

Trenco 818 Soundside Rd Edenton, NC 27932

Re: J0325-1350 Lot 3 Mabry Ridge

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I71968847 thru I71968875

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



March 13,2025

Galinski, John

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	A1-GE	COMMON SUPPORTED GAB	1	1	Job Reference (optional)	171968847

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:48 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



30	-3-0	

Scale = 1:67 Plate Offsets (X, Y): [9:0-2-4.Edge], [19:0-2-4.Edge]

	, .). [,	[
Loading TCLL (roof) TCDL BCLL BCDL		(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	4	CSI TC BC WB Matrix-S	0.05 0.02 0.10	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.01	(lo 2	c) l/defl - n/a - n/a 26 n/a	L/d 999 999 n/a	PLATES MT20 Weight: 329 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD WEBS	2x6 SP No 2x6 SP No 2x4 SP No SPF No.2(Structural 6-0-0 oc p Rigid ceilin bracing. T-Brace: Fasten (2) of web with 2	 b.1 c.2 *Excep (flat) wood sheaurlins. ng directly X) T and I h 10d (0.1 in minimum 	t* 0-0,0-0,0-0,0-0,0-0: athing directly applied applied or 10-0-0 oc 2x4 SPF No.2 - 14-39 13-40, 12-41, 15-37, 16-36 braces to narrow edg 31*x3") nails, 6in p ord distance	2x4 For FORCES TOP CHOR	I RD	Max Grav 2=: 28: 30: 32: 34: 36: 39: 41: 43: 45: (Ib) - Maximur Tension 1-2=0/11, 2-3 4-5=-217/194	209 (LC 20), 2 =158 (LC 20), =119 (LC 20), =118 (LC 20), =118 (LC 20), =118 (LC 20), =118 (LC 20), =121 (LC 20), =183 (LC 13), =120 (LC 19), =118 (LC 19), =118 (LC 19), =169 (LC 19) m Compression =-344/259, 3- 5-6=-189/17	Re=162 (LC 29=111 (LC 31=117 (LC 33=117 (LC 33=117 (LC 37=103 (LC 40=120 (LC 40=120 (LC 40=118 (LC 44=118 (LC 46=117 (LC 48=112 (LC on/Maximum 4=-250/2122 6 6-7=-174	22), 2 20), 2 20), 2 20), 2 20), 2 10, 2 19), 2 19, 2	NOT 1) (2) (1)	ES Jnbalance his design Wind: ASC /asd=103 Cat. II; Ex WWFRS (i 0-9-7 to 3	14-33 11-42 7-453 4-483 16-33 20-33 23-30 ed roof EE 7-16 mph; To C; Er envelop -7-6, E 9-6-5	9=-168/69, 13-40= 2=-91/80, 10-43=- =-91/74, 6-46=-91 =-91/76, 3-49=-12 5=-95/82, 17-35=- 3=-91/74, 21-32=- 0=-91/73, 24-29=- live loads have be \$; Vult=130mph (3 CDL=6.0psf; BCE closed; Gable Ro be) exterior zone a xterior(2N) 3-7-61 Evator(2N) 19-6	93/25, 12-41=-93/81, 91/75, 8-44=-91/74, /74, 5-47=-90/73, 7/132, 15-37=-76/0, 93/82, 18-34=-91/75, 91/74, 22-31=-91/74, 89/75, 25-28=-117/123 en considered for i-second gust) b1=6.0psf; h=15ft; of; Common Truss; and C-C Corner(3E) to 15-1-8, Commer(3R) -5 to 31-0-7 zener(3R)
REACTIONS	o.c.,with 3 Brace mi (size) Max Horiz Max Uplift	in minimu ust cover § 2=30-3-0, 32=30-3-0 32=30-3-0 33=30-3-0 42=30-3-0 42=30-3-0 42=30-3-0 42=30-3-0 42=30-3-0 2=-103 (L0 28=-112 (30=-58 (L 32=-58 (L 41=-65 (L 41=-65 (L 41=-65 (L 41=-55 (L 41=-58 (L 41=-58 (L 41=-58 (L 41=-58 (L 41=-58 (L 41=-58 (L 41=-58 (L 41=-58 (L) 41=-58 (L) 4	m end distance. 90% of web length. 26=30-3-0, 28=30-3- 9, 30=30-3-0, 31=30-3 9, 33=30-3-0, 34=30-3 9, 40=30-3-0, 37=30-3 9, 40=30-3-0, 41=30-3 9, 43=30-3-0, 44=30-3 9, 44=30-3-0, 47=30-3 9, 49=30-3-0 C 11), 44=30-3 0, 46=30-3-0, 47=30-3 1, 49=30-3-0 C 11), 29=-57 (LC 12 C 13), 29=-57 (LC 13 C 13), 31=-58 (LC 13 C 13), 35=-66 (LC 13 C 13), 40=-9 (LC 9), C 12), 42=-64 (LC 12 C 12), 44=-58 (LC 12 C 12), 48=-58 (LC 12) C 12), 48=	0, -0, -0, -0, -0, BOT CHOR 3),),),),),),	RD	$\begin{array}{l} 4-5=-217/194, 5-6=-189/176, 6-7=-174/163,\\ 7-8=-160/160, 8-10=-146/180,\\ 10-11=-132/209, 11-12=-138/240,\\ 12-13=-166/272, 13-14=-169/271,\\ 14-15=-169/271, 15-16=-166/266,\\ 16-17=-138/214, 17-18=-108/164,\\ 18-20=-80/116, 20-21=-64/78, 21-22=-74/56,\\ 22-23=-85/67, 23-24=-125/85,\\ 24-25=-176/103, 25-26=-264/141,\\ 26-27=0/11\\ 2-49=-115/243, 48-49=-115/243,\\ 45-46=-115/243, 46-47=-115/243,\\ 43-44=-115/243, 40-41=-115/243,\\ 41-42=-115/243, 40-41=-115/243,\\ 39-40=-115/243, 35-36=-115/243,\\ 34-35=-115/243, 33-34=-115/243,\\ 34-35=-115/243, 31-32=-115/243,\\ 30-31=-115/243, 31-32=-115/243,\\ 30-31=-115/243, 26-28=-115/243,\\ 28-29=-115/243, 26-28=-115/243\\ \end{array}$			74/56,	 -0-9-7 to 3-7-6, Exterior(2N) 3-7-6 to 15-1-6 15-1-8 to 19-6-5, Exterior(2N) 19-6-5 to 31- C for members and forces & MWFRS for re shown; Lumber DOL=1.60 plate grip DOL= 3) Truss designed for wind loads in the plane only. For studs exposed to wind (normal to see Standard Industry Gable End Details a or consult qualified building designer as pe 4) All plates are 2x4 MT20 unless otherwise in SEAL 28677 			-5 to 31-0-7 zone,C- RS for reactions p DOL=1.60 e plane of the truss ormal to the face), Details as applicable, er as per ANSI/TPI 1. etwise indicated.	

