1:7
			10			
	11-2-10 9 9		¹³ 9 W4 29 23 22 30	14 14 17 6x10 17 6x10 19 16 180 19 10 10 10 10 10 10 10 10 10 10	<u>1-11-10</u> 2-2-2	
		5x6 = 8x8 =	4x6 = 7x8 =	8x8 = 5x6	=	
		4-8-6 9-3-0	13-9-10	18-6-0		
Plate Offsets (X,Y) [1:0	-6-8,0-1-7], [19:0-6-8,0-1-7]	<u> </u>	<u>4-6-10</u> , [24:0-3-8,0-4-8]	4-8-6		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TPI	2-0-0 CSI. 1.15 TC 0.76 1.15 BC 0.47 NO WB 0.96 I2014 Matrix-MSH	Vert(CT)	in (loc) l/defl -0.06 21-23 >999 -0.12 21-23 >999 0.02 20 n/a	L/d 240 180 n/a	PLATES MT20 GRIP 244/190 Weight: 649 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No	2 *Except*		BRACING- TOP CHORD	Structural wood she	athing directly apr	plied or 6-0-0 oc purlins, excep
T2,T3: 2x6 BOT CHORD 2x6 SP DS WEBS 2x4 SP No	SP No.2 S		BOT CHORD JOINTS	end verticals. Rigid ceiling directly 1 Brace at Jt(s): 11,	applied or 10-0-0	
Max Horz Max Uplift	25=8158/0-3-8 (min. 0-2-1) 25=226(LC 41) 25=-511(LC 11), 20=-414(L 25=9063(LC 3), 20=7682(L					
TOP CHORD 1-2=-531 12-14=-5 1-3=-401 9-11=-16 18-19=-4	4/264, 2-5=-5106/314, 5-7= 114/367, 14-17=-5107/317, 2/495, 3-4=-4012/495, 4-6= 35/120, 11-13=-1635/120, 176/503	50 (lb) or less except when shown. 5115/365, 7-10=-4957/399, 10-12 17-19=-5314/266, 1-25=-6685/38 1635/120, 6-8=-1635/120, 8-9=-1 13-15=-1635/120, 15-16=-1635/120	=-4957/401, 3, 19-20=-6766/388 635/120, 0, 16-18=-4176/503	· ,		
23-29=-6	17/7553, 22-23=-547/7755,	24-27=-261/1332, 24-28=-617/755 22-30=-547/7755, 21-30=-547/775	,	,		
WEBS 11-23=-3		. 16-21=-162/1522, 4-23=-2557/41 ⁻ 17-18=-145/253, 10-11=-498/6663,				
Top chords connected Bottom chords connect Webs connected as fol 2) All loads are considere	ted as follows: 2x6 - 2 rows lows: 2x4 - 1 row at 0-9-0 o d equally applied to all plies	-9-0 oć, 2x6 - 2 rows staggered at 0 staggered at 0-4-0 oc.	ck (B) face in the LC	DAD CASE(S) section t. II; Exp B; Enclosed r DOL=1.60 plate gri Plate DOL=1.15); Is= ds. gle 3-6-0 tall by 1-0-0 ilding designer shoul	n. Ply to ply d; MWFRS p DOL=1.60 1.0; Rough 0 wide will fit	SEAL 28147

Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN WAY ANGI	ER, NC
25-3559-R01	R15	COMMON GIRDER	1	3	Job Reference (optional) #	58836
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Apr 24 22: DezVNhM-UeM7824MOT6VJumW6i7ECgvd5yUAm7	

NOTES- (14)

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 25=511, 20=414.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 27 lb down and 60 lb up at 0-4-11, and 27 lb down and 60 lb up at 18-1-5 on top chord, and 1512 lb down and 104 lb up at 0-1-12, 29 lb down and 31 lb up at 2-3-4, 1717 lb down and 63 lb up at 2-3-12, 29 lb down and 31 lb up at 4-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1717 lb down and 63 lb up at 10-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-12, 29 lb down and 31 lb up at 10-3-12, 29 lb down and 63 lb up at 12-3-4, 1717 lb down and 62 lb up at 12-3-12, 29 lb down and 31 lb up at 12-3-12, 29 lb down and 31 lb up at 12-3-12, 29 lb down and 31 lb up at 12-3-4, 1771 lb down and 62 lb up at 12-3-12, 29 lb down and 31 lb up at 14-3-4, 1771 lb down and 62 lb up at 14-3-12, and 29 lb down and 31 lb up at 16-3-4, and 1771 lb down and 62 lb up at 16-3-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-10=-60, 10-19=-60, 20-25=-20, 1-19=-6(F)

