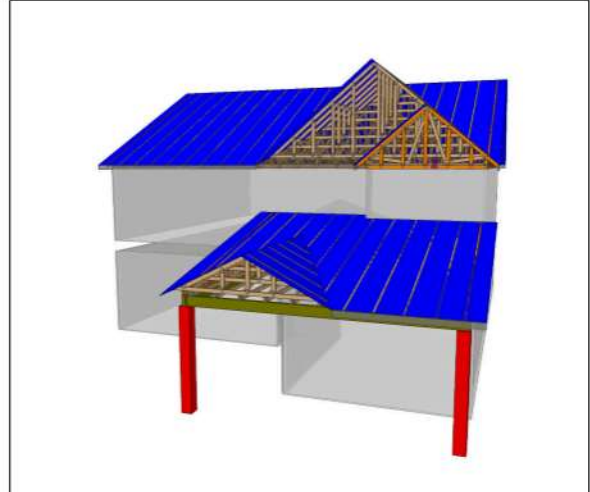




Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450

Builder: DR Horton Inc
Model: 13 Eagle Creek -
Edisto - C



THE PLACEMENT PLAN NOTES:

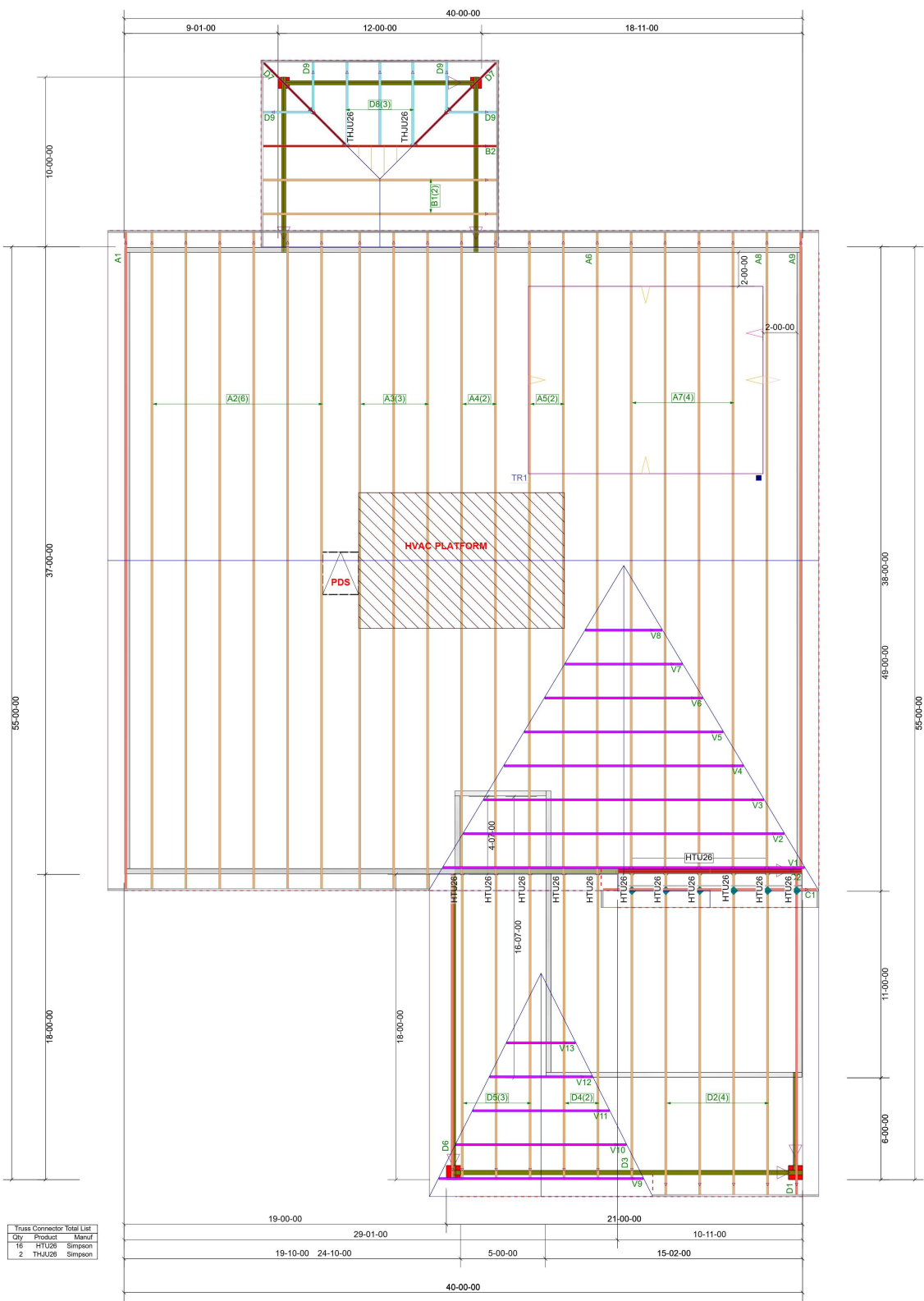
1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

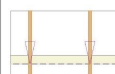
Date: _____

ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS

FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS. DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT. TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.



Truss Connector Total List		
Qty	Product	Manuf
16	HTU26	Simpson
2	THJU26	Simpson



Truss Drawing Let
End Indicator

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS. ** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH. ** All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI-1, all uplift connectors are the responsibility of the bldg designer and or contractor.

Course:	NTS
Date:	4/24/2025
Designer:	Nate Donaldson
Project Number:	25040106-B
Sheet Number:	1/1

DR Horton Inc
13 Eagle Creek - Edisto - C
ROOF PLACEMENT PLAN



THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Trusses" available from the Truss Plate Institute, 583 D'Orofino Drive, Madison, WI 53179

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

RE: 25040106
13 Eagle Creek - Edisto C - Roof

Trenco
818 Soundside Rd
Edenton, NC 27932

Site Information:

Customer: DR Horton Inc Project Name: 25040106
Lot/Block: 13 Model: Edisto C
Address: Subdivision: Eagle Creek
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2021/TPI2014 Design Program: MiTek 20/20 8.7
Wind Code: ASCE 7-16 Wind Speed: 130 mph
Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 35 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#	Truss Name	Date
1	I72956711	A1	4/23/2025	21	I72956731	D8	4/23/2025
2	I72956712	A2	4/23/2025	22	I72956732	D9	4/23/2025
3	I72956713	A3	4/23/2025	23	I72956733	V1	4/23/2025
4	I72956714	A4	4/23/2025	24	I72956734	V2	4/23/2025
5	I72956715	A5	4/23/2025	25	I72956735	V3	4/23/2025
6	I72956716	A6	4/23/2025	26	I72956736	V4	4/23/2025
7	I72956717	A7	4/23/2025	27	I72956737	V5	4/23/2025
8	I72956718	A8	4/23/2025	28	I72956738	V6	4/23/2025
9	I72956719	A9	4/23/2025	29	I72956739	V7	4/23/2025
10	I72956720	B1	4/23/2025	30	I72956740	V8	4/23/2025
11	I72956721	B2	4/23/2025	31	I72956741	V9	4/23/2025
12	I72956722	C1	4/23/2025	32	I72956742	V10	4/23/2025
13	I72956723	C2	4/23/2025	33	I72956743	V11	4/23/2025
14	I72956724	D1	4/23/2025	34	I72956744	V12	4/23/2025
15	I72956725	D2	4/23/2025	35	I72956745	V13	4/23/2025
16	I72956726	D3	4/23/2025				
17	I72956727	D4	4/23/2025				
18	I72956728	D5	4/23/2025				
19	I72956729	D6	4/23/2025				
20	I72956730	D7	4/23/2025				

The truss drawing(s) referenced above have been prepared by
Truss Engineering Co. under my direct supervision
based on the parameters provided by Carter Components (Sanford, NC)).
Truss Design Engineer's Name: Galinski, John
My license renewal date for the state of North Carolina is December 31, 2025.
North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

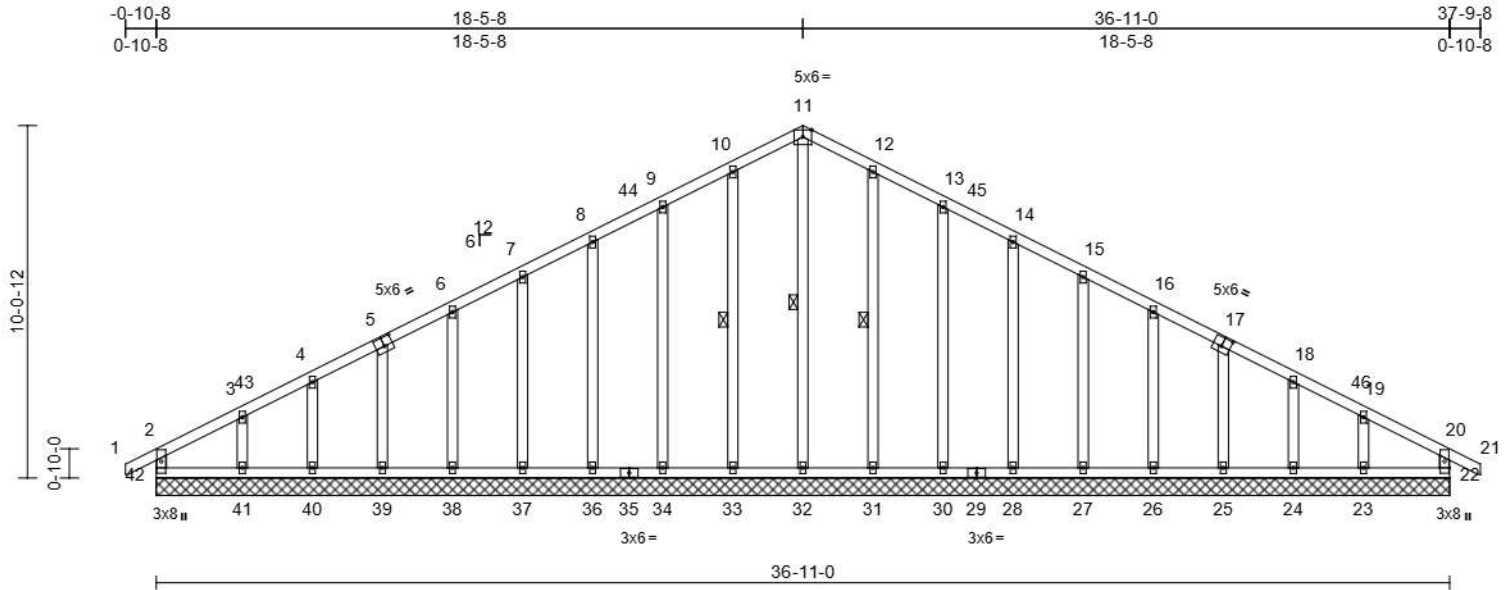


Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	I72956711
25040106	A1	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:38
ID:zOwdqVY5fLpY7Y_EniUSDzO8oG-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWRCdoi7J4zJC?f

Page: 1



Scale = 1:65.8

Plate Offsets (X, Y): [5:0-3-0,0-3-0], [17:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.00	22	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
Weight: 254 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6'-0" oc bracing.

WEBS 1 Row at midpt 11-32, 10-33, 12-31

REACTIONS (size) 22=36-11-0, 23=36-11-0, 24=36-11-0, 25=36-11-0, 26=36-11-0, 27=36-11-0, 28=36-11-0, 30=36-11-0, 31=36-11-0, 32=36-11-0, 33=36-11-0, 34=36-11-0, 36=36-11-0, 37=36-11-0, 38=36-11-0, 39=36-11-0, 40=36-11-0, 41=36-11-0, 42=36-11-0
Max Horiz 42=-115 (LC 13)
Max Uplift 22=-6 (LC 12), 23=-48 (LC 16), 24=-6 (LC 16), 25=-18 (LC 16), 26=-15 (LC 16), 27=-16 (LC 16), 28=-15 (LC 16), 30=-19 (LC 16), 31=-10 (LC 16), 33=-11 (LC 15), 34=-19 (LC 15), 36=-15 (LC 15), 37=-16 (LC 15), 38=-15 (LC 15), 39=-19 (LC 15), 40=-4 (LC 15), 41=-56 (LC 15), 42=-26 (LC 11)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-42=-136/74, 1-2=0/33, 2-3=-94/68, 3-4=-75/59, 4-6=-70/111, 6-7=-71/157, 7-8=-82/202, 8-9=-100/247, 9-10=-120/295, 10-11=-136/334, 11-12=-136/334, 12-13=-120/295, 13-14=-100/247, 14-15=-82/202, 15-16=-71/157, 16-18=-59/111, 18-19=-54/35, 19-20=-75/40, 20-21=0/33, 20-22=-136/73
BOT CHORD 41-42=-34/98, 40-41=-34/98, 39-40=-34/98, 38-39=-34/98, 37-38=-34/98, 36-37=-34/98, 34-36=-34/98, 33-34=-34/98, 32-33=-34/98, 31-32=-34/98, 30-31=-34/98, 28-30=-34/98, 27-28=-34/98, 26-27=-34/98, 25-26=-34/98, 24-25=-34/98, 23-24=-34/98, 22-23=-34/98
WEBS 11-32=-225/58, 10-33=-165/65, 9-34=-140/83, 8-36=-126/76, 7-37=-126/78, 6-38=-127/77, 5-39=-128/80, 4-40=-123/69, 3-41=-147/139, 12-31=-165/65, 13-30=-140/83, 14-28=-126/76, 15-27=-126/78, 16-26=-127/77, 17-25=-128/80, 18-24=-122/69, 19-23=-145/139

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-1 to 2-10-3, Exterior(2N) 2-10-3 to 18-5-8, Corner(3R) 18-5-8 to 22-1-13, Exterior(2N) 22-1-13 to 37-9-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.



April 23, 2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinet.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacompnents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	I72956711
25040106	A1	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:38
ID:zOwdqVY5FoLp?Y_EniUSDzO8oG-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 2

- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) All bearings are assumed to be SP No.2 .
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 42, 6 lb uplift at joint 22, 11 lb uplift at joint 33, 19 lb uplift at joint 34, 15 lb uplift at joint 36, 16 lb uplift at joint 37, 15 lb uplift at joint 38, 19 lb uplift at joint 39, 4 lb uplift at joint 40, 56 lb uplift at joint 41, 10 lb uplift at joint 31, 19 lb uplift at joint 30, 15 lb uplift at joint 28, 16 lb uplift at joint 27, 15 lb uplift at joint 26, 18 lb uplift at joint 25, 6 lb uplift at joint 24 and 48 lb uplift at joint 23.