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Continued on page 2 WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and RCSI Building Component Safety Information available from the Structural Building Component Association (www.stearonponent.scom) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



J0325-1350 A1-GE COMMON SUPPORTED GAB 1 1 Job Reference (optional)	Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
	J0325-1350	A1-GE	COMMON SUPPORTED GAB	1	1	Job Reference (optional)	171968847

- 5) Gable requires continuous bottom chord bearing.
- Gable studs spaced at 1-4-0 oc. 6)
- This truss has been designed for a 10.0 psf bottom 7)
- chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 30.0psf 8)
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 9) All bearings are assumed to be SP No.1 crushing capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 26, 103 lb uplift at joint 2, 9 lb uplift at joint 40, 65 lb uplift at joint 41, 64 lb uplift at joint 42, 59 lb uplift at joint 43, 58 lb uplift at joint 44, 58 lb uplift at joint 45, 58 lb uplift at joint 46, 58 lb uplift at joint 47, 58 lb uplift at joint 48, 122 lb uplift at joint 49, 66 lb uplift at joint 36, 66 lb uplift at joint 35, 59 lb uplift at joint 34, 58 lb uplift at joint 33, 58 lb uplift at joint 32, 58 lb uplift at joint 31, 58 lb uplift at joint 30, 57 lb uplift at joint 29 and 112 lb uplift at joint 28.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	A2	COMMON	7	1	Job Reference (optional)	171968848

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:49 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

31-2-0 0-11-0 7-8-5 22-6-11 30-3-0 15-1-8 7-8-5 7-5-3 7-5-3 7-8-5 5x5= 5 12 8 4x6 🖌 4x6、 2x4 13 14 6 4 2x4 / 3 7 10-11-0 11-0-13 8 0-10-0 9 L. Ø 15 16 12 17 11 18 10 19 20 4x4= 4x4= 3x4= 4x6= 3x4= 10-2-1 20-0-15 30-3-0 10-2-1 9-10-14 10-2-1

Scale = 1:68.2												
Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.25	Vert(LL)	-0.12	10-12	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.18	2-12	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.37	Horz(CT)	0.04	8	n/a	n/a		
BCDL	10.0	Code	IRC2021/TPI2014	Matrix-S		Wind(LL)	0.04	2-12	>999	240	Weight: 211 lb	FT = 20%
		•		•								

LUMBER		
TOP CHORD	2x6 SP N	0.1
BOT CHORD	2x6 SP N	0.1
WEBS	2x4 SP N	0.2
BRACING		
TOP CHORD	Structura	I wood sheathing directly applied or
	5-2-12 oc	purlins.
BOT CHORD	Rigid ceil	ing directly applied or 10-0-0 oc
	bracing.	
REACTIONS	(size)	2=0-3-8, 8=0-3-8
	Max Horiz	2=-254 (LC 10)
	Max Uplift	2=-75 (LC 12), 8=-75 (LC 13)
	Max Grav	2=1563 (LC 19), 8=1563 (LC 20)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	·
TOP CHORD	1-2=0/11,	2-3=-2071/348, 3-5=-1941/443,
	5-7=-194	1/443, 7-8=-2072/348, 8-9=0/11
BOT CHORD	2-12=-174	4/1793, 10-12=0/1181,
	8-10=-15	8/1620
WEBS	5-10=-16	1/1011, 7-10=-411/295,
	5-12=-16	1/1011. 3-12=-411/295

NOTES

- Unbalanced roof live loads have been considered for 1) this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C 2) Exterior(2E) -0-9-7 to 3-7-6, Interior (1) 3-7-6 to 15-1-8, Exterior(2R) 15-1-8 to 19-6-5, Interior (1) 19-6-5 to 31-0-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf 4) on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) All bearings are assumed to be SP No.1 crushing

capacity of 565 psi.

Provide mechanical connection (by others) of truss to 6) bearing plate capable of withstanding 75 lb uplift at joint 2 and 75 lb uplift at joint 8.

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	A3	COMMON	4	1	Job Reference (optional)	171968849

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Plate Offsets (X, Y): [9:0-3-4,0-1-0]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	-0.12	10-12	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.18	2-12	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.37	Horz(CT)	0.04	9	n/a	n/a		
BCDL	10.0	Code	IRC2021/TPI2014	Matrix-S		Wind(LL)	0.03	2-12	>999	240	Weight: 214 lb	FT = 20%

LUMBER

NOTES

2)

3)

this design.

DOL=1.60

Scale = 1:68.2

TOP CHORD	2x6 SP N	0.1
BOT CHORD	2x6 SP N	0.1
WEBS	2x4 SP N	0.2
SLIDER	Right 2x4	SP No.2 4-4-7
BRACING		
TOP CHORD	Structura	I wood sheathing directly applied or
	5-2-14 oc	purlins.
BOT CHORD	Rigid ceili	ing directly applied or 10-0-0 oc
	bracing.	
REACTIONS	(size)	2=0-3-8, 9= Mechanical
	Max Horiz	2=253 (LC 9)
	Max Uplift	2=-75 (LC 12), 9=-61 (LC 13)
	Max Grav	2=1557 (LC 19), 9=1504 (LC 20)
FORCES	(lb) - Max	imum Compression/Maximum
	Tension	·
TOP CHORD	1-2=0/11,	2-3=-2061/347, 3-5=-1930/442,
	5-7=-188	6/436, 7-9=-2024/348
BOT CHORD	2-12=-18	5/1781, 10-12=0/1170,
	9-10=-164	4/1566
WEBS	$5-10 = -15^{\circ}$	1/949 7-10=-370/289

5-12=-161/1012, 3-12=-407/294

1) Unbalanced roof live loads have been considered for

Wind: ASCE 7-16; Vult=130mph (3-second gust)

Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C

Exterior(2E) -0-9-7 to 3-7-6, Interior (1) 3-7-6 to 15-1-8, Exterior(2R) 15-1-8 to 19-6-5, Interior (1) 19-6-5 to

29-11-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

chord live load nonconcurrent with any other live loads.

This truss has been designed for a 10.0 psf bottom

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Bearings are assumed to be: Joint 2 SP No.1 crushing capacity of 565 psi.
- 6) Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 75 lb uplift at joint 2 and 61 lb uplift at joint 9.
- LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	A3A	COMMON	9	1	Job Reference (optional)	171968850

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Scale = 1:74.7

TOP CHORD

BOT CHORD

FORCES

TOP CHORD

BOT CHORD

this design.