Concentrated Loads (lb)

Vert: 22=-1503(F=4, B=-1507) 1=20(F) 19=20(F) 25=-1416(B) 26=-1503(F=4, B=-1507) 27=-1503(F=4, B=-1507) 28=-1503(F=4, B=-1507) 29=-1503(F=4, B=-1507) 30=-1503(F=4, B=-1507) 31=-1503(F=4, B=-1507) 32=-1503(F=4, B=-1507) 32=-1503(F



Job	Truss		Truss Type		Qty	Ply	LOT 0.0002 CAMPBELL RID	GE 102 ALDEN WAY ANGIER, NC
25-3559-R01	R16		Common Girder		1	1		# 58836
					Run: 8.630 s Jul	 12 2024 Pri	Job Reference (optional) nt: 8.630 s Jul 12 2024 MiTek I	ndustries, Inc. Thu Apr 24 22:19:20 2025 Page 1 nEMw2LifPeTktSsbMuwVls13UP1xVzNNzl
		-0-	0-8 2-10-0	4-10-0 6-10-	0 8-10-0 10	-10-0	13-8-0 14-6-8	
		0-1	0-8 2-10-0	2-0-0 2-0-0		-0-0 '	2-10-0 0-10-8	
					4x4 =			Scale = 1:50.1
					8			
		2-8-2 2-1-2 1	2	2x4 6 3 J1 W3 40 5 2x4 W4 W2 7x8 =	9 9 ₩7 ₩5 ₩8 ₩7 ₩5 ₩8 ₩7 ₩8	2x4 12V3 2x4W		
		- //	18	19 17	20 ²¹ 16	22	23 🖾 🔍 15	
			3x4	$4x4 \equiv$	4x8 =	=	3x4	
				7-13	9-0-3		8-0 -13	
Plate Offsets (2	X,Y) [5:0-4-0,0-2	-0]	4-	7-13	4-4-5	4-7	-10	
LOADING (psf) TCLL (roof) Snow (Pf) TCDL BCLL	20.0 20.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 NO	CSI. TC 0.39 BC 0.25 WB 0.20	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l 0.03 16 -0.06 16 0.01	-17 >999 240	PLATES GRIP MT20 244/190
BCDL	10.0	Code IRC2021/TF	12014	Matrix-MSH				Weight: 124 lb FT = 20%
	2x4 SP No.2 2x4 SP No.3 *Exca W1: 2x6 SP No.2				BRACING- TOP CHORD BOT CHORD JOINTS	end ver Rigid ce 1 Brace MiTek be ins	ticals. Biling directly applied or 1 at Jt(s): 4, 7, 12 recommends that Stabil	tly applied or 6-0-0 oc purlins, except 10-0-0 oc bracing. izers and required cross bracing on, in accordance with Stabilizer
	Max Horz 18=-175 Max Uplift18=-46(/0-3-8 (min. 0-1-8) 5(LC 34) LC 10), 15=-46(LC (LC 2), 15=631(LC	11)	(min. 0-1-8)				
TOP CHORD	2-3=-408/5, 3-6= 2-4=-532/317, 4- 12-13=-543/318,	-337/42, 6-8=-303/ 5=-528/315, 5-7=-5 2-18=-602/67, 13-	87, 8-9=-303/8 41/323, 7-10= 15=-602/67	except when shown. 7, 9-11=-342/43, 11- -541/323, 10-12=-540	13=-412/6, D/316,			
BOT CHORD WEBS	,	17-19=-163/339, 1 16=-244/251, 13-1		3, 20-21=-260/788, 16	6-21=-260/788			
 Wind: ASCE (envelope) g TCLL: ASCE Cat B; Partia This truss ha non-concurra Provide adea This truss ha Th	roof live loads have 7-16; Vult=120mp able end zone; ca 7-16; Pr=20.0 ps illy Exp.; Ce=1.0; (as been designed the twith other live quate drainage to as been designed bas been designed bas been designed than connection rhan connection of the connection own and 21 lb up at 4-4-12, 6 lb do b up at 12-4-12, posibility of others.	ntilever left and rigf f (roof LL: Lum DO Cs=1.00; Ct=1.10 for greater of min ro loads. prevent water pond for a 10.0 psf botto d for a live load of 3 any other member n (by others) of frus n does not depict th n device(s) shall be at 13-4-12 on top of won and 4 lb up at and 7 lb down and	Vasd=95mph; tt exposed ; ei _=1.15 Plate I pof live load of ing. n chord live lo 0.0psf on the s. s to bearing p e size or the c provided suff bhord, and 7 lb 6-4-12, 6 lb do 4 lb up at 13	TCDL=5.0psf; BCDL d vertical left and rig iOL=1.15); Pf=20.0 p 12.0 psf or 2.00 time ad nonconcurrent wit bottom chord in all an late capable of withst rientation of the purlir cient to support conc down and 4 lb up at 8-6	ht exposed; Lumbe sf (Lum DOL=1.15 s flat roof load of 2 h any other live loa eas where a rectar anding 100 lb uplif h along the top and entrated load(s) 35 0-2-12, 6 lb down and The design/selec	er DOL=1 Plate DO 0.0 psf o ads. agle 3-6-0 t at joint(s /or bottoo 5 lb down and 4 lb 4 lb un	C C	SEAL 28147
Coaid Case(s	aget and ard							4/24/2025
Warning !V	erify design parame	ters and read notes b	efore use. This	lesign is based only upor	parameters shown, a	nd is for a	n individual building compon	ent to be installed and loaded

vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is to fail individual building component to be instanted and based only upon parameters shown, and is to fail individual building component to be instanted and based only upon parameters shown, and is to fail individual building component to be instanted and based only upon parameters shown, and is to fail individual building component to be shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN	WAY ANGIER, NC
25-3559-R01	R16	Common Girder	1	1	Job Reference (optional)	# 58836
		Ri	in: 8,630 s. Jul 1	12 2024 Pri	nt: 8,630 s. Jul 12 2024 MiTek Industries Inc. Th	u Apr 24 22:19:20 2025 Page 2

Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 Millek Industries, Inc. Thu Apr 24 22:19:20 2025 Page 2 ID:qqqJulBALaBkGN7NwUcS9ezVNhM-yrwWLO4_9nEMw2LifPeTktSsbMuwVls13UP1xVzNNzL

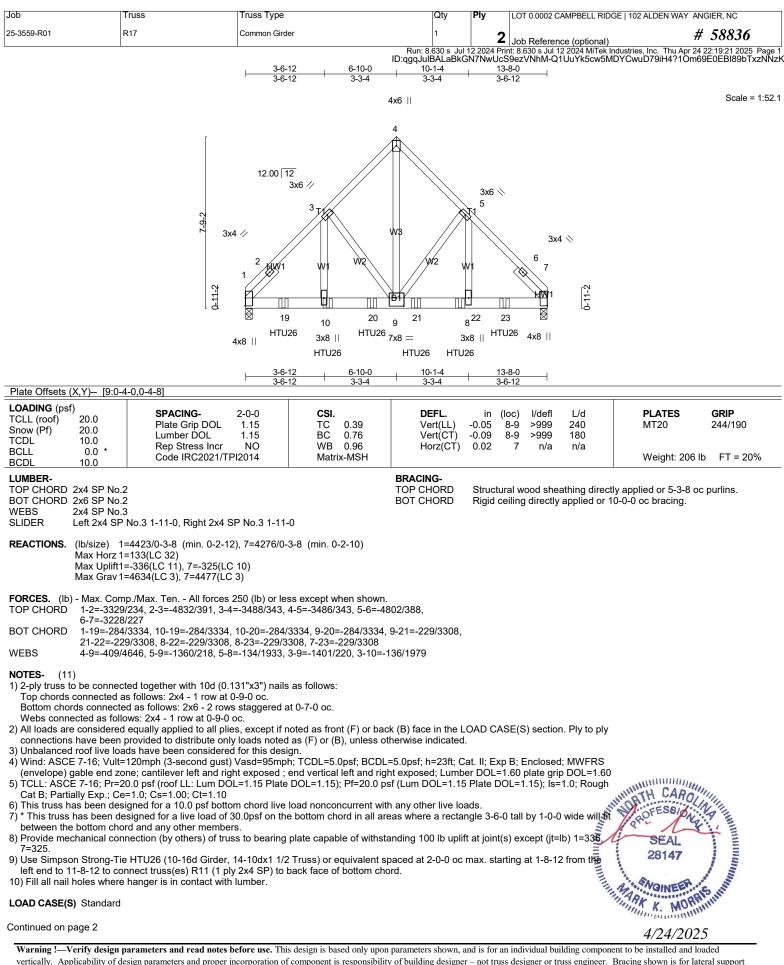
LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-8=-60, 8-13=-60, 13-14=-60, 15-18=-20, 2-13=-8(F) Concentrated Loads (lb)