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

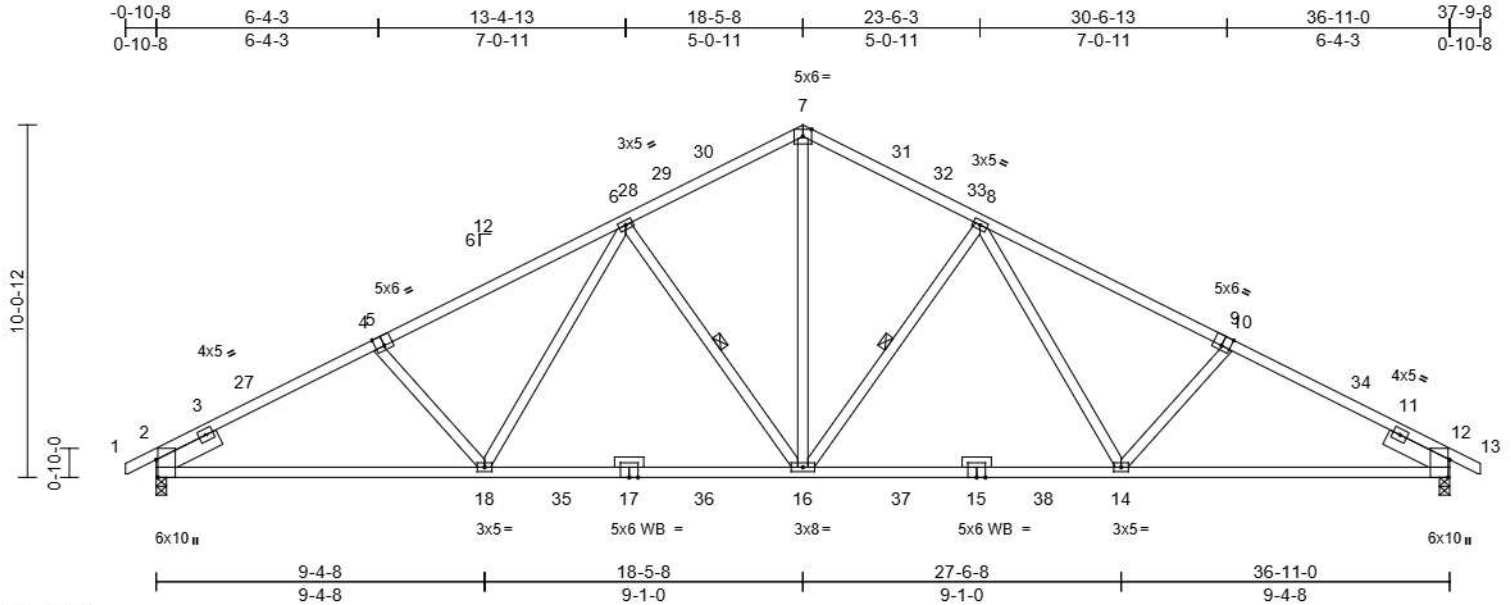
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956712
25040106	A2	Common	6	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:65.8

Plate Offsets (X, Y): [2:0-6-1,Edge], [5:0-3-0,Edge], [9:0-3-0,Edge], [12:0-6-1,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.66	Vert(LL)	-0.26	14-16	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.61	Vert(CT)	-0.48	14-16	>922	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.62	Horz(CT)	0.12	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 208 lb FT = 20%											

LUMBER

TOP CHORD	2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP 2400F 2.0E
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
SLIDER	Left 2x6 SP 2400F 2.0E -- 2-0-0, Right 2x6 SP 2400F 2.0E -- 2-0-0

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-5-15 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 8-16, 6-16

REACTIONS

(size)	2=0-3-8, 12=0-3-8
Max Horiz	2=98 (LC 14)
Max Grav	2=1772 (LC 3), 12=1772 (LC 3)

FORCES

	(lb) - Maximum Compression/Maximum Tension
TOP CHORD	1-2=0/28, 2-4=-3207/64, 4-6=-3046/65, 6-7=-2336/110, 7-8=-2336/110, 8-10=-3046/65, 10-12=-3207/64, 12-13=0/28
BOT CHORD	2-18=-57/2766, 16-18=0/2361, 14-16=0/2361, 12-14=-59/2766
WEBS	7-16=-75/1681, 8-16=-769/123, 8-14=-13/530, 10-14=-253/157, 6-16=-769/123, 6-18=-13/530, 4-18=-253/157

NOTES

- 1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-10-3, Interior (1) 2-10-3 to 18-5-8, Exterior(2R) 18-5-8 to 22-1-13, Interior (1) 22-1-13 to 37-9-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 6) 200.0lb AC unit load placed on the top chord, 18-5-8 from left end, supported at two points, 5-0-0 apart.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) All bearings are assumed to be SP 2400F 2.0E.

LOAD CASE(S) Standard



April 23, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

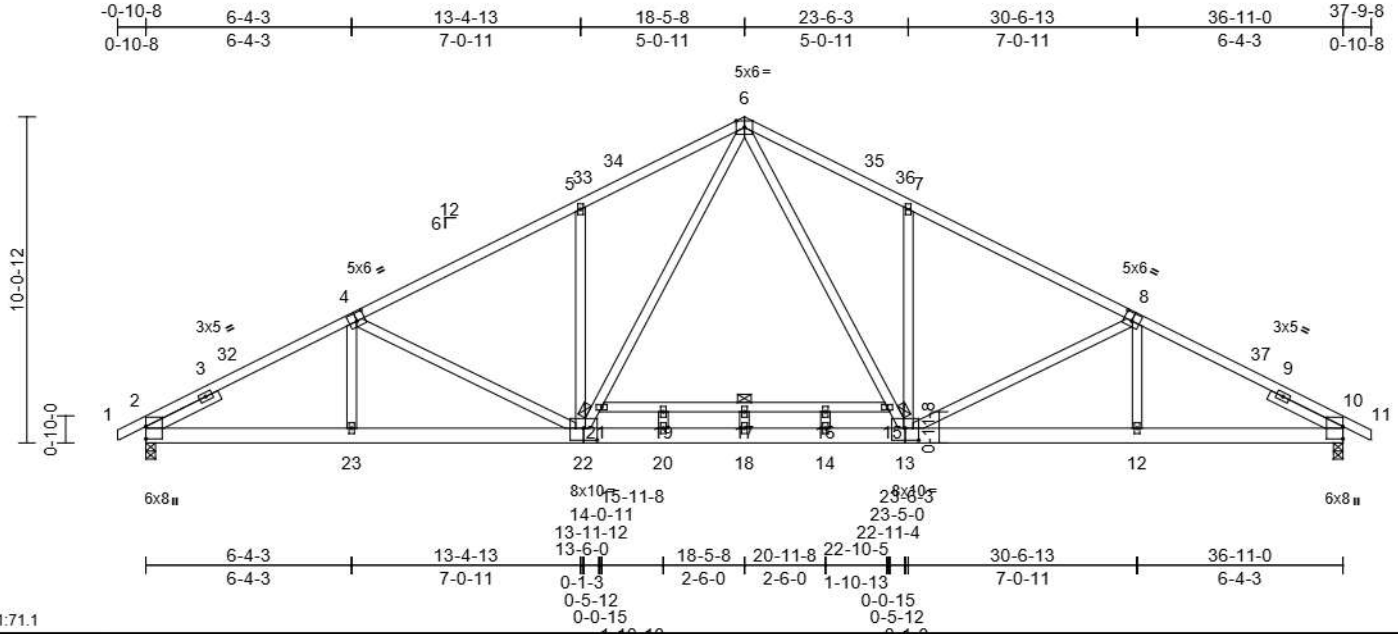
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956713
25040106	A3	Common	3	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:39
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Page: 1



Scale = 1:71.1

Plate Offsets (X, Y): [4:0-3-0,0-3-0], [8:0-3-0,0-3-0], [13:0-5-0,0-4-8], [22:0-5-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	-0.22	17	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.46	Vert(CT)	-0.45	17	>976	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(CT)	0.07	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 258 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E *Except* 21-15:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 22-6,13-6:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 10=0-3-8
Max Horiz 2=98 (LC 14)
Max Grav 2=1946 (LC 3), 10=1946 (LC 3)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=0/28, 2-5=-3540/0, 5-6=-3310/0, 6-7=-3310/0, 7-10=-3540/0, 10-11=0/28
BOT CHORD 2-23=0/3098, 20-23=0/3100, 18-20=0/2235, 14-18=0/2235, 12-14=0/3100, 10-12=0/3099, 19-21=-108/0, 17-19=-108/0, 16-17=-108/0, 15-16=-108/0
WEBS 5-22=-421/178, 21-22=0/1395, 6-21=0/1495, 4-22=-333/171, 4-23=-68/12, 7-13=-421/178, 6-15=0/1495, 13-15=0/1395, 8-13=-333/171, 8-12=-68/12, 19-20=-108/0, 17-18=-113/0, 14-16=-108/0

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-10-3, Interior (1) 2-10-3 to 18-5-8, Exterior(2R) 18-5-8 to 22-1-13, Interior (1) 22-1-13 to 37-9-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 18-5-8 from left end, supported at two points, 5-0-0 apart.
- All plates are 2x4 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP 2400F 2.0E.

LOAD CASE(S) Standard



April 23, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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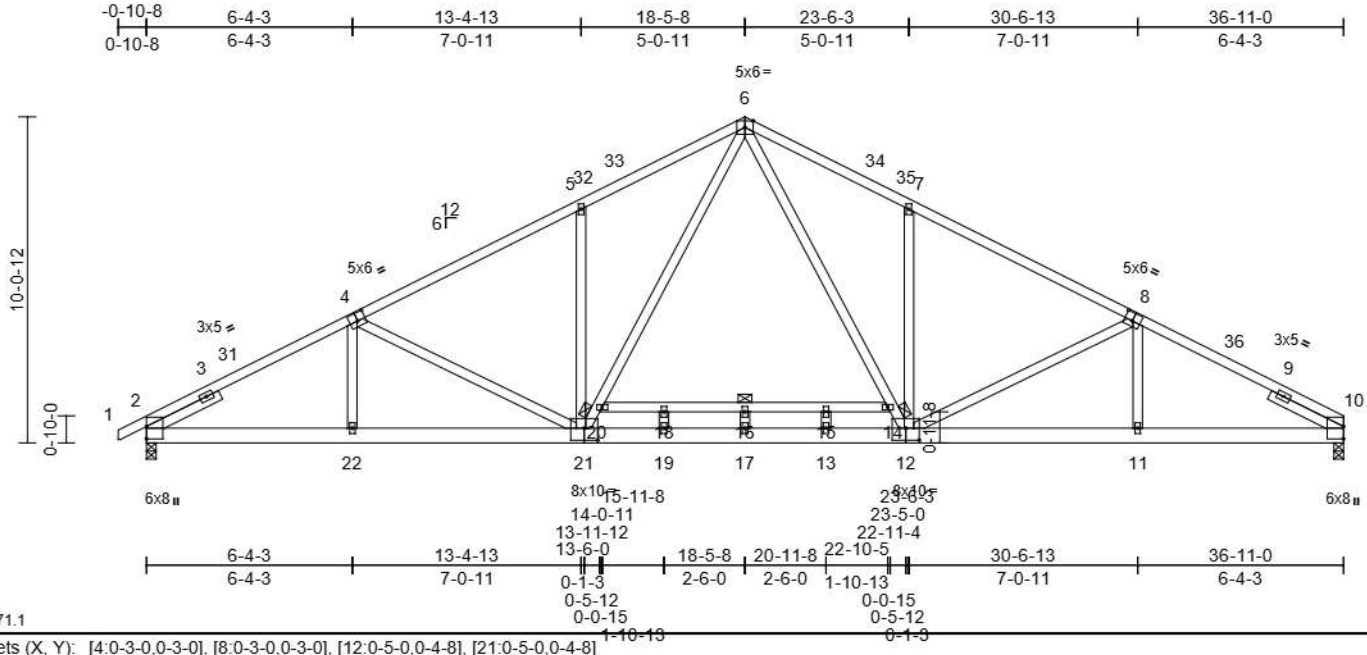
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956714
25040106	A4	Common	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:39
ID:gc7G9x02nGC9Tnb1sEC5TQzO8am-RfC?PsB70Hq3NSgPqnL8w3uITxbGKwRCDoi7J4zJC?r

Page: 1



Scale = 1:71.1

Plate Offsets (X, Y): [4:0-3-0,0-3-0], [8:0-3-0,0-3-0], [12:0-5-0,0-4-8], [21:0-5-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	-0.22	16	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.46	Vert(CT)	-0.45	16	>976	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(CT)	0.07	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
Weight: 256 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E *Except* 20-14:2x4 SP No.2
WEBS 2x4 SP No.3 *Except* 21-6,12-6:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-2-2 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 2=0-3-8, 10=0-3-8
Max Horiz 2=100 (LC 12)
Max Grav 2=1946 (LC 3), 10=1903 (LC 3)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/28, 2-5=-3541/0, 5-6=-3311/0, 6-7=-3312/0, 7-10=-3545/0
BOT CHORD 2-22=0/3097, 19-22=0/3099, 17-19=0/2234, 13-17=0/2234, 11-13=0/3102, 10-11=-2/3101, 18-20=-108/0, 16-18=-108/0, 15-16=-108/0, 14-15=-108/0
WEBS 5-21=-421/178, 20-21=0/1395, 6-20=0/1496, 4-21=-333/171, 6-14=0/1497, 12-14=0/1396, 7-12=-421/178, 8-12=-338/178, 4-22=-68/12, 8-11=-65/12, 18-19=-108/0, 16-17=-113/0, 13-15=-108/0

NOTES

1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-10-3, Interior (1) 2-10-3 to 18-5-8, Exterior(2R) 18-5-8 to 22-1-13, Interior (1) 22-1-13 to 36-11-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- 200.0lb AC unit load placed on the bottom chord, 18-5-8 from left end, supported at two points, 5-0-0 apart.
- All plates are 2x4 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP 2400F 2.0E.

LOAD CASE(S) Standard



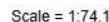
April 23, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

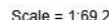
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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Page: 1818 Soundside Road
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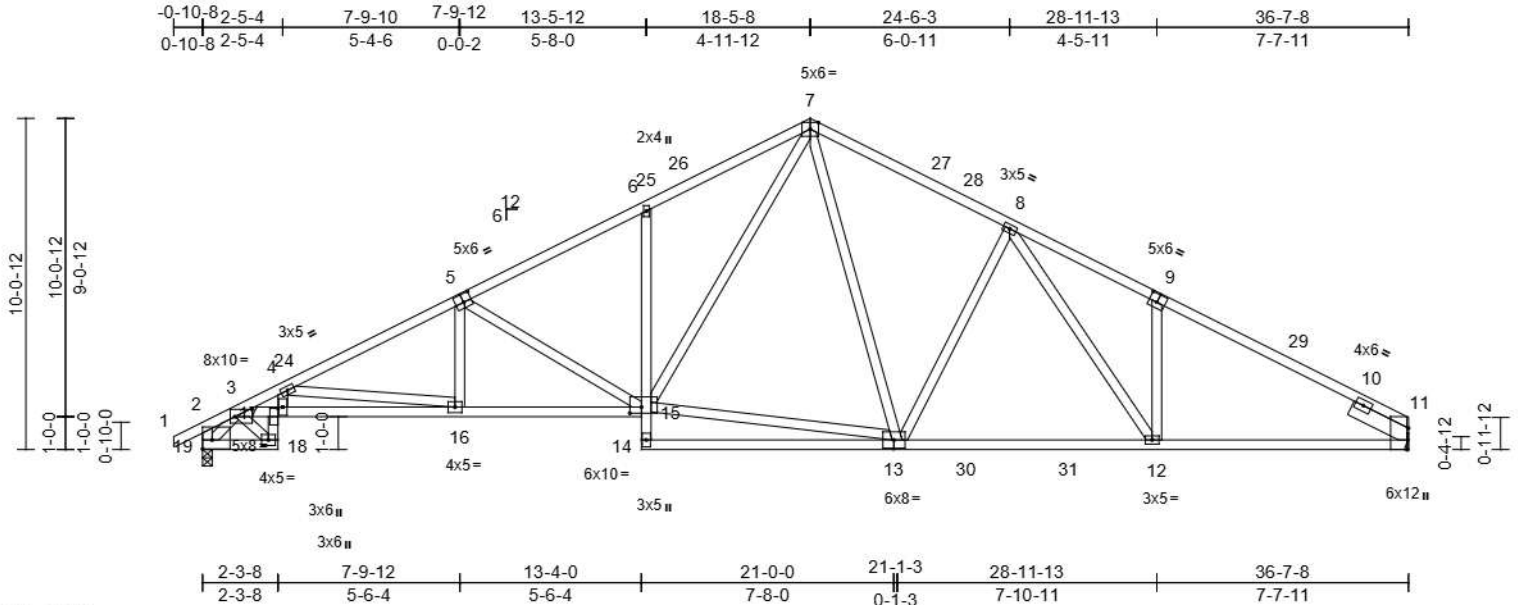
Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956717
25040106	A7	Roof Special	4	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:40

