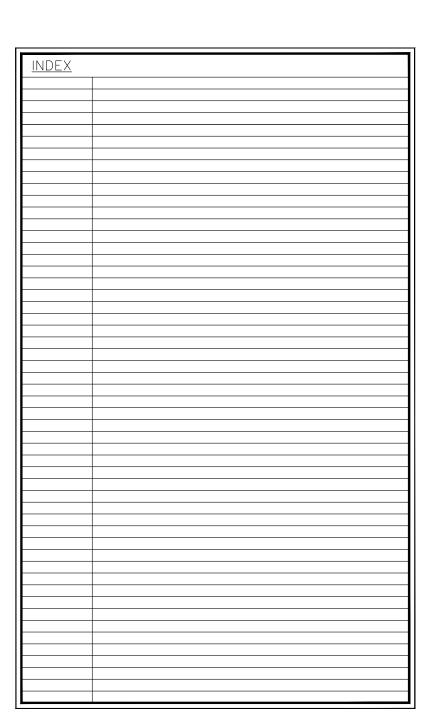
STONEHAVEN-RALE

RALEIGH- LOT 00.0116 BLAKE POND SF

(MODEL# 3656) ELEVATION 1 - GR



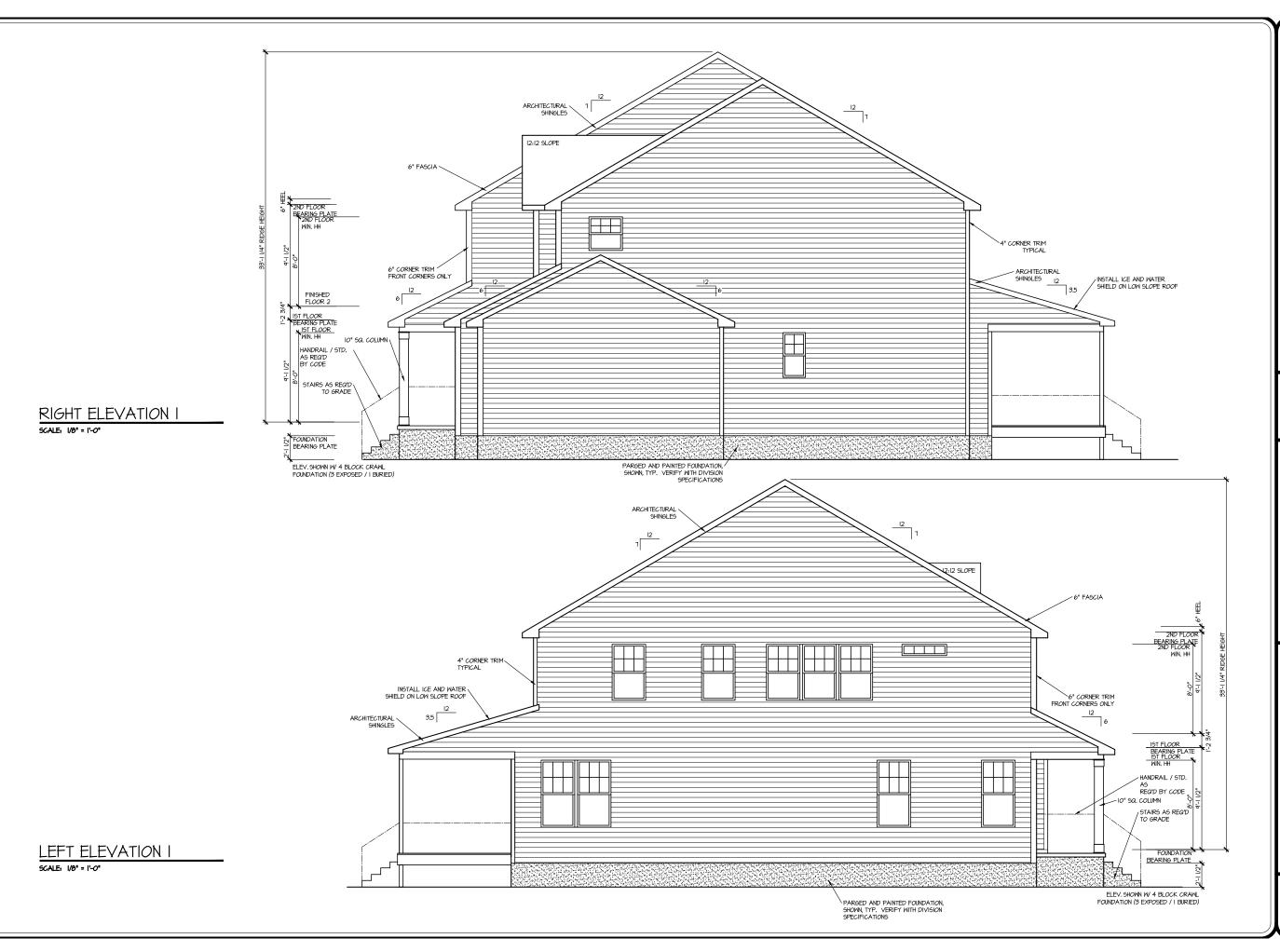


ADEA CALCULATIONS				
<u>AREA CALCULATIONS</u>			OOVEDED /	
ELEVATION 1		HEATED	COVERED / UNHEATED	UNCOVERED
			UNHEATED	UNCOVERED
FIRST FLOOR		1724 SF		
GARAGE			487 SF	
FRONT PORCH - ELEVATION 1			169 SF	
SECOND FLOOR		1939 SF		
OPTIONS				
3RD CAR GARAGE			264 SF	
COVERED PORCH			150 SF	
COVERED PORCH			150 SF	
	TOTAL	3663 SF	1070 SF	
<u> </u>				

22 CELTIC LANE

LOT	SPECIFIC	
1	LOT 00.0116	BLAKE POND SF
		STONEHAVEN R2 ELEVATION 1 22 CELTIC LANE LILLINGTON, NC 27546
2	ADDRESS	22 CELTIC LANE LILLINGTON, NC 27546
-		
-		
		





DRAWN BY: DATE: 04/01/2025 PLAN NO. 3656

0 8 0 HOUSE NAME:
STONEHAVEN
DRAWING TITLE <u>...</u> ₩

SHEET No.

<u>Ø</u> ∓±

ROOF VENTILATION CALCULATIONS:
ROOF AREA = 241 SO. FT.
CHEALERSEE OPERILATION.
CHEALERSEE OPERILATION.
1 TO 50 0 = 142 SO. FT.
1 TO 50 0 = 141 SO. FT.
50-008 IN TOP HIRD: = 357 - 5.11 SO. FT. (1 TO 300)
NET FREE AREA OF VENTES OPERIT = 5.15 SO. IN / LINEAR FT.
NET FREE AREA OF RIDGE VENT = 15 SO. IN/ LINEAR FT.

LOPER VENTINS. (BOTTOM 2/3 RDS)

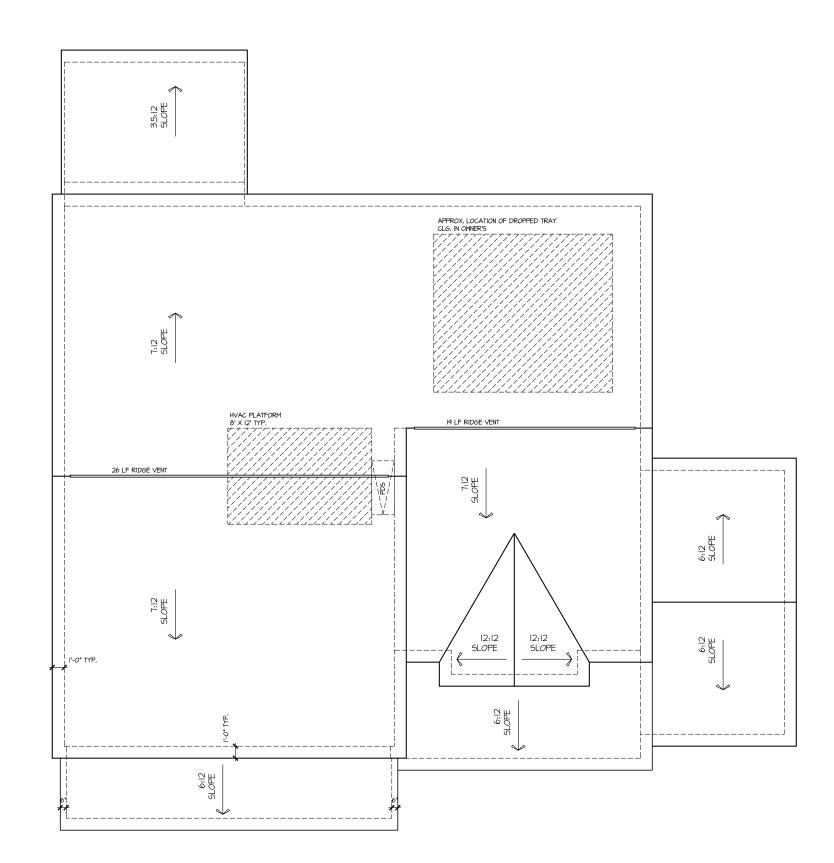
86 LIREAR FET OF SOFFIT X S.1 SQ. IN. = 3.40 SQ. FT.

19 LIREAR VENTINS. (TOP 1/3 RD)

19 LIREAR FET OF RIDGE X IS SQ. IN. = 5.69 SQ. FT.

10 SQ. ALLONED)

TOTAL ROOF VENTILATION. 4.03 SQ. FT. > 1.4 SQ. FT. (RQD)