Vert: 18=4(F) 15=4(F) 17=4(F) 19=4(F) 20=4(F) 21=4(F) 22=4(F) 23=4(F)





Job	Truss	Truss Type	Qty	Ply	LOT 0.0002 CAMPBELL RIDGE 102 ALDEN WAY	ANGIER, NC
25-3559-R01	R17	Common Girder	1	2	Job Reference (optional)	# 58836
	Run: 8.630 s. Jul 12 2024 Print: 8.630 s. Jul 12 2024 MiTek Industries, Inc. Thu Apr 24 22:19:21 2025, Page					· 24 22·19·21 2025 Page 2

Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Apr 24 22:19:21 2025 Page 2 ID:qqqJuIBALaBkGN7NwUcS9ezVNhM-Q1UuYk5cw5MDYCwuD79iH4?10m69E0EBI89bTxzNNzK

LOAD CASE(S) Standard

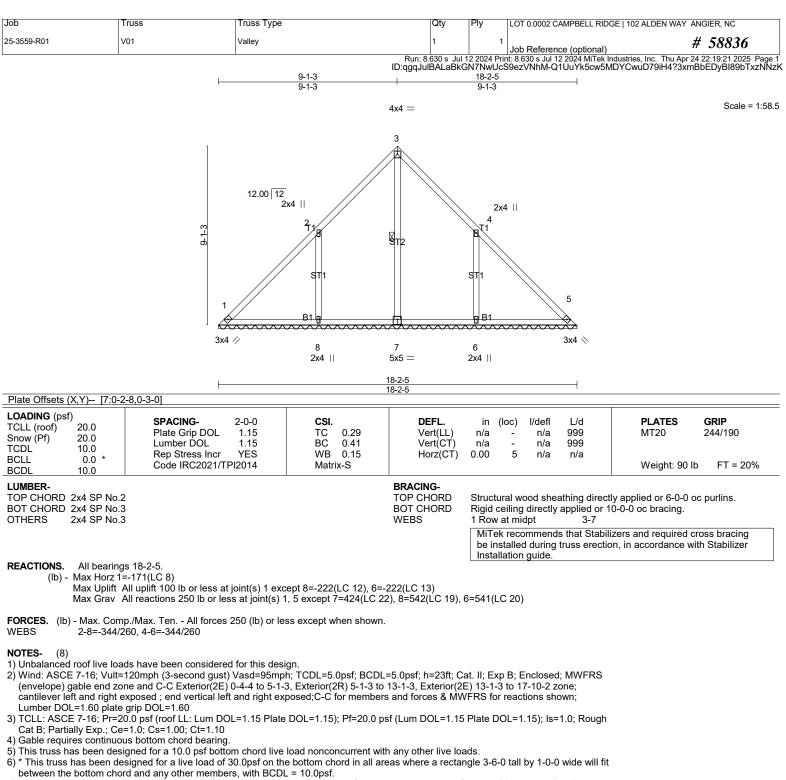
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 10=-1268(B) 19=-1268(B) 20=-1268(B) 21=-1268(B) 22=-1268(B) 23=-1268(B)