DOL=1.60

WFBS

NOTES

2)

3)

REACTIONS (size)

Plate Offsets	(Х,	Y):	[9:0-3-4,0-1-0]
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Structural wood sheathing directly applied or

2=0-3-8, 9= Mechanical

Rigid ceiling directly applied or 10-0-0 oc

Max Uplift 2=-75 (LC 12), 9=-61 (LC 13) Max Grav 2=1550 (LC 19), 9=1497 (LC 20)

(lb) - Maximum Compression/Maximum

2-12=-185/1771, 10-12=0/1163,

5-10=-151/941, 7-10=-371/289,

1) Unbalanced roof live loads have been considered for

Wind: ASCE 7-16; Vult=130mph (3-second gust)

Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-7 to 3-7-6, Interior (1) 3-7-6 to 15-1-8,

Exterior(2R) 15-1-8 to 19-6-5, Interior (1) 19-6-5 to

This truss has been designed for a 10.0 psf bottom

29-11-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip

chord live load nonconcurrent with any other live loads.

5-12=-161/1005, 3-12=-408/294

1-2=0/11, 2-3=-2048/347, 3-5=-1917/442, 5-7=-1874/436, 7-9=-2012/348

5-2-15 oc purlins.

Max Horiz 2=253 (LC 9)

bracing.

Tension

9-10=-164/1556

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.25	Vert(LL)	-0.12	10-12	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.52	Vert(CT)	-0.18	2-12	>999	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.37	Horz(CT)	0.04	9	n/a	n/a		
BCDL	10.0	Code	IRC2021/TPI2014	Matrix-S		Wind(LL)	0.03	2-12	>999	240	Weight: 233 lb	FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER BRACING	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 Right 2x4 SP No.2	- 4-4-7	 4) * This trus on the bott 3-06-00 ta chord and 5) Bearings a capacity or 	s has been desig om chord in all a Il by 2-00-00 wid any other memb re assumed to b 565 psi.	gned for a liv areas where e will fit betw bers, with BC be: Joint 2 SF	e load of 30. a rectangle veen the bott DL = 10.0ps P No.1 crush	Opsf tom if. ing					

6)

Refer to girder(s) for truss to truss connections. Provide mechanical connection (by others) of truss to 7) bearing plate capable of withstanding 75 lb uplift at joint 2 and 61 lb uplift at joint 9.

LOAD CASE(S) Standard



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818 Soundside Road

Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	A4-GE	COMMON SUPPORTED GAB	1	1	Job Reference (optional)	171968851

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:67 Plate Offsets (X, Y): [9:0-2-4,Edge], [19:0-2-4,Edge]

,		. ,												
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Mate	ix-S	0.09 0.01 0.20	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 27	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 325 lb	GRIP 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD WEBS	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 *Excep SPF No.2(flat) Structural wood shea 6-0-0 oc purlins. Rigid ceiling directly bracing. T-Brace: Fasten (2X) T and I of web with 10d (0.1 o.c.,with 3in minimur Brace must cover 9	t* 0-0,0-0,0-0,0-0,0-0 athing directly applied applied or 10-0-0 oc 2x4 SPF No.2 - 14-3 13-40, 12-41, 15-37, 16-36 braces to narrow edg 31*x3") nails, 6in m end distance. 10% of web length.	:2x4 1 or 9,	Max Up	ift 2=-224 (L 29=-21 (L 31=-59 (L 33=-57 (L 35=-73 (L 39=-83 (L 41=-66 (L 43=-59 (L 43=-59 (L 45=-58 (L 47=-58 (L 47=-58 (L 28=218 (l 30=120 (l 32=118 (l 34=118 (l 34=118 (l 39=358 (l	.C 8), 2 .C 13), .C 13), .C 13), .C 13), .C 12), .C 12), .C 12), .C 12), .C 12), .C 12), .C 12), .C 20), .C 20, .C	8=-122 (LC 1 30=-59 (LC 1 34=-58 (LC 1 34=-59 (LC 1 36=-71 (LC 1 40=-21 (LC 9 42=-66 (LC 1 44=-58 (LC 1 44=-58 (LC 1 44=-58 (LC 1 44=-57 (LC 3), 29=65 (LC 2 31=119 (LC 33=117 (LC 35=122 (LC 37=121 (LC 40=147 (LC	(3), (3), (3), (3), (3), (3), (2), (2), (2), (2), (2), (20), (20), (20), (20), (20), (20), (22), (22), (22),	BOT CF WEBS NOTES 1) Unt this	IORD palanced design.	2-49=1 45-46: 41-42: 36-37: 32-33: 28-29: 14-39: 12-41: 8-44=: 5-47=: 15-37: 18-34: 22-31: 25-28: d roof li	V0.98.4-95 V0.98.4-95 V0.94.4-55 V0.94.4-55 V0.90 V0.90 V0.90 V0.90 V0.90 V0.90 V0.97-28-0/0 V0.97-28-0/0 V0.97-28-0/0 V0.97.28-0/0 V0.97.28-0 V1.42 V0.97.2 V0.16-36-9 V0.91/75, 20-33= -92/75, 23-30= -185/235 V0.045 have be	7-48=0/0, 46-47= 43-44=0/0, 42-3 39-40=0/0, 37-35 34-35=0/0, 33-34 30-31=0/0, 29-30)=-120/41, 88/83, 10-43=-91 /74, 6-46=-91/74, /75, 3-49=-141/12 6/87, 17-35=-95/8 91/73, 21-32=-91 93/75, 24-29=-88	=0/0, 3=0/0,)==0/0, 1=0/0,)=0/0, /75, , 22, 89, /74, 3/36,
REACTIONS	(size) 2=29-11-8 28=29-11- 30=29-11- 32=29-11- 34=29-11- 39=29-11- 41=29-11- 43=29-11- 45=29-11- 45=29-11- 45=29-11- 49=29-11- Max Horiz 2=-314 (Li	3, 27=29-11-8, -8, 29=29-11-8, -8, 33=29-11-8, -8, 33=29-11-8, -8, 35=29-11-8, -8, 40=29-11-8, -8, 42=29-11-8, -8, 44=29-11-8, -8, 44=29-11-8, -8, 44=29-11-8, -8, 44=29-11-8, -8, 44=29-11-8, -8, 46=29-11-8, -8, 46=29-11-8,-8, 46=29-11-8, -8, 46=29-11-8,-8, 46=29-11-8,-8, 46=29-10,-8, 46=29-10,-8,	FORCES TOP CHORD	(lb) - M Tensic 1-2=0/ 4-5=-3 7-8=-2 10-11= 12-13= 14-15= 16-17= 18-200 21-22= 23-24=	41-117 (l 43=117 (l 43=117 (l 43=118 (l 47=118 (l 47=118 (l 49=184 (l 13,2-3=-383/ 18/342,5-6=- 77/396,8-10- -249/453,11- -268/523,13- -258/475,15- -238/447,17- -179/345,20- -125/251,22- -69/154,24-2	LC 19), LC 19), LC 19), LC 19), LC 19), LC 19), ID pression 400, 3- 305/34 -12=-2; -14=-2; -14=-2; -16=-26 -21=-1; -23=-97 25=-64/	42=115 (LC 44=118 (LC 46=117 (LC 48=114 (LC bn/Maximum 4=-335/349, 0, 6-7=-291/3 24, 38/488, 58/480, 58/480, 58/480, 58/200, 17/393, 52/298, 7/203, 148, 25-26=0	19), 19), 19), 19), 19), 368,		. antitution	and a state of the	SEA 2867	EER. St.	ANNO DE

March 13,2025

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Continued on page 2 WARNING - Verify of

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	A4-GE	COMMON SUPPORTED GAB	1	1	Job Reference (optional)	171968851

Comtech. Inc. Favetteville, NC - 28314.

