Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 58535 JOB: 25-3294-R01 JOB NAME: LOT 155 PROVIDENCE CREEK Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 35 These truss designs comply with IRC 2015 as well as IRC 2018. 22 Truss Design(s)

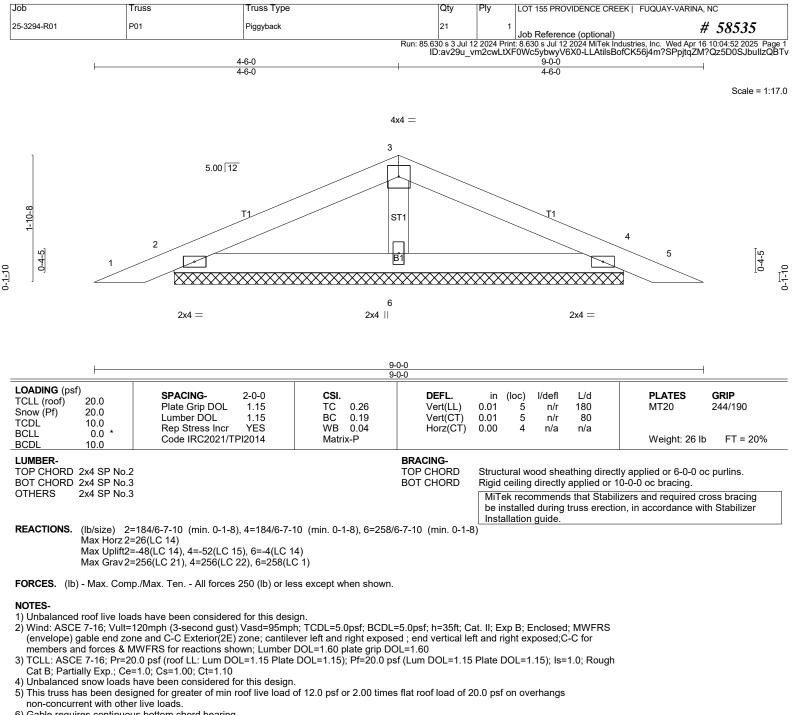
Trusses:

P01, R01, R02, R02A, R02B, R02C, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, V01, V02, V03, V04, V05



My license renewal date for the state of North Carolina is 12/31/2025

Warning !--- Verify design parameters and read notes before use.



- Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

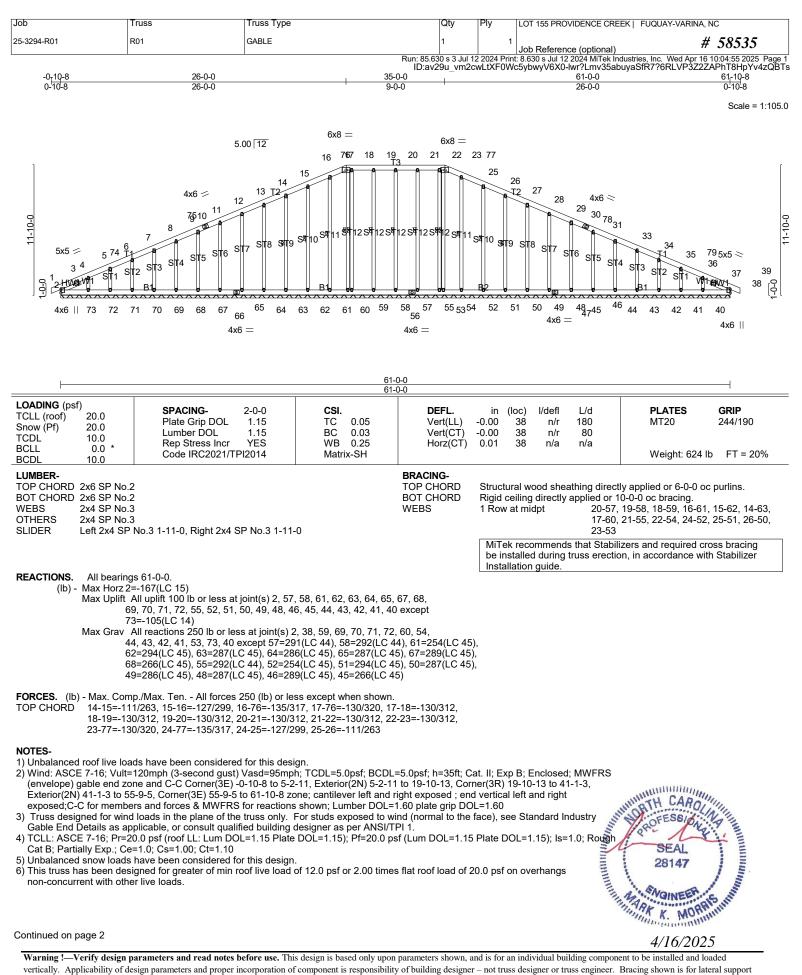
8)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 2, 52 lb uplift at joint 4 and 4 lb uplift at joint 6.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building desianer.

LOAD CASE(S) Standard





Continued on page 2

Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

4/16/2025

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUAY-VARINA, NC			
25-3294-R01	R01	GABLE	1	1	Job Reference (optional) # 5	58535		
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 Mick Industries, Inc. Wed Apr 16 10:04:56 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-E7PNY6vhsujlZk1r?rWL_Z2apzOovsxcNxZ6RWzQBTr								

7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. 8) Provide adequate drainage to prevent water ponding.

9) All plates are 2x4 MT20 unless otherwise indicated.

10) Gable requires continuous bottom chord bearing.

11) Gable studs spaced at 2-0-0 oc.

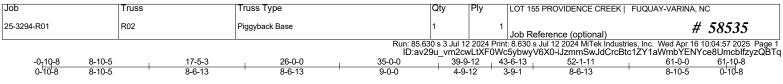
12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

13) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

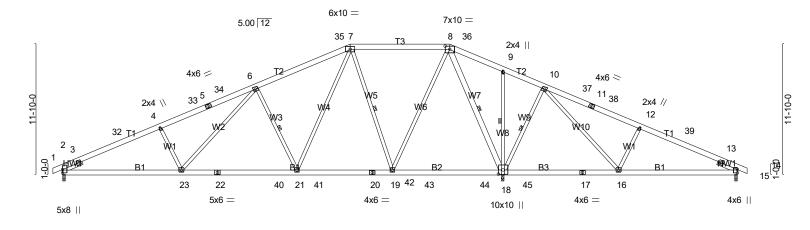
14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 57, 58, 61, 62, 63, 64, 65, 67, 68, 69, 70, 71, 72, 55, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40 except (jt=lb) 73=105.

LOAD CASE(S) Standard





Scale = 1:104.1



LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCDL 0.0 * BCDL 10.0 LUMBER- TOP CHORD 2x6 SP NG BOT CHORD 2x6 SP NG BOT CHORD 2x6 SP NG BOT CHORD 2x6 SP NG SLIDER Left 2x4 SP Max Horz Max Uplift Max Grav	0-5-0,0-3-7], [18:0-4-8,0-5-0] SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014 0.2 0.2 0.3 *Except*	-3-8 (min. 0-1-8), 18=3 232(LC 11) 124(LC 45) ess except when shown	· ·	MiTek recommends that Stabi be installed during truss erect Installation guide.	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCDL 0.0 * BCDL 10.0 LUMBER- TOP CHORD 2x6 SP NG BOT CHORD 2x6 SP NG BOT CHORD 2x6 SP NG WEBS 2x4 SP NG W7: 2x6 S SLIDER Left 2x4 S REACTIONS. (lb/size) Max Uplift Max Uplift	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014 0.2 0.3 *Except* SP DSS SP No.3 1-11-0, Right 2x4 SP No.3 1-11 2=1412/0-3-8 (min. 0-1-14), 14=461/0 :2=-167(LC 15) 12=-222(LC 14), 14=-145(LC 15), 18=-2 /2=1577(LC 39), 14=583(LC 55), 18=47 pmp./Max. Ten All forces 250 (lb) or left	-0 -3-8 (min. 0-1-8), 18=3 232(LC 11) 124(LC 45) co. 0 -3 except when shown	Vert(LL) Vert(CT) Horz(CT) BRACING- TOP CHORD BOT CHORD WEBS	-0.31 21-23 >999 240 -0.47 21-23 >999 180 0.05 18 n/a n/a Structural wood sheathing direc Rigid ceiling directly applied or 1 Row at midpt 6-21, MiTek recommends that Stabi be installed during truss erect Installation guide.	MT20 244/190 Weight: 466 lb FT = 20% thy applied or 2-10-2 oc purlins. 6-0-0 oc bracing. 7-19, 8-18, 10-18, 9-18 lizers and required cross bracing
LUMBER- TOP CHORD 2x6 SP No BOT CHORD 2x6 SP No WEBS 2x4 SP No W7: 2x6 S SLIDER Left 2x4 S REACTIONS. (Ib/size) Max Horz Max Uplift Max Grav	o.2 o.3 *Except* SP DSS SP No.3 1-11-0, Right 2x4 SP No.3 1-11 2=1412/0-3-8 (min. 0-1-14), 14=461/0 : 2=-167(LC 15) t2=-222(LC 14), 14=-145(LC 15), 18=-2 /2=1577(LC 39), 14=583(LC 55), 18=4 pmp./Max. Ten All forces 250 (lb) or left	-3-8 (min. 0-1-8), 18=3 232(LC 11) 124(LC 45) ess except when shown	TOP CHORD BOT CHORD WEBS 112/0-3-8 (min. 0-	Rigid ceiling directly applied or 1 Row at midpt 6-21, MiTek recommends that Stabi be installed during truss erect Installation guide.	6-0-0 oc bracing. 7-19, 8-18, 10-18, 9-18 ilizers and required cross bracing
Max Uplif Max Grav	t2=-222(LC 14), 14=-145(LC 15), 18=-2 /2=1577(LC 39), 14=583(LC 55), 18=4 omp./Max. Ten All forces 250 (lb) or le	124(LC 45) ess except when shown	I.		
5-34=-26 8-36=01 11-38=-2 BOT CHORD 2-23=-43 20-41=-2 WEBS 4-23=-42 8-19=-13 9-18=-34	613/395, 6-34=-2510/412, 6-35=-1782/3 1500, 9-36=0/1429, 9-10=-26/1462, 10- 286/363, 12-38=-388/332, 12-39=-432/3 33/2561, 22-23=-250/1951, 22-40=-250 26/1001, 20-42=-26/1001, 19-42=-26/10 256/220, 18-45=-905/158, 17-45=-905/ 21/229, 6-23=-131/727, 6-21=-1190/324 38/1835, 8-18=-2747/243, 10-18=-1061	341, 7-35=-1533/343, 7 37=-256/493, 11-37=-2 315, 13-39=-582/270, 1 /1951, 21-40=-250/195 001, 19-43=-256/220, 4 158, 16-17=-905/158, 1 4, 7-21=-217/1520, 7-19	-8=-646/242, 80/374, 3-14=-402/2 1, 21-41=-26/1001 3-44=-256/220, 4-16=-249/483 9=-1292/235,	,	
2) Wind: ASCE 7-16; Vul (envelope) gable end 2 43-6-6 to 55-9-5 Exter	oads have been considered for this des It=120mph (3-second gust) Vasd=95mp zone and C-C Exterior(2E) -0-10-8 to 5- rior(2E) 55-9-5 to 61-10-8 zone; cantile's MWFRS for reactions shown; Lumber =20.0 psf (roof LL: Lum DOL=1.15 Plate Ce=1.0; Cs=1.00; Ct=1.10 ds have been considered for this design esigned for greater of min roof live load ther live loads. span truss requires extreme care and de , see Guide to Good Practice for Handl SCA and TPI. The building owner or the esign and inspection of the temporary in ek assumes no responsibility for truss r inage to prevent water ponding.	oh; TCDL=5.0psf; BCDL -2-11, Interior(1) 5-2-11 ver left and right expose	to 17-4-8, Exterior ed · end vertical lef	r(2R) 17-4-8 to 43-6-6, Interior(1) ft and right exposed C-C for	SEAL 28147