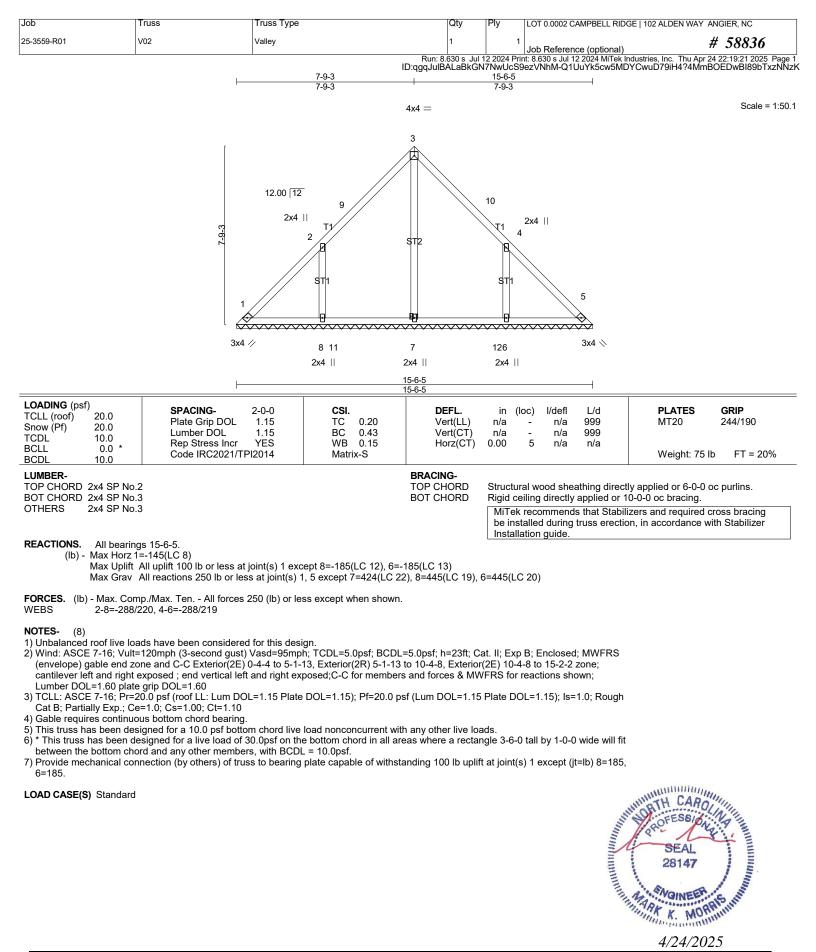


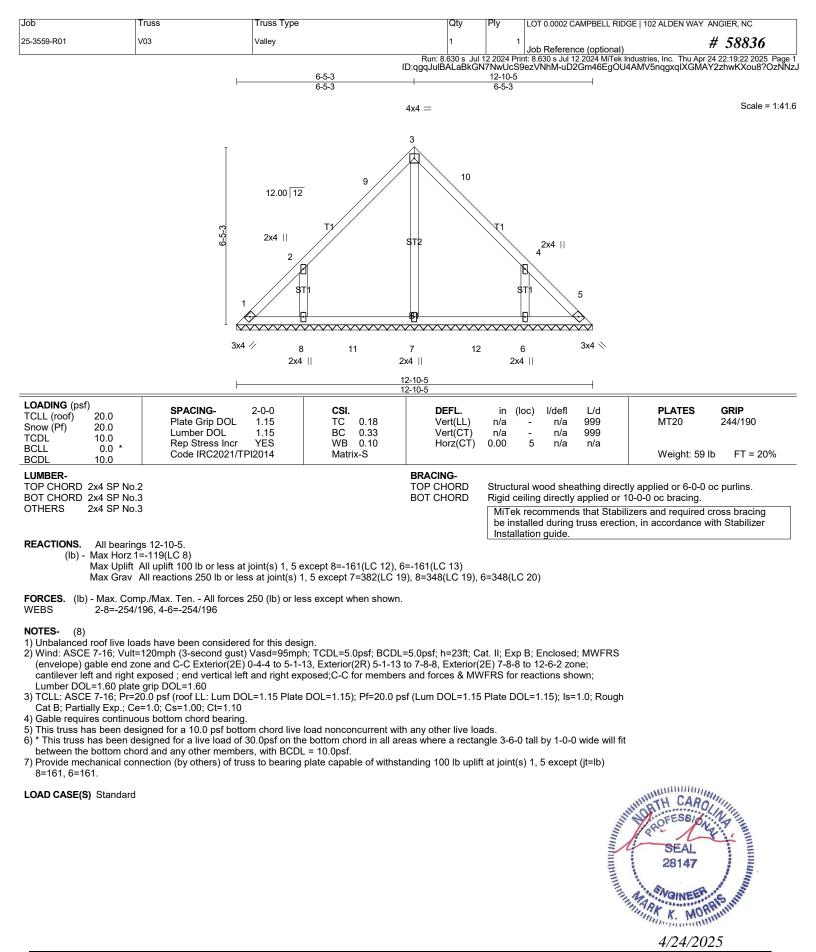


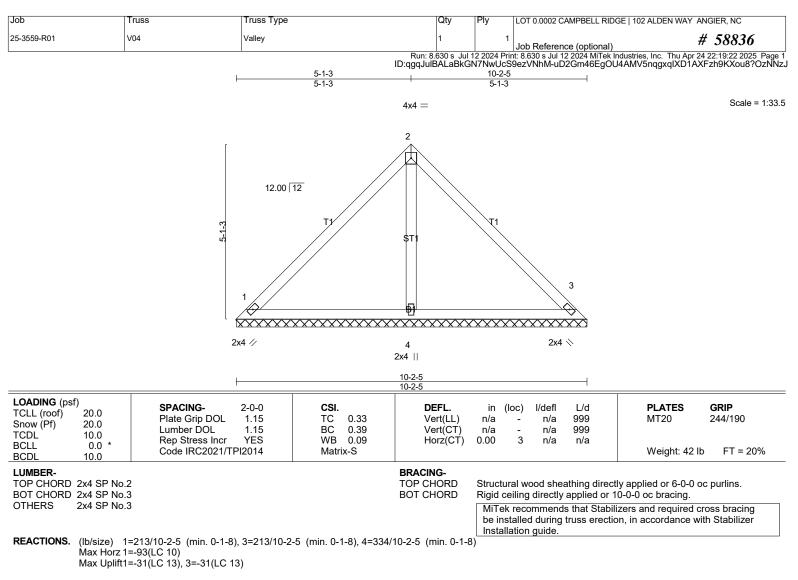
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=222, 6=222.

LOAD CASE(S) Standard









FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

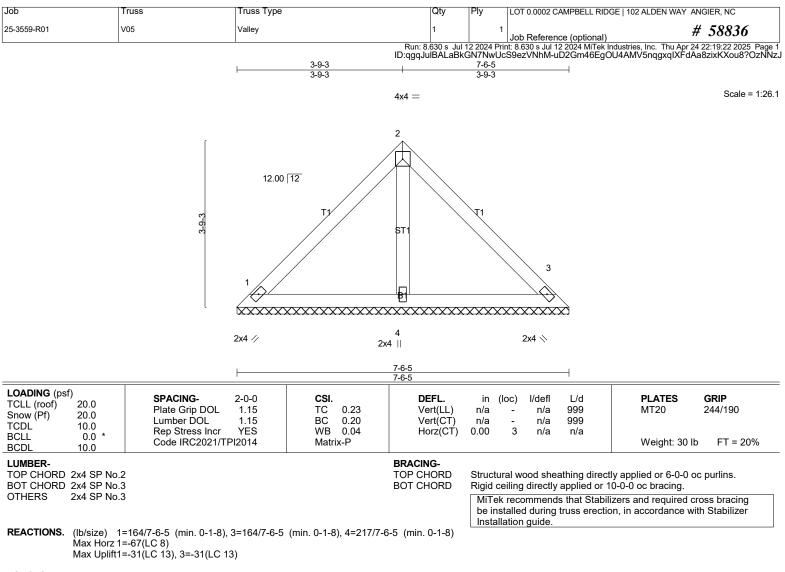
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

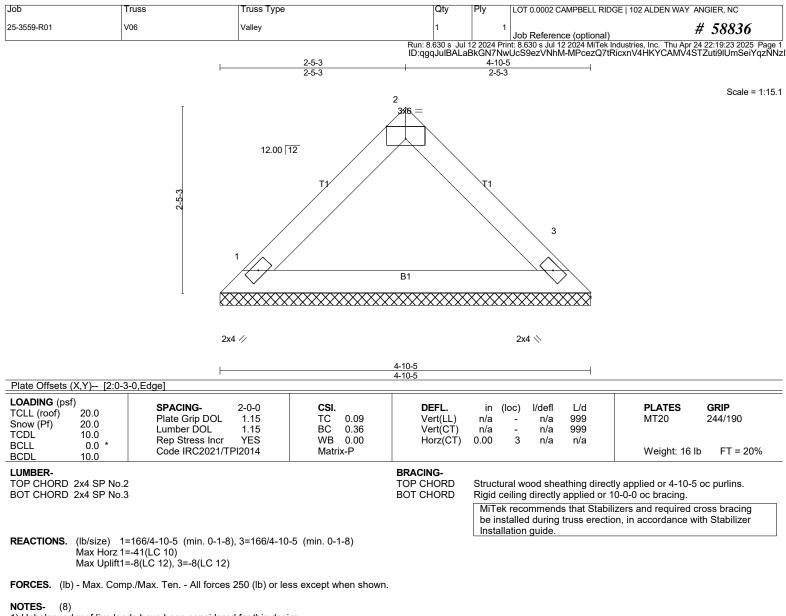
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