- Wind: ASCE 7-16: Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-9-7 to 3-7-6, Exterior(2N) 3-7-6 to 15-1-8, Corner(3R) 15-1-8 to 19-6-5, Exterior(2N) 19-6-5 to 29-11-8 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated. Gable requires continuous bottom chord bearing.
- 5) 6) Gable studs spaced at 1-4-0 oc.
- 7)
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf 8) on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- All bearings are assumed to be SP No.1 crushing 9) capacity of 565 psi.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 224 lb uplift at joint 2, 83 lb uplift at joint 39, 21 lb uplift at joint 40, 66 lb uplift at joint 41, 66 lb uplift at joint 42, 59 lb uplift at joint 43, 58 lb uplift at joint 44, 58 lb uplift at joint 45, 58 lb uplift at joint 46, 58 lb uplift at joint 47, 57 lb uplift at joint 48, 111 lb uplift at joint 49, 71 lb uplift at joint 36, 73 lb uplift at joint 35, 59 lb uplift at joint 34, 57 lb uplift at joint 33, 58 lb uplift at joint 32, 59 lb uplift at joint 31, 59 lb uplift at joint 30, 21 lb uplift at joint 29 and 122 lb uplift at joint 28.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

LOAD CASE(S) Standard

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	B1-GE	GABLE	1	1	Job Reference (optional)	171968852

Scale = 1:89.4

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Plate Offsets (X, Y): [7:0-6-10,Edge], [15:0-6-10,Edge], [26:0-7-12,0-1-8], [28:0-7-12,0-1-8]

Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-S	0.58 0.60 0.34	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.18 -0.31 0.01 0.11	(loc) 26-28 26-28 20 28-29	l/defl >999 >901 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 262 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD JOINTS REACTIONS FORCES TOP CHORD	2x6 SP No.1 *Except 2400F 2.0E 2x10 SP No.1 2x6 SP No.1 2x4 SP No.2 Structural wood sheat 5-8-3 oc purlins. Rigid ceiling directly bracing. 1 Brace at Jt(s): 33 (size) 2=0-3-8, 2 Max Horiz 2=382 (LC Max Grav 2=1531 (L (lb) - Maximum Comp Tension 1-2=0/35, 2-3=-1911) 4-5=-1760/0, 5-6=-19 7-8=-1155/196, 8-10 11-12=0/452, 12-14= 14-15=-1155/196, 15 16-17=-1925/32, 17- 18-19=-1836/0 19-21	* 1-9,21-13:2x6 SP athing directly applied applied or 10-0-0 oc 0=0-3-8 :11) C 20), 20=1531 (LC . pression/Maximum /0, 3-4=-1836/0, 026/32, 6-7=-2448/90 =-119/114, 10-11=0/4 :-119/114, 10-11=0/4 :-119/114, 10-11=0/4 :-119/114, 10-11=0/4 :-119/100, 20-21=0/2	2) d or 3) 21) 6) 7) 452, 8) 35 0)	Wind: ASCE Vasd=103mp Cat. II; Exp C MWFRS (env- -0-9-6 to 3-7- (3R) 11-11-8 zone;C-C for reactions sho DOL=1.60 Truss design only. For stu see Standard or consult qu All plates are Gable studs : This truss ha on the bottom 3-06-00 tall b chord and ar Ceiling dead 8-34, 33-34, member(s).7 Bottom obser	7-16; Vult=130mpl h; TCDL=6.0psf; E c; Enclosed; Gable velope) exterior zon 7, Exterior(2N) 3-7 to 16-4-5, Exterior members and forc own; Lumber DOL= ed for wind loads in ds exposed to wind I ndustry Gable Er alified building des 2x4 MT20 unless spaced at 1-4-0 oc s been designed for dn onconcurrent w as been designed n chord in all areas y 2-00-00 wide wil y other members. load (10.0 psf) on 33-35, 14-35; Wal -28, 15-26	h (3-sec 3CDL=6 Roof; C ne and 0 7-7 to 11 (2N) 16 es & M -1.60 pl: -1.60 pl	ond gust) .0psf; h=15ft common Trus C-C Corner(3 -11-8, Corner -4-5 to 24-8-6 WFRS for ate grip ane of the tru at to the face ils as applica is per ANSI/TI se indicated. 0 psf bottom other live load e load of 30.0 a rectangle reen the bottor r(s). 7-8, 14 bod (5.0psf) of dditional bott	; ss; E) rr 6 ss ble, Pl 1. Opsf om 15, on				WH CA	Boy
BOT CHORD WEBS NOTES 1) Unbalance this design	2-32=0/1299, 31-32= 29-30=0/1299, 28-29 25-26=0/1295, 24-25 22-23=0/1299, 20-22 8-34=-1458/284, 33- 33-35=-1453/284, 14 7-28=0/1723, 15-26= 10-34=-17/523, 6-29: 4-31=-34/109, 3-32=: 16-25=-812/71, 17-2: 18-23=-34/109, 19-2: ed roof live loads have 1	0/1299, 30-31=0/129 =0/1295, 26-28=0/12 =0/1295, 26-28=0/12 =0/1298, 23-24=0/12 =0/1298, 23-24=0/12 =0/1284,	299, 295, 10, 299, 11, (10, (96, 22,	All bearings a capacity of 5 Attic room ch	and (10.0 psf) appl are assumed to be 65 psi. ecked for L/360 de Standard	of and a lied only SP No.	to room. 26- 1 crushing	28			in V States	SEA 2867	ERSK III

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	B2	ATTIC	4	1	Job Reference (optional)	171968853

Scale = 1:81.5

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Plate Offsets (X, Y):	[5:0-3-0,Edge], [6:0-3-0,Edge], [7:0-3-0,Edge],	[12:0-7-0,0-1-8],	[14:0-7-0,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	1.00	Vert(LL)	-0.28	12-14	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-0.52	12-14	>549	240		
BCLL	0.0*	Rep Stress Incr	YES	WB	0.12	Horz(CT)	0.01	10	n/a	n/a		
BCDL	10.0	Code	IRC2021/TPI2014	Matrix-S		Wind(LL)	0.12	12-14	>999	240	Weight: 222 lb	FT = 20%
LUMBER 6) Bottom chord live load (40.0 psf) and additional bottom												