Page: 1

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Scale = 1:70.1									
Plate Offsets (X, Y): [2:Edge,0-3-4], [3:0-6-4,0-2-11], [5:0-3-0,0-3-0], [9:0-3-0,0-3-0], [11:0-7-13,Edge], [15:0-4-4,0-2-4], [17:0-0-8,0-1-12]									
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in (loc)	l/defl	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	-0.26 12-13	>999	240
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-0.47 12-13	>938	180
TCDL	10.0	Rep Stress Incr	YES	WB	0.85	Horz(CT)	0.25 11	n/a	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH					
BCDL	10.0								
									Weight: 230 lb FT = 20%

LUMBER	
TOP CHORD	2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP 2400F 2.0E *Except* 18-17,6-14:2x4 SP No.3
WEBS	2x4 SP No.3
SLIDER	Right 2x6 SP 2400F 2.0E -- 2-0-0
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS (size) 11= Mechanical, 19=0-3-8	
Max Horiz 19=110 (LC 12)	
Max Grav 11=1582 (LC 3), 19=1626 (LC 3)	
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/33, 2-3=-396/49, 3-4=-5438/349, 4-6=-3627/266, 6-7=-2782/356, 7-8=-2233/286, 8-11=-2805/316, 2-19=-502/103
BOT CHORD	18-19=-107/1438, 17-18=-87/1374, 3-17=-292/4523, 16-17=-320/4693, 15-16=-163/3185, 14-15=0/122, 6-15=-323/152, 12-14=-79/2126, 11-12=-136/2383
WEBS	4-16=-1514/159, 5-16=0/501, 5-15=-957/96, 13-15=-14/1533, 7-15=-154/1375, 7-13=-42/694, 8-13=-597/161, 8-12=-69/460, 9-12=-223/142, 4-17=0/863, 3-18=-1679/121, 3-19=-1777/116

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-9-14, Interior (1) 2-9-14 to 18-5-8, Exterior(2R) 18-5-8 to 22-1-7, Interior (1) 22-1-7 to 36-7-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 19 SP 2400F 2.0E .
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard



April 23,2025

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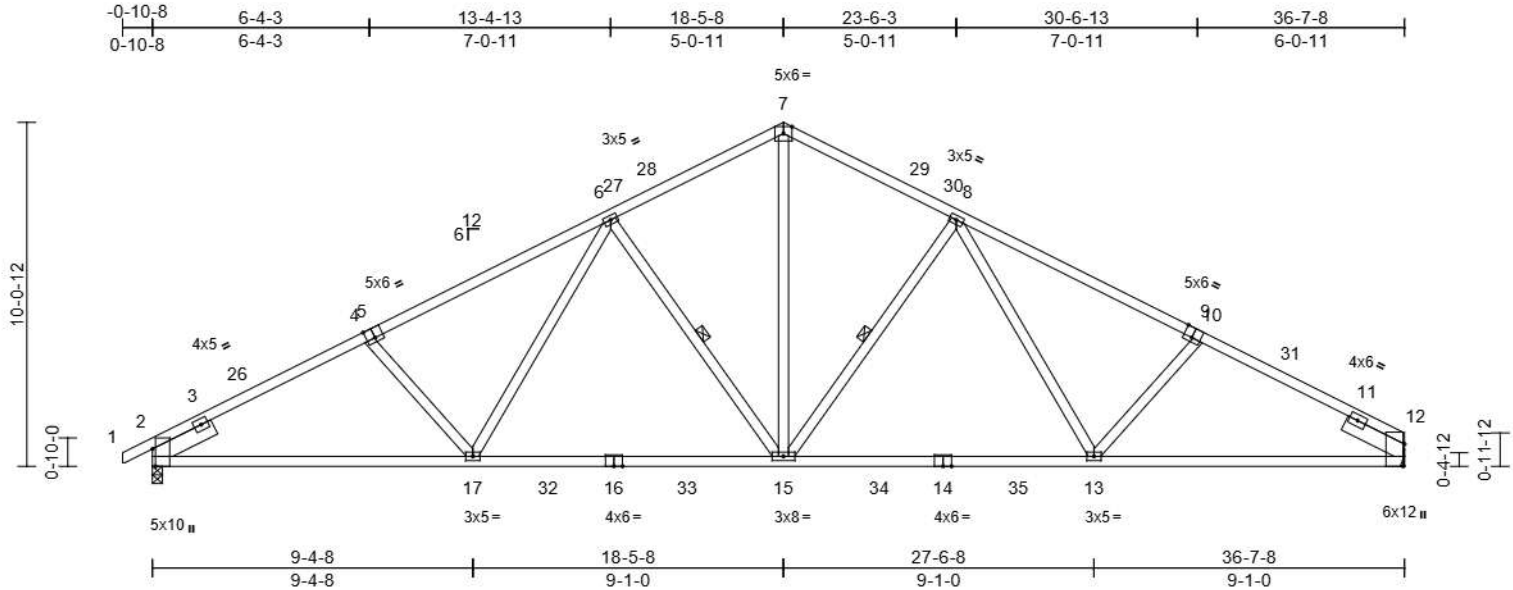
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956718
25040106	A8	Common	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:40
ID:IBmKzq9yX8AJQIGkG7rGlzO8DK-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoI7J4zJC?f

Page: 1



Scale = 1:67.5

Plate Offsets (X, Y): [2:0-6-1,Edge], [5:0-3-0,Edge], [9:0-3-0,Edge], [12:0-7-13,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.79	Vert(LL)	-0.29	13-15	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.66	Vert(CT)	-0.50	13-15	>882	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.58	Horz(CT)	0.14	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 203 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3
SLIDER Left 2x6 SP 2400F 2.0E -- 2-0-0, Right 2x6 SP 2400F 2.0E -- 2-0-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-4 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 8-15, 6-15

REACTIONS

(size) 2=0-3-8, 12= Mechanical
Max Horiz 2=101 (LC 12)
Max Grav 2=1660 (LC 3), 12=1618 (LC 3)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/28, 2-4=-2989/253, 4-6=-2823/258, 6-7=-2097/273, 7-8=-2099/275, 8-10=-2743/259, 10-12=-2890/302
BOT CHORD 2-17=-161/2577, 15-17=-85/2144, 13-15=-73/2126, 12-13=-156/2466
WEBS 7-15=-139/1597, 8-15=-700/158, 8-13=0/467, 10-13=-224/136, 6-15=-733/160, 6-17=0/555, 4-17=-275/136

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-10-1 to 2-9-14, Interior (1) 2-9-14 to 18-5-8, Exterior(2R) 18-5-8 to 22-1-7, Interior (1) 22-1-7 to 36-7-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP 2400F 2.0E .
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard



April 23, 2025

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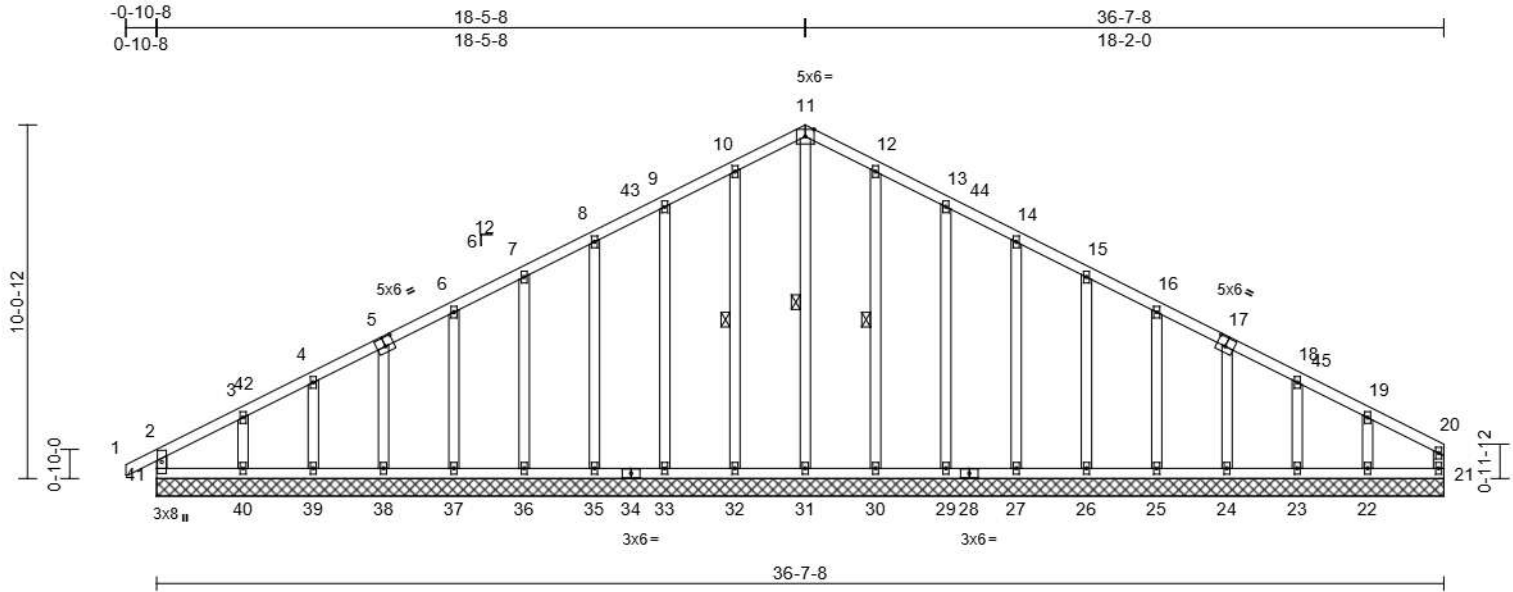
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	I72956719
25040106	A9	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:40
ID:0v0EKOIPXjpZ?6Z5P9GaGMzO89H-RfC?PsB70Hq3NSgPqnL8w3uITxbGKwrcDoi7J4zJC?f

Page: 1



Scale = 1:65.6

Plate Offsets (X, Y): [5:0-3-0,0-3-0], [17:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	n/a	-	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.00	21	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
Weight: 252 lb FT = 20%											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

WEBS 1 Row at midpt 11-31, 10-32, 12-30

REACTIONS (size)
21=36-7-8, 22=36-7-8, 23=36-7-8,
24=36-7-8, 25=36-7-8, 26=36-7-8,
27=36-7-8, 29=36-7-8, 30=36-7-8,
31=36-7-8, 32=36-7-8, 33=36-7-8,
35=36-7-8, 36=36-7-8, 37=36-7-8,
38=36-7-8, 39=36-7-8, 40=36-7-8,
41=36-7-8

Max Horiz 41=119 (LC 14)

Max Uplift 22=52 (LC 16), 23=6 (LC 16),
24=18 (LC 16), 25=15 (LC 16),
26=16 (LC 16), 27=15 (LC 16),
29=19 (LC 16), 30=10 (LC 16),
32=11 (LC 15), 33=19 (LC 15),
35=15 (LC 15), 36=16 (LC 15),
37=15 (LC 15), 38=19 (LC 15),
39=3 (LC 15), 40=57 (LC 15),
41=31 (LC 11)

Max Grav 21=86 (LC 36), 22=176 (LC 40),
23=156 (LC 2), 24=161 (LC 40),
25=160 (LC 2), 26=160 (LC 40),
27=160 (LC 2), 29=180 (LC 23),
30=205 (LC 23), 31=166 (LC 36),
32=205 (LC 22), 33=180 (LC 22),
35=160 (LC 2), 36=160 (LC 39),
37=160 (LC 2), 38=161 (LC 39),
39=156 (LC 2), 40=180 (LC 33),
41=163 (LC 34)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 2-41=-138/77, 1-2=0/33, 2-3=-101/73,
3-4=-82/66, 4-6=-77/116, 6-7=-77/162,
7-8=-89/207, 8-9=-104/252, 9-10=-121/300,
10-11=-136/339, 11-12=-136/339,
12-13=-121/300, 13-14=-104/252,
14-15=-89/207, 15-16=-77/162,
16-18=-65/116, 18-19=-54/32, 19-20=-69/33,
20-21=-62/6
BOT CHORD 40-41=-32/77, 39-40=-32/77, 38-39=-32/77,
37-38=-32/77, 36-37=-32/77, 35-36=-32/77,
33-35=-32/77, 32-33=-32/77, 31-32=-32/77,
30-31=-32/77, 29-30=-32/77, 27-29=-32/77,
26-27=-32/77, 25-26=-32/77, 24-25=-32/77,
23-24=-32/77, 22-23=-32/77, 21-22=-32/77
WEBS 11-31=230/58, 10-32=165/64,
9-33=140/84, 8-35=-126/76, 7-36=-126/77,
6-37=-127/77, 5-38=-128/80, 4-39=-123/69,
3-40=-146/137, 12-30=-165/64,
13-29=-140/84, 14-27=-126/76,
15-26=-126/77, 16-25=-127/77,
17-24=-127/79, 18-23=-122/83,
19-22=-143/155

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-1 to 2-9-14, Exterior(2N) 2-9-14 to 18-5-8, Corner(3R) 18-5-8 to 22-1-7, Exterior(2N) 22-1-7 to 36-5-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.



April 23, 2025

Continued on page 2

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof
25040106	A9	Common Supported Gable	1	1	Job Reference (optional)

I72956719

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 2

- 12) All bearings are assumed to be SP No.2 .
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 41, 11 lb uplift at joint 32, 19 lb uplift at joint 33, 15 lb uplift at joint 35, 16 lb uplift at joint 36, 15 lb uplift at joint 37, 19 lb uplift at joint 38, 3 lb uplift at joint 39, 57 lb uplift at joint 40, 10 lb uplift at joint 30, 19 lb uplift at joint 29, 15 lb uplift at joint 27, 16 lb uplift at joint 26, 15 lb uplift at joint 25, 18 lb uplift at joint 24, 6 lb uplift at joint 23 and 52 lb uplift at joint 22.

LOAD CASE(S) Standard

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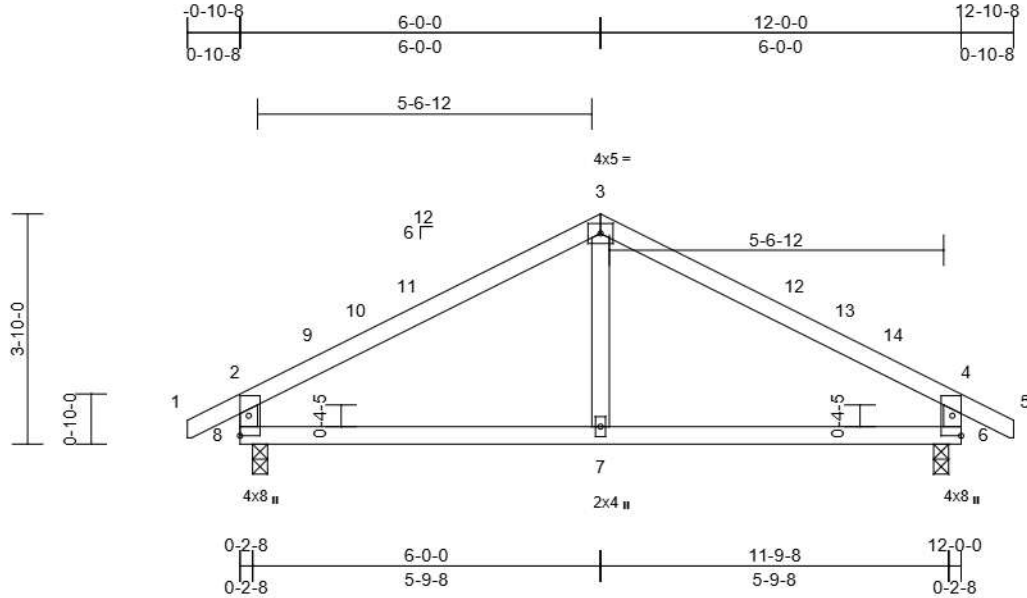
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956720
25040106	B1	Common	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:40
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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.74	Vert(LL)	-0.02	7-8	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	-0.05	7-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 47 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)

6=0-3-0, 8=0-3-0
Max Horiz 8=51 (LC 14)
Max Grav 6=528 (LC 2), 8=528 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=0/33, 2-3=-571/188, 3-4=-571/188, 4-5=0/33, 2-8=-489/231, 4-6=-489/231
BOT CHORD	7-8=-68/431, 6-7=-68/431
WEBS	3-7=0/123

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-10-1 to 2-1-15, Interior (1) 2-1-15 to 6-0-0, Exterior(2R) 6-0-0 to 9-0-0, Interior (1) 9-0-0 to 12-10-1 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.