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUAY-VARI	NA, NC		
25-3294-R01	R02	Piggyback Base	1	1	Job Reference (optional)	# 58535		
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:04:57 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-iJzmmSwJdCrcBtc1ZY1aWmbYENYce8UmcblfzyzQBTq								

8) All plates are 5x5 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

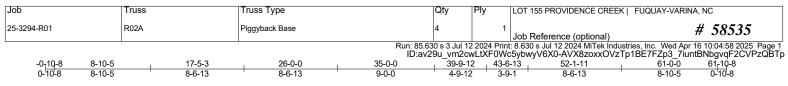
11) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

12) Provide metal plate or equivalent at bearing(s) 18 to support reaction shown.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=222, 14=145, 18=232.

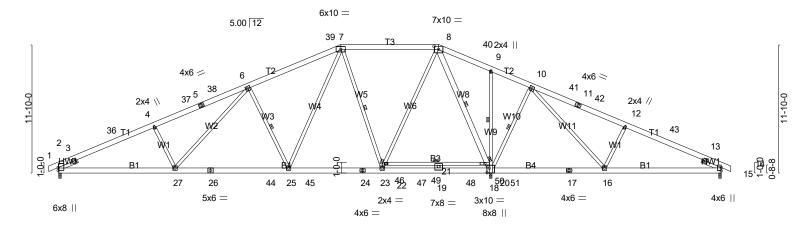
LOAD CASE(S) Standard





Scale = 1:106.0

4/16/2025



	-8-14 <u>21-2-4</u> -8-14 10-5-6		5-0-0 <u>39-9-</u> 5-2-4 4-9-1			<u>61-0-0</u> 10-8-14		
Plate Offsets (X,Y)	8:0-5-0,0-3-7], [18:0-5-4,0-2-8]							
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.85 BC 0.87 WB 0.93 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.33 25-27 >999 -0.50 25-27 >957 0.06 18 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 480 lb	GRIP 244/190 FT = 20%	
WEBS 2x4 SP W8: 2x		T E V	BRACING- TOP CHORD BOT CHORD WEBS	Structural wood shea Rigid ceiling directly 6-0-0 oc bracing: 20- 1 Row at midpt MiTek recommends be installed during	applied or 6-0-0 -22 6-25, 7-23, s that Stabilizers	oc bracing. Exce , 8-20, 10-18, 9-18 s and required cross	pt: ss bracing	
Max H Max U) 2=1429/0-3-8 (min. 0-1-14), 14=448/0 orz 2=-167(LC 15) Jift2=-212(LC 14), 14=-148(LC 15), 18=-1 av 2=1593(LC 39), 14=572(LC 55), 18=4	44(LC 11)	8/0-3-8 (min. 0-3	Installation guide. 3-2)]	
TOP CHORD 2-3=- 5-38= 8-40= 11-42	Comp./Max. Ten All forces 250 (lb) or le 984/0, 3-36=-2959/350, 4-36=-2867/366, 4 -2707/372, 6-38=-2604/388, 6-39=-1877/ 0/1545, 9-40=0/1475, 9-10=-21/1508, 10- =-258/426, 12-42=-360/394, 12-43=-404/ 412/2646, 26-27=-227/2039, 26-44=-227	1-37=-2803/367, 5-37=-272 16, 7-39=-1631/318, 7-8= 41=-228/556, 11-41=-252/ 377, 13-43=-554/330, 13-1	-774/211, /438, 4=-402/0					
19-48 18-51 WEBS 4-27= 22-23	24-45=-1/1105, 24-46=-1/1105, 23-46=-1/1105, 23-47=-24/423, 19-47=-24/423, 19-48=-24/423, 18-48=-24/423, 22-49=-326/9, 21-49=-326/9, 21-50=-326/9, 20-50=-326/9, 18-51=-953/153, 17-51=-953/153, 16-17=-953/153, 14-16=-304/457							
 Wind: ASCE 7-16; (envelope) gable en 43-6-6 to 55-9-5, E: members and force TCLL: ASCE 7-16; Cat B; Partially Exp Unbalanced snow I 	e loads have been considered for this des /ult=120mph (3-second gust) Vasd=95m d zone and C-C Exterior(2E) -0-10-8 to 5- terior(2E) 55-9-5 to 61-10-8 zone; cantile s & MWFRS for reactions shown; Lumber Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate ; Ce=1.0; Cs=1.00; Ct=1.10 yads have been considered for this design designed for greater of min roof live load other live loads.	h; TCDL=5.0psf; BCDL=5 2-11, Interior(1) 5-2-11 to /er left and right exposed ; DOL=1.60 plate grip DOL DOL=1.15); Pf=20.0 psf (17-4-8, Exterior ; end vertical left =1.60 (Lum DOL=1.15	(2R) 17-4-8 to 43-6-6, t and right exposed;C- Plate DOL=1.15); ls=	; MWFRS Interior(1) C for 1.0; Rough	SEAL 28147	ALL A TANK AND	
Continued on page 2						4/16/2025		

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUAY-VARINA, NC			
25-3294-R01	R02A	Piggyback Base	4	1	Job Reference (optional) # 58535			
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:04:58 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-AVX8zoxxOVzTp1BE7FZp3_7iuntBNbgvqF2CVPzQBTp								

6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

7) Provide adequate drainage to prevent water ponding.

8) All plates are 5x5 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide metal plate or equivalent at bearing(s) 18 to support reaction shown.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=212, 14=148, 18=144.