TOP CHORD 2x6 SP No.1 BOT CHORD 2x10 SP No.1 2x6 SP No.1 WEBS BRACING TOP CHORD Structural wood sheathing directly applied. BOT CHORD Rigid ceiling directly applied or 9-6-8 oc bracing. REACTIONS (size) 2=0-3-8, 10=0-3-8 Max Horiz 2=-305 (LC 10) Max Grav 2=1536 (LC 20), 10=1536 (LC 21) FORCES (Ib) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/35, 2-3=-2011/0, 3-4=-1127/135, 4-6=0/291, 6-8=0/292, 8-9=-1126/135, 9-10=-2011/0, 10-11=0/35

- BOT CHORD 2-14=0/1237, 12-14=0/1237, 10-12=0/1237 WEBS 4-8=-1508/158, 3-14=0/900, 9-12=0/900 NOTES
- 1) Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-6 to 3-7-7, Interior (1) 3-7-7 to 11-11-8, Exterior(2R) 11-11-8 to 16-4-5, Interior (1) 16-4-5 to 24-8-6 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; 5) Wall dead load (5.0psf) on member(s).3-14, 9-12

- chord dead load (10.0 psf) applied only to room. 12-14
- All bearings are assumed to be SP No.1 crushing 7)
- capacity of 565 psi.
- 8) Attic room checked for L/360 deflection.
- LOAD CASE(S) Standard



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Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	B3	ATTIC	6	1	Job Reference (optional)	171968854

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Plate Offsets (X, Y): [4:0-3-0,Edge], [5:0-3-0,Edge], [6:0-3-0,Edge], [11:0-7-0,0-1-8]

Loading (ps	sf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof) 20.	0.0	Plate Grip DOL	1.15	TC	1.00	Vert(LL)	-0.29	11-13	>990	360	MT20	244/190
TCDL 10.	0.0	Lumber DOL	1.15	BC	0.79	Vert(CT)	-0.53	11-13	>539	240		
BCLL 0.	0.0*	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.01	9	n/a	n/a		
BCDL 10.	0.0	Code	IRC2021/TPI2014	Matrix-S		Wind(LL)	0.12	11-13	>999	240	Weight: 219 lb	FT = 20%

LUMBER	
TOP CHORD	2x6 SP No.1
BOT CHORD	2x10 SP No.1
WEBS	2x6 SP No.1
BRACING	
TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied or 9-3-11 oc
	bracing.
REACTIONS	(size) 1=0-3-8, 9=0-3-8
	Max Horiz 1=-299 (LC 8)
	Max Grav 1=1486 (LC 21), 9=1537 (LC 21)
FORCES	(Ib) - Maximum Compression/Maximum
	Tension
TOP CHORD	1-2=-1981/0, 2-3=-1130/138, 3-5=0/293,
	5-7=0/300, 7-8=-1123/134, 8-9=-2013/0,
	9-10=0/35
BOT CHORD	1-13=0/1237, 11-13=0/1237, 9-11=0/1237

NOTES

Scale = 1:81.5

- 1) Unbalanced roof live loads have been considered for this design
- Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-1-12 to 4-6-9, Interior (1) 4-6-9 to 11-11-8, Exterior(2R) 11-11-8 to 16-4-5, Interior (1) 16-4-5 to 24-8-6 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom 3) chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Ceiling dead load (10.0 psf) on member(s). 2-3, 7-8, 3-7; 5) Wall dead load (5.0psf) on member(s).2-13, 8-11

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

All bearings are assumed to be SP No.1 crushing 7)

capacity of 565 psi.

Attic room checked for L/360 deflection. 8)

LOAD CASE(S) Standard



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	C1-GE	KINGPOST	1	1	Job Reference (optional)	171968855

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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Scale = 1:73.7	
Plate Offsets (X, Y):	[13:0-5-0,0-4-8], [14:1-2-14,0-2-0]

		-											
Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202 ²	1/TPI2014	CSI TC BC WB Matrix-S	0.15 0.17 0.10	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.01 -0.02 0.01 0.01	(loc) 2-22 2-22 14 2-22	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 200 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS OTHERS SLIDER BRACING TOP CHORD BOT CHORD JOINTS REACTIONS	2x6 SP No.1 2x6 SP No.1 2x6 SP No.1 *Excep 2x4 SP No.2 Left 2x4 SP No.2 No.2 1-8-7 Structural wood she 6-0-0 oc purlins. Rigid ceiling directly bracing. 1 Brace at Jt(s): 23 (size) 2=0-3-8,	t* 4-22:2x4 SP No.2 4-1-15, Right 2x4 SP athing directly applie applied or 10-0-0 oc 14=6-8-0, 16=6-8-0,	WI NC 1) d or 2)	EBS 4 2 2 5 1 DTES Unbalanced 1 this design. Wind: ASCE Vasd=103mp Cat. II; Exp C MWFRS (env -0-9-10 to 3-7 9-1-12 to 13-2	-24=-543/307, 23-2 :3-25=-532/275, 25 :0-26=-562/319, 4-2 :-24=-29/17, 7-25=- :-19=-156/147, 10-1 1-17=-105/137, 13 roof live loads have 7-16; Vult=130mph h; TCDL=6.0psf; B ; Enclosed; Gable (elope) exterior zon r-3, Interior (1) 3-7- 6.9 Interior (1) 3-7-	24=-53 -26=-5: 22=0/44 -30/440, 18=-83/ -16=-1: been of CDL=6 Roof; C ie and of 3 to 9- 6-9 to 1	7/287, 36/289, 42, 6-23=-60/ 8-26=-89/88, 100, 39/215 considered fo cond gust) .0psf; h=15ft common Trus C-C Exterior(1-12, Exterior	/54, , ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ; ;					
	INS (size) 2=0-3-8, 14=6-8-0, 16=6-8-0, 17=6-8-0, 20=6-8-0, 20=6-8-0 9-112 to 13-6-9, Interior (1) 03-6-9 to 19-1-2 zone;C-C Max Horiz 2=-298 (LC 10) 9-112 to 13-6-9, Interior (1) 03-6-9 to 19-1-2 zone;C-C Max Horiz 2=-298 (LC 10) 7 Max Uplift 2=-37 (LC 12), 14=-71 (LC 11), 16=-211 (LC 13), 19=-272 (LC 18), 20=-81 (LC 12) 3 Max Grav 2=696 (LC 19), 14=404 (LC 13), 19=-138 (LC 20), 18=-160 (LC 20), 17=138 (LC 20), 18=160 (LC 20), 19=-1 (LC 1), 4 Max Grav 2=696 (LC 20), 19=-1 (LC 1), 18=-160 (LC 20), 19=-1 (LC 1), 18=160 (LC 20), 19=-1 (LC 1), 4									ROUT			
FORCES	(lb) - Maximum Com Tension	pression/Maximum	7)	* This truss h on the bottom	as been designed f n chord in all areas	for a liv where	e load of 30.0 a rectangle	0psf			ÎX	O' ESS	1. A
TOP CHORD BOT CHORD	1-2=0/10, 2-4=-669/ 5-6=-157/122, 6-7=- 8-9=-176/6, 9-10=-1 11-13=-349/126, 13 14-15=0/10 2-22=-122/542, 20-2 19-20=-143/351, 18 17-18=-142/350, 16 14-16=-136/340	47, 4-5=-218/119, 127/80, 7-8=-164/43, 89/43, 10-11=-247/7 -14=-528/209, 22=-122/542, -19=-142/350, -17=-141/349,	1, 8) 9)	 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 8) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 71 lb uplift at joint 14, 81 lb uplift at joint 20, 37 lb uplift at joint 12, 272 lb uplift at joint 19, 74 lb uplift at joint 18, 121 lb uplift at joint 16. 								7 ER Hum	
			LC	OAD CASE(S)	Standard							L. G.	ALIM