ROOF PLAN ELEV. I

HOUSE NAME:
STONEHAVEN
DRAWING TITLE
ROOF PLAN

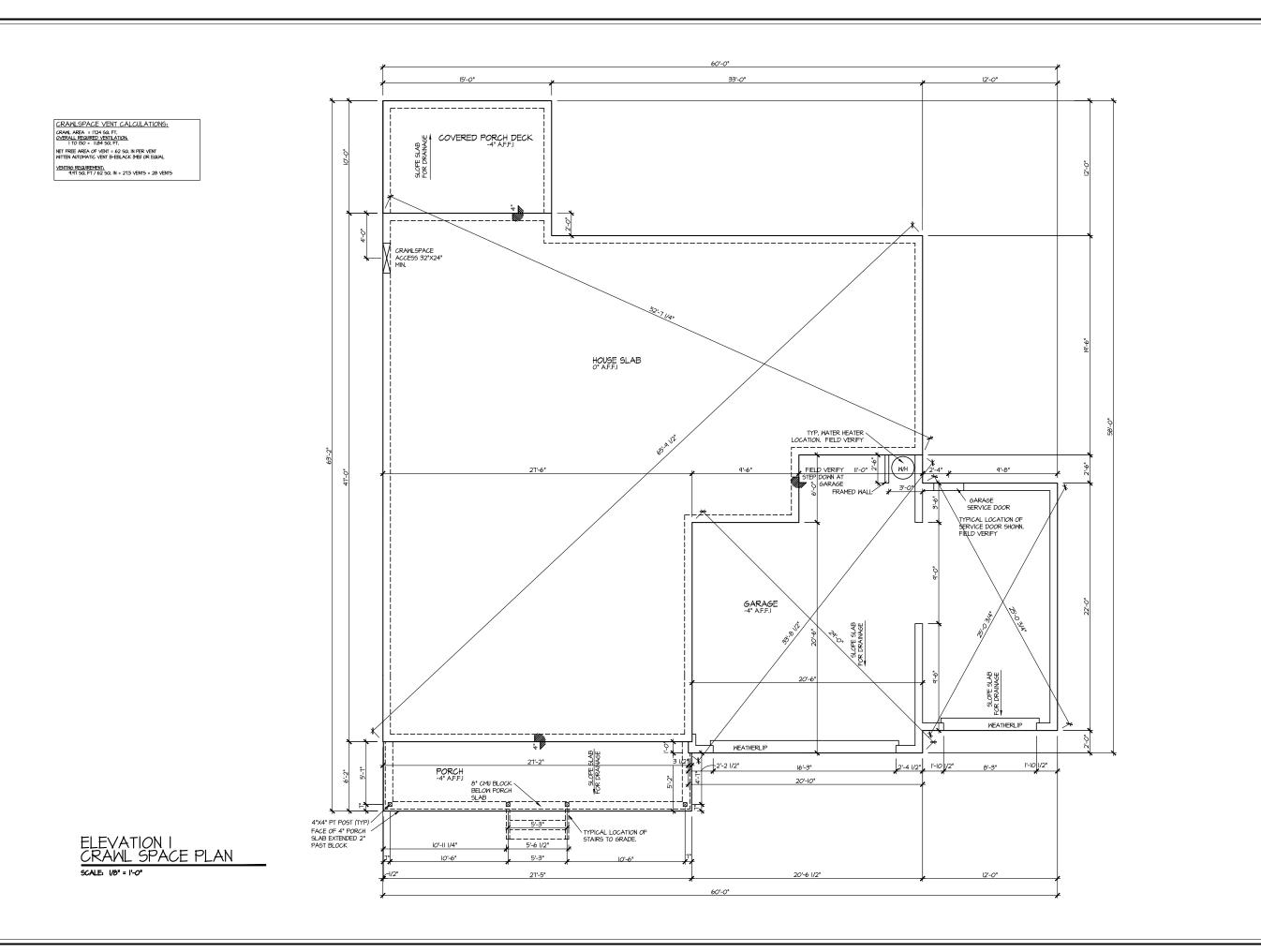
UPDATED DATE 09-13-2024

DRAWN BY:

DATE: 04/01/2025 PLAN NO. 3656

SHEET No.

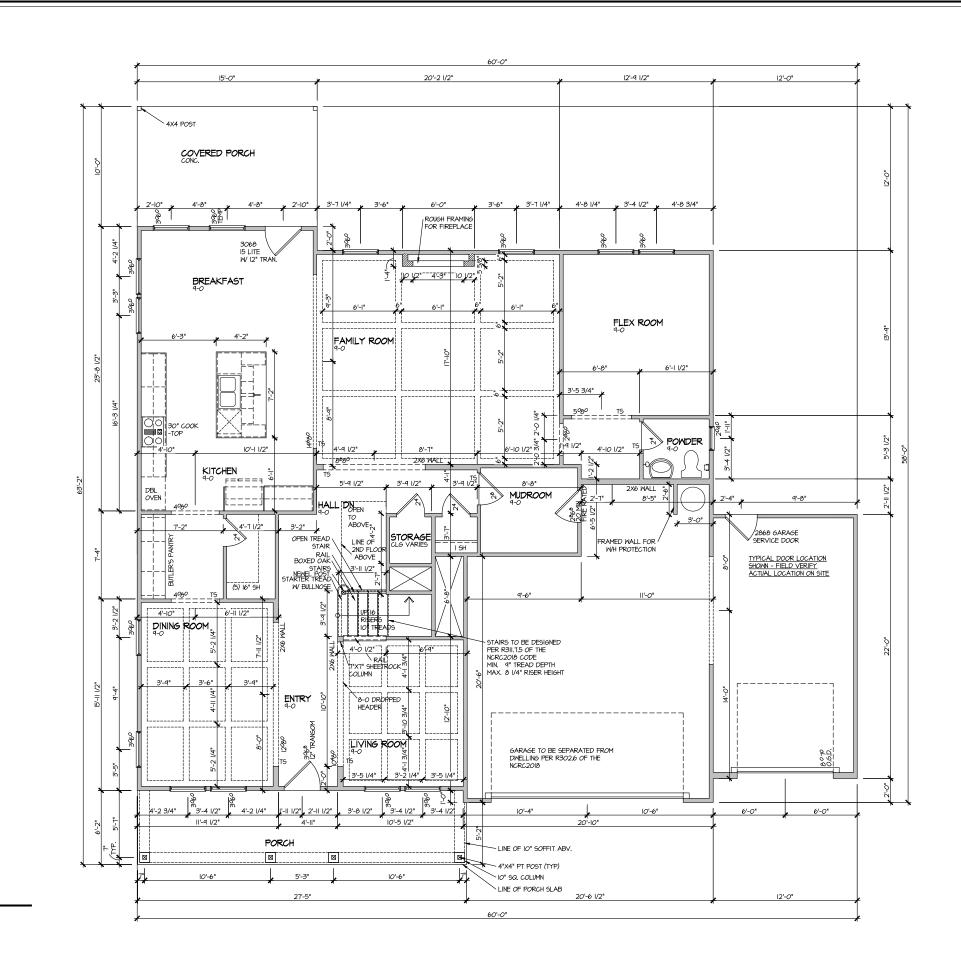
AI.3



UPDATED DATE 09-13-2024 DRAWN BY: DATE: 04/01/2025 PLAN NO. 3656

HOUSE NAME:
STONEHAVEN
DRAWING TITLE SPACE ORANL JAKIN

SHEET No. A2.



ELEVATION I FIRST FLOOR PLAN SCALE: 1/8" = 1'-0"

HOUSE NAME:
STONEHAVEN
DRAWING TITLE
FIRST FLOOR PLA

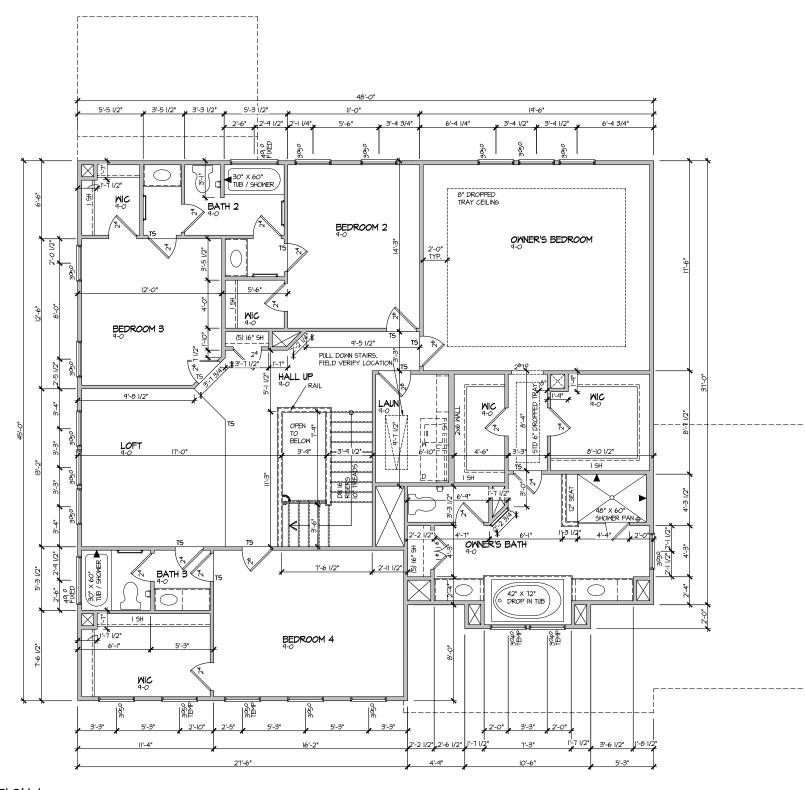
DRAWN BY:

PLAN NO. 3656

DATE: 04/01/2025

SHEET No.

A3.



ELEVATION I SECOND FLOOR PLAN SCALE, 1/8" = 1'-0" STONEHAVEN
DRAWING TITLE
SECOND FLOOR PL,

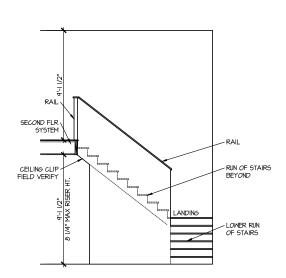
DRAWN BY:

PLAN NO. 3656

DATE: 04/01/2025

SHEET No.

Lot 00.0116.dwg DATE: 4/1/2025 12:20 PM



STAIR SECTION SCALE: 1/8" = 1'-0"

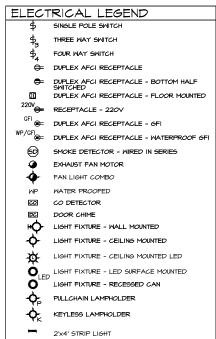
DRAWN BY:

DATE: 04/01/2025 PLAN NO. 3656

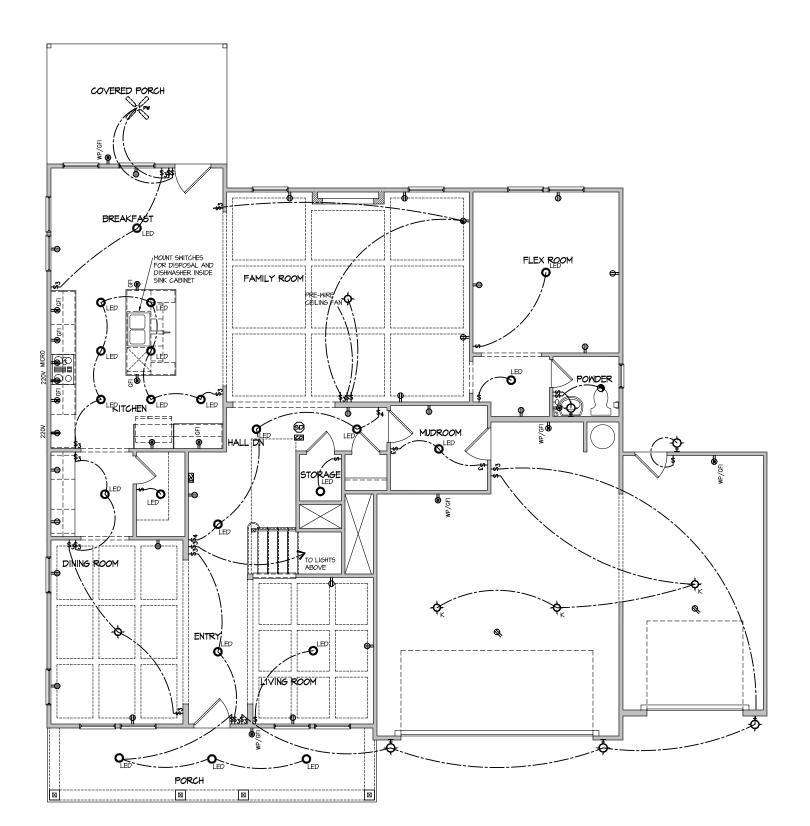


HOUSE NAME:
STONEHAVEN
DRAWING TITLE
BUILDING SECTION

SHEET No. **A4**.