LOAD CASE(S) Standard



Job 25-3294-R01		russ 02B	Truss Type Piggyback Base		Qty 2	Ply 1	LOT 155 PROVI	DENCE CREE	ek Fuquay-var	RINA, NC # 58535
		-		F	Run: 85.630 s 3 Jul 12	2 2024 Prin	Job Reference	(optional) 024 MiTek Ind	ustries, Inc. Wed A	pr 16 10:04:59 2025 Page
	-0 ₁ 10-8 7-1	1-0 , 13-9-0 ,	21-2-4 _ 26-0		ID:av29u_vm2cwL	_tXF0Wc5	ybwyV6X0-eh5	WA8ya9p5K	.QBmQgz42bBgu	_BBx62423vnm2rzQBT 61 _r 10-8
	0-10-8 7-1		7-5-4 4-9-						8-10-5	0-10-8
			5.00 12	6x10 =						Scale = 1:116.
					7x10 =	2x4				
T			5x5 =	46 8 T4	9 	47	11			Ţ
			'T3				5x5 ≳ 11 4			
		3x6 8x8 ≤ 5 ⁴	6 ⁴⁵					4x6 <>		
11-10-0	3	8x8 43 54	W		W10	v	1/2 12		(4 // 3	11-10-0
Ę	5	4 42 B		\mathbb{N}	\parallel		WA3	4	50	5x5 ≈
	2 II	W1 W2		\mathbb{N}	/ \			W74	-rec -	14
		B2	W4 B5		B7		B8		B9 🙀	
-		28 ¹ 29 29 2	7 26 54	25 23	24 51 56 52 20 51 56	2 21 ⁵⁷ 19	53 18	17		
	$6x8 = \frac{2x^2}{2}$	2x4	5 5x8 =	5x5 =	= 7x8 = 3	3x10 = 8x8	4x6 =	= 5x5 =		4x6
	3x6 =	6x8	3 =	2x4 = 4x6 =	=	0.00 11				
	7x8 =									
		7-11-0 13-9-0	21-2-4		35-0-0 39-9-1		50-3-2		61-0-0	
Plate Offsets		<u>5-7-8 5-10-0</u> -8,0-0-7], [3:0-0-6,0-0-0]		<u>8-7-8</u> 5-0,0-3-7], [19:0-{	<u>5-2-4</u> 4-9-12 5-4,0-2-8], [28:0-		10-5-6 4]		10-8-14	
OADING (ps		SPACING-	2-0-0 CS	si.	DEFL.	in (le	oc) l/defl	L/d	PLATES	GRIP
CLL (roof) Snow (Pf)	20.0 20.0	Plate Grip DOL	1.15 TC	0.83	Vert(LL)	-0.27 22-	23 >999	240	MT20	244/190
FCDL`´ BCLL	10.0 0.0 *	Lumber DOL Rep Stress Incr	1.15 BC YES W		Vert(CT) Horz(CT)	-0.42 22- 0.12	23 >999 19 n/a	180 n/a		
BCEL BCDL	10.0	Code IRC2021/T	PI2014 Ma	atrix-MSH	. ,				Weight: 5	519 lb FT = 20%
UMBER-		·-			BRACING-	o				
OP CHORD	2x6 SP No.2 T1: 2x8 SP N	*Except* lo.2			TOP CHORD BOT CHORD				y applied or 3-10 0-0 oc bracing.	
BOT CHORD	2x6 SP No.2	*Except* lo.2, B7,B4,B3: 2x4 SP	No 2 B6: 2x6 SP No	1 1	WEBS		bracing: 21-2	3	-21, 11-19, 10-1	
WEBS	2x4 SP No.3	*Except*	10.2, 00. 200 01 10.	1	WEBO			,	, ,	ed cross bracing
SLIDER	W10: 2x6 SF Right 2x4 SF						alled during tru ation quide.	uss erectior	n, in accordance	e with Stabilizer
REACTIONS.	(lh/size) 2=	1399/0-3-8 (min. 0-1-1	3) $15=369/0-3-8$ (min	0-1-8) 19=3420	0/0-3-8 (min 0-?		dion guido.			
LACHONO.	Max Horz 2=	-167(LC 19)		<i>,</i> .	0/0-0-0 (11111. 0-0)				
		-184(LC 14), 15=-155(L 1562(LC 39), 15=530(L								
		./Max. Ten All forces								
FOP CHORD	2-3=-555/1	63, 3-42=-3363/397, 4-4	42=-329́3/413, 4-43=-2	609/299, 5-43=-2						
		//395, 6-44=-2634/407, //343, 8-9=-660/191, 9-4								
		/603, 12-48=-157/503, /401, 14-15=-357/16	12-49=-164/496, 13-49)=-265/469, 13-5	0=-310/452,					
BOT CHORD	23-51=-313	/11, 22-51=-313/11, 22								
		5/200, 17-18=-1005/20 8, 25-55=0/948, 24-55=	, ,	,	,	68.				
	19-57=-12	0/368, 5-28=-603/204, 3	3-29=-469/3088, 28-29	=-467/3100		,				
VEBS		1571, 8-24=-1284/262, \8/203, 11-19=-1062/26								
		/95, 7-26=-1407/353, 2								
	4 00 040/	203							unununu.	Ilter.
IOTES	4 00 040/	203							111 -01 1-10 0	30 111
NOTES- I) Unbalance	4 00 040/	203 Is have been considere	d for this design.					and and	IN ATH CAA	OLINIU
IOTES-) Unbalance) Wind: ASC	4 00 040/	203 Is have been considere 20mph (3-second gust is and C-C Exterior(25)	d for this design.) Vasd=95mph; TCDL= _0-8-4 to 5-5-0 Interio	:5.0psf; BCDL=5	i.0psf; h=35ft; Ca 1-8 Exterior(2P)	at. II; Exp 17-4-8 tr	B; Enclosed; I	MWFRS	PROFESSI	OL IN A HAL
IOTES-) Unbalance) Wind: ASC (envelope) 43-6-6 to 5	4 00 040/	203 Is have been considere 20mph (3-second gust ie and C-C Exterior(2E) (2E) 55-9-5 to 61-10-8	d for this design.) Vasd=95mph; TCDL= -0-8-4 to 5-5-0, Interio zone; cantilever_left an	:5.0psf; BCDL=5 r(1) 5-5-0 to 17-4 d right exposed ;	i.0psf; h=35ft; Ca 4-8, Exterior(2R) ; end vertical left	at. II; Exp 17-4-8 to and right	B; Enclosed; I 43-6-6, Interio exposed;C-C	MWFRS	PROFESSI SEAL	O NA
NOTES- 1) Unbalance 2) Wind: ASC (envelope) 43-6-6 to 5 members a 3) TCLL: ASC	4-28=-848/ d roof live load E 7-16; Vult= ⁻⁷ gable end zor 5-9-5, Exterior and forces & M	is have been considere 20mph (3-second gust ie and C-C Exterior(2E) (2E) 55-9-5 to 61-10-8 : WFRS for reactions sh	d for this design.) Vasd=95mph; TCDL= -0-8-4 to 5-5-0, Interio zone; cantilever left an own; Lumber DOL=1.6 DL=1.15 Plate DOL=1.	=5.0psf; BCDL=5 r(1) 5-5-0 to 17-4 d right exposed ; 0 plate grip DOL 5); Pfe_20.0 psf (i.0psf; h=35ft; Ca 4-8, Exterior(2R) ; end vertical left =1.60 (Lum DOL=1.15	at. II; Exp 17-4-8 to and right Plate DC	B; Enclosed; I 43-6-6, Interio exposed;C-C L=1.15); Is=1.	MWFRS or(1) for 0; Rough	SEAL 28147	
Cat B; Par	4-28=-848/ d roof live load E 7-16; Vult= 5-9-5, Exterior and forces & M CE 7-16; Pr=20 tially Exp.; Ce=	is have been considere 20mph (3-second gust he and C-C Exterior(2E) (2E) 55-9-5 to 61-10-8 WFRS for reactions sh 0.0 psf (roof LL: Lum DC :1.0; Cs=1.00; Ct=1.10	DL=1.15 Plate $DOL=1.$	5.0psf; BCDL=5 r(1) 5-5-0 to 17-4 d right exposed ; 0 plate grip DOL l5); Pf=20.0 psf (5.0psf; h=35ft; Ca 4-8, Exterior(2R) ; end vertical left =1.60 (Lum DOL=1.15	at. II; Exp 17-4-8 to and right Plate DC	B; Enclosed; I 43-6-6, Interi exposed;C-C L=1.15); Is=1.	MWFRS or(1) for 0; Rough	SEAL 28147	A Contraction of the Contraction
Cat B; Pari Cat B; Pari (1) Unbalance (5) This truss	4-28=-848/ d roof live load E 7-16; Vult= gable end zor 5-9-5, Exterior and forces & M CE 7-16; Pr=20 tially Exp.; Ce= d snow loads has been desig	Is have been considere 20mph (3-second gust te and C-C Exterior(2E) (2E) 55-9-5 to 61-10-8 WFRS for reactions sh 0.0 psf (roof LL: Lum DC :1.0; Cs=1.00; Ct=1.10 nave been considered f gned for greater of min	or this design.	15); PI=20.0 pSI ((Lum DOL=1.15	Plate DC	L=1.15); IS=1.	MWFRStrift for 0; Rottgh	SEAL 28147	
Cat B; Pari Cat B; Pari) Unbalance) This truss	4-28=-848/ d roof live load E 7-16; Vult=- gable end zor 5-9-5, Exterior and forces & M CE 7-16; Pr=20 tially Exp.; Ce= d snow loads	Is have been considere 20mph (3-second gust te and C-C Exterior(2E) (2E) 55-9-5 to 61-10-8 WFRS for reactions sh 0.0 psf (roof LL: Lum DC :1.0; Cs=1.00; Ct=1.10 nave been considered f gned for greater of min	or this design.	15); PI=20.0 pSI ((Lum DOL=1.15	Plate DC	L=1.15); IS=1.	MWFRS or(1) for 0; Rough	SEAL 28147	RA SHITTER
Cat B; Pari Cat B; Pari Unbalance) This truss	4-28=-848/ d roof live load E 7-16; Vult=- gable end zor 5-9-5, Exterior and forces & M CE 7-16; Pr=20 tially Exp.; Ce d snow loads has been designed	Is have been considere 20mph (3-second gust te and C-C Exterior(2E) (2E) 55-9-5 to 61-10-8 WFRS for reactions sh 0.0 psf (roof LL: Lum DC :1.0; Cs=1.00; Ct=1.10 nave been considered f gned for greater of min	or this design.	15); PI=20.0 pSI ((Lum DOL=1.15	Plate DC	L=1.15); IS=1.	MWFRS or(1) 0; Rough	SEAL 28147	BRANCHING DECEMBER OF THE PARTY