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818 Soundside Road Edenton, NC 27932

March 13,2025

Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	C2	COMMON	1	1	Job Reference (optional)	171968856

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:50 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TF	PI2014	CSI TC BC WB Matrix-S	0.39 0.28 0.10	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.03 -0.07 0.01 0.04	(loc) 6-9 6-9 6 2-9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 143 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS SLIDER	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 Left 2x4 SP No.2 6 6-5-0	i-5-0, Right 2x4 SP N	5) A ca 6) P ba lo.2 6 LOAE	II bearings a apacity of 56 rovide mech earing plate and 31 lb u D CASE(S)	are assumed to be 65 psi. nanical connection capable of withsta plift at joint 2. Standard	SP No. (by oth anding 3	1 crushing ers) of truss to 1 lb uplift at jo	o pint					
BRACING	Structural wood she	athing directly applied	lor										
	6-0-0 oc purlins.	atting directly applied											
BOT CHORD	Rigid ceiling directly	applied or 10-0-0 oc											
REACTIONS	bracing. REACTIONS (size) 2=0-3-8, 6=0-3-8 Max Horiz 2=-238 (LC 10) Max Uplift 2=-31 (LC 12), 6=-31 (LC 13) Max Gray 2=778 (LC 1).												
FORCES	(lb) - Maximum Com	pression/Maximum											
TOP CHORD	1-2=0/9, 2-4=-770/20 6-7=0/9	09, 4-6=-770/209,											
BOT CHORD WEBS	2-9=-2/435, 6-9=-2/4 4-9=0/434	35											
NOTES													
1) Unbalance	ed roof live loads have	been considered for										minin	1111
this design 2) Wind: ASC Vasd=103 Cat. II; Exp Exterior(2E Exterior(2E 19-0-14 zc for reaction	n. CE 7-16; Vult=130mph mph; TCDL=6.0psf; BC oc; Enclosed; MWFR3 E) -0-9-6 to 3-7-7, Inter R) 9-1-12 to 13-6-9, Int ne;C-C for members a ns shown; Lumber DOI	(3-second gust) CDL=6.0psf; h=15ft; S (envelope) and C-C ior (1) 3-7-7 to 9-1-1: erior (1) 13-6-9 to und forces & MWFRS L=1.60 plate grip	2,								New	OR TH CA	ROUNA T

DOL=1.603) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

 * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. SEAL 28677 VGINEER March 13,2025

Page: 1

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSUTP11 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	C3	Common Girder	1	2	Job Reference (optional)	1/1968857

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:62.6

Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP		
TCLL (roof)	20.0	Plate Grip DOL	1.15		тс	0.64	Vert(LL)	-0.06	6-8	>999	360	MT20	244/190		
TCDL	10.0	Lumber DOL	1.15		BC	0.51	Vert(CT)	-0.11	6-8	>999	240				
BCLL	0.0*	Rep Stress Incr	NO		WB	0.79	Horz(CT)	0.02	5	n/a	n/a				
BCDL	10.0	Code	IRC2021	/TPI2014	Matrix-S		Wind(LL)	0.03	6-8	>999	240	Weight: 338 lb	FT = 20%		
			2)	Linkalanaad	a of live loods how		a naida na d fa								
LUMBER			3)	this design	oor live loads hav	e been o	considered to	ſ							
TOP CHORD	2X6 SP N0.1		4)	Wind ASCE	7 16: \/ult_120mn	h (2 coc	ond quet)								
BOICHORD	2X8 SP 2400F 2.0E		4)	Vasd-103mr	h: TCDI -6 0psf: I		One gusi)								
WEDGE	2X4 SP N0.2			Cat II: Exp C	: Enclosed: MWFI	RS (env	elone): Lumb	er							
WEDGE	Dight: 2v9 SP No.1			Call, Exp C, Enclosed, Inverse (inverse), Lumber											
	Right. 2x6 SF NU. 1		5)	This truss ha	s been designed f	ora 10 () psf bottom								
BRACING	0		•) -	chord live loa	d nonconcurrent v	vith anv	other live loa	ds.							
TOP CHORD	Structural wood snea	atning directly applie	a or 6)	* This truss h	as been designed	for a liv	e load of 30.0	Dpsf							
	6-0-0 oc punins.	applied or 10 0 0 as	-,	on the botton	n chord in all areas	s where	a rectangle								
BOT CHORD	kigiu celling ullectly	applied of 10-0-0 00		3-06-00 tall b	y 2-00-00 wide wi	I fit betv	veen the botto	om							
DEACTIONS				chord and an	y other members.										
REACTIONS	(SIZE) I=0-3-8, 3)=U-3-8	7)	All bearings a	are assumed to be	SP 240	0F 2.0E crus	hing							
		20) 0 0) 5 070 (1 0 0)		capacity of 8	05 psi.										
	Max Opliπ 1=-331 (L	C 9), 5=-378 (LC 8)	8)	Provide mech	nanical connection	(by oth	ers) of truss t	0							
		.C 16), 5=7832 (LC 1	15)	bearing plate	capable of withsta	anding 3	31 lb uplift at								
FORCES	(lb) - Maximum Com	pression/Maximum		joint 1 and 37	'8 lb uplift at joint \$	5.									
		5040/004	9)	Hanger(s) or	other connection	device(s) shall be								
TOP CHORD	1-2=-7494/410, 2-3=	-5049/381,		provided suff	icient to support co	oncentra	ited load(s) 1	4//							
	3-4=-5050/301, 4-5=	207/4001		Ib down and	73 lb up at 1-10-4	, 1477 1	b down and 7	3 10							
BOT CHORD	6-8-290/4901, 0-9=	-297/4991, -201/4820		up at 3-10-4	14// ID down and		177 lb dours	ام مر م							
WEBS	3-8437/6724 4-8-	-20174023		72 lb up at 0	10 4 1477 lb dov	-10-4, 1 vn and 7	4// ID dOWN	and					1912 - C		
WEBS	4-6=-116/3340 2-8=	-2140/261		11-10-4 147	-10-4, 1477 10 00v 7 lb down and 73 l	hun at	3 ID UP at 13-10-4 and						1111		
	2-9=-121/3451	2110/201,		1477 lb dowr	and 73 lb un at 1	5-10-4	and 1484 lb					IN TH UA	Roile		
NOTES	20 12/0101			down and 69	Ib un at 18-1-12 (on hotto	m chord The	2			1	A	··· ////		
1) 2 ply truck	e to be connected toget	bor with 10d		design/select	ion of such conne	ction de	vice(s) is the				22	C. FLOO	Phillips		
(0 131"x3	") nails as follows:			responsibility	of others.						\sim		11: 2		
Top chore	is connected as follows:	: 2x6 - 2 rows	LO	AD CASE(S)	Standard					-		TXN JO	- u -		
staggereg	at 0-9-0 oc	210110	1)	Dead + Roc	of Live (balanced):	Lumber	Increase=1	15				SEA	1 1 2		
Bottom ch	nords connected as follo	ows: 2x8 - 2 rows	•)	Plate Increa	se=1.15	Lambol	11010000-11	10,		=		JLA	<u>-</u> : :		
staggered	at 0-7-0 oc.			Uniform Loa	ads (lb/ft)					=		2867	7 : 3		
Web conr	nected as follows: 2x4 -	1 row at 0-9-0 oc.		Vert: 1-3=	=-60, 3-5=-60, 1-5	=-20				-					
2) All loads a	are considered equally	applied to all plies,		Concentrate	ed Loads (lb)						-	N	1. 1. 2		
except if r	except if noted as front (F) or back (B) face in the LOAD Vert: 6=-1172 (F), 5=-1179 (F), 10=-1172 (F).											FRULS			
CASE(S)	CASE(S) section. Ply to ply connections have been 11=-1172 (F), 12=-1172 (F), 13=-1172 (F), 14=-1172											S.S.			
provided t	to distribute only loads	noted as (F) or (B),		(F), 15=-	1172 (F), 16=-117	2 (F)					1	VI C	ALICIN		
unless oth	nerwise indicated.											111 L. GI			
													11.		