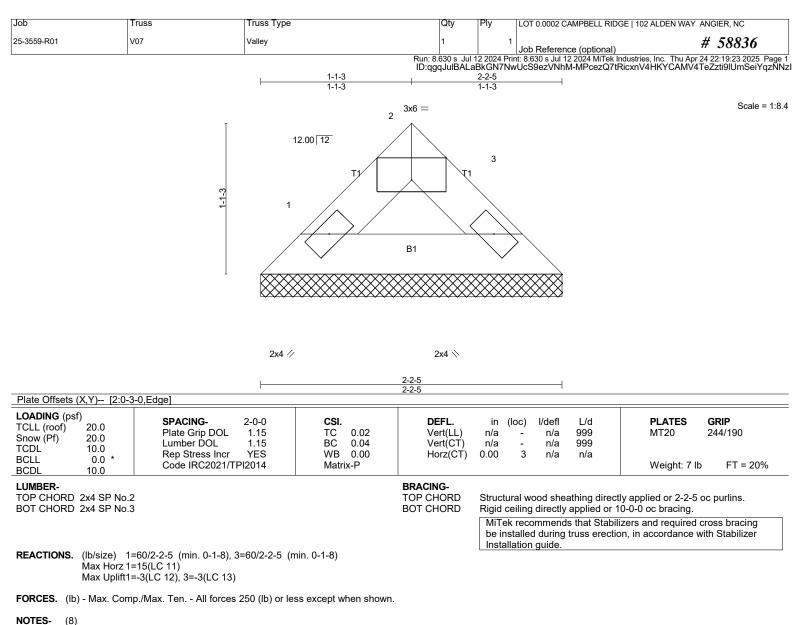
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

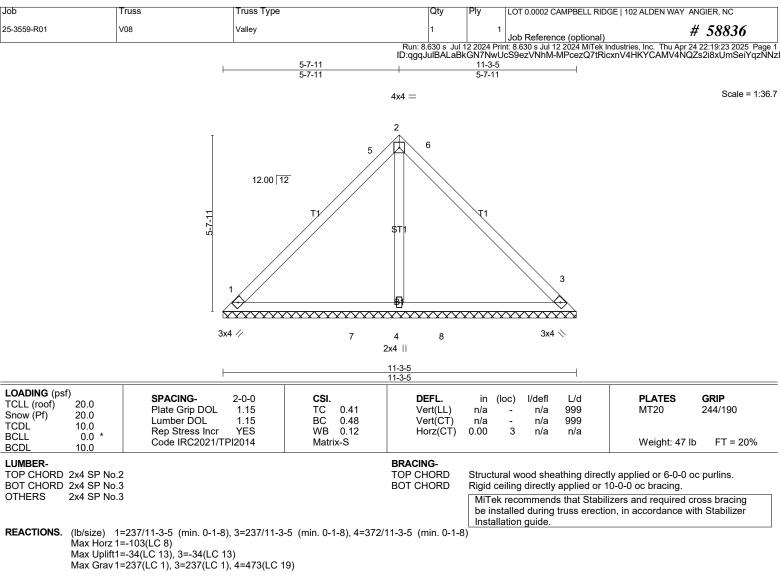
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-1-8, Exterior(2E) 6-1-8 to 10-11-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

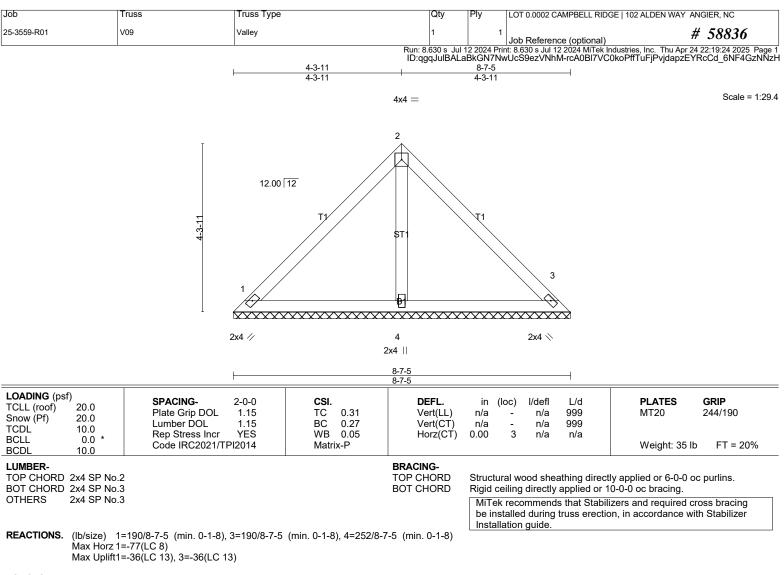
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