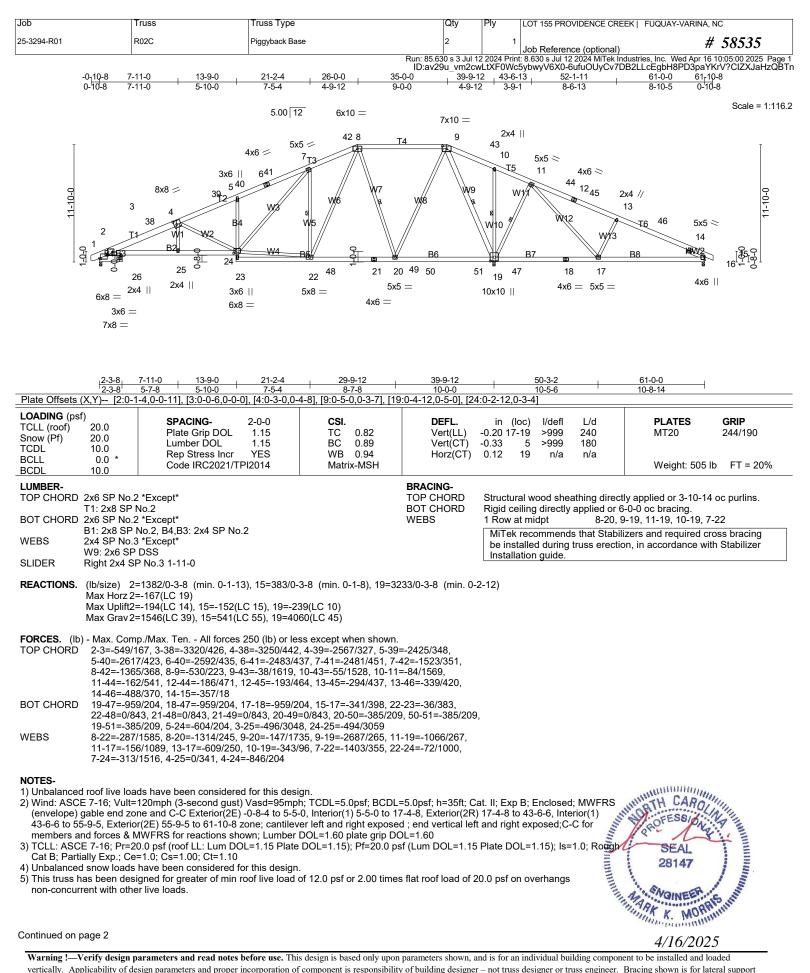
vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQU	AY-VARINA, NC		
25-3294-R01	R02B	Piggyback Base	2	1	Job Reference (optional)	# 58535		
Run: 85 630 s 3 Jul 12 2024 Print: 8 630 s Jul 12 2024 MITek Industries, Inc. Wed Apr 16 10:04:59 2025 Page 2 ID:av29u vm2cwLtXF0Wc5ybwyV6X0-eh5WA8ya9p5KQBmQgz42bBqu BBx62423vnm2rzQBTo								

- 6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 7) Provide adequate drainage to prevent water ponding.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide metal plate or equivalent at bearing(s) 19 to support reaction shown.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=184, 15=155, 19=152.

LOAD CASE(S) Standard



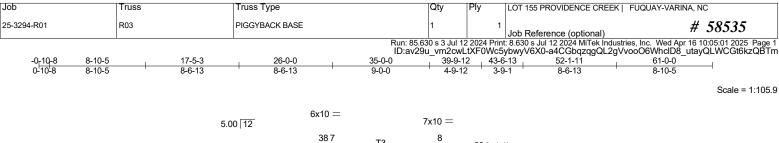


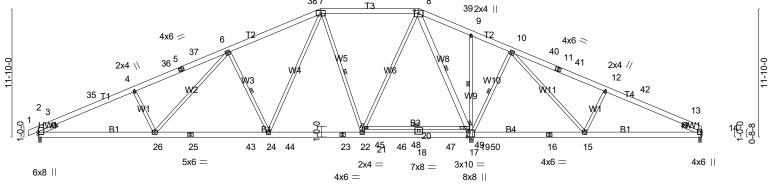
Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUAY-VARINA, NC			
25-3294-R01	R02C	Piggyback Base	2	1	Job Reference (optional) # 5	8535		
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:05:00 2025 Page 2 ID:av29u vm2cwLtXF0Wc5ybwyV6X0-6ufuQUyCv7DB2LLcEgbH8PD3paYKrV?CIZXJaHzQBTn								

- 6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 7) Provide adequate drainage to prevent water ponding.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Bearing at joint(s) 19 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 11) Provide metal plate or equivalent at bearing(s) 19 to support reaction shown.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (it=lb) 2=194, 15=152, 19=239.

LOAD CASE(S) Standard







	<u> </u>	21-2		29-9-12	<u>35-0-0 39-9</u> 5-2-4 4-9-		<u>50-3-2</u> 10-5-6		61-0-0	
Plate Offsets		-0,0-3-7], [17:0-5-4,0-2-8		8-7-8	5-2-4 4-9-	12	10-5-6		10-8-14	
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TF	2-0-0 1.15 1.15 YES Pl2014	CSI. TC 0.85 BC 0.87 WB 0.93 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.33 24-26 -0.50 24-26 0.06 17	l/defl >999 >956 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 478 I	GRIP 244/190 b FT = 20%
LUMBER- TOP CHORD BOT CHORD	2x6 SP No.2	*Except* lo.2, B2: 2x6 SP No.1			BRACING- TOP CHORD BOT CHORD		g directly a	pplied or 6-	/ applied or 2-7-0 oc 0-0 oc bracing. Exc	
WEBS	2x4 SP No.3	*Except*			WEBS	1 Row at mi			22, 8-19, 10-17, 9-1	17
SLIDER	W8: 2x6 SP Left 2x4 SP I	DSS No.3 1-11-0, Right 2x4 S	SP No.3 1-11-0				d during ti		ers and required cr n, in accordance wit	
REACTIONS.	Max Horz 2= Max Uplift2=	1430/0-3-8 (min. 0-1-14 172(LC 14) -211(LC 14), 14=-129(L 1594(LC 39), 14=520(L	C 15), 17=-147(LC 11)	295/0-3-8 (min. 0		<u>- guide</u>			
FORCES. (Ib TOP CHORD	2-3=-985/0 5-37=-2709 8-39=0/154	0./Max. Ten All forces 3-35=-2961/349, 4-35= /370, 6-37=-2606/387, (0, 9-39=0/1470, 9-10=-/ /418, 12-41=-369/391	:-2869/365, 4-36 5-38=-1879/314, 24/1504, 10-40=	=-2806/366, 5-36=- 7-38=-1629/316, 7 -237/547, 11-40=-2	2725/369, -8=-773/208, 61/430,					
BOT CHORD	11-41=-267/418, 12-41=-369/391, 12-42=-413/374, 13-42=-563/326, 13-14=-411/0 BOT CHORD 2-26=-416/2648, 25-26=-231/2042, 25-43=-231/2042, 24-43=-231/2042, 24-44=-5/1102, 23-44=-5/1102, 23-45=-5/1102, 22-46=-28/418, 18-46=-28/418, 18-47=-28/418, 17-47=-28/418, 21-48=-326/9, 20-49=-326/9, 19-49=-326/9, 19-49=-326/9, 17-50=-948/143, 16-50=-948/143, 15-16=-948/143, 14-15=-301/466									
WEBS	4-26=-418/ 21-22=-118	/143, 10-30-340/143, 230, 6-26=-132/726, 6-2 /1830, 8-21=-97/2041, 8 /1064, 12-15=-604/251,	4=-1190/323, 7- 3-19=-2954/203,	24=-224/1483, 7-22 17-19=-3171/184,						
NOTES- 1) Unbalance 2) Wind: ASC (envelope) 43-6-6 to 5	d roof live load E 7-16; Vult=1 gable end zor 4-10-13 Exter	Is have been considered 20mph (3-second gust) e and C-C Exterior(2E) ior(2E) 54-10-13 to 61-0	d for this design. Vasd=95mph; 1 -0-10-8 to 5-2-1)-0 zone: cantiley	CDL=5.0psf; BCDL 1, Interior(1) 5-2-11 ver left and right ext	.=5.0psf; h=35ft; 0 to 17-4-8, Exterio	Cat. II; Exp B; I r(2R) 17-4-8 to al left and right	Enclosed; o 43-6-6, li t exposed;	MWFRS nterior(1)	TH CARO	NIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII

43-6-6 to 54-10-13, Exterior(2E) 54-10-13 to 61-0-0 zone; cantilever left and right exposed; end vertical left and right exposed;C