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



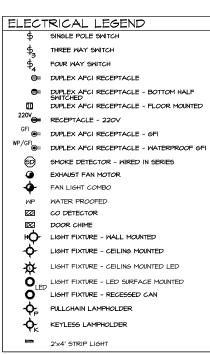
ELECTRICAL PLAN FIRST FLOOR - ELEV. I SCALE: 1/8' = 1'-0'

HOUSE NAME:
STONEHAVEN
DRAWING TITLE
FIRST FLOOR ELECTR

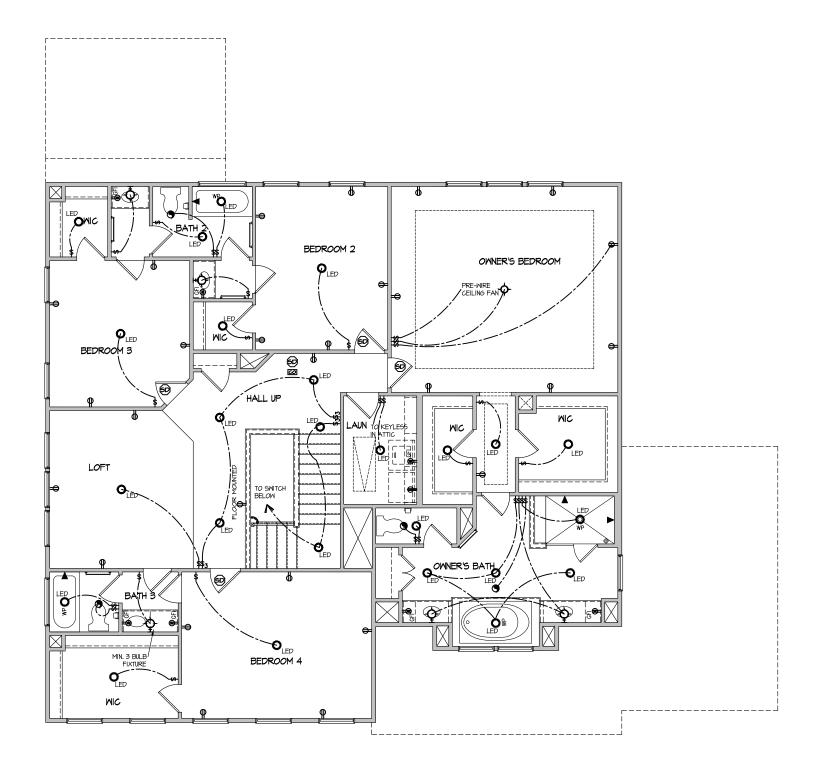
DRAWN BY:

DATE: 04/01/2025 PLAN NO. 3656

SHEET No.



NOTE. ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



ELECTRICAL PLAN SECOND FLOOR - ELEV. I SCALE: 1/0" = 1'-0"

FILE: Lot 00.0116.dwg DATE: 4/1/2025 12:20 PM

HOUSE NAME:
STONEHAVEN
DRAWING TITLE

O

ロコロ

SECOND

DRAWN BY:

PLAN NO. 3656

DATE: 04/01/2025

SHEET No.

DESCRIPTION OF BLDG, ELEMENT	3"x0.131" NAILS	3"x0.120" NAILS	
JOIST TO SOLE PLATE	(3) TOENAILS	(3) TOENAILS*	
SOLE PLATE TO JOIST/BLK'G.	(3) NAILS @ 4" o.c.	(3) NAILS 0 4" o.c.	
STUD TO SOLE PLATE	(2) TOENAILS	(3) TOENAILS*	
TOP OR SOLE PLATE TO STUD	(2) NAILS	(3) NAILS	
RIM TO TOP PLATE	TOENAILS @ 8" O.C.	TOENAILS @ 6" o.c.*	
BLK'G. BTWN. JOISTS TO TOP PL.	(3) TOENAILS	(3) TOENAILS*	
DOUBLE STUD	NAILS @ 24" o.c.	NAILS @ 16" O.C.	
DOUBLE TOP PLATE	NAILS @ 24" o.c.	NAILS @ 16" o.c.	
DOUBLE TOP PLATE LAP SPLICE	(9) NAILS IN LAPPED AREA	(II) NAILS IN LAPPED AREA	
TOP PLATE LAP @ CORNERS €	(2) NAILS	(2) NAILS	
INTERSECTING WALLS			
* 4/1 0 10 10 11 1 4 4 5 5 1 1 1 1 4 5 5 1 1 1 1 1 1			

2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE * ARE SHOWN)

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL. INCLUDING, BUT NOT LIMITED TO, FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY OR WARRANTY TO FRANCES.

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS. FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS NOTED OTHERWISE ON PLAN

TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN AD JACENT BEAMS DO NOT EXCEED THE FOLLOWING:

- I/4" DEAD LOAD
- FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS: I/8" DEAD LOAD
- FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEELECTION TO 3/16" DEAL LOAD. (NOT DIFFERENTIAL DEFLECTION)

GENERAL STRUCTURAL NOTES

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE:
- WOOD FRAME ENGINEERING IS BASED ON NDS, "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS

ROOF DEAD = 7 PSF T.C., IO PSF B.C.

LOAD DURATION FACTOR = 1.25 LIVE = 40 PSF (30 PSF @ SLEEPING AREAS)

DEAD = 10 PSF (1-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES)

(ADD'L IO PSF @ TILE)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

GENERAL FRAMING

- ALL TYP, NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS, ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION. ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.
- REFER TO FASTENING SCHEDULE TABLE R602.3(I) FOR ALL CONNECTIONS, TYP, U.N.O.
- EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS,
 I6" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. • WALLS OVER 12' TALL SHALL BE PER PLAN.
- ALL HEADERS, BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRICE-PINE-FIR #2 (SPF) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KILN-DRIED). ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY, CODE TABLES HAVE NOT BEEN USED.
- ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED @ 16" O.C. (MAX. UN.O.) • HEADERS IN NON-LOAD BEARING WALLS SHALL BE:
- (1)2x4/6 FLAT @ OPENINGS UP TO 4'. (2)2x4/6 FLAT UP TO 8'
- ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15).
- ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING
- "LSL" Fb=2325 psi: Fv=3I0 psi: E=I,55xI0^6 psi
- 'LVL' Fb=2600 psi; Fv=285 psi; E=2.0xl0^6 psi 'PSL' - FB=2900 PSI; FV=290 PSI; E=2.0XI0^6 PSI
- M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO M+K FOR STRUCTURAL REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
- FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 8" O/C OR 2 ROWS 1/4"x3/5" SIMPSON SDS SCREMS (OR 3½" TRUSSLOK SCREMS) © 16" O/C. USE A MINIMUM OF 3 ROMS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE SOLID 3 K" OR 5 K" BEAMS ARE ACCEPTABLE. USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8 MEMBERS.
- FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROMS OF $\frac{1}{4}$ "x6" SIMPSON SDS SCREWS (OR 6 $\frac{3}{4}$ " TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER, APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE, A SOLID 7" BEAM IS ACCEPTABLE
- ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x KING STUD, MINIMUM.
- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O.,
- ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER W/ 3"X0.131" NAILS @ 24" O.C. (MIN.), EACH PLY.
- PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND,/BEARING. BLOCKING TO MATCH POST ABOVE
- FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR I/2" DIA. BOLTS @ 48" O.C. STAGGERED.
- ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

FLOOR FRAMING

- I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT M&K FOR MARBLE FLOOR DESIGNS)
- AT I-JOIST FLOORS, PROVIDE I I/8" MIN. OSB RIM BOARD.
- METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
- FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR' 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND
- 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12"o.c. FIELD. - 2 3 × 0.120" NAILS @ 4" O.C. @ PANEL EDGES & @ 8" O.C. FIELD.
- 2 3" x 0,113" NAILS @ 3" O.C. @ PANEL EDGES & @ 6" O.C. IN FIELD.
- #6 x 2" MIN. SCREWS @ 6" O.C. @ PANEL EDGES & @ 12" O.C. FIELD.

ROOF FRAMING

- BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.
- FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) @ ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H2 5T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS
- METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
- ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
- SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO
 - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS) - TRUSS VERTICALS W/ (3) 3"x0.131" NAILS @ 19.2" O.C. MAX. (FLOOR TRUSSES)
- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS
- W/ 2 ½" x 0.131" NAILS @ 6"o.c. @ PANEL EDGES € @ 12" O.C. FIELD. - W/ 2 (8 × 0.120" NAILS @ 4"0.c. @ PANEL EDGES \$ @ 8" O.C. FIELD.
- W/ 2 3" x 0.113" NAILS @ 3"O.C. @ PANEL EDGES & @ 6" O.C. FIELD.

VENEER LINTEL SCHEDULE

SPAN (MAX)	HEIGHT OF VENEER ABOVE LINTEL	STEEL ANGLE SIZE
3'-0"	20 FT. MAX	L3"x3"x¼"
6'-0"	3 FT. MAX	L3"x3"x/4"
	I2 FT. MAX	L4"x3"x/4"
	20 FT. MAX	L5"x31½"x5%"
	3 FT. MAX	L4"x4"x½" *
8'-0"	I2 FT. MAX	L5"x31½"x5%"
	I6 FT. MAX	L6"x31/2"x3%"
q'-6"	I2 FT. MAX	L6"x3½"x¾6"
16'-0"	2 FT. MAX	L7"x4"x/2" **
	3 FT. MAX	L8"x4"x½" **

SHALL SUPPORT 2 %" - 3 ½" VENEER w/ 40 pef MAXIMUM WEIGHT.

- : 16' SHALL HAVE 8" MIN. BEARING : 16' SHALL NOT BE FASTENED BACK TO HEADER.
- 16' SHALL BE EASTENED BACK TO WOOD HEADER IN WALL 448" W/½" DIA. x 3 ½" LONG LAG SCREMS IN 2" LONG VERTICALLY SLOTTED HOI FG.
- MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
 WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE
- HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE $3\,\text{\ensuremath{\mathbb{Z}}}^*$ WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT FINISHING. HE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT
- NCOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTE FASTENED BACK TO BEAM, FASTENERS SHALL MAINTAIN A 2/2" (MINIMUM) CLEAR DISTANCE FROM BOTTOM OF BEAM. FOR QUEEN VENEER USE L4x3x1/a
- * FOR 3/5" VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

LATERAL BRACING & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RO

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER IRC R301.2.1.1) EXP. B, RISK CAT. 2 & SEISMIC CAT. A/B.

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NCSBC-RC OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY ACCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST

THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NCSBC:RC SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602 3 5& R802 II

EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W 2 3/8"x0.II3" NAILS @ 6" O.C. AT EDGES & ● 12" O.C. IN THE PANEL FIELD. TYP, U.N.C.
- HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.
- ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.
- ALT. STAPLE CONNECTION SPEC: 1½" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6" O.C IN FIELD.

BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 36" x 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 1 34" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6" O.C IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING w/8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
- DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O.
- ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING
- PRE-MANUFACTURED PANELIZED WALLS FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

INDICATES EXTENT OF INT. OSB SHEARWAI OR 3" O.C. OSB SHEARWALL.