LOAD CASE(S) Standard



April 23, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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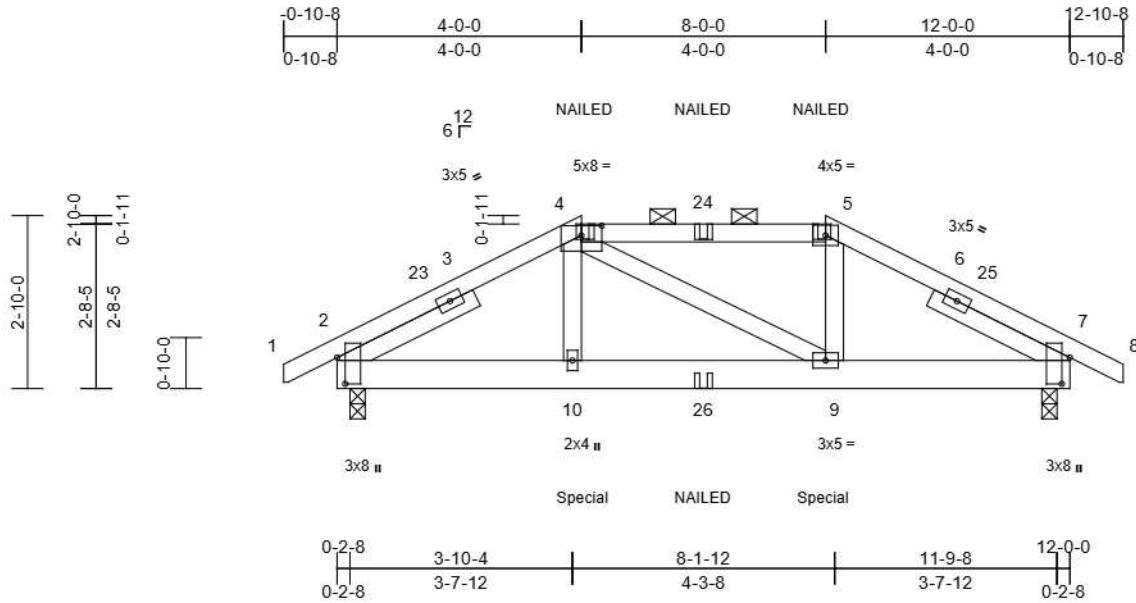
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956721
25040106	B2	Hip Girder	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:41
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Page: 1



Scale = 1:37.8

Plate Offsets (X, Y): [2:0-5-3,0-1-9], [4:0-4-0,0-1-15], [7:0-5-3,0-1-9]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.57	Vert(LL)	-0.03	9-10	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.22	Vert(CT)	-0.05	9-10	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.09	Horz(CT)	0.01	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 71 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 2-6-0, Right 2x4 SP No.3 -- 2-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-3-9 oc purlins, except 2-0-0 oc purlins (5-0-15 max.): 4-5.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size) 2=0-3-0, 7=0-3-0
Max Horiz 2=-24 (LC 9)
Max Uplift 2=-43 (LC 11), 7=-43 (LC 12)
Max Grav 2=949 (LC 36), 7=949 (LC 36)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-4=-1096/71, 4-5=-943/77, 5-7=-1091/70, 7-8=0/32
BOT CHORD 2-10=-39/959, 9-10=-42/947, 7-9=-24/954
WEBS 4-10=0/211, 4-9=-46/37, 5-9=0/208

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft, Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.

- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP 2400F 2.0E .
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 43 lb uplift at joint 2 and 43 lb uplift at joint 7.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 230 lb down and 15 lb up at 4-0-0, and 230 lb down and 15 lb up at 7-11-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-48, 4-5=-58, 5-8=-48, 11-17=-20
Concentrated Loads (lb)
Vert: 5=-74 (B), 10=-230 (B), 9=-230 (B), 4=-74 (B), 24=-69 (B), 26=-25 (B)



April 23, 2025

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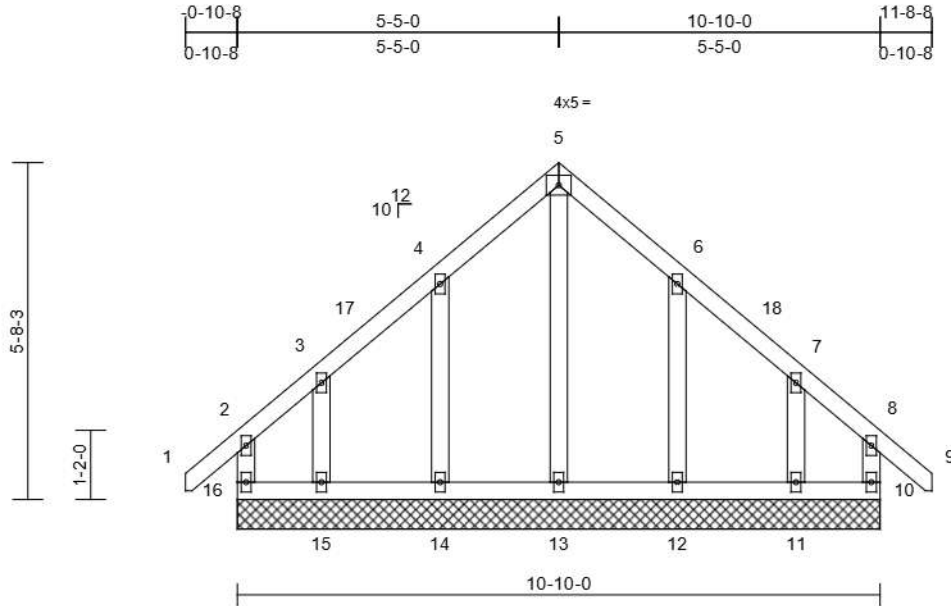
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956722
25040106	C1	Common Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:41
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Scale = 1:38.9

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	n/a	-	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.00	10	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0										
										Weight: 66 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size)	10=10-10-0, 11=10-10-0, 12=10-10-0, 13=10-10-0, 14=10-10-0, 15=10-10-0, 16=10-10-0
Max Horiz	16=-124 (LC 11)
Max Uplift	10=-56 (LC 10), 11=-62 (LC 9), 12=-36 (LC 14), 14=-36 (LC 13), 15=-68 (LC 10), 16=-67 (LC 9)
Max Grav	10=133 (LC 29), 11=166 (LC 30), 12=180 (LC 30), 13=162 (LC 32), 14=179 (LC 29), 15=171 (LC 29), 16=142 (LC 30)

FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-16=-114/113, 1-2=0/47, 2-3=-77/79, 3-4=-60/143, 4-5=-116/256, 5-6=-116/255, 6-7=-61/145, 7-8=-66/69, 8-9=0/47, 8-10=-109/112
BOT CHORD	15-16=-63/71, 14-15=-63/71, 13-14=-63/71, 12-13=-63/71, 11-12=-63/71, 10-11=-63/71
WEBS	5-13=-241/47, 4-14=-176/152, 3-15=-141/145, 6-12=-176/152, 7-11=-141/145

NOTES

- Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-9-14 to 2-2-2, Exterior(2N) 2-2-2 to 5-5-0, Corner (3R) 5-5-0 to 8-5-0, Exterior(2N) 8-5-0 to 11-7-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 67 lb uplift at joint 16, 56 lb uplift at joint 10, 36 lb uplift at joint 14, 68 lb uplift at joint 15, 36 lb uplift at joint 12 and 62 lb uplift at joint 11.

LOAD CASE(S) Standard



April 23, 2025

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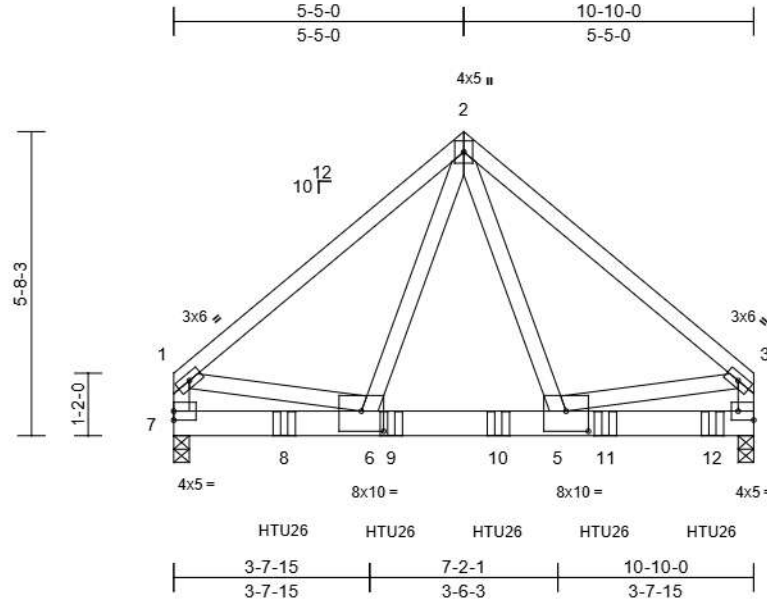
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956723
25040106	C2	Common Girder	1	2	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:41
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Page: 1



Scale = 1:43.1									
Plate Offsets (X, Y): [4:Edge,0-2-0], [5:0-5-0,0-4-8], [6:0-5-0,0-4-8]									
Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.44	Vert(LL)	-0.03	5-6	>999
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.06	5-6	>999
TCDL	10.0	Rep Stress Incr	NO	WB	0.54	Horz(CT)	0.00	4	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH					
BCDL	10.0								
Weight: 148 lb FT = 20%									

LUMBER	
TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP 2400F 2.0E
WEBS	2x4 SP No.3 *Except* 7-1,4-3:2x4 SP No.2
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS	
(size)	4=0-3-8, 7=0-3-8
Max Horiz	7=103 (LC 7)
Max Grav	4=4751 (LC 3), 7=3814 (LC 3)
FORCES	
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-3862/0, 2-3=-3899/0, 1-7=-3166/0, 3-4=-3178/0
BOT CHORD	6-7=-39/505, 5-6=0/2167, 4-5=0/506
WEBS	1-6=0/2597, 3-5=0/2549, 2-6=0/2398, 2-5=0/2487

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 3 rows staggered at 0-8-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope); cantilever left and right exposed; end vertical left and right exposed;
Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP 2400F 2.0E .
- Use Simpson Strong-Tie HTU26 (10-16d Girder, 14-10dx1 1/2 Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-12 from the left end to 10-0-12 to connect truss(es) to back face of bottom chord.
- Fill all nail holes where hanger is in contact with lumber.

- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-2=-46, 2-3=-46, 4-7=-19
Concentrated Loads (lb)
Vert: 8=-1220 (B), 9=-1215 (B), 10=-1215 (B), 11=-1215 (B), 12=-1219 (B)



April 23, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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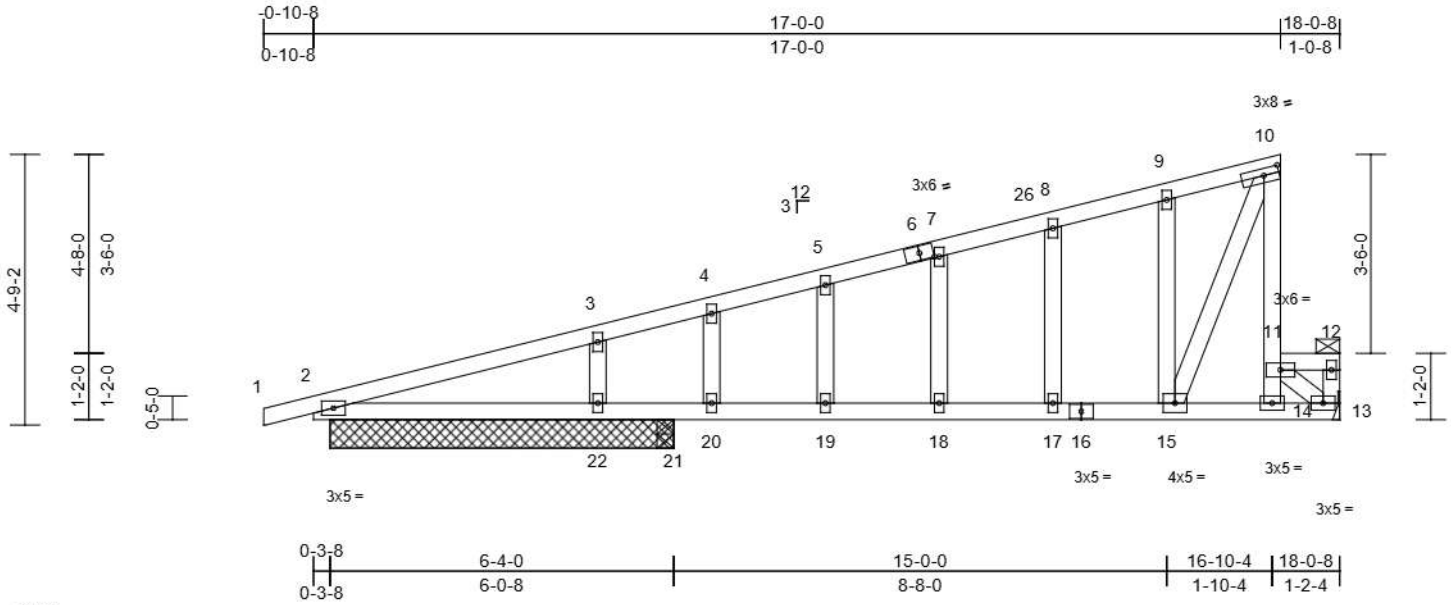
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956724
25040106	D1	Half Hip Supported Gable	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 E May 9 2024 Print: 8.730 E May 9 2024 MiTek Industries, Inc. Wed Apr 23 16:15:50
ID:p12iSP7BQcIN33S46ITSliZ085d-aSNE3v3CoxDFWZDGH?UNQ4jTnFvmjwTD8q2yWzNo07

Page: 1



Scale = 1:40.6

Plate Offsets (X, Y): [6:0-2-15,0-1-8], [10:0-3-4,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.71	Vert(LL)	-0.11	18	>999	240	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.51	Vert(CT)	-0.20	18	>683	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.42	Horz(CT)	0.01	13	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 92 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP 2400F 2.0E *Except* 11-12:2x4 SP No.2
BOT CHORD	2x4 SP 2400F 2.0E
WEBS	2x4 SP No.3 *Except* 10-14:2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-4-3 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 11-14, 11-12.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size)	2=296/6-0-8, 13=696/ Mechanical, 21=568/0-3-8, 22=3/6-0-8, 23=296/6-0-8
Max Horiz	2=188 (LC 15), 23=188 (LC 15)
Max Uplift	2=-12 (LC 11), 13=-26 (LC 15), 21=-26 (LC 11), 22=-11 (LC 15), 23=-12 (LC 11)
Max Grav	2=381 (LC 41), 13=767 (LC 2), 21=777 (LC 41), 22=16 (LC 41), 23=381 (LC 41)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=0/16, 2-3=-531/23, 3-4=-517/61, 4-5=-566/99, 5-7=-529/115, 7-8=-508/133, 8-9=-499/157, 9-10=-540/192, 11-14=-694/281, 10-11=-1190/383, 11-12=-88/46, 12-13=-137/50
BOT CHORD	2-22=-186/558, 21-22=-186/558, 20-21=-186/558, 19-20=-186/558, 18-19=-186/558, 17-18=-186/558, 15-17=-186/558, 14-15=-60/160, 13-14=-186/584
WEBS	9-15=-315/138, 8-17=-101/72, 7-18=-53/52, 5-19=-13/44, 4-20=-348/132, 3-22=-227/139, 11-13=-637/195, 10-15=-332/1017

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) -0-10-5 to 2-1-11, Exterior(2N) 2-1-11 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.

- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 12 lb uplift at joint 2, 11 lb uplift at joint 22, 26 lb uplift at joint 13, 26 lb uplift at joint 21 and 12 lb uplift at joint 2.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 404 lb down and 100 lb up at 16-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-10=-48, 11-12=-58, 13-23=-20
Concentrated Loads (lb)
Vert: 10=-300



April 23, 2025

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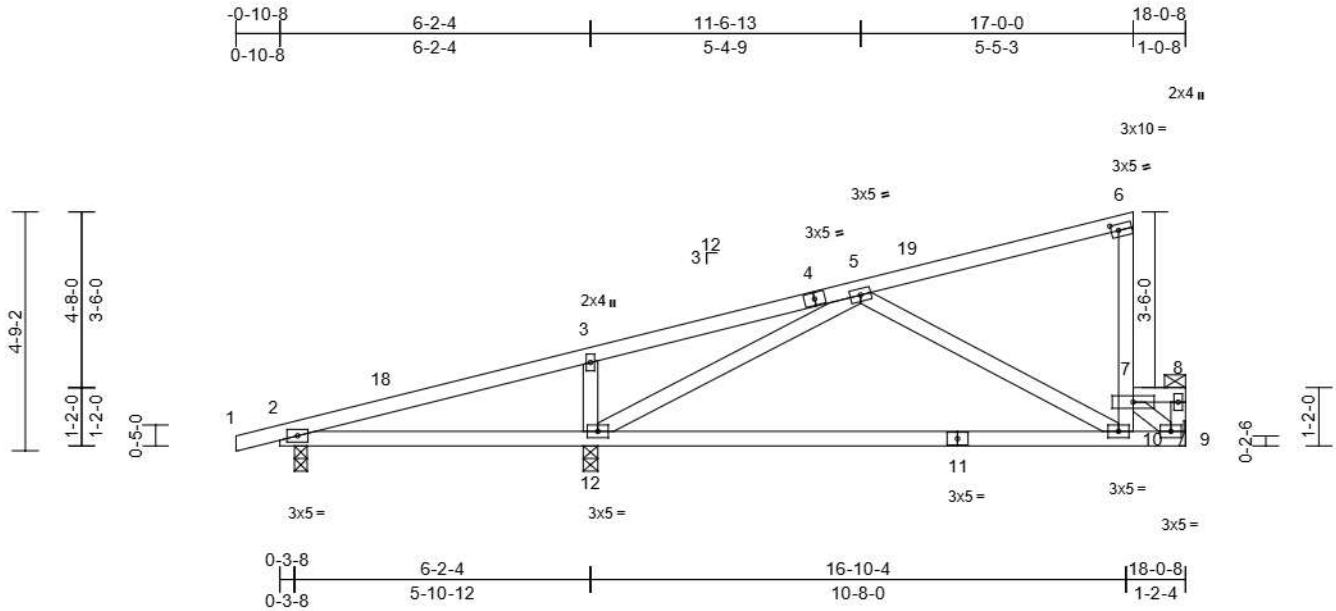
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956725
25040106	D2	Half Hip	4	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:41
ID:3upO1vafwlf?Gyfbf4lgCWztCnV-RfC?PsB70Hq3NSgPqnL8w3uITXbGKwRCDot7J4zJC?f

Page: 1



Scale = 1:46
Plate Offsets (X, Y): [6:0-1-12,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.07	10-12	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.34	Vert(CT)	-0.31	10-12	>458	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.53	Horz(CT)	0.00	9	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 84 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* 7-8:2x4 SP No.2
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 6-10:2x4 SP 2400F 2.0E

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.); 7-10, 7-8.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 2=0-3-0, 9= Mechanical, 12=0-3-8
Max Horiz 2=188 (LC 15)
Max Uplift 2=-47 (LC 11), 9=-53 (LC 15)
Max Grav 2=209 (LC 41), 9=680 (LC 2), 12=1040 (LC 41)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-78/323, 3-5=-28/297, 5-6=-301/31, 7-10=0/251, 6-7=-501/230, 7-8=-67/32, 8-9=-220/39
BOT CHORD 2-12=-347/110, 10-12=-324/475, 9-10=-237/733
WEBS 7-9=-864/262, 3-12=-446/145, 5-10=-270/210, 5-12=-721/82

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-10-5 to 2-1-11, Interior (1) 2-1-11 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- All plates are 3x5 MT20 unless otherwise indicated.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 2 SP 2400F 2.0E, Joint 12 SP 2400F 2.0E.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 9 and 47 lb uplift at joint 2.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 379 lb down and 132 lb up at 16-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S)

- Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-6=-48, 7-8=-58, 9-13=-20
Concentrated Loads (lb)
Vert: 6=-300



April 23, 2025

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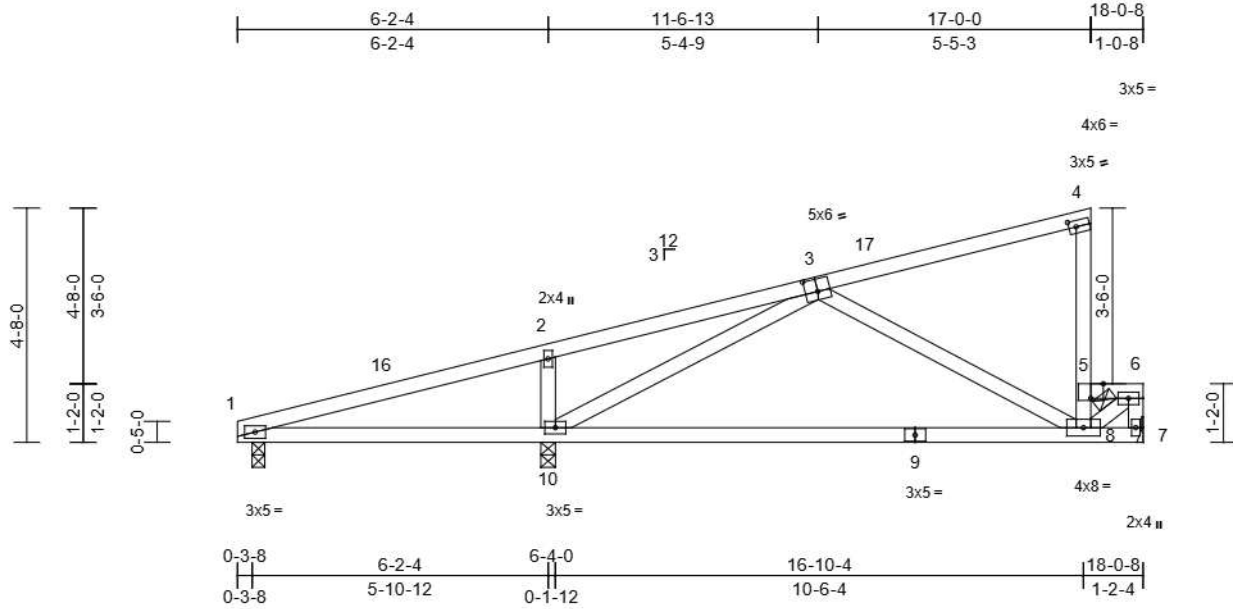
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956726
25040106	D3	Half Hip	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:41
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Page: 1



Scale = 1:46

Plate Offsets (X, Y): [3:0-3-0,0-3-0], [4:0-1-12,0-1-8], [5:0-3-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	1.00	Vert(LL)	-0.08	8-10	>999	240	MT20	244/190
Snow (Pf/Pg)	18.9/20.0	Lumber DOL	1.15	BC	0.36	Vert(CT)	-0.33	8-10	>424	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.56	Horz(CT)	0.00	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 82 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* 5-6:2x4 SP No.2
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* 4-8:2x4 SP No.1

BRACING

TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-8, 5-6.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size) 1=0-3-0, 7= Mechanical, 10=0-3-8
Max Horiz 1=184 (LC 15)
Max Uplift 1=-26 (LC 11), 7=-55 (LC 15)
Max Grav 1=154 (LC 40), 7=673 (LC 2), 10=1069 (LC 40)

FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-71/375, 2-4=-292/348, 5-8=-327/218, 4-5=-505/230, 5-6=-660/204, 6-7=-735/200
BOT CHORD 1-10=-393/122, 8-10=-328/453, 7-8=-41/84
WEBS 6-8=-240/776, 2-10=-449/152, 3-8=-255/214, 3-10=-757/63

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=18.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10, Lu=50-0-0
- Unbalanced snow loads have been considered for this design.
- Provide adequate drainage to prevent water ponding.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 1 SP 2400F 2.0E, Joint 10 SP 2400F 2.0E.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 7 and 26 lb uplift at joint 1.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 379 lb down and 132 lb up at 16-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S)

- Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-4=-48, 5-6=-58, 7-11=-20
Concentrated Loads (lb)
Vert: 4=-300



April 23, 2025

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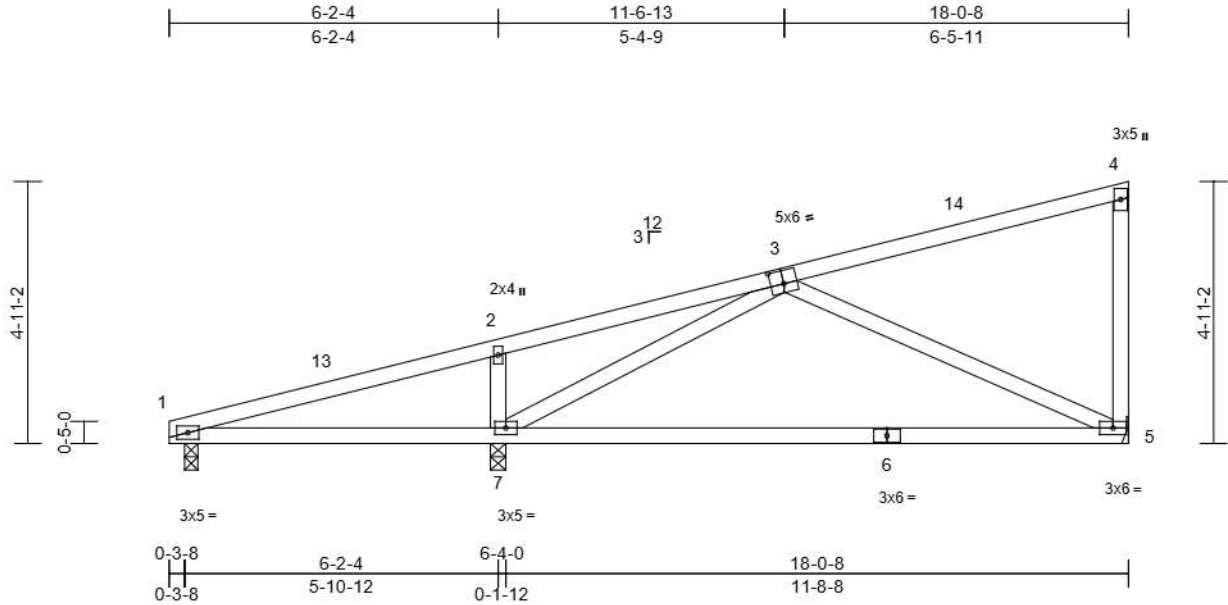
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956727
25040106	D4	Monopitch	2	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Page: 1



Scale = 1:43.4									
Plate Offsets (X, Y): [3:0-3-0,0-3-0]									
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	L/d
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.53	Vert(LL)	-0.02	7-12	>999
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	-0.36	5-7	>387
TCDL	10.0	Rep Stress Incr	YES	WB	0.44	Horz(CT)	0.01	5	n/a
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH					
BCDL	10.0								
					PLATES		GRIP		
					MT20		244/190		
					Weight: 82 lb		FT = 20%		

LUMBER	
TOP CHORD	2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP 2400F 2.0E
WEBS	2x4 SP No.3
BRACING	
TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
REACTIONS	
(size)	1=0-3-0, 5= Mechanical, 7=0-3-8
Max Horiz	1=133 (LC 14)
Max Uplift	1=-29 (LC 11), 5=-31 (LC 15)
Max Grav	1=232 (LC 2), 5=478 (LC 21), 7=746 (LC 2)
FORCES	
(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-206/106, 2-4=-221/158, 4-5=-188/102
BOT CHORD	1-7=-219/224, 5-7=-267/502
WEBS	2-7=-338/149, 3-7=-360/55, 3-5=-467/241

- NOTES**
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 3) Unbalanced snow loads have been considered for this design.

- 4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 5) Bearings are assumed to be: Joint 1 SP 2400F 2.0E, Joint 7 SP 2400F 2.0E.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 5 and 29 lb uplift at joint 1.

LOAD CASE(S) Standard



April 23, 2025

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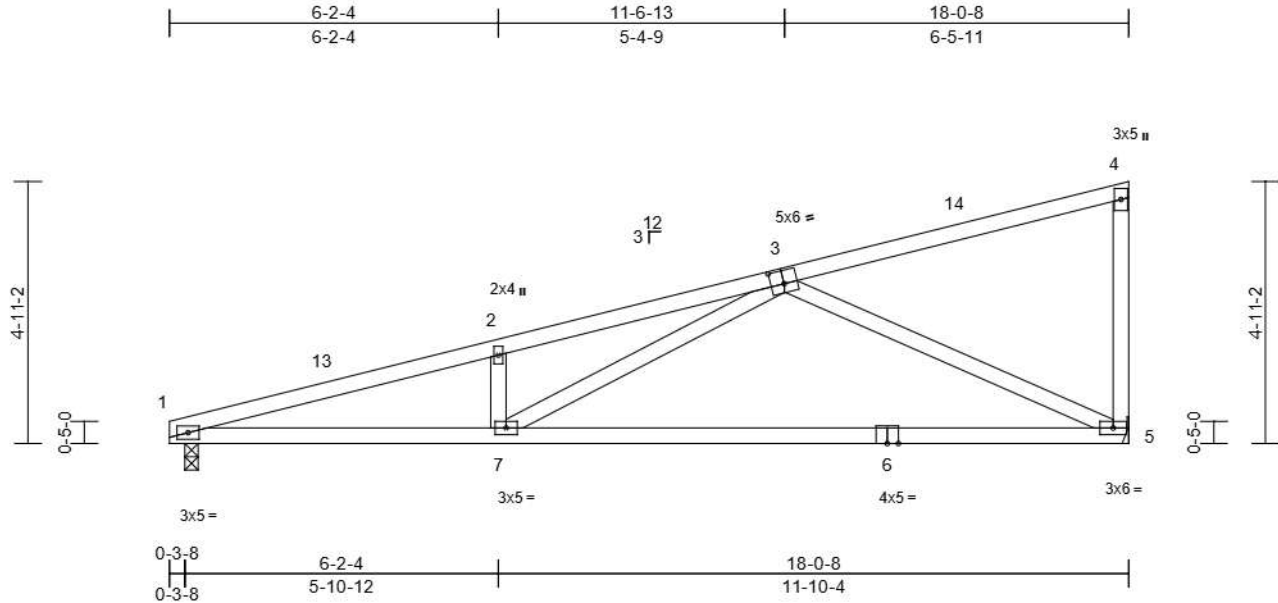
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956728
25040106	D5	Monopitch	3	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:43.4

Plate Offsets (X, Y): [3:0-3:0,0-3:0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.57	Vert(LL)	-0.06	5-7	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.46	5-7	>462	180	
TCDL	10.0	Rep Stress Incr	YES	WB	1.00	Horz(CT)	0.02	5	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 82 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-9-13 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=0-3-0, 5= Mechanical
Max Horiz 1=133 (LC 14)
Max Uplift 1=-8 (LC 11), 5=-21 (LC 15)
Max Grav 1=728 (LC 2), 5=704 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-1945/221, 2-4=-1944/272, 4-5=-183/102
BOT CHORD 1-7=-301/1878, 5-7=-293/1030
WEBS 2-7=-269/144, 3-5=-1048/270, 3-7=-34/975

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft, Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 3-0-0, Interior (1) 3-0-0 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- Bearings are assumed to be: Joint 1 SP 2400F 2.0E.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 5 and 8 lb uplift at joint 1.