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



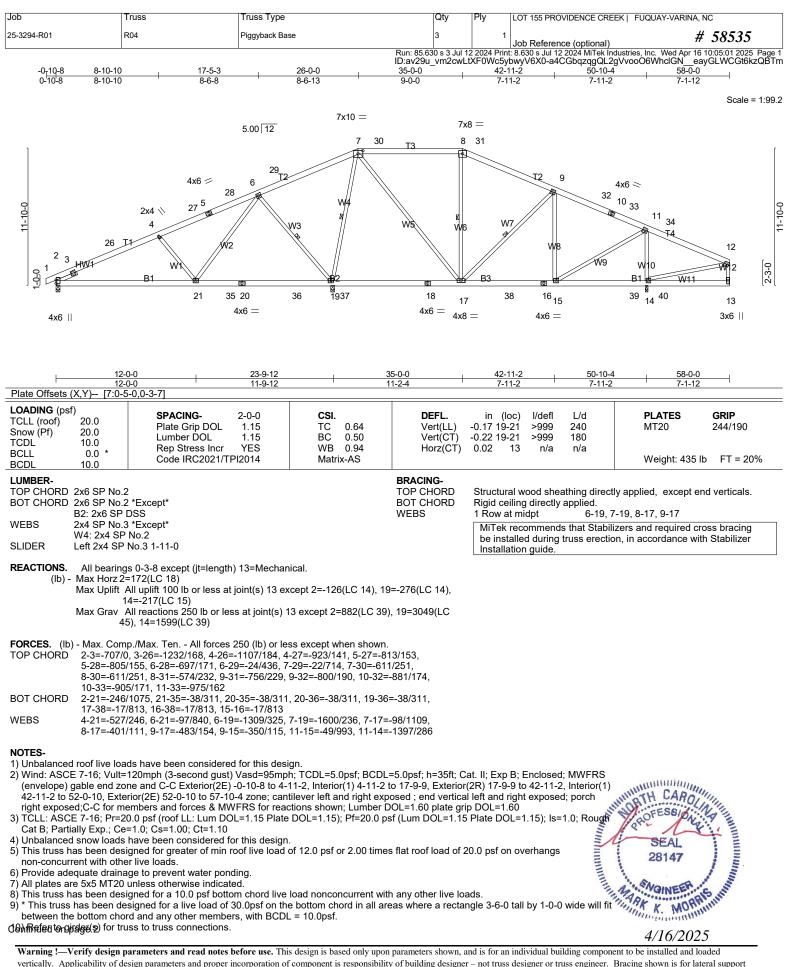
Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUAY-VAR	INA, NC		
25-3294-R01	R03	PIGGYBACK BASE	1	1	Job Reference (optional)	# 58535		
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:05:01 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-a4CGbqzqgQL2gVvooO6WhclD8_utayQLWCGt6kzQBTm								

- 6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 5x5 MT20 unless otherwise indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide metal plate or equivalent at bearing(s) 17 to support reaction shown.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=211, 14=129, 17=147.

LOAD CASE(S) Standard





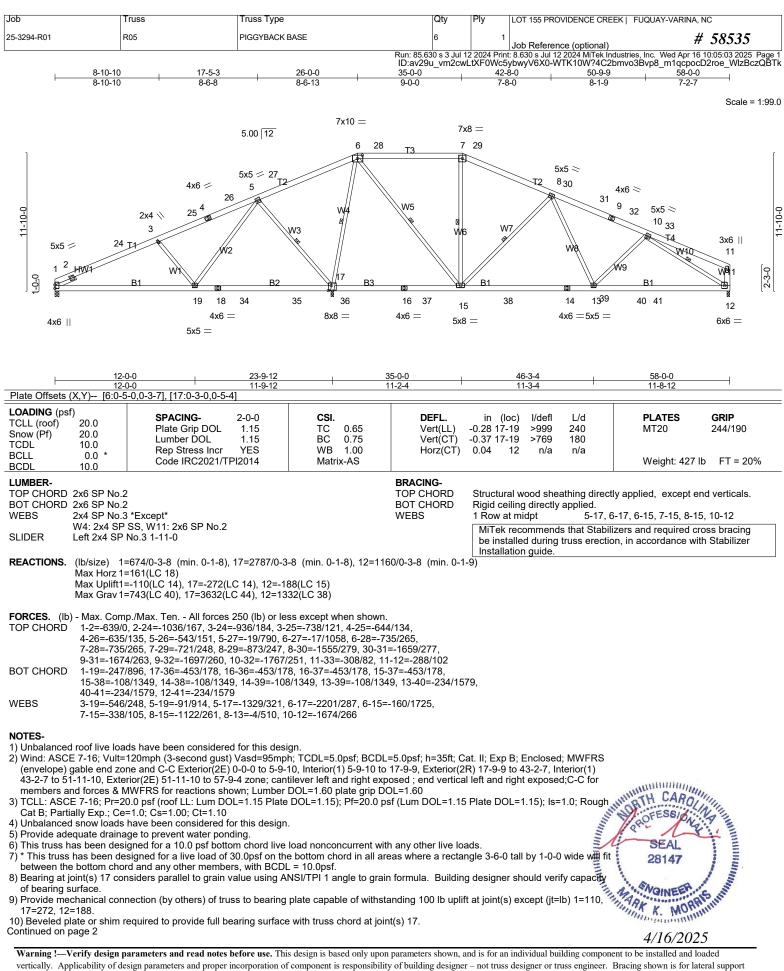
Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUG	QUAY-VARINA, NC
25-3294-R01	R04	Piggyback Base	3	1	Job Reference (optional)	# 58535
					: 8.630 s Jul 12 2024 MiTek Industries, I	

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 2=126, 19=276, 14=217.

12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



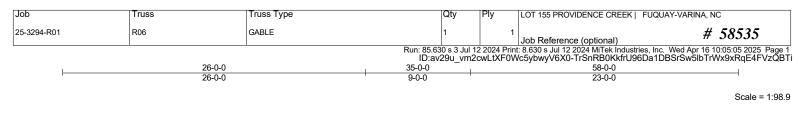


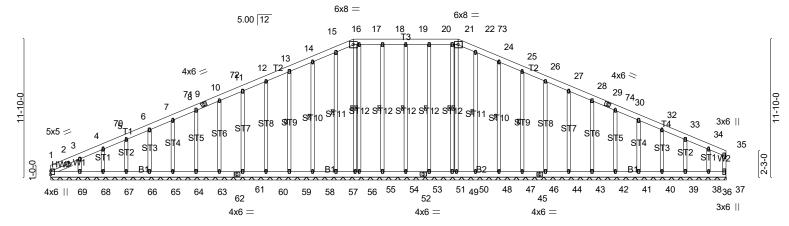
[Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUA	Y-VARINA, NC		
	25-3294-R01	R05	PIGGYBACK BASE	6	1	Job Reference (optional)	# 58535		
	Run: 85 630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:05:03 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-WTK10W?4C2bmvo3Bvp8_m1qcpocD2roe_WlzBczQBTk								

NOTES-11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard







			8-0-0 8-0-0		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.10 BC 0.04 WB 0.25 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d n/a - n/a 999 n/a - n/a 999 0.00 36 n/a n/a	PLATES GRIP MT20 244/190 Weight: 603 lb FT = 20%
LUMBER- TOP CHORD 2x6 SP No BOT CHORD 2x6 SP No WEBS 2x4 SP No OTHERS 2x4 SP No	0.2 0.3		BRACING- TOP CHORD BOT CHORD WEBS	end verticals. Rigid ceiling directly applied or 1 Row at midpt 19-53 16-56 22-49 MiTek recommends that Stab	3, 18-54, 17-55, 15-57, 14-58, 13-59, 6, 20-51, 21-50, 23-48, 24-47, 25-46,
(lb) - Max Horz Max Uplift Max Grav FORCES. (lb) - Max. Co TOP CHORD 10-72=-6 14-15=-1 19-20=-1	All uplift 100 lb or less at joint(s) 1, 5 65, 66, 67, 68, 51, 48, 47, 46, 44, 43, 14) All reactions 250 lb or less at joint(s) 39, 38, 37, 49, 36, 69 except 53=291(58=294(LC 44), 59=287(LC 44), 60=2 64=266(LC 44), 51=292(LC 43), 48=2 44=286(LC 44), 43=287(LC 44), 42=2 mp./Max. Ten All forces 250 (lb) or 1 18/259, 11-72=-64/266, 11-12=-82/300 28/405, 15-16=-136/424, 16-17=-131/ 31/410, 20-21=-131/410, 21-22=-131/	42, 41, 40, 39, 38, 37 ex 1, 55, 65, 66, 67, 68, 56 LC 43), 54=292(LC 43), 86(LC 44), 61=287(LC 4 55(LC 44), 47=294(LC 4 89(LC 44), 41=266(LC 4 ess except when shown , 12-13=-97/335, 13-14 411, 17-18=-131/410, 1 411, 22-73=-130/423, 2	xcept 69=-103(LC 6, 50, 40, 57=255(LC 44), 44), 63=289(LC 44 44), 46=287(LC 44 44) 1. 1. 1. 1. 1. 1. 1. 1. 1. 1.		
NOTES- 1) Unbalanced roof live lc 2) Wind: ASCE 7-16; Vult (envelope) gable end z 40-6-0 to 52-0-10, Corr members and forces & 3) Truss designed for wir Gable End Details as a 4) TCLL: ASCE 7-16; Pr= Cat B; Partially Exp.; C 5) Unbalanced snow load	28/406, 24-25=-112/370, 25-26=-97/3 bads have been considered for this de t=120mph (3-second gust) Vasd=95m cone and C-C Corner(3E) 0-0-0 to 5-9- ner(3E) 52-0-10 to 57-10-4 zone; canti MWFRS for reactions shown; Lumbe d loads in the plane of the truss only. applicable, or consult qualified building :20.0 psf (roof LL: Lum DOL=1.15 Plat be=1.0; Cs=1.00; Ct=1.10 Is have been considered for this desig nage to prevent water ponding.	sign. ph; TCDL=5.0psf; BCDI 10, Exterior(2N) 5-9-10 lever left and right expo r DOL=1.60 plate grip D For studs exposed to v designer as per ANSI/ e DOL=1.15); Pf=20.0 p	L=5.0psf; h=35ft; C to 20-2-6, Corner(3 sed ; end vertical le iOL=1.60 vind (normal to the TPI 1.	at. II; Exp B; Enclosed; MWFRS 3R) 20-2-6 to 40-6-0, Exterior(2N eft and right exposed;C-C for face), see Standard Industry 5 Plate DOL=1.15); Is=1.0; Rough	SEAL : =