► INDICATES HOLDOWN BELOW

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODE: RESIDENTIAL CODE.
- FOOTING DESIGN 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED. BUILDER/CONTRACTOR MUST VERIFY.
- FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX, FROM PLATE ENDS - UTILIZING
- 1/2" DIA. ANCHOR BOLTS 6'-0" O.C, 7" MIN. EMBEDMENT
- (CONC), 15" MIN, EMBEDMENT (CMU) SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC)
- SIMPSON MAB23 ANCHOR STRAPS @ 2'-8" O.C. (CMU) (REFER TO DETAILS FOR IO' TALL WALL ANCHOR REQUIREMENTS)
- ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.
- BUILDER TO VERIEY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT W/ PRESERVATIVE-TREATED WOOD, CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.
- BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 € 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER.
- CONCRETE DESIGN BASED ON ACL 318 CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.: 4,000 psi: FOUNDATION WALLS 2,500 psi: FOOTINGS & INTERIOR SLABS ON GRADE 3,000 psi: GARAGE & EXTERIOR SLABS ON GRADE
- 60,000 psi BASEMENT FOUNDATION WALL DESIGN BASED ON:
- · 9' OR 10' HEIGHT (AS NOTED ON PLANS) - TALLER WALLS MUST BE ENGINEERED.
- NOMINAL WIDTH (91/3" FOR 10" THICK WALL).
- BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC. ML-CL, OR CL).
- BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL Ist FLOOR DECK.
- PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL WITH 2" CLEAR, REINFORCEMENT
- SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.

 FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 w/ (2)2x6 JACK STUDS, U.N.O
- LARGER OPENINGS SHALL BE PER PLAN.
- ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT
- ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.
- FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 95% COMPACTED FILL.
- PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY
- TO DEVELOP • JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR
- 15'-0" O.C. (MAXIMUM) JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS
- POSSIBLE (I.I RATIO) WITH A MAXIMUM OF I.15 RATIO · CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL
- CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (F'm=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 \$ 530.I.
- CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
- PROVIDE 2x8 x 16" LONG PT PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID. PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS.
- FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE

DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

• BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES, METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

HOLD-DOWN SCHEDULE

SYMBOL	SPECIFICATION
► HD-I	SIMPSON HTT4 HOLD-DOWN * (5/6" DIA. ANCHOR)
► HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM U.N.O.) -OR- MSTC66B3 ALTERNATE
▶ HD-3	SIMPSON STHDI4/STHDI4RJ

* UTILIZE THE SSTB24 ANCHOR BOLT @ ALL MONOSLAB & INTERIOR RAISED 5LAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24" MIN.

OOTING THICKNESS REQUIRED EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FASTEN THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE IO" (FOR 5/8" DIA.) OR

15" (FOR 7/8" DIA.) MIN. EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D. <u>DO NOT LOCATE ANCHORS WITHIN I 3/4" OF EDGE OF CONCRETE</u>

a: 4/10/25 CAR PROFESSION, O. ENGINE SEPHT. R

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I&K project numbe 126-2306

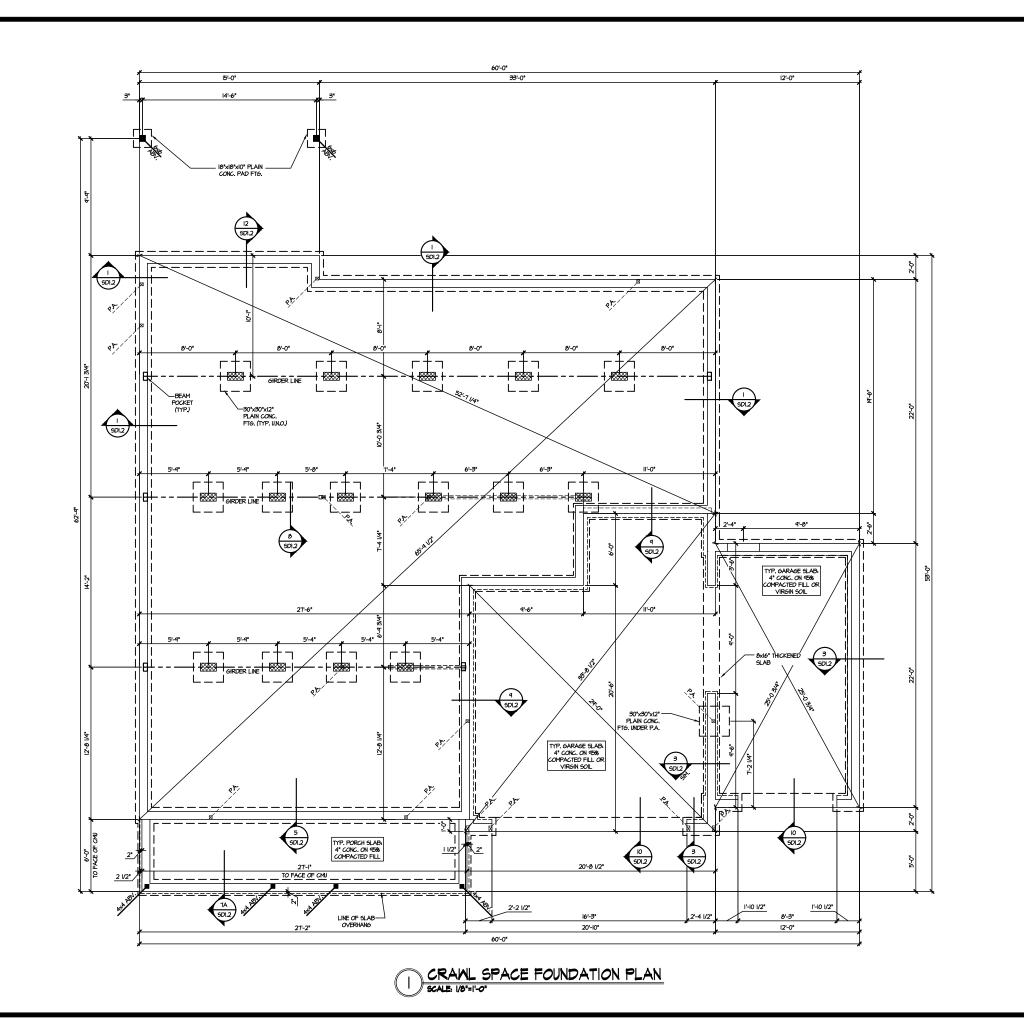
JTR rawn by: JAC sue date: 04-08-2

REVISIONS

initial:



COMMUNIT AVEN 1 O POND CC STONEHAVE BLAKE LOT 116 -RALEIGH,



eal: 4/10/25

MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERI

M&K project number: 126-23061

JTR drawn by: issue date: 04-08-2

BLAKE POND COMMUNITY Lot 116 - Stonehaven 1 Raleigh, nc **OUNDATION PLANS**

S1.0

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

LEGEND

• = = INDICATES SHEAR WALL & EXTENT

* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

• IIIIII INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE • BEAM / HEADER

• EXTENT OF OVERFRAMING

JL METAL HANGER



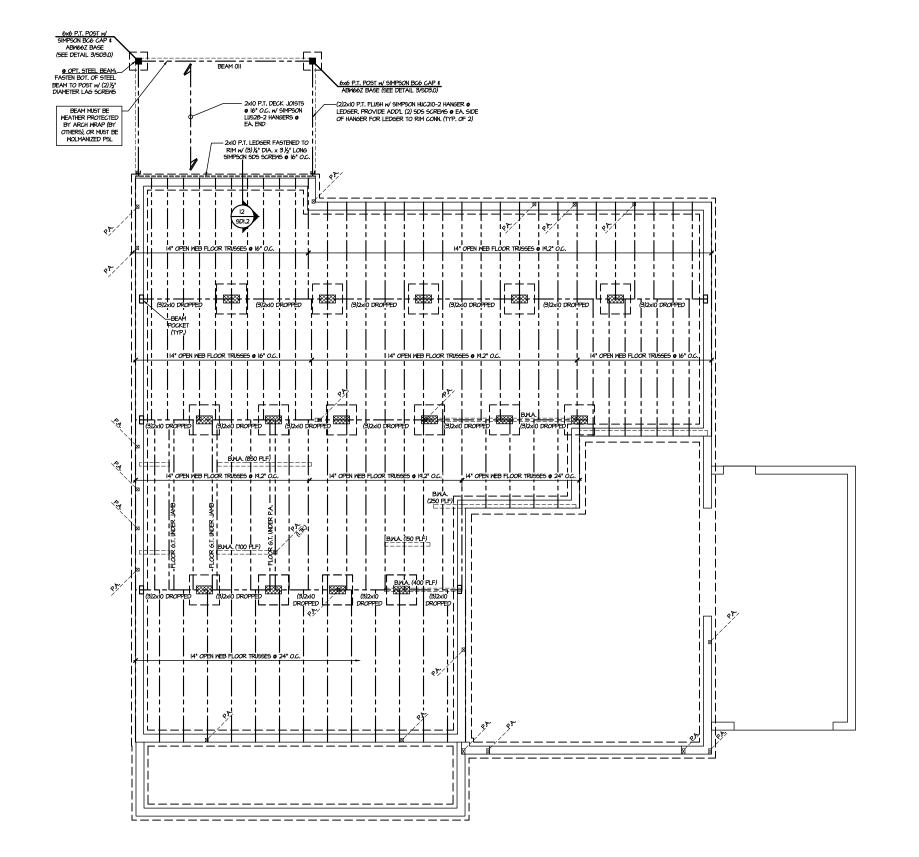
MULHERN+KUL
RESIDENTIAL STRUCTURAL ENGINEERI

I&K project number: 126-2306

JTR drawn by: **JAD** ssue date: 04-08-2

REVISIONS: initial:

BLAKE POND COMMUNITY Lot 116 - Stonehaven 1 Raleigh, nc OR



	ENGINEERED BEAM MATERIAL SCHEDULE					
BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION	
001	(2)134"x914" - F	3½"x9¼" - F	N/A	(2)2xI0 + (I) ¼"x4¼" STEEL FLITCH PLATES - F	W8xIO − F	
002	(2)194"x18" - FB	3½"xl8" - FB	N/A	(2)2xi2 + (i) i"xii¼" steel Flitch plates - F	WI2xI9 - F	
002A	(3)194"x16" - FB	5¼"xl6" - FB	N/A	(3)2xi2 + (2) ½"xii¼" STEEL FLITCH PLATES - F	WI2xI9 - F	
003	(2)13/4"×14" - F	3½"x 4" - F	(2)i¾"xi4" - F	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATES - F	WI2xI4 - F	
004	(2)13/4"x14" - F	3½"x 4" - F	(3)19/4"×14" - F	(2)2xi2 + (i) %"xi以" STEEL FLITCH PLATES - F	WI2xI4 - F	
005	(4)13/4"x18" - D	7"x18" - D	N/A	(4)2x12 + (3) %"xII¼" STEEL FLITCH PLATES - D	WI2x30 - D	
006	(2)194"xII%" - H	3½"xII%" - H	(3)1¾"x11%" - H	N/A	N/A	
001	(2)13/4"×14" - H	3½"x 4" - H	(3)13/4"×14" - H	(2)2xi2 + (i) %"xi以" STEEL FLITCH PLATES - H	N/A	
000	(3)1¾"x14" - F	34"×14" - F 5¼"×14" - F	N/A	(3)2xi0 + (2) 以"xii以" STEEL FLITCH PLATES - F	WI2xI4 - F	
009	(3)13/4"×18" - H	5¼"×18" - H	N/A	(3)2xi2 + (2) ½"xi以" STEEL FLITCH PLATES - H	N/A	
010	(2)19%"×9%" - H	3½"x9¼" - H	(2)134"×94" - H	(2)2xi0 + (i) ¼"xq¼" Steel Flitch Plates - H	MØxIO − H	
OII	(3)1¾"×4¼" - F	5¼"x9¼" - F	(3)134"×94" - F	(3)2xi0 + (2) 以"xi以" STEEL FLITCH PLATES - F	W8×IO − F	
012	(4)1¾"×11½" - D	7"xII%" - D	N/A	(4)2xi2 + (3)以"xi以" STEEL FLITCH PLATES - D	WI2xI4 - D	
013	(3)13/4"x16" - FT 5/4"x16" - FT		N/A	(3)2xi2 + (2) ½"xi以" STEEL FLITCH PLATES - F	WI2xI9 - F	
014	(2)13/4"×14" - F	3½"x 4" - F	(3)13/4"×14" - F	(2)2xl2 + (l) %"xl以" STEEL FLITCH PLATES - F	WI2xI4 - F	