LOAD CASE(S) Standard



April 23, 2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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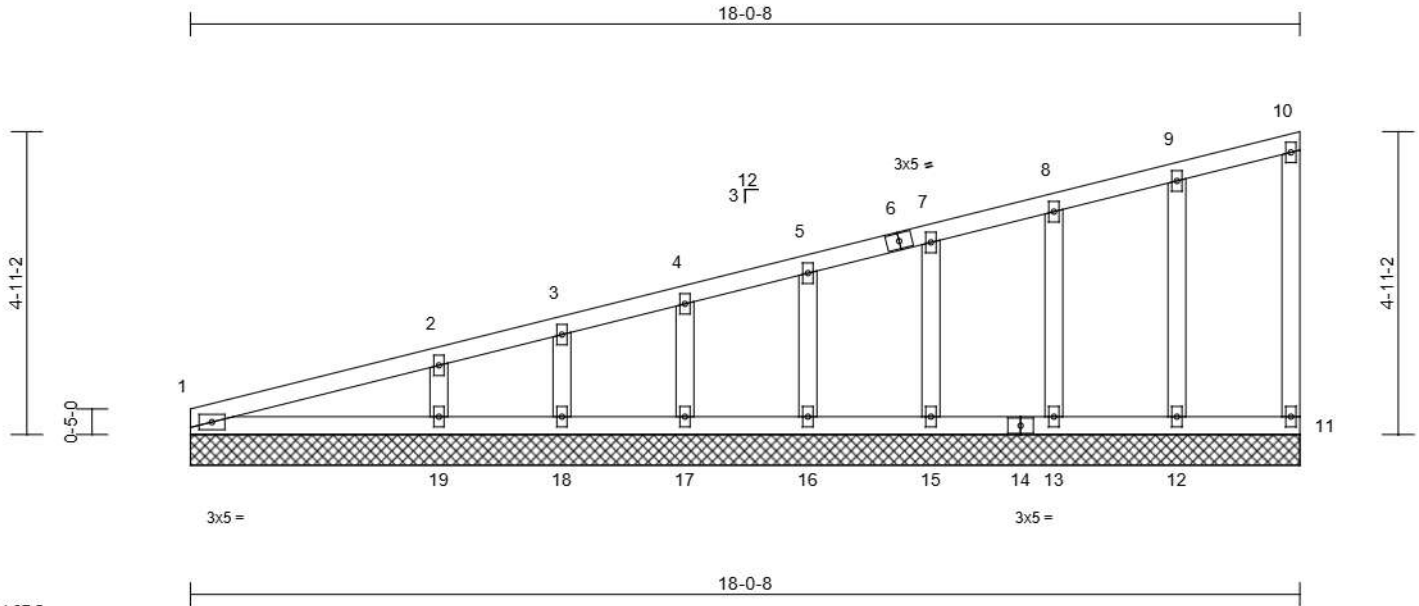
818 Soundside Road
Edenton, NC 27932

Job 25040106	Truss D6	Truss Type Monopitch Supported Gable	Qty 1	Ply 1	13 Eagle Creek - Edisto C - Roof Job Reference (optional)	172956729
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Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Page: 1



Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.15	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 86 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)
1=18-0-8, 11=18-0-8, 12=18-0-8,
13=18-0-8, 15=18-0-8, 16=18-0-8,
17=18-0-8, 18=18-0-8, 19=18-0-8
Max Horiz 1=133 (LC 14)
Max Uplift 11=-4 (LC 12), 12=-6 (LC 11),
13=-6 (LC 15), 15=-7 (LC 11),
16=-6 (LC 15), 17=-8 (LC 15),
18=-1 (LC 11), 19=-18 (LC 15)
Max Grav 1=132 (LC 2), 11=68 (LC 21),
12=184 (LC 21), 13=177 (LC 21),
15=167 (LC 21), 16=155 (LC 2),
17=179 (LC 2), 18=86 (LC 2),
19=330 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-262/153, 2-3=-219/130, 3-4=-199/129,
4-5=-172/120, 5-7=-147/114, 7-8=-121/106,
8-9=-96/100, 9-10=-80/96, 10-11=-52/33
BOT CHORD 1-19=-128/130, 18-19=-79/106,
17-18=-79/106, 16-17=-79/106,
15-16=-79/106, 13-15=-79/106,
12-13=-79/106, 11-12=-79/106
WEBS 9-12=-147/103, 8-13=-137/89, 7-15=-126/83,
5-16=-120/81, 4-17=-133/88, 3-18=-80/60,
2-19=-228/142

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust)
Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) 0-0-0 to 3-0-0, Exterior(2N) 3-0-0 to 17-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.2.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 11, 6 lb uplift at joint 12, 6 lb uplift at joint 13, 7 lb uplift at joint 15, 6 lb uplift at joint 16, 8 lb uplift at joint 17, 1 lb uplift at joint 18 and 18 lb uplift at joint 19.

LOAD CASE(S) Standard



April 23, 2025

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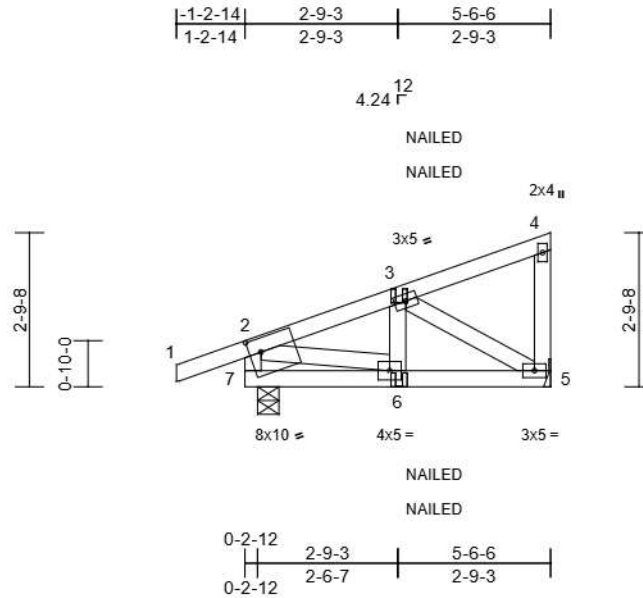
Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof
25040106	D7	Diagonal Hip Girder	2	1	Job Reference (optional)

I72956730

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Page: 1



Scale = 1:41.7

Plate Offsets (X, Y): [7:0-2-8,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	0.00	6	>999	240	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	0.00	6-7	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.10	Horz(CT)	0.00	5	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										
										Weight: 32 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or
 5-6-6 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc
 bracing.

REACTIONS (size) 5= Mechanical, 7=0-4-10

Max Horiz 7=79 (LC 8)
 Max Uplift 5=-8 (LC 11), 7=-38 (LC 7)
 Max Grav 5=233 (LC 18), 7=315 (LC 18)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 2-7=-292/50, 1-2=0/34, 2-3=-275/2,
 3-4=-49/20, 4-5=-78/14
 BOT CHORD 6-7=-77/10, 5-6=-17/230
 WEBS 2-6=0/236, 3-6=0/43, 3-5=-265/18

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust)
 Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat.
 II; Exp B; Enclosed; MWFRS (envelope); cantilever left
 and right exposed; end vertical left and right exposed;
 Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15
 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum
 DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully
 Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this
 design.
- This truss has been designed for greater of min roof live
 load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on
 overhangs non-concurrent with other live loads.

- * This truss has been designed for a live load of 20.0psf
 on the bottom chord in all areas where a rectangle
 3-06-00 tall by 2-00-00 wide will fit between the bottom
 chord and any other members.
- Bearings are assumed to be: Joint 7 SP No.2 .
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to
 bearing plate capable of withstanding 38 lb uplift at joint
 7 and 8 lb uplift at joint 5.
- "NAILED" indicates 3-10d (0.148"x3") or 2-12d
 (0.148"x3.25") toe-nails per NDS guidelines.
- In the LOAD CASE(S) section, loads applied to the face
 of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate
 Increase=1.15
 Uniform Loads (lb/ft)
 Vert: 1-2=-48, 2-4=-48, 5-7=-20
 Concentrated Loads (lb)
 Vert: 6=-1 (F=0, B=0)



April 23, 2025

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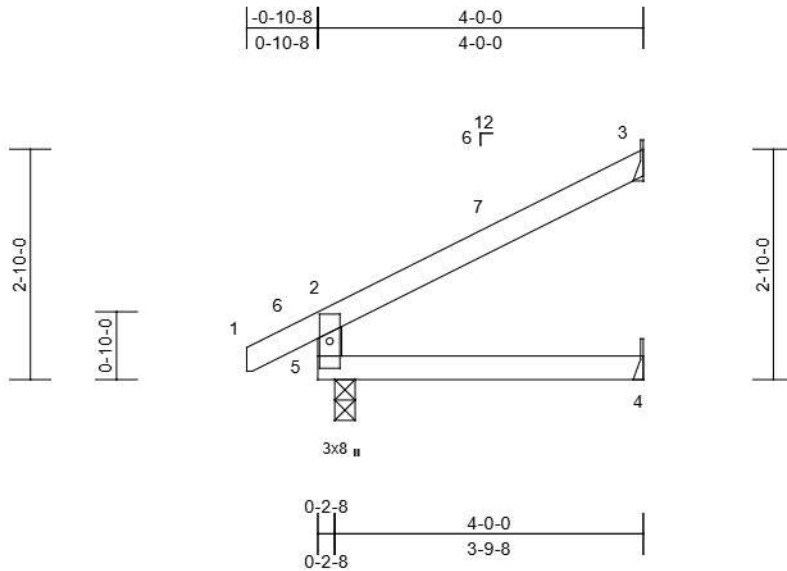
818 Soundside Road
 Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956731
25040106	D8	Jack-Open	3	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Page: 1



Scale = 1:28.4

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	0.01	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.19	Vert(CT)	-0.02	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0											
											Weight: 15 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)	3= Mechanical, 4= Mechanical, 5=0-3-0
Max Horiz	5=55 (LC 15)
Max Uplift	3=-35 (LC 15)
Max Grav	3=127 (LC 22), 4=45 (LC 22), 5=266 (LC 22)

FORCES (lb) - Maximum Compression/Maximum Tension	
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TOP CHORD	2-5=-236/137, 1-2=0/33, 2-3=-75/50
BOT CHORD	4-5=0/0

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-10-1 to 2-1-15, Interior (1) 2-1-15 to 3-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06'-00 tall by 2'-00'-00 wide will fit between the bottom chord and any other members.
- 6) Bearings are assumed to be: Joint 5 SP No.2.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 3.

LOAD CASE(S) Standard



April 23, 2025

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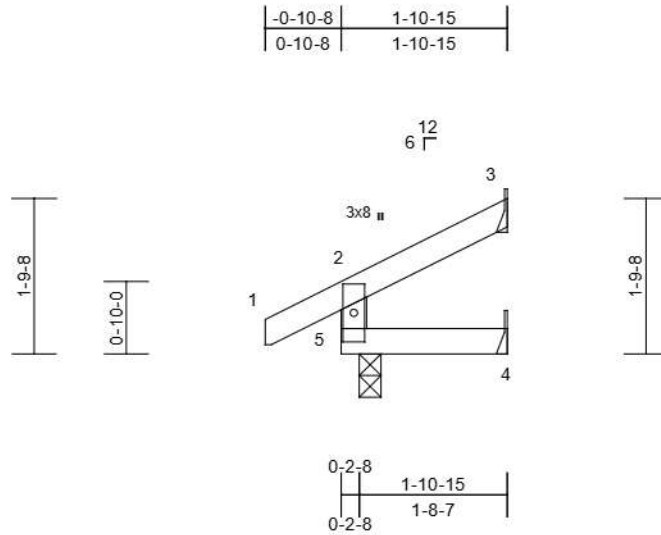
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956732
25040106	D9	Jack-Open	4	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Page: 1



Scale = 1:26.6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 8 lb	FT = 20%

LUMBER		
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
WEBS	2x4 SP No.3	
BRACING		
TOP CHORD	Structural wood sheathing directly applied or 1-10-15 oc purlins, except end verticals.	
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.	
REACTIONS	(size)	3= Mechanical, 4= Mechanical, 5=0-3-0
	Max Horiz	5=32 (LC 12)
	Max Uplift	3=18 (LC 15)
	Max Grav	3=44 (LC 22), 4=18 (LC 13), 5=165 (LC 22)
FORCES	(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	2-5=-145/98, 1-2=0/33, 2-3=-34/22	
BOT CHORD	4-5=0/0	

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 13.9 psf on overhangs non-concurrent with other live loads.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Bearings are assumed to be: Joint 5 SP No.2.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 3.