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK F	UQUAY-VARINA, NC
25-3294-R01	R06	GABLE	1	1	Job Reference (optional)	# 58535
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:05:05 2025 Page 2 ID:av29u vm2cwLtXF0Wc5ybwyV6X0-TrSnRB0KkfrU96Da1DBSrSw5lbTrWx9xRgE4FVzQBTi						

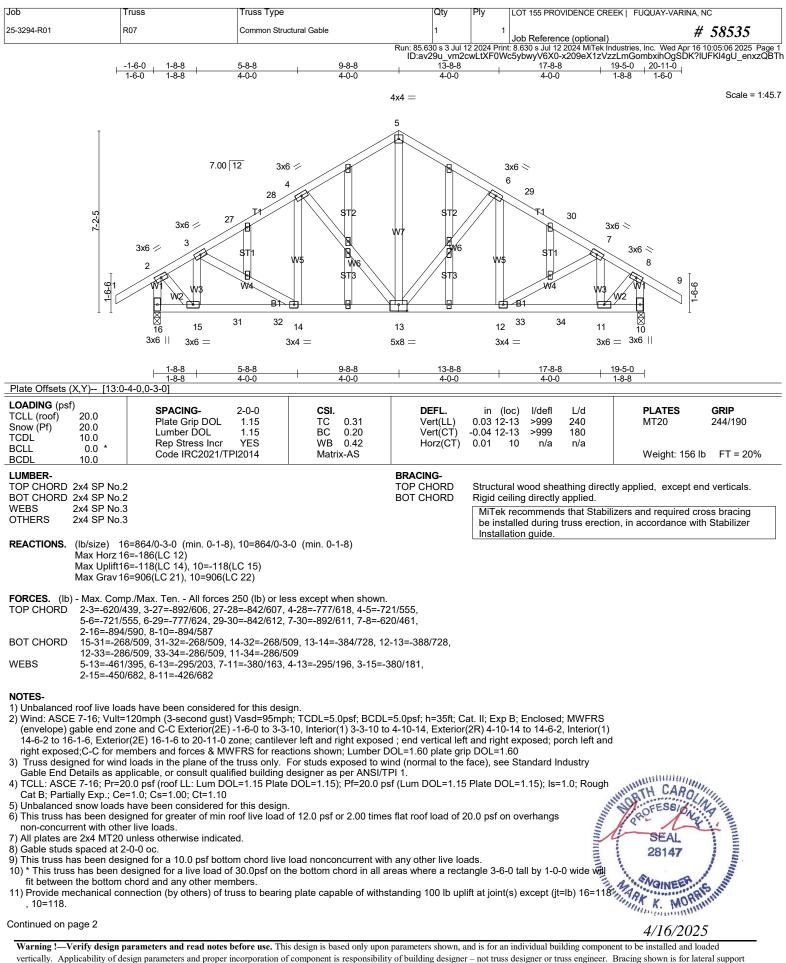
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

11) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 53, 54, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 51, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38, 37 except (jt=lb) 69=103.

LOAD CASE(S) Standard



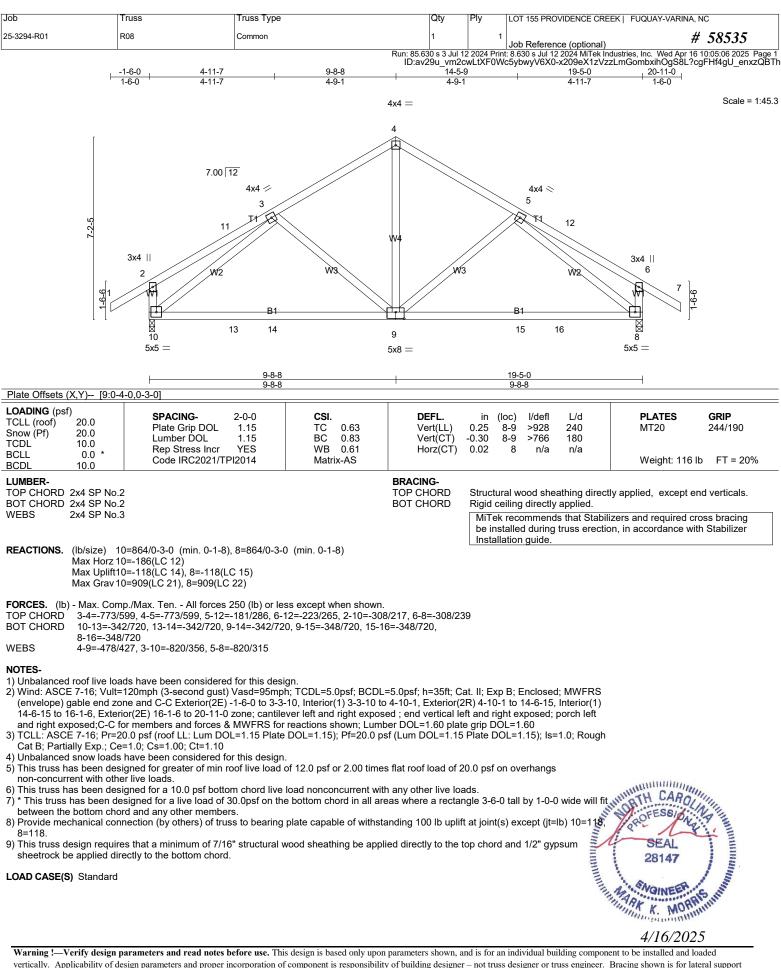


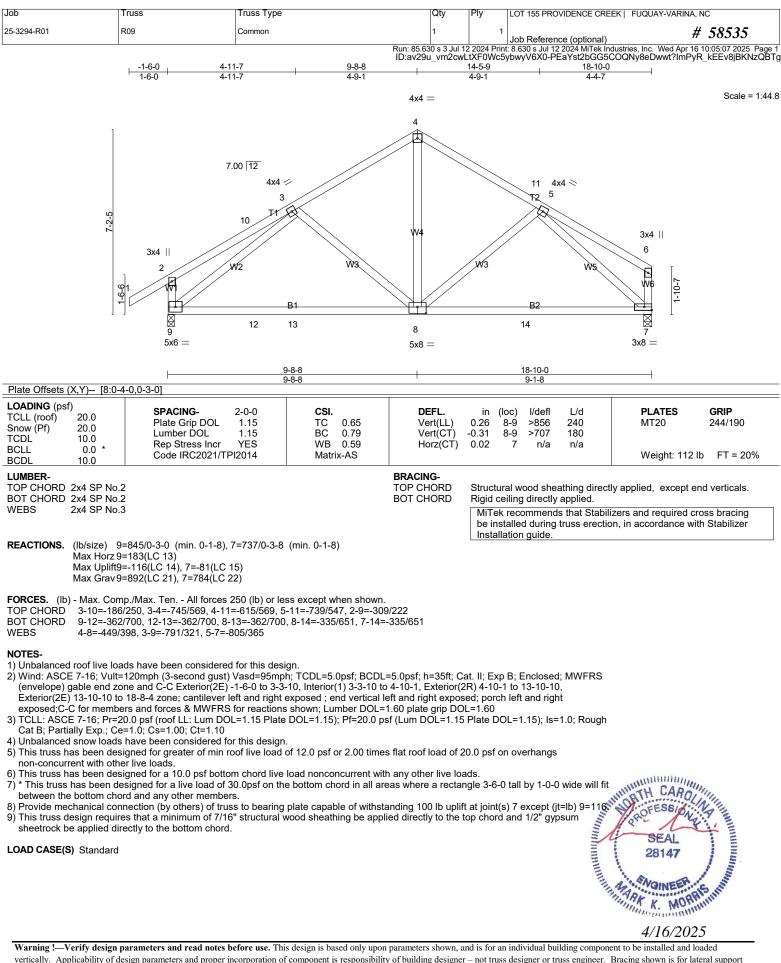
Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK	FUQUAY-VARINA, NC
25-3294-R01	R07	Common Structural Gable	1	1	Job Reference (optional)	# 58535
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:05:06 2025 Page 2 ID:av29u vm2cwLtXF0Wc5ybwyV6X0-x209eX1zVzzLmGombxihOqSDK?IUFKI4qU enxzQBTh						