- BEAM NOTATION: "F" INDICATES FLUSH BEAM
- "FT" INDICATES FLUSH TOP BEAM "FB" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM

- "D" INDICATES DROPPED ØPENNG HEADER

 "H" INDICATES DROPPED ØPENNG HEADER

 REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS

 REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS

 FOR FLUSH TOP BEANS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D, FASTEN

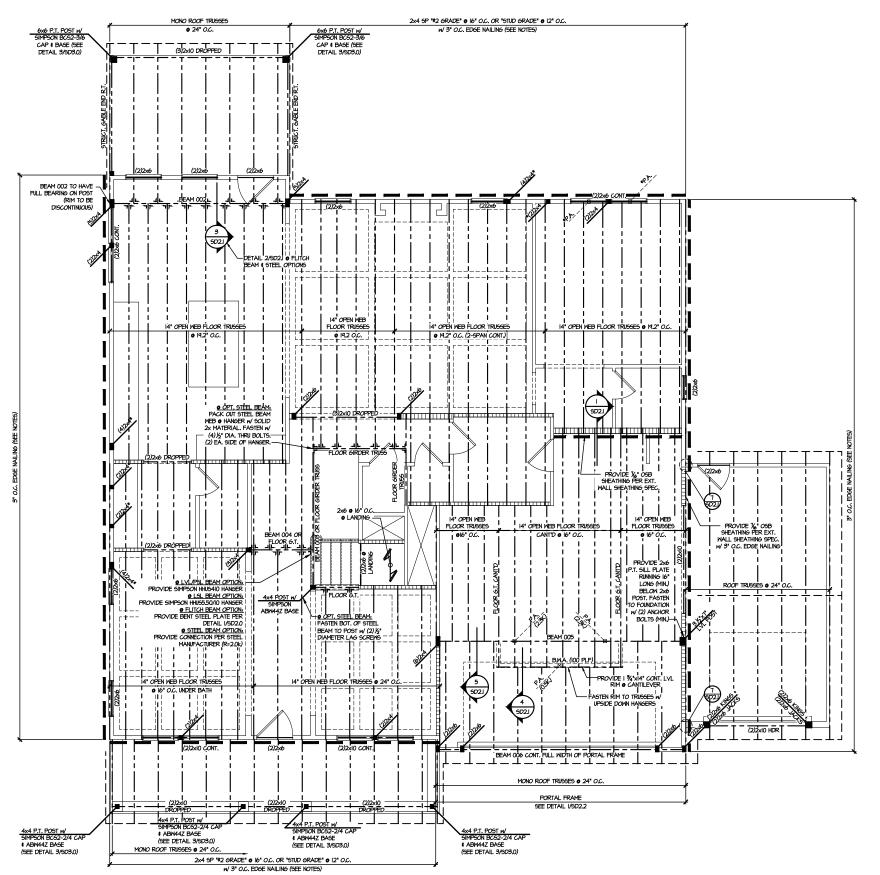
 PLATES IN SUCCESSION W (2) 3"X0.120" NAILS © 8" O.C.
- FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C.

LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- * INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES \$ SCHEDULES

IST FLOOR FRAMING PLAN



2ND FLOOR FRAMING PLAN

ENGINEERED BEAM MATERIAL SCHEDULE BEAM LVL OPTION FLITCH OPTION STEEL OPTION 2)2x10 + (1) ¼"x4¼" STEEI FLITCH PLATES - F 001 (2)13/4"x9 1/4" - F 3½"x9¼" - F W8xIO - F (2)2xl2 + (I) I"xII¼" STEEL FLITCH PLATES - F 002 (2)13/4"x18" - FB 3%"xi8" - FB MIONIA - I (3)2xl2 + (2) 片"xl以" STEEI FLITCH PLATES - F 002A (3)13/4"x16" - FB 5¼"xl6" - FB WI2xI4 - I (2)2x12 + (1) ¼"x1¼" STEEI FLITCH PLATES - F 003 (2)|3/4"×|4" - F 3½"x14" - F (2)13/4"×14" -(2)2xl2 + (I) %"xll4" STEEL FLITCH PLATES - F 004 (2)|3/"v|4" - F 3½"xl4" - F (3)13/4"x14" -1)2x12 + (3) %"xII¼" STEE FLITCH PLATES - D 005 (4)13/4"x18" - D 7"xI8" - D WI2x30 - D N/A 006 N/A (2)13/4"×117/6" - H (3)134"x1136" - H N/A 3½"xII%" - H (2)2xl2 + (l) **%**"xl以" STEEI FLITCH PLATES - H (2)13/4"×14" - H (3)13/4"×14" - H 3½"xl4" - H (3)2x10 + (2) ¼"x1¼" STEE FLITCH PLATES - F 800 (3)13/4"×14" - F 5½"xl4" - F N/A WI2xI4 - F (3)2xi2 + (2) 片"xi以" STEEL FLITCH PLATES - H 009 (3)13/4"x18" - H 5½"xl8" - H N/A N/A (2)2x10 + (1) ¼"x4¼" STEEL FLITCH PLATES - H 010 M8xIO - H (2)134"x914" - H 3K"x9¼" - H (2)134"×94" - H (3)2x10 + (2) ¼"x1¼" STEEL FLITCH PLATES - F OII 5¼"x9¼" - F W∂xI0 - F (4)2xl2 + (3) ¼"xll¼" STEEL FLITCH PLATES - D 012 (4)13/4"x113/6" - D 7"xII%" - D WI2xI4 - D (3)2xi2 + (2) ½"xi以" STEEI FLITCH PLATES - F 013 (3)13/4"x16" - FT 5¼"xl6" - FT WI2xI9 - F N/A (2)2x12 + (1) %"xII¼" STEEL FLITCH PLATES - F 014 (2)13/4"×14" - F 3½"xl4" - F (3)13/4"×14" - F WI2xI4 - F

BEAM NOTATION:
- "F" INDICATES FLUSH BEAM

- "FT" INDICATES FLUSH TOP BEAM
- "FB" INDICATES FLUSH BOTTOM BEAM
- "D" INDICATES DROPPED BEAM
- "H" INDICATES DROPPED OPENING HEADER

REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN

PLATES IN SUCCESSION W (2) 3"XO(120" NAILS • 8" O.C.
FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION w/ (2) 3"x0.120" NAILS @ 8" O.C.

AND RIVE

a: 4/10/25 CAR ENGINEE EPH T. R

MULHERN+KULP RESIDENTIAL STRUCTURAL ENGINEERING



M&K project number: 126-2306

JTR frawn by: JAD ssue date: 04-08-2

REVISIONS

initial:

S POND COMMUNITY STONEHAVEN 1

OOR

BLAKE I LOT 116 - S RALEIGH,

LEGEND

● □===□ BEARING WALL ABOVE BEAM / HEADER

EXTENT OF OVERFRAMING

JL METAL HANGER

* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

• IIIIIII INTERIOR BEARING WALL

• = = INDICATES SHEAR WALL & EXTENT

INDICATES HOLD-DOWN OR STRAP.

S3.

MULHERN+KULP
RESIDENTIAL STRUCTURAL ENGINEERING

M&K project number: 126-23061

JTR drawn by: issue date: 04-08-2

REVISIONS:

initial:

LEGEND

• == INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

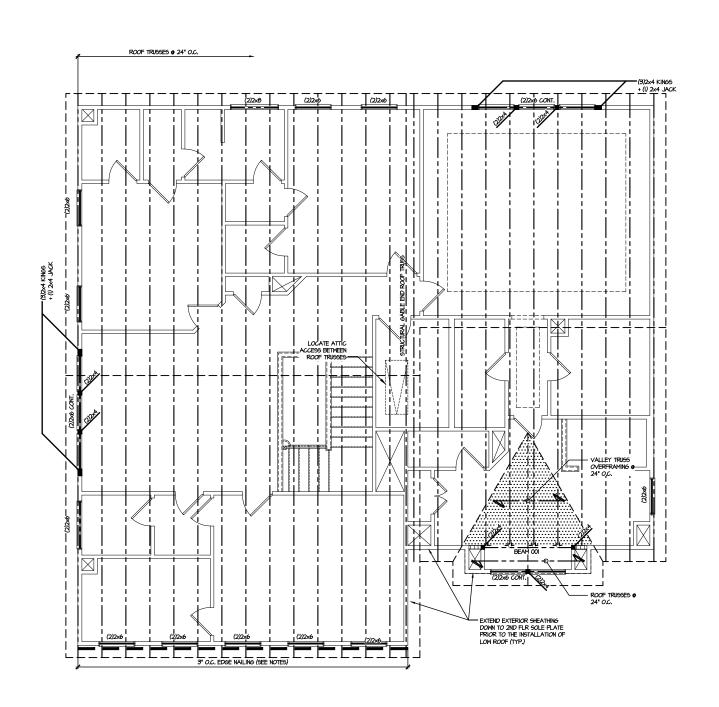
REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

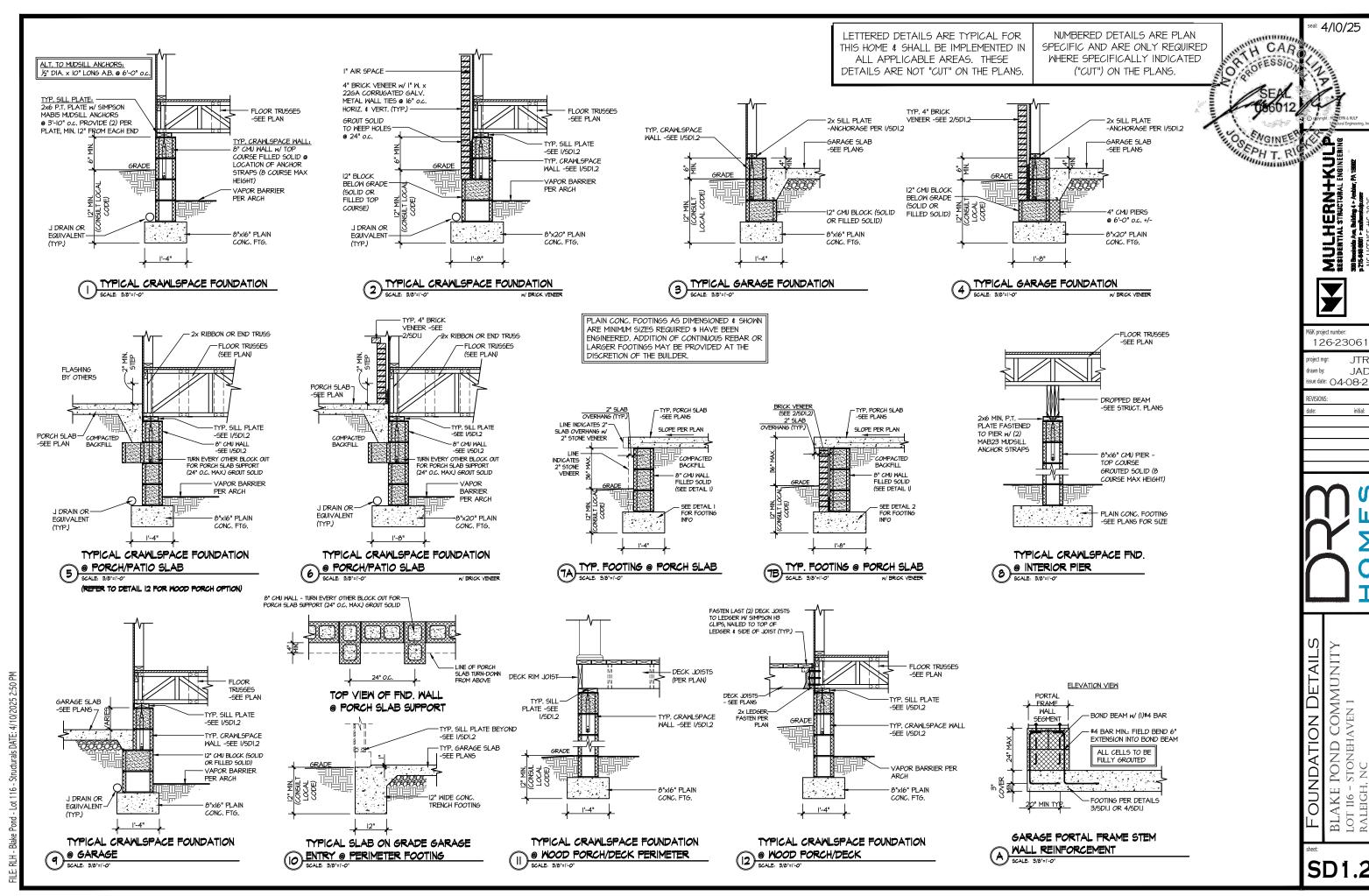
 INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE BEAM / HEADER