LOAD CASE(S) Standard



April 23, 2025

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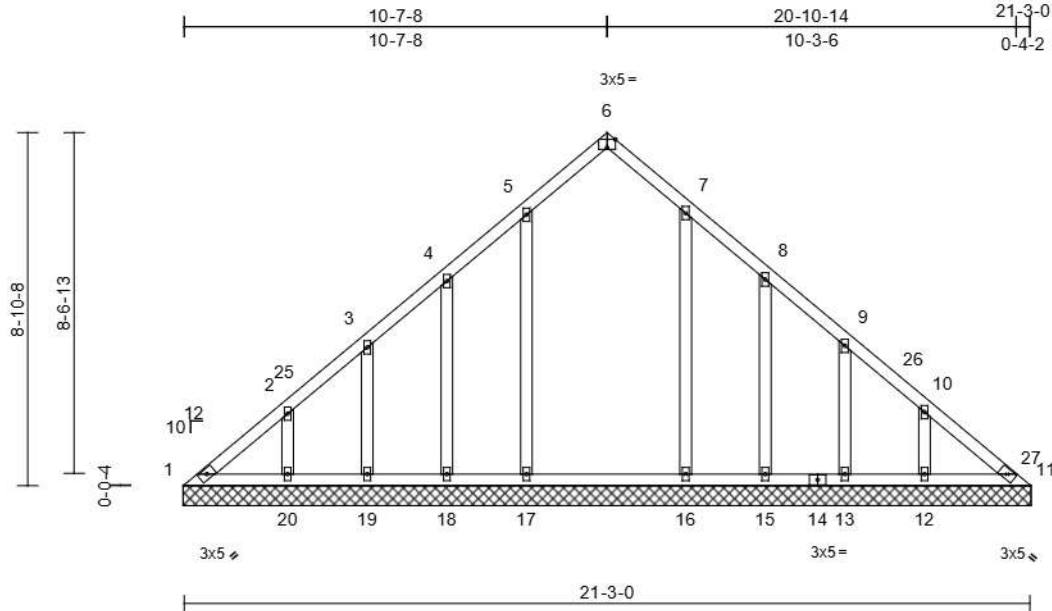
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956733
25040106	V1	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Page: 1



Scale = 1:58

Plate Offsets (X, Y): [6:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.07	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.14	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horiz(TL)	0.01	11	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 121 lb FT = 20%	

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size)
1=21-3-10, 11=21-3-10,
12=21-3-10, 13=21-3-10,
15=21-3-10, 16=21-3-10,
17=21-3-10, 18=21-3-10,
19=21-3-10, 20=21-3-10
Max Horiz 1=163 (LC 10)
Max Uplift 1=-2 (LC 11), 12=-14 (LC 14),
13=-43 (LC 14), 15=-52 (LC 14),
16=-4 (LC 14), 17=-10 (LC 13),
18=-51 (LC 13), 19=-41 (LC 13),
20=-21 (LC 13)
Max Grav 1=152 (LC 30), 11=134 (LC 31),
12=264 (LC 29), 13=188 (LC 29),
15=166 (LC 29), 16=303 (LC 29),
17=312 (LC 28), 18=163 (LC 28),
19=186 (LC 28), 20=273 (LC 28)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-273/127, 2-3=-191/76, 3-4=-131/49,
4-5=-117/30, 5-6=-128/61, 6-7=-127/61,
7-8=-109/20, 8-9=-128/40, 9-10=-190/75,
10-11=-273/127
BOT CHORD 1-20=-102/251, 19-20=-102/251,
18-19=-102/251, 17-18=-102/251,
16-17=-102/251, 15-16=-102/251,
13-15=-102/251, 12-13=-102/251,
11-12=-102/251

WEBS
5-17=-159/80, 4-18=-151/114,
3-19=-143/104, 2-20=-177/106,
7-16=-157/77, 8-15=-152/115,
9-13=-142/104, 10-12=-179/109

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Corner (3E) 0-0-0 to 3-0-0, Exterior(2N) 3-0-0 to 10-7-13, Corner(3R) 10-7-13 to 13-7-13, Exterior(2N) 13-7-13 to 20-10-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1, 10 lb uplift at joint 17, 51 lb uplift at joint 18, 41 lb uplift at joint 19, 21 lb uplift at joint 20, 4 lb uplift at joint 16, 52 lb uplift at joint 15, 43 lb uplift at joint 13 and 14 lb uplift at joint 12.

11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 11.

LOAD CASE(S) Standard



April 23, 2025

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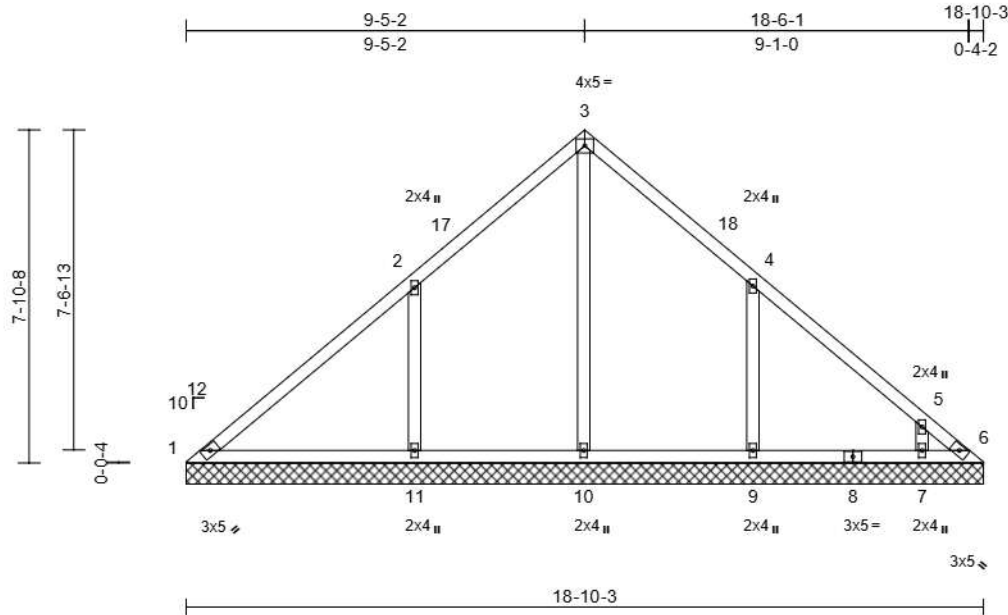
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956734
25040106	V2	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:42
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.33	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.28	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.26	Horiz(TL)	0.01	11	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 87 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=18-10-3, 6=18-10-3, 7=18-10-3, 9=18-10-3, 10=18-10-3, 11=18-10-3
Max Horiz	1=144 (LC 10)
Max Uplift	1=-35 (LC 9), 7=-5 (LC 14), 9=-104 (LC 14), 11=-110 (LC 13)
Max Grav	1=217 (LC 29), 6=0 (LC 29), 7=238 (LC 29), 9=490 (LC 29), 10=427 (LC 31), 11=597 (LC 28)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-277/215, 2-3=-145/189, 3-4=-137/179, 4-5=-76/100, 5-6=-77/101
BOT CHORD	1-11=-47/193, 10-11=-47/56, 9-10=-47/56, 7-9=-47/56, 6-7=-47/56
WEBS	3-10=-243/18, 2-11=-401/192, 4-9=-342/181, 5-7=-229/120

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 9-5-6, Exterior(2R) 9-5-6 to 12-5-6, Interior (1) 12-5-6 to 18-5-15 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 1, 110 lb uplift at joint 11, 104 lb uplift at joint 9 and 5 lb uplift at joint 7.

LOAD CASE(S) Standard



April 23, 2025

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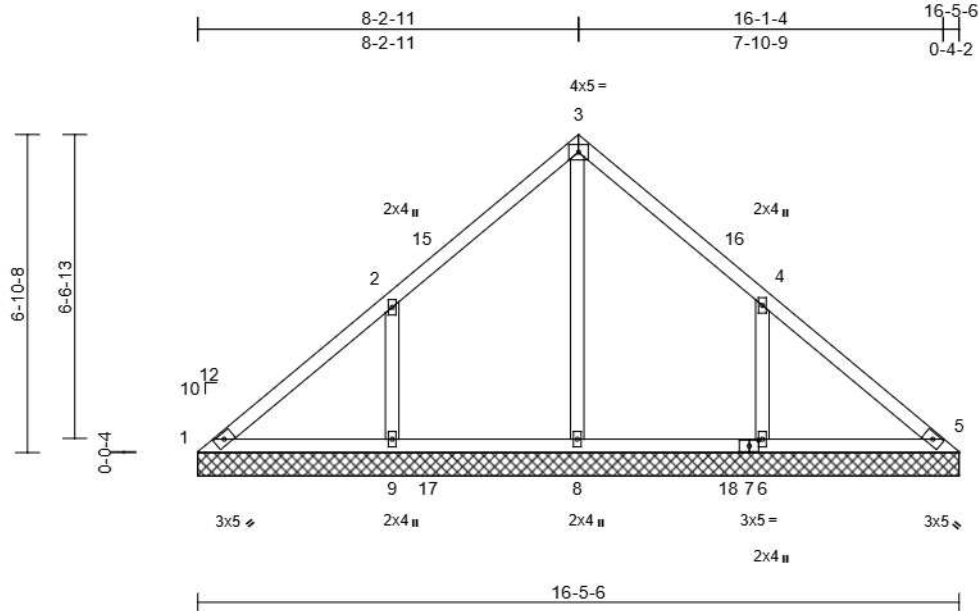
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	I72956735
25040106	V3	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:43
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Scale = 1:49.8

Plate Offsets (X, Y): [7:0-2-7,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.28	Vert(LL)	n/a	-	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.17	Vert(TL)	n/a	-	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.36	Horiz(TL)	0.00	5	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 73 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (size) 1=16-5-6, 5=16-5-6, 6=16-5-6, 8=16-5-6, 9=16-5-6
Max Horiz 1=125 (LC 10)
Max Uplift 1=-46 (LC 9), 6=-89 (LC 14), 9=-94 (LC 13)
Max Grav 1=77 (LC 34), 5=0 (LC 29), 6=484 (LC 29), 8=637 (LC 28), 9=482 (LC 28)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-83/339, 2-3=-4/282, 3-4=-5/266, 4-5=-97/287
BOT CHORD 1-9=-189/57, 8-9=-189/50, 6-8=-187/50, 5-6=-187/50
WEBS 3-8=-447/0, 2-9=-340/179, 4-6=-340/177

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 8-3-0, Exterior(2R) 8-3-0 to 11-3-0, Interior (1) 11-3-0 to 16-1-3 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 1, 94 lb uplift at joint 9 and 89 lb uplift at joint 6.

LOAD CASE(S) Standard



April 23, 2025

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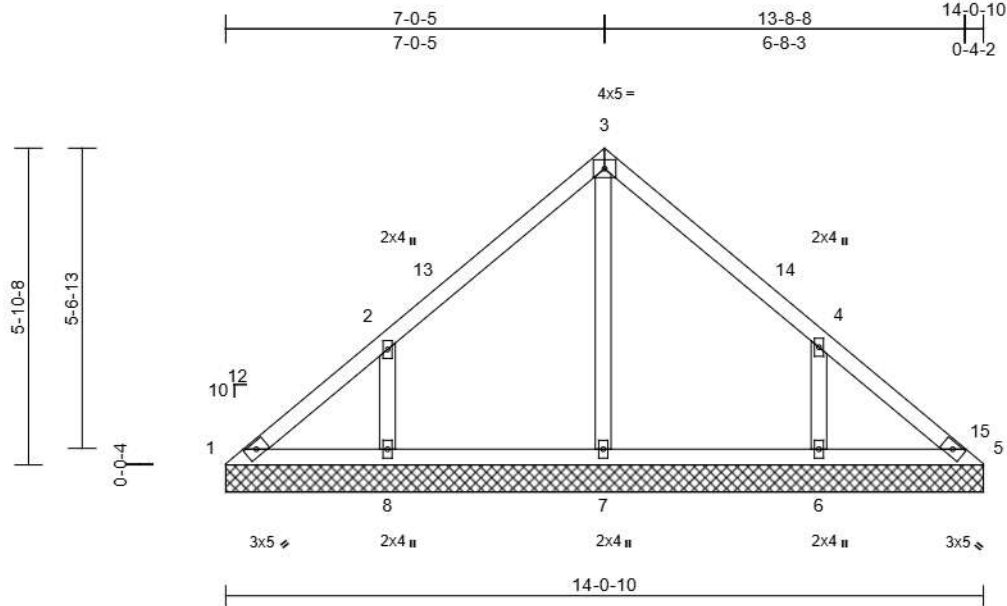
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956736
25040106	V4	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:43
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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.08	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.12	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 60 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=14-0-10, 5=14-0-10, 6=14-0-10, 7=14-0-10, 8=14-0-10
Max Horiz	1=107 (LC 10)
Max Uplift	1=-14 (LC 9), 6=-75 (LC 14), 8=-78 (LC 13)
Max Grav	1=108 (LC 29), 5=77 (LC 28), 6=344 (LC 29), 7=281 (LC 2), 8=348 (LC 28)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-132/110, 2-3=-121/113, 3-4=-121/108, 4-5=-101/82
BOT CHORD	1-8=-44/105, 7-8=-44/70, 6-7=-44/70, 5-6=-44/79
WEBS	3-7=-203/0, 2-8=-310/190, 4-6=-307/187

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 7-0-10, Exterior(2R) 7-0-10 to 10-0-10, Interior (1) 10-0-10 to 13-8-6 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 1, 78 lb uplift at joint 8 and 75 lb uplift at joint 6.

LOAD CASE(S) Standard



April 23, 2025

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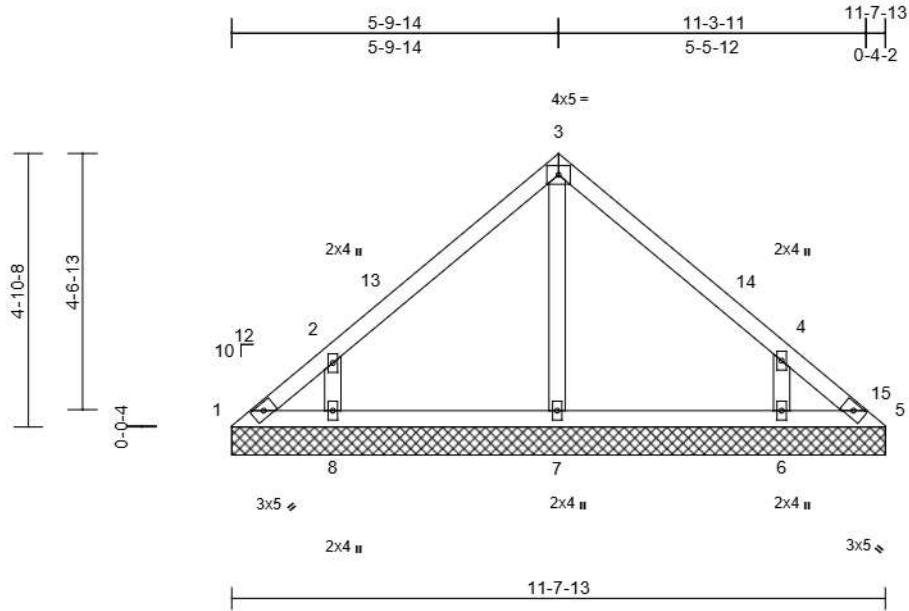
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956737
25040106	V5	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:43
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Scale = 1:41.1

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.09	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	5	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										
										Weight: 48 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(size)	1=11-7-13, 5=11-7-13, 6=11-7-13, 7=11-7-13, 8=11-7-13
Max Horiz	1=88 (LC 10)
Max Uplift	1=-26 (LC 9), 5=-9 (LC 12), 6=-66 (LC 14), 8=-70 (LC 13)
Max Grav	1=72 (LC 29), 5=46 (LC 28), 6=308 (LC 29), 7=236 (LC 2), 8=316 (LC 28)

FORCES

(lb) - Maximum Compression/Maximum Tension	
TOP CHORD	1-2=-97/82, 2-3=-164/114, 3-4=-164/110, 4-5=-82/52
BOT CHORD	1-8=-23/60, 7-8=-18/60, 6-7=-18/61, 5-6=-20/61
WEBS	3-7=-149/0, 2-8=-326/239, 4-6=-316/231

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf, BCDL=6.0psf, h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 5-10-3, Exterior(2R) 5-10-3 to 8-10-3, Interior (1) 8-10-3 to 11-3-9 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 1, 9 lb uplift at joint 5, 70 lb uplift at joint 8 and 66 lb uplift at joint 6.