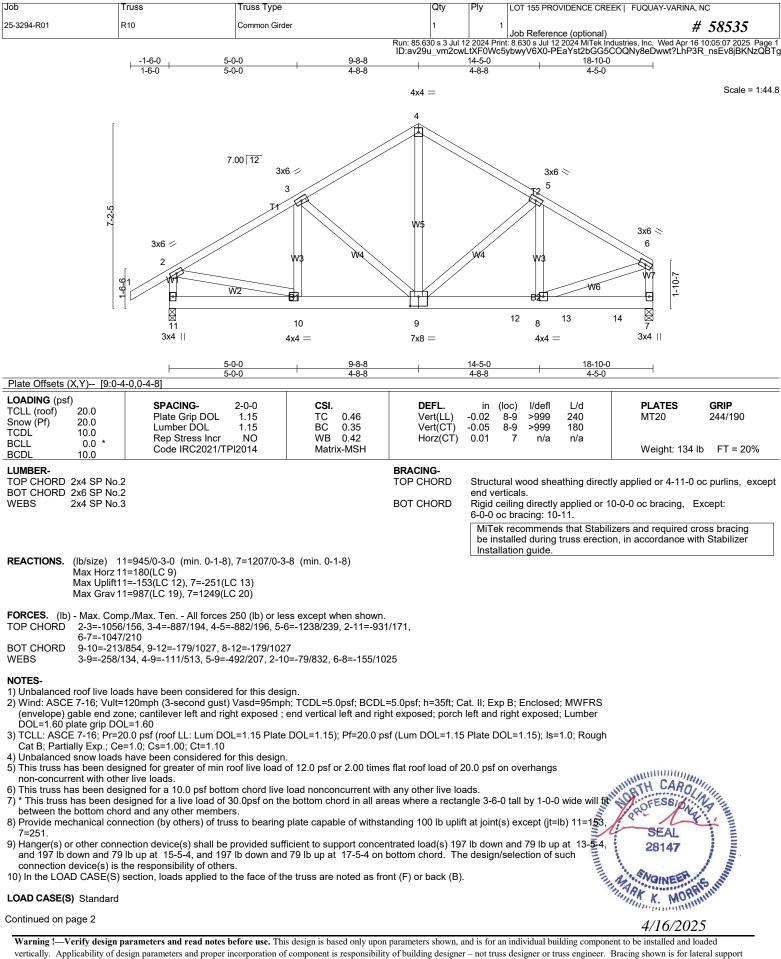
NOTES-12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard









vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and loaded vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and loaded of individual web members only. Additional perparameters and read notes before use. This responsibility of building designer – not truss designer or truss engineer. Bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 155 PROVIDENCE CREEK FUQUAY-VARINA, NC	
25-3294-R01	R10	Common Girder	1	1	Job Reference (optional) # 58	535
Run: 85.630 s 3 Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Apr 16 10:05:08 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-tQ8w3D3D1aD20ay9iMk9T5YWRpPgjE6N7oTksqzQBT						

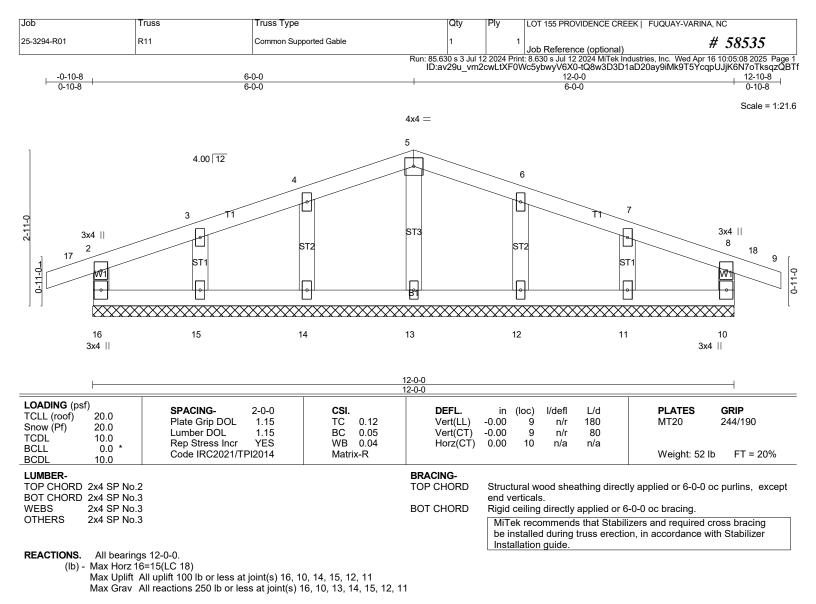
LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-60, 2-4=-60, 4-6=-60, 7-11=-20

Concentrated Loads (lb)

Vert: 12=-190(B) 13=-190(B) 14=-190(B)





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 8-0-0, Corner(3E) 8-0-0 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B: Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

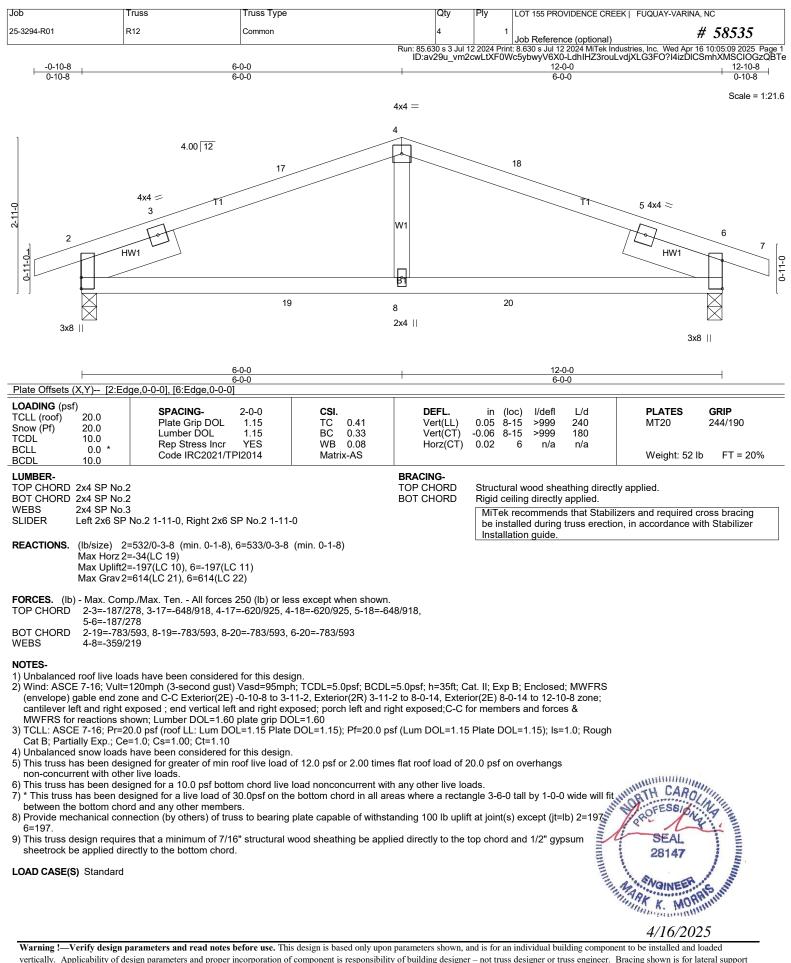
- All plates are 2x4 MT20 unless otherwise indicated.

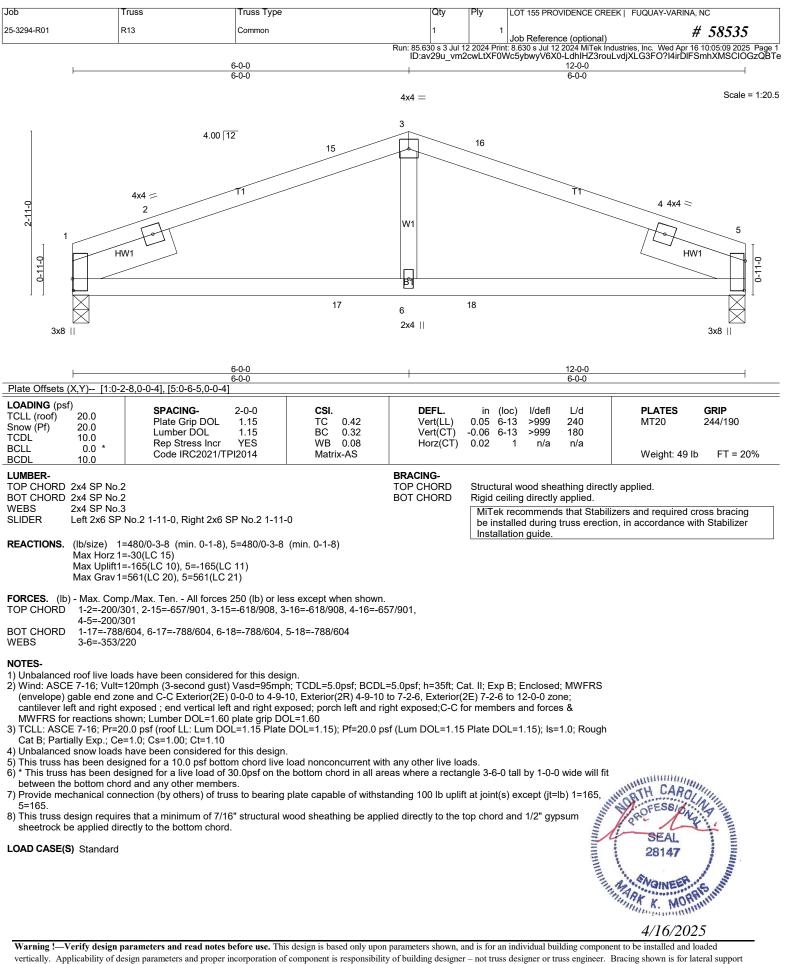
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the bottom chord and any other members.
 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 the standing 100 the stand