JL METAL HANGER

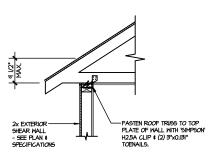
BLAKE POND COMMUNITY Lot 116 – Stonehaven 1 Raleigh, nc ROOF



ROOF FRAMING PLAN SCALE: 1/0"=1'-0"

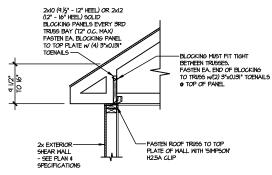


initial:



TYPICAL SHEAR

TRANSFER DETAIL @ ROOF HEEL HEIGHT LESS THAN 4½" NO BLOCKING REQ'D

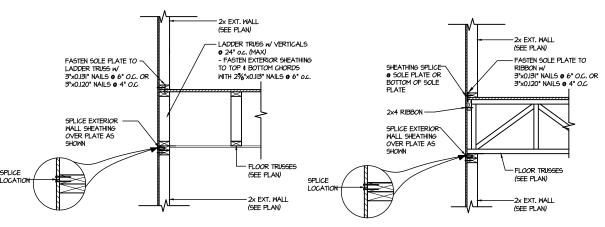


TYPICAL SHEAR

TRANSFER DETAIL @ ROOF

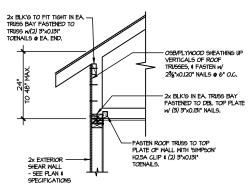
SCALE: 3/8"=1"-0" HEEL HEIGHT BETWEEN 9,





TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL
SCALE SIGNAL FROM

TYPICAL SHEAR TRANSFER DETAIL BETWEEN FLOORS @ EXTERIOR WALL
SCALE 989*1-0" FEMPENDIAL AR FRAME

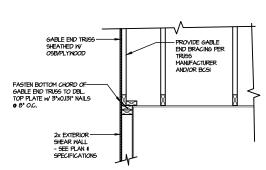


TYPICAL SHEAR TRANSFER

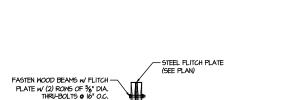
DETAIL @ RAISED HEEL TRUSS

2x WOOD BEAM

(SEE PLAN)

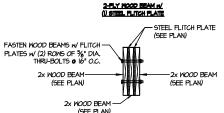


TYPICAL GABLE END DETAIL SOLLE: 3/8"=1"-0"



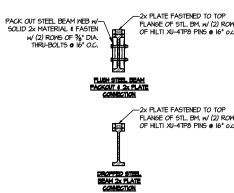
-2x WOOD BEAM

(SEE PLAN)

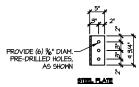


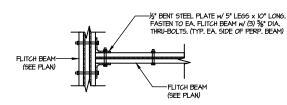
S-FLY OR MORE WOOD BEAM W/ (2 OR MORE) STEEL FLITCH FLATES

TYPICAL FLITCH BEAM CONNECTION DETAIL



TYPICAL STEEL BEAM CONNECTION DETAIL





TYPICAL FLITCH BEAM TO FLITCH BEAM CONNECTION DETAIL

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

FLANGE OF STL. BM. w/ (2) ROWS OF HILTI XU-47P8 PINS @ 16" o.c. -2x PLATE FASTENED TO TOP FLANGE OF STL. BM. w/ (2) ROWS OF HILTI XU-4TP8 PINS @ 16" o.c.

DETAILS Ŋ

BLAKE POND COMMUNIT Lot 116 - Stonehaven 1 Raleigh, nc RAMIN

a: 4/10/25

MULHERN+KULP HESIDENTIAL STRUCTURAL ENGINEERING

M&K project number:

drawn by:

REVISIONS

126-2306

issue date: 04-08-2

JTR

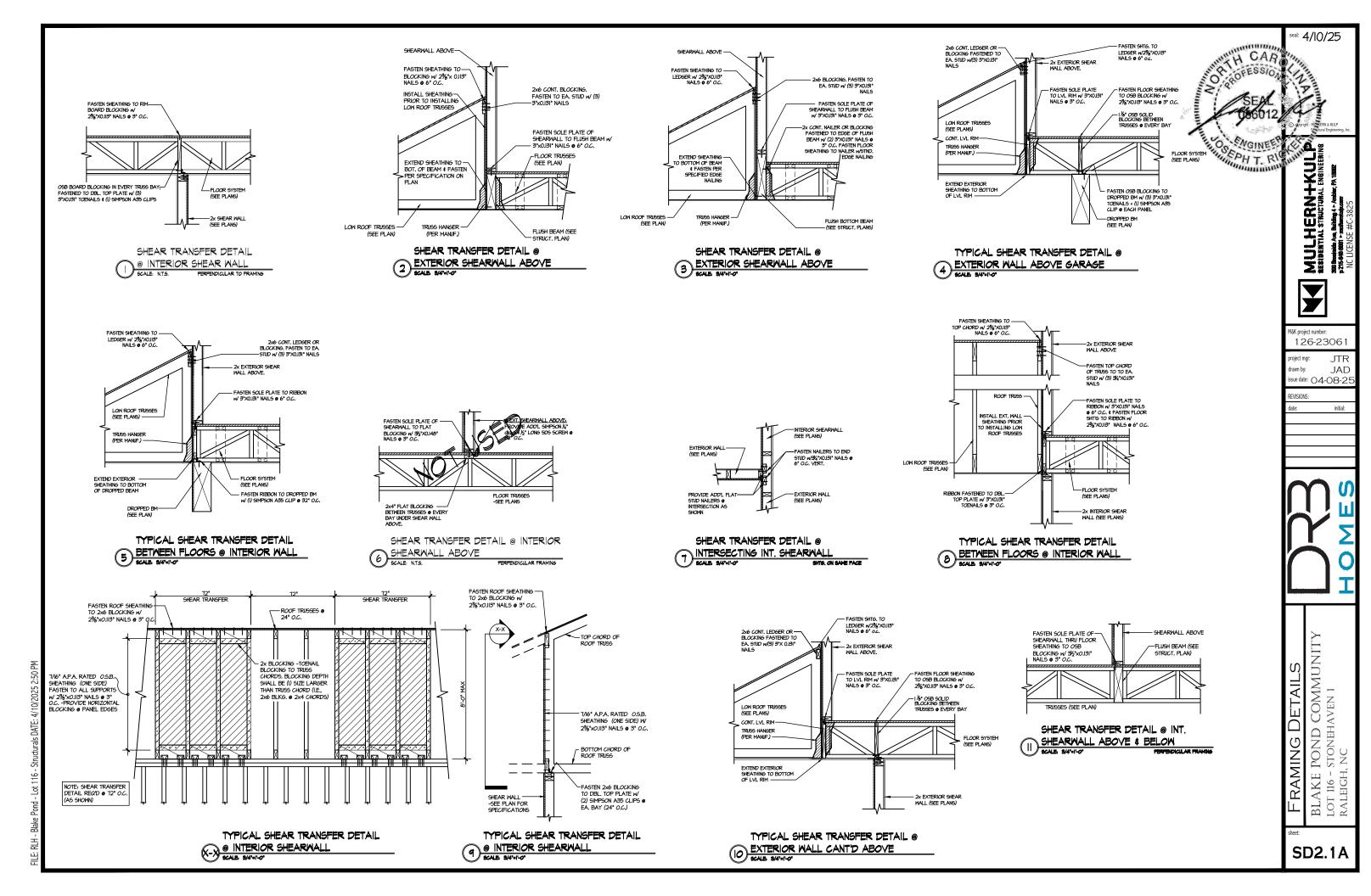
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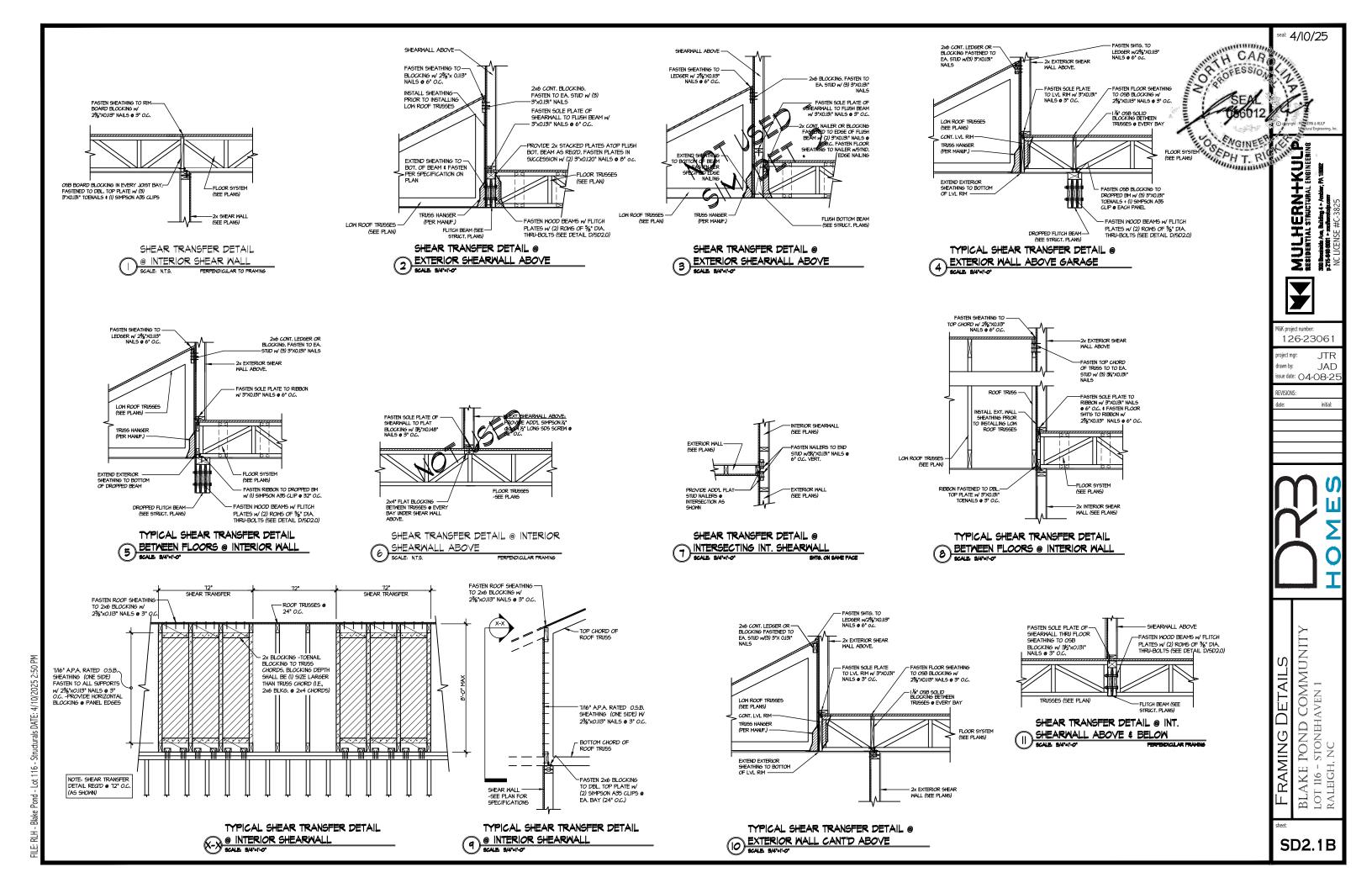
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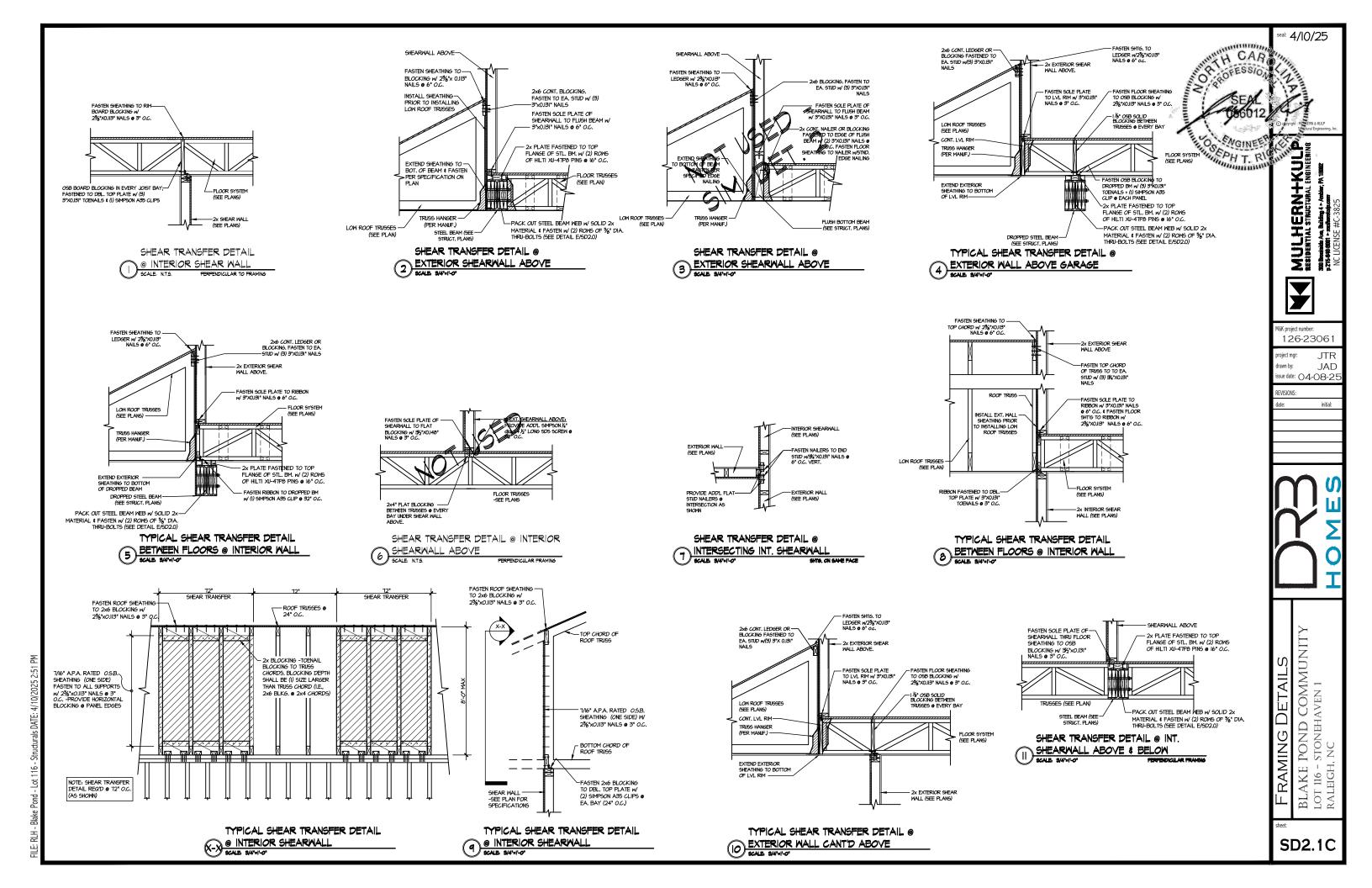
TH CAR