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March 13,2025

Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	D1-GE	COMMON SUPPORTED GAB	1	1	Job Reference (optional)	171968858

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:46.1		
Plate Offsets (X, Y): [2:Edge,0-2-3], [12:Edge,0-2-3]		

Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC20	21/TPI2014	CSI TC BC WB Matrix-S	0.04 0.04 0.11	DEFL Vert(LL) Vert(CT) Horz(CT)	in n/a n/a 0.00	(loc) - - 12	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 119 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS SLIDER BRACING TOP CHORD BOT CHORD REACTIONS	2x6 SP No.1 2x6 SP No.2 *Excep Left 2x4 SP No.2 *C No.2 0-8-12 Structural wood shea 6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 2=11-11- (size) 2=11-11- 14=11-11- 16=11-11- 18=11-11- 20=11-11 Max Horiz 2=-202 (LC 14=-187 (L 19=-91 (LC 14=184 (L 16=117 (L 18=126 (L 20=161 (L	t* 2-3,12-11:2x4 SP f)-8-12, Right 2x4 SP athing directly applied applied or 10-0-0 oc), 12=11-11-0, -0, 15=11-11-0, -0, 15=11-11-0, -0, 17=11-11-0, -0, 19=11-11-0, -0 C 10) :8), 12=-25 (LC 9), LC 13), 15=-95 (LC 1 C 12), 20=-166 (LC 1 C 12), 20=-166 (LC 1 C 21), 12=180 (LC 22 C 20), 15=120 (LC 2 C 20), 19=117 (LC 1 C 19)	No.3 1 1 2 1 or 3), 4 (), 5 (), 7 0), 8 (), 7 0), 8 (), 8 ()	 WEBS 7 WIDBAL VOTES Unbalanced I this design. Wind: ASCE Vasd=103mp Cat. II; Exp C Wind: ASCE Vasd=103mp Cat. II; Exp C MWFRS (env -0-9-6 to 3-7- 5-11-8 to 10- for members. Lumber DOL Truss design only. For stu see Standarc or consult qu All plates are Gable require Gable studs are Gable studs as This truss ha chord live load * This truss ha on the bottom 3-06-00 tall b 	-17=-157/51, 6-1 -20=-153/242, 8- 0-14=-153/241 roof live loads hav 7-16; Vult=130mp h; TCDL=6.0psf; ;; Enclosed; Gabl velope) exterior zc 7, Exterior(2N) 3- 4-5, Exterior(2N) 3- 5, Exterior(2N) 3- 4-5, Exterior(2N) 3- 5, Exterior(2N) 3- 6, Exterior(2N) 3- 5, Exterior(2N) 3- 6, Exterior(2N) 3- 7, Exte	8=-97/85 16=-88/8 ve been of bh (3-see BCDL=6 e Roof; C bone and 7-7 to 5- 10-4-5 to FRS for DOL=1.60 in the pla nd norm End Deta signer as s otherwit tom chor c. for a 10.0 with any d for a liv s where ill fit betw	 5-19=-111/1 3, 9-15=-112 considered fo cond gust) .0psf; h=15ft; common Trus C-C Corner(3) 11-8, Corner(0) 11-8, Corner(0) ane of the true al to the face, ils as applicated. d bearing. D psf bottom other live load of 30.00 a rectangle veen the bottom 	80, /181, r s; E) 3R) ;C-C ss ;, ole, Pl 1. ds. opsf om				ORTH CA	ROULIN
FORCES TOP CHORD BOT CHORD	(lb) - Maximum Com Tension 1-2=0/28, 2-3=-215/ 4-5=-121/87, 5-6=-11 7-8=-109/200, 8-9=- 10-11=-199/94, 11-1 2-20=-104/293, 19-2 18-19=-96/283, 17-1 16-17=-96/283, 15-1 14-15=-96/282, 12-1	pression/Maximum 117, 3-4=-214/138, 03/131, 6-7=-109/199 79/130, 9-10=-88/45, 2=-199/72, 12-13=0/: 0=-96/283, 6=-96/283, 4=-103/293	9), 1 28 1 L	 All bearings a capacity of 56 Provide mech bearing plate 2, 25 lb uplift uplift at joint 1 joint 16, 95 lb 14. See Standard Detail for Cor consult qualif LOAD CASE(S) 	are assumed to be something of the second capable of withsi at joint 12, 58 lb 1 9, 166 lb uplift at uplift at joint 15 a d Industry Piggyb nection to base t ied building desig Standard	e SP No. and (by oth and ing 2 uplift at ju i joint 20, and 187 ack Trus russ as a ner.	1 crushing ers) of truss t (3 lb uplift at j point 18, 91 lb 51 lb uplift at join lb uplift at join s Connection applicable, or	o bint t			Summer State	SEA 2867	EEP. St.