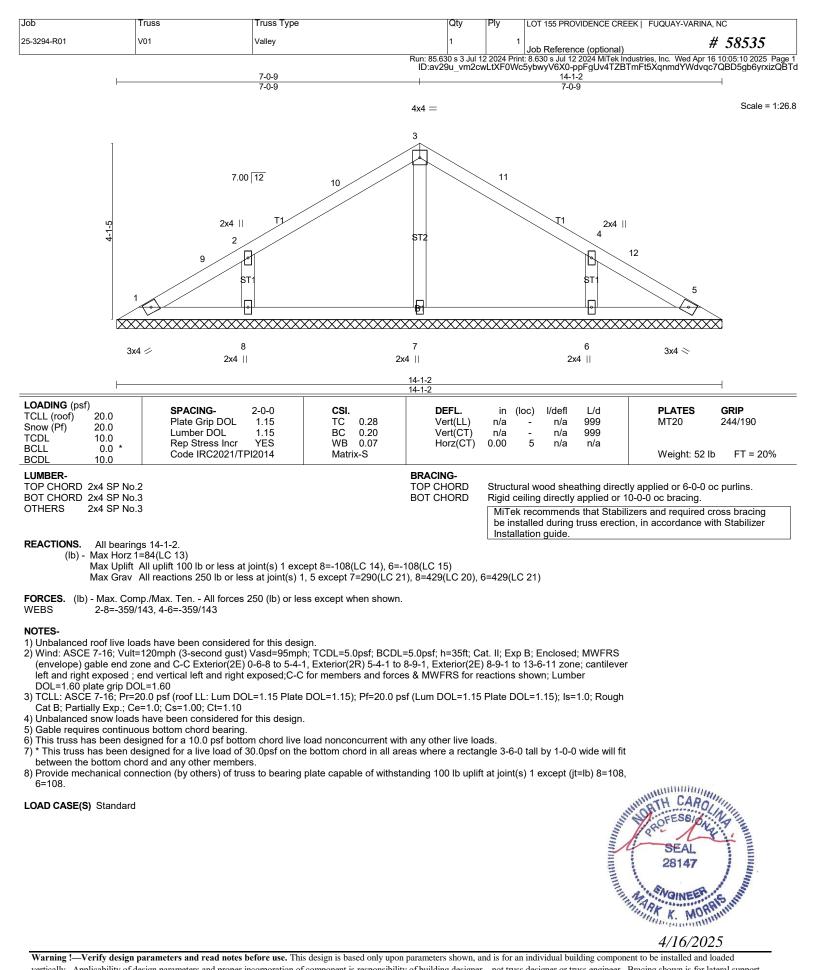
LOAD CASE(S) Standard

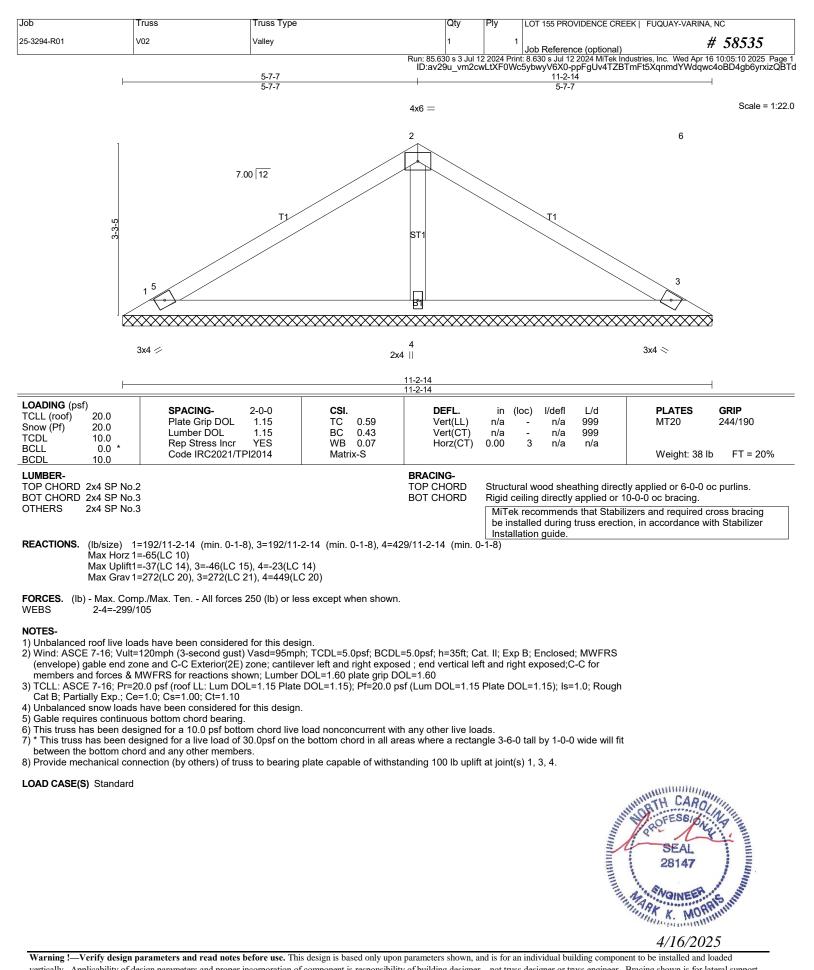


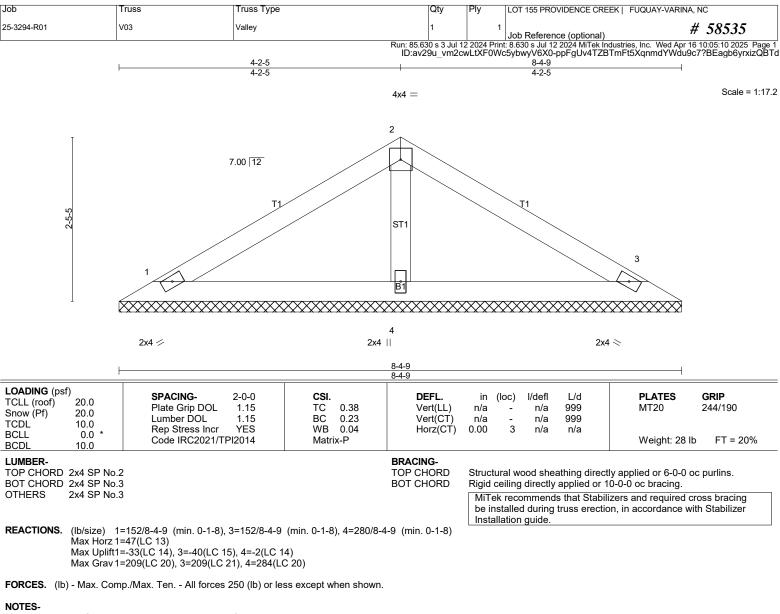
¹⁾ Unbalanced roof live loads have been considered for this design.











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3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

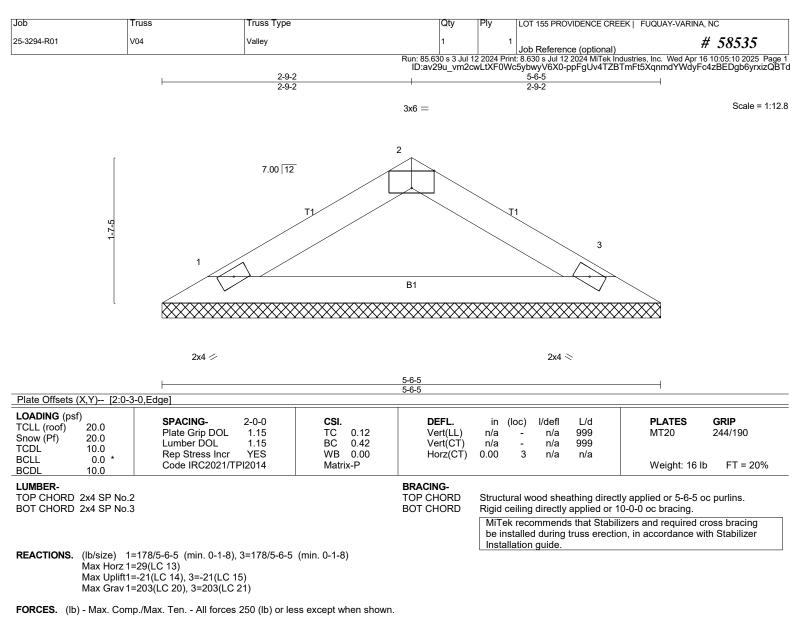
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





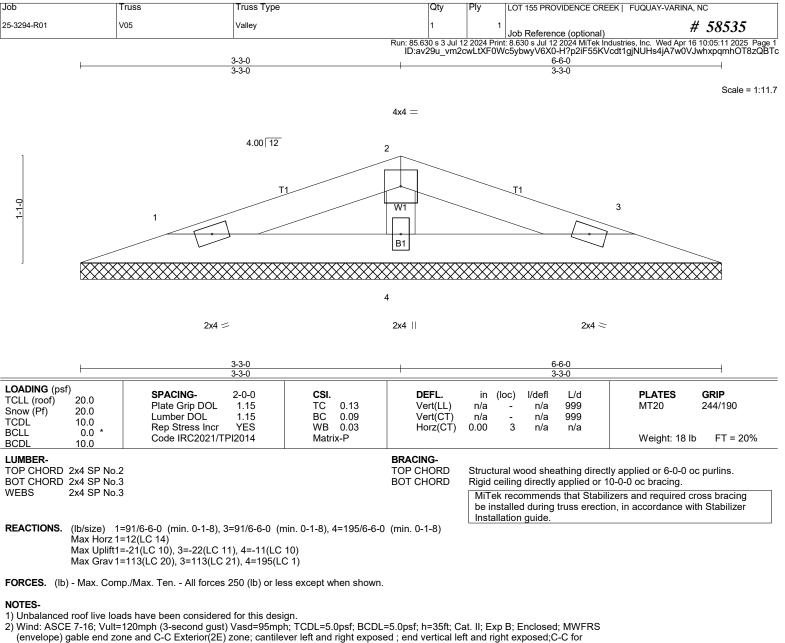
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- 4) Unbalanced snow loads have been considered for this design.

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- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard