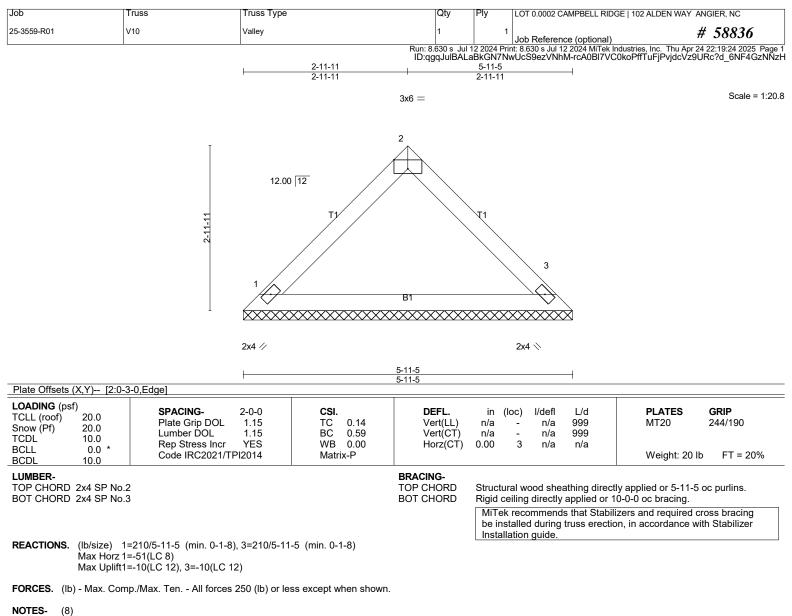
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

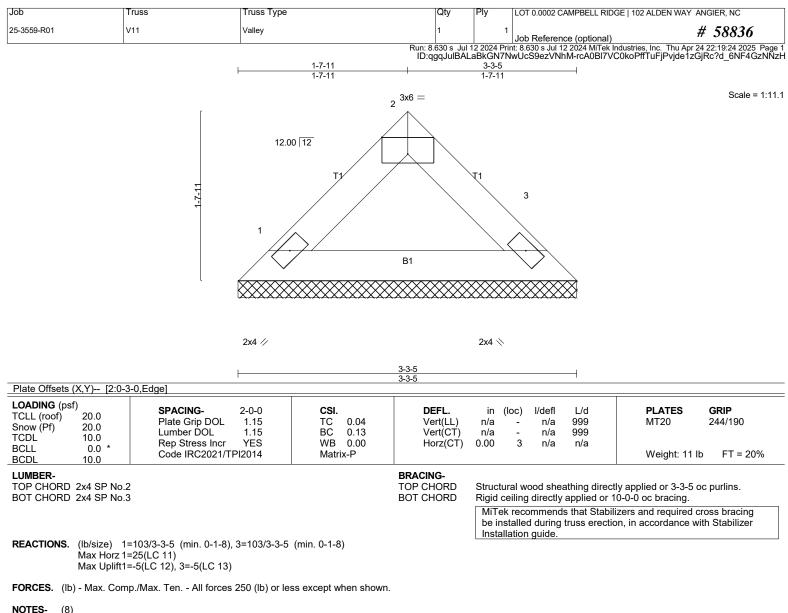
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

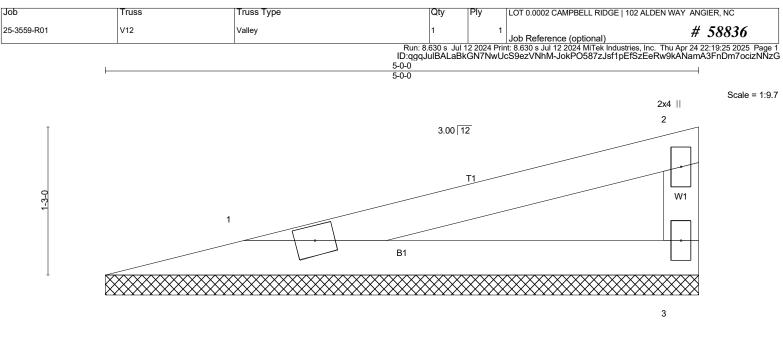
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





3x4 ⋍

2x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.33 BC 0.27 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 3	n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 14 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3				Structural wood sheathing directly applied or 5-0-0 oc purlins, exc end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.				c purlins, excep
			ed during		lizers and required co on, in accordance wi			

REACTIONS. (lb/size) 1=147/5-0-0 (min. 0-1-8), 3=147/5-0-0 (min. 0-1-8) Max Horz 1=29(LC 11) Max Uplift1=-18(LC 10), 3=-21(LC 14) Max Grav 1=184(LC 20), 3=184(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