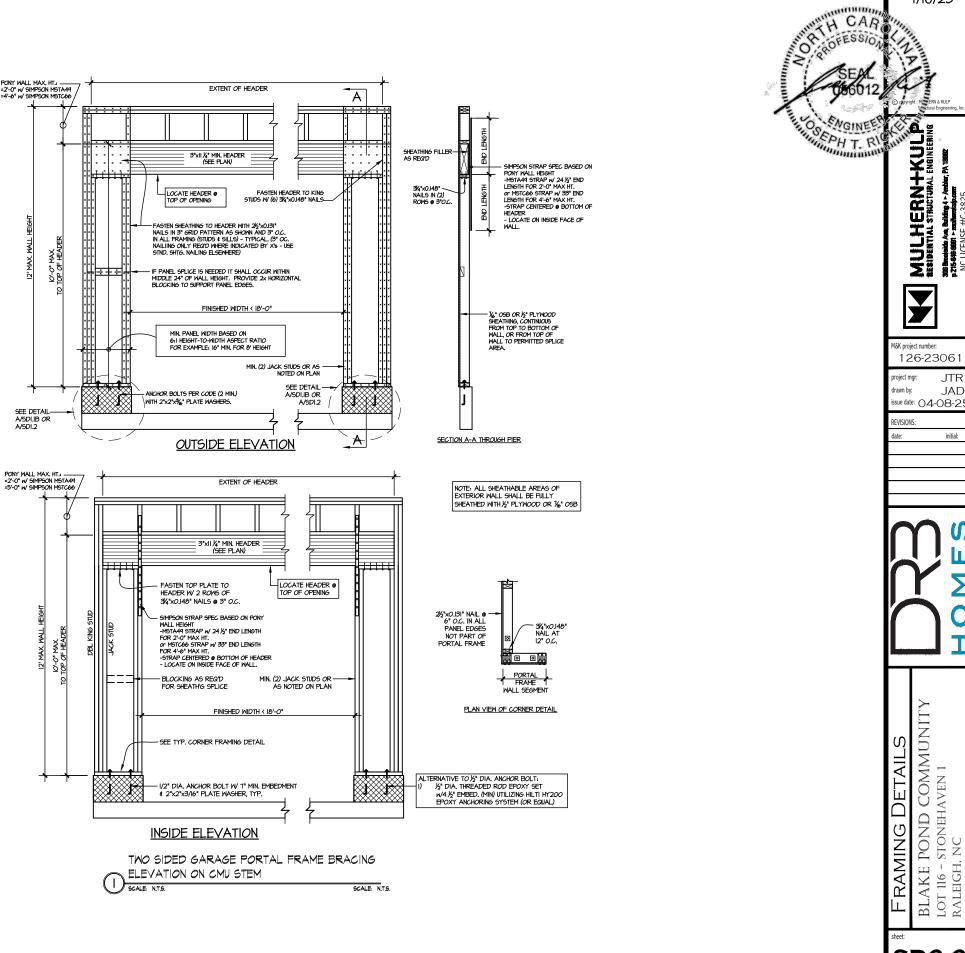
OSEPH T. RI

SD2.0









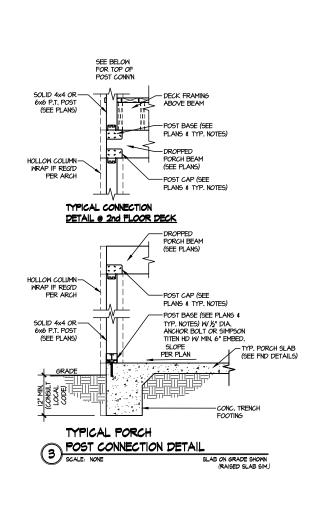
BLAKE POND COMMUNITY LOT 116 - STONEHAVEN 1 RALEIGH, NC

eal: 4/10/25

JTR

JAD

initial:



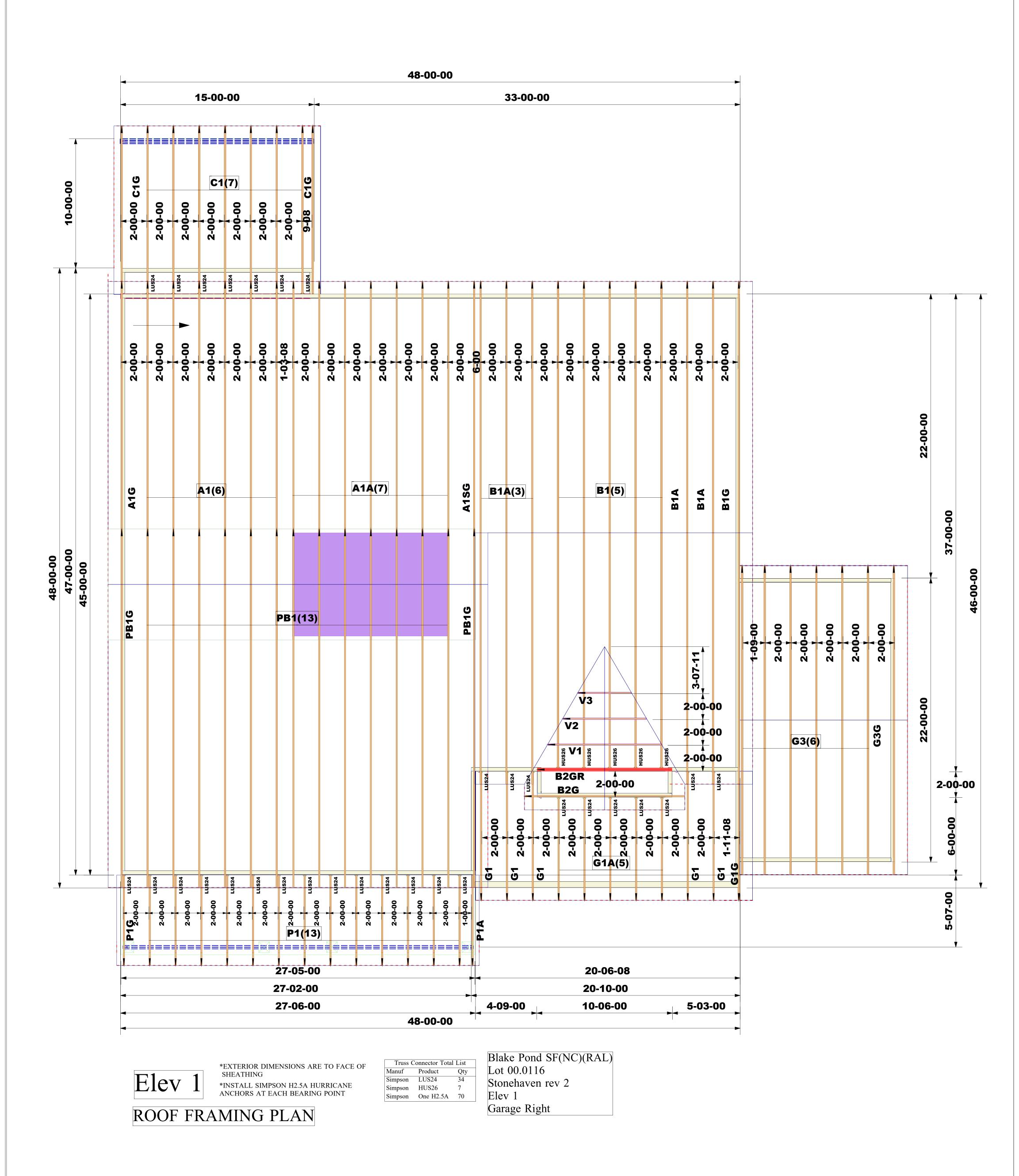
eal: 4/10/25 MULHERN+KUL

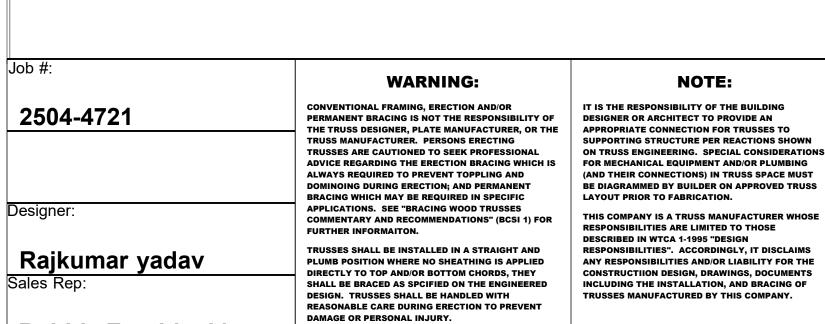
MESIDENTIAL STRUCTURAL ENGINEERI

M&K project number: 126-23061

JTR drawn by: JAD issue date: 04-08-2‡

BLAKE POND COMMUNITY LOT 116 - STONEHAVEN 1 RALEIGH, NC

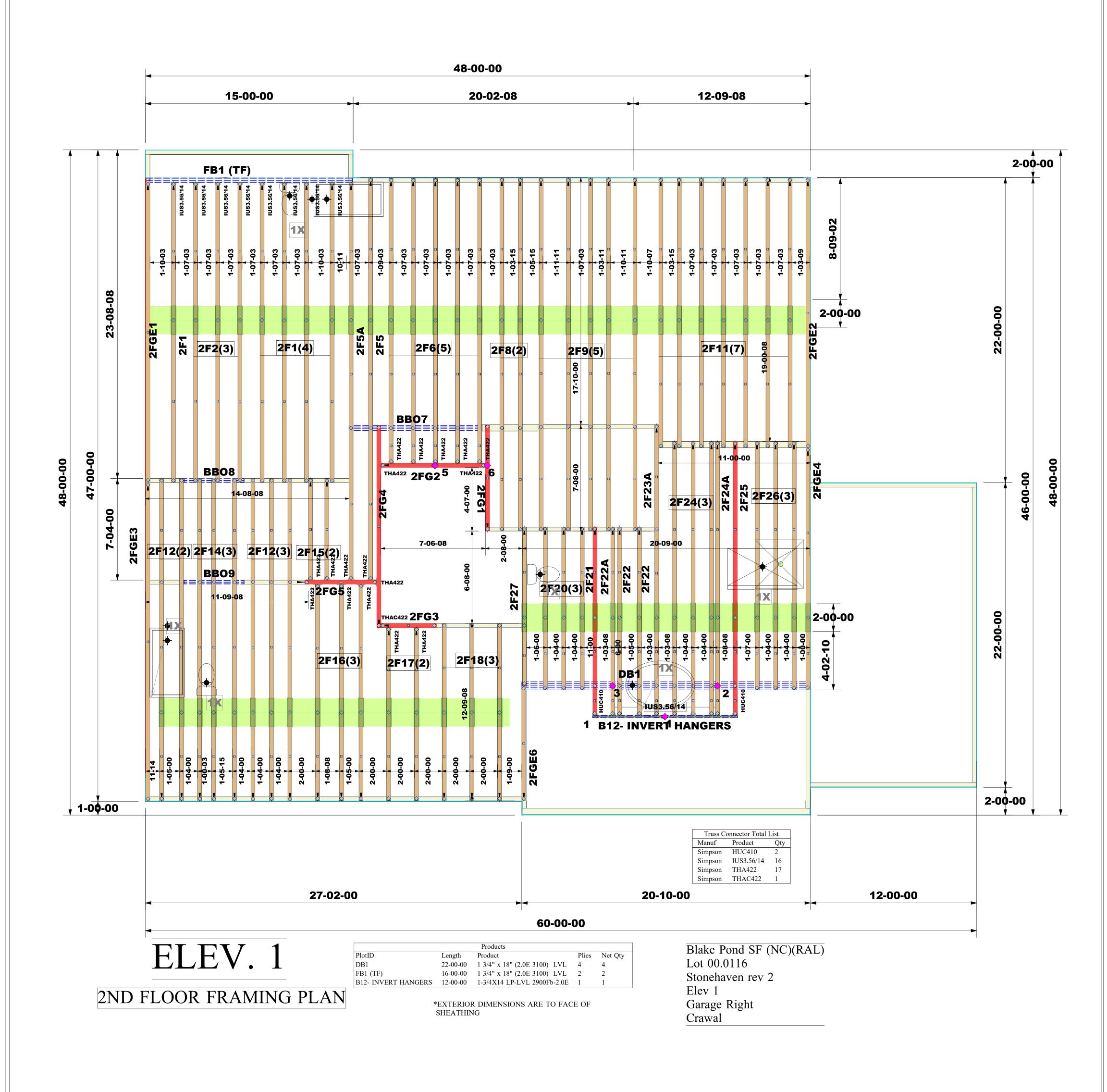




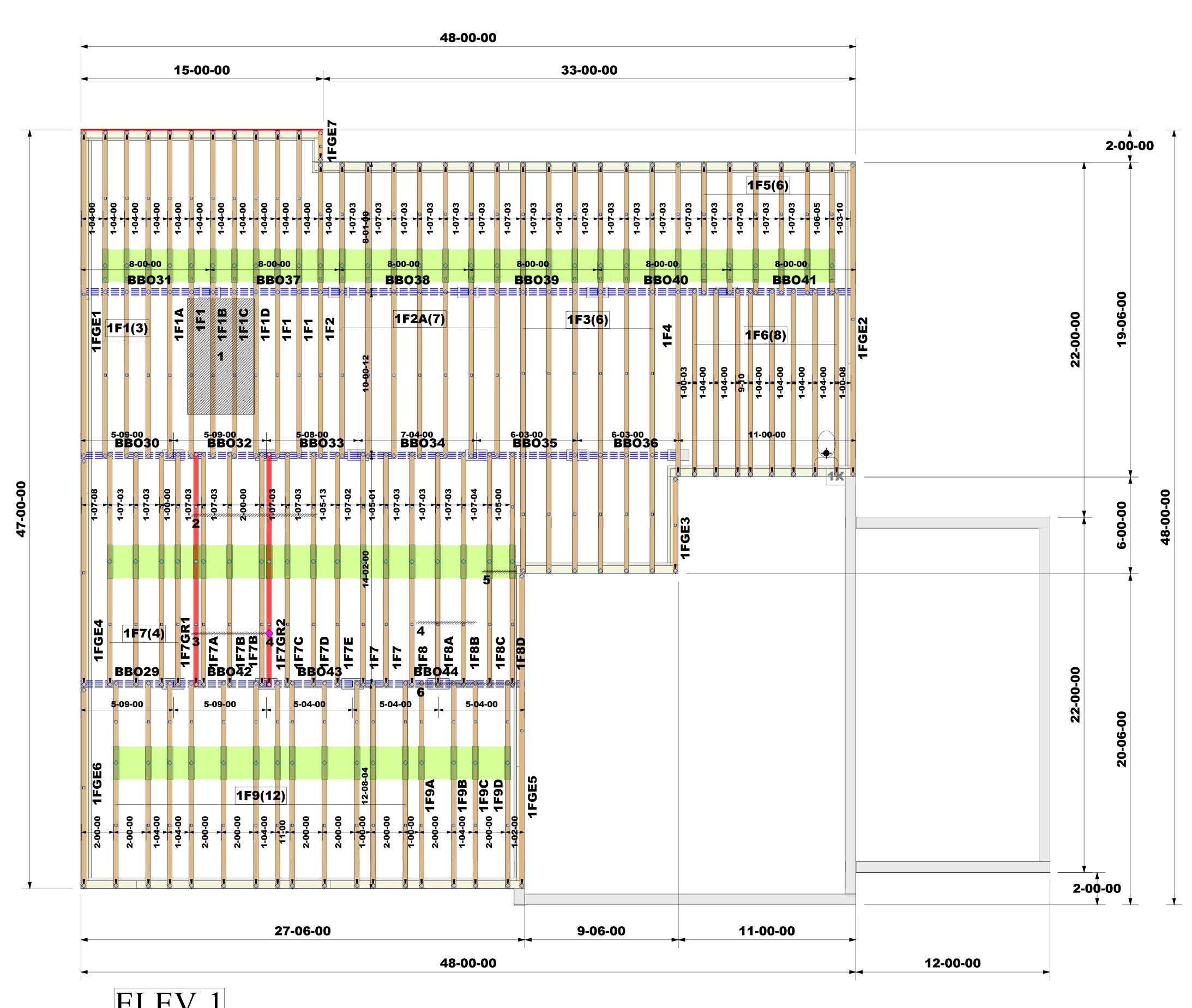
Robbie Zarobinski

	Customer: DRB Raleigh					
NS S	Job Name: Blake Pond Lot 00.0116 Roof	7				
E	Lot #: Lot 00.0116					
	Model Name: Stonehaven					









ELEV 1

1ST FLOOR FRAMING

Job #: 2504-4720	WARNING: CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE	NOTE: IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO	Customer: DRB Raleigh	
	TRUSS MANUFACTURER. PERFONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	Job Name: Blake Pond Lot 00.0116 OWF	Third-Party Quality Assurance Licensee TPI Plant W974
Designer: Rajkumr yadav	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON. TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS. THEY	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS	Lot #: Lot 00.0116	Structural, LLC 201 Poplar Avenue Thurmont, MD 21788
Sales Rep: Robbie Zarobinski	SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.	INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.	Model Name: Stonehaven	Phone: 301-271-7591