LOAD CASE(S) Standard



April 23, 2025

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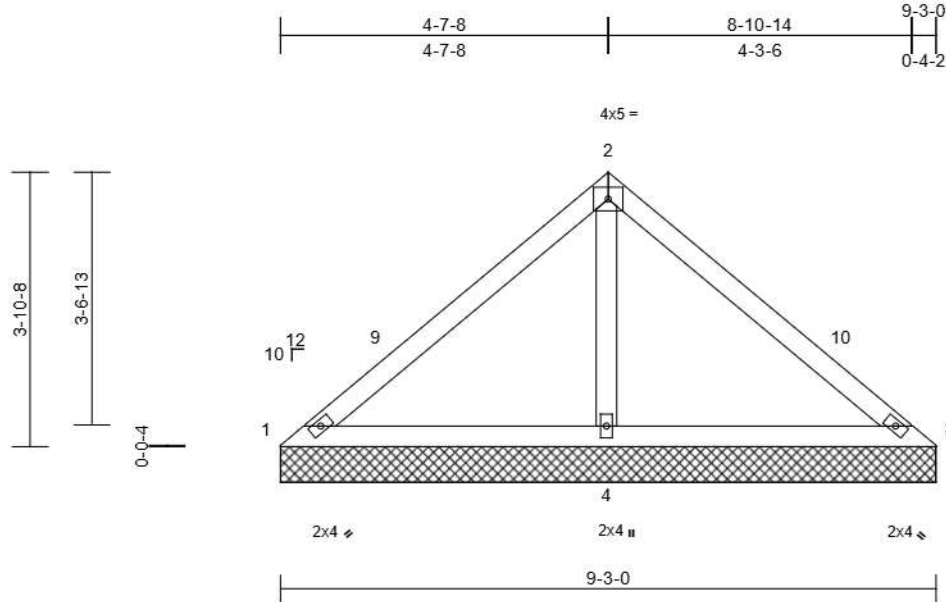
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956738
25040106	V6	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:43
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Scale = 1:32.6

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.24	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.23	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.15	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 35 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 9-3-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=9-3-0, 3=9-3-0, 4=9-3-0
Max Horiz	1=69 (LC 12)
Max Uplift	1=-22 (LC 35), 3=-19 (LC 34), 4=-15 (LC 13)
Max Grav	1=71 (LC 34), 3=74 (LC 35), 4=669 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-118/311, 2-3=-111/306
BOT CHORD	1-4=-228/174, 3-4=-224/172
WEBS	2-4=-598/262

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-5 to 3-0-5, Interior (1) 3-0-5 to 4-7-13, Exterior(2R) 4-7-13 to 7-7-13, Interior (1) 7-7-13 to 9-3-5 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 1, 19 lb uplift at joint 3 and 15 lb uplift at joint 4.

LOAD CASE(S) Standard



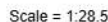
April 23, 2025

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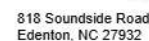
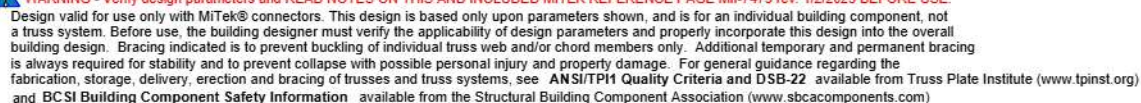
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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April 23, 2025

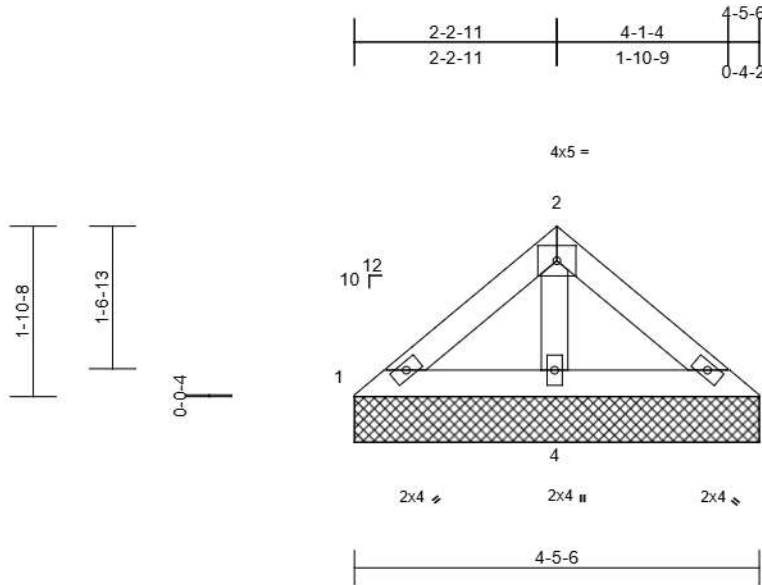


Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956740
25040106	V8	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.06	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 16 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-5-6 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=4-5-6, 3=4-5-6, 4=4-5-6
Max Horiz	1=32 (LC 12)
Max Grav	1=58 (LC 34), 3=60 (LC 35), 4=257 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-51/77, 2-3=-54/74
BOT CHORD	1-4=-72/78, 3-4=-69/76
WEBS	2-4=-179/88

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- All bearings are assumed to be SP No.2.

LOAD CASE(S) Standard



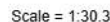
April 23, 2025

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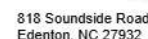
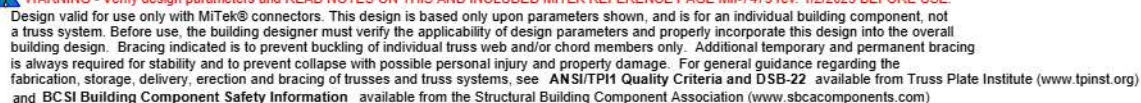
818 Soundside Road
Edenton, NC 27932

Page: 1LUMBER

BRACING

NOTES

- LOAD CASE(S) Standard

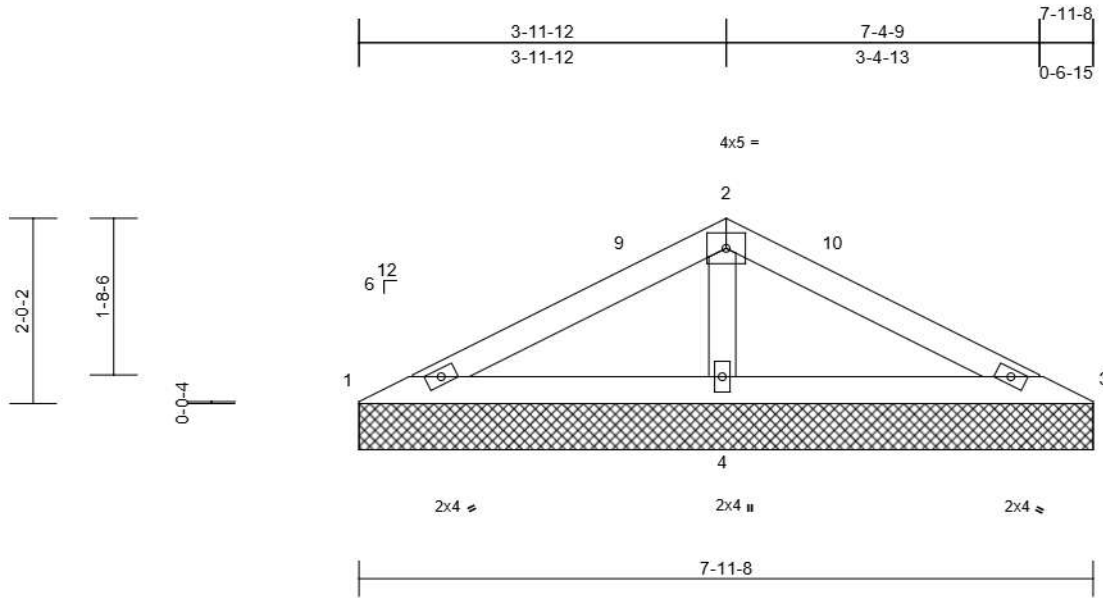


Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956743
25040106	V11	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Scale = 1:25

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.23	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
											Weight: 25 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 7-11-8 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=7-11-8, 3=7-11-8, 4=7-11-8
Max Horiz	1=-19 (LC 13)
Max Uplift	1=-2 (LC 39), 3=-5 (LC 16)
Max Grav	1=89 (LC 21), 3=95 (LC 22), 4=530 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-126/276, 2-3=-113/266
BOT CHORD	1-4=-245/173, 3-4=-235/168
WEBS	2-4=-402/219

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-8 to 3-0-8, Interior (1) 3-0-8 to 4-0-4, Exterior(2R) 4-0-4 to 6-9-11, Interior (1) 6-9-11 to 8-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 5 lb uplift at joint 3.

LOAD CASE(S) Standard



April 23, 2025

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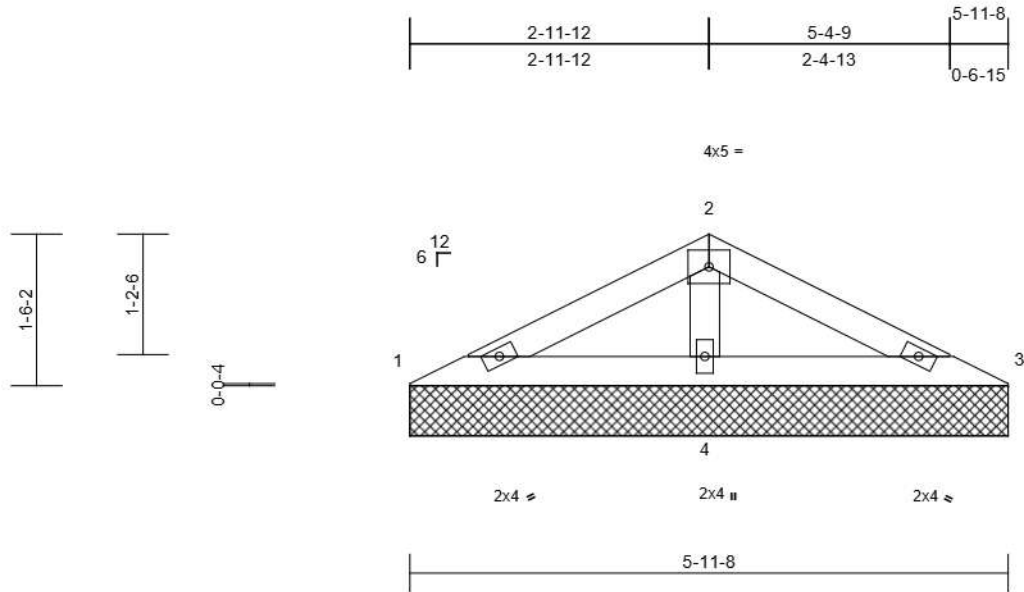
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956744
25040106	V12	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

Run: 8.73 S Feb 19 2025 Print: 8.730 S Feb 19 2025 MiTek Industries, Inc. Wed Apr 23 07:39:43
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Page: 1



Scale = 1:23

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.10	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.12	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	4	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										
										Weight: 18 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-11-8 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS

(size)	1=5-11-8, 3=5-11-8, 4=5-11-8
Max Horiz	1=14 (LC 14)
Max Uplift	1=-2 (LC 15), 3=-4 (LC 16)
Max Grav	1=79 (LC 21), 3=84 (LC 22), 4=360 (LC 2)

FORCES

(lb) - Maximum Compression/Maximum Tension

TOP CHORD	1-2=-82/158, 2-3=-92/149
BOT CHORD	1-4=-144/114, 3-4=-135/109
WEBS	2-4=-250/154

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf, Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

- Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 4-0-0 oc.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - All bearings are assumed to be SP No.2.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2 lb uplift at joint 1 and 4 lb uplift at joint 3.
- LOAD CASE(S)** Standard



April 23, 2025

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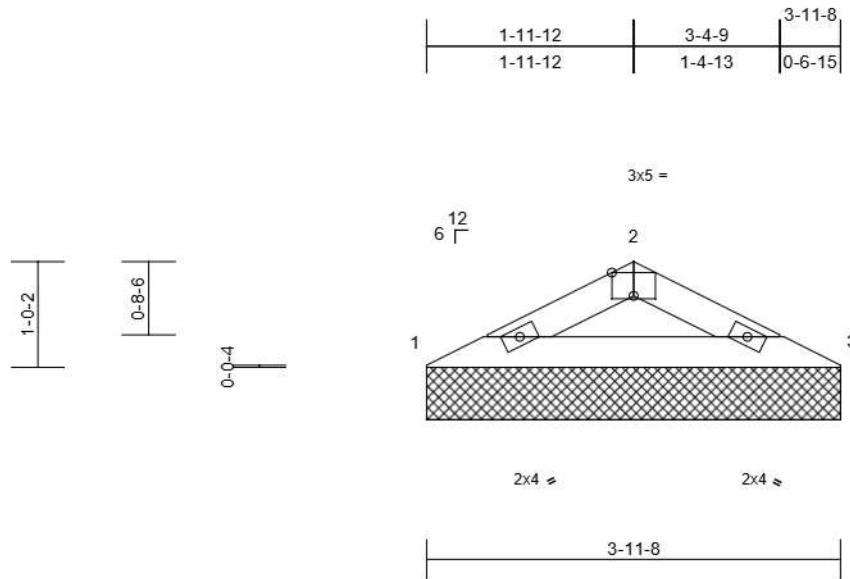
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	13 Eagle Creek - Edisto C - Roof	172956745
25040106	V13	Valley	1	1	Job Reference (optional)	

Carter Components (Sanford, NC), Sanford, NC - 27332,

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Page: 1



Scale = 1:22.1

Plate Offsets (X, Y): [2:0-2-8,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf/Pg)	13.9/20.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0											
Weight: 11 lb											FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-11-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (size) 1=3-11-8, 3=3-11-8
Max Horiz 1=9 (LC 13)
Max Grav 1=158 (LC 2), 3=158 (LC 2)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-304/165, 2-3=-304/154
BOT CHORD 1-3=-143/262

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=13.9 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.

- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.2.

LOAD CASE(S) Standard



April 23, 2025

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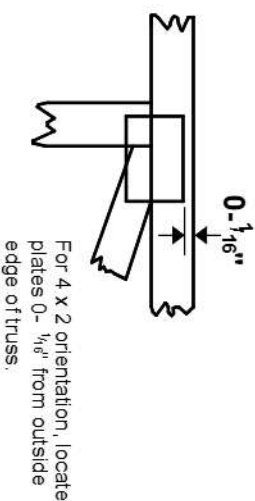
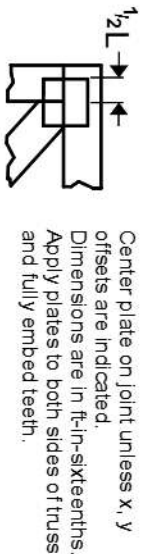
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

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818 Soundside Road
Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

PLATE SIZE

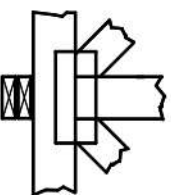
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

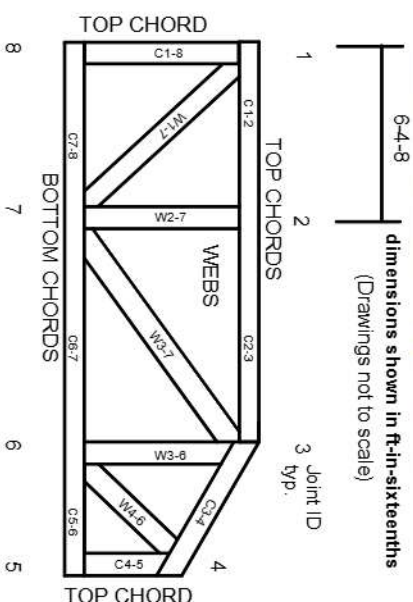


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TFP 1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:
ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TFP 1 section 6.3. These truss designs rely on lumber values established by others.

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MITek Engineering Reference Sheet, MIL-7473 rev. 1/2/2023

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and ware at joint locations are regulated by ANSI/TFP 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TFP 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TFP 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.