March 13,2025

Page: 1

S AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.	1
ased only upon parameters shown, and is for an individual building component, not	
oplicability of design parameters and properly incorporate this design into the overall	
al truss web and/or chord members only. Additional temporary and permanent bracing	
e personal injury and property damage. For general guidance regarding the	
russ systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org)	818
om the Structural Building Component Association (www.shcacomponents.com)	510

WARNING - Verify design parameters and READ NOTES ON THIS. Design valid for use only with MITek® connectors. This design is bas a truss system. Before use, the building designer must verify the app building design. Bracing indicated is to prevent buckling of individua is always required for stability and to prevent collapse with possible fabrication, storage, delivery, erection and bracing of trusses and tru and BCSI Building Component Safety Information available from .spcacompone

Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge				
J0325-1350	E1	MONOPITCH	3	1	Job Reference (optional)	171968859			





Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51

Scale = 1:27.5

Plate Offsets (X, Y): [2:0-2-12,Edge]

2-3-14

-													
Loading		(psf)	Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roo	f)	20.0	Plate Grip DOL	1.15	TC	0.27	Vert(LL)	-0.03	2-4	>999	360	MT20	244/190
TCDL		10.0	Lumber DOL	1.15	BC	0.28	Vert(CT)	-0.05	2-4	>999	240		
BCLL		0.0*	Rep Stress Incr	YES	WB	0.00	Wind(LL)	0.09	2-4	>922	240		
BCDL		10.0	Code	IRC2021/TPI2014	Matrix-P							Weight: 38 lb	FT = 20%
				7) Provide mec	hanical connection	(bv oth	ers) of truss t	0					
TOP CHO	RD 2x6 SP No.	1		bearing plate	capable of withsta	nding 1	28 lb uplift at						
BOT CHO	RD 2x6 SP No.	1		joint 2 and 1) 08 lb uplift at joint 4								
WEBS	2x6 SP No.	1		LOAD CASE(S)	Standard								
BRACING	ì			()									
TOP CHO	RD Structural w	ood she	athing directly applie	d or									
	6-0-0 oc pu	rlins, exe	cept end verticals.										
BOT CHO	RD Rigid ceiling	g directly	applied or 10-0-0 oc	:									
	bracing.												
REACTIO	NS (size) 2	=0-3-0, 4	1=0-1-8										
	Max Horiz 2	=63 (LC	8)										
	Max Uplift 2	=-128 (L	C 8), 4=-108 (LC 8)										
	Max Grav 2	=320 (LC	C 1), 4=263 (LC 1)										
FORCES	(lb) - Maxim	num Com	pression/Maximum										
TODOUO	Tension		o / / / o o / o / o										
	RD 1-2=0/8, 2-	3=-80/40,	, 3-4=-196/246										
BOLCHO	RD 2-4=0/0												
NOTES			(a										
1) Wind:	ASCE 7-16; Vult=	=130mph	(3-second gust)										
Cat II	· Evp C: Enclosed	. upsi, bu	SDL=0.0psi, n= ron, S (envelope) and C-I	r i									
Evteri	or(2E) -0-8-11 to 1	R-8-2 Inte	c (envelope) and c^{-1}	.4									111.
zone:	porch left and right	t expose	d:C-C for members	and								White CA	Dall
forces	& MWFRS for re	actions sl	hown: Lumber									att	70, 14
DOL=	1.60 plate grip DC	DL=1.60	,								S.	O'LESS	M.M.
2) This ti	uss has been des	igned for	a 10.0 psf bottom								5 5	4.1 1	No. 7 ·
chord	live load noncond	urrent wi	th any other live load	ls.							: /	St 2	R: =
 3) * This 	truss has been de	esigned for	or a live load of 30.0	psf						-	1	054	· · · · ·
on the	bottom chord in a	all areas	where a rectangle							=		SEA	L 🕴 🗄
3-06-0	0 tall by 2-00-00	wide will	fit between the botto	m								2867	7 : 2
	and any other me	mbers.								-		. 2007	1 2
4) All De	annys are assume ity of 565 psi		or NO. I Crushing									N	1 3
5) Bearin	ng of 505 pai. ng at ioint(s) 4 con	siders no	arallel to grain value								30	S.ENIO	CR. DS
using	ANSI/TPI 1 angle	to grain f	formula. Building								1	GIN	St. St.
desia	ner should verify c	apacity o	of bearing surface.								1	NIA	1111
6) Provid	le mechanical cor	nection (by others) of truss to)								11, L. G.	ALINI
hearin	a nlate at ioint(s)	4											111.

6) bearing plate at joint(s) 4.

March 13,2025

Page: 1

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge				
J0325-1350	E2	MONOPITCH	5	1	Job Reference (optional)	171968860			





Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51

Page: 1

Scale = 1:26.5

Plate Offsets (X, Y): [2:0-2-12,Edge]

2-0-14

Loading TCUL (cool) (pf) 100 Spacing Parta Gr pOL 100 2-0-0 100 CSI 100 0 PLATES 100 GRIP Code PLATES BC GRIP Code Vert(L1) 0.0 Vest 2-4 >989 240 BCDL 10.0 Rp Stress Inor 1000 YES BC 0.00 Vert(C1) 0.00 2-4 >989 240 Weight: 33 ib FT = 20% LUMBER TO CHORD 2x6 SP No.1 To Code 7) Provide mechanical connection (by others) of truss to 2 and 91 b uplift at joint 4. Vert(L1) 0.0 5.4 >989 240 Weight: 33 ib FT = 20% LUMBER TO CHORD 2x6 SP No.1 LOAD CASE(S) Standard LOAD CASE(S) Standard BT CHORD 3x6 SP No.1 LOAD CASE(S) Standard Standard Standard BT CHORD 3x6 SP No.1 LOAD CASE(S) Standard Standard Standard BT CHORD 3x6 SP No.1 LOAD CASE(S) Standard Standard Standard BT CHORD Standard LOAD CASE(S) Standard Standard St														
TCLL (root) 200 Plate Grip DOL 1.15 TC 0.19 Vert(CT) -0.00 24 -999 240 BCL 10.0 Code Itember DOL 1.15 BC 0.20 Vert(CT) -0.06 24 >999 240 BCL 10.0 Code Item Carpot WES 0.00 Wind(LL) 0.05 2.4 >999 240 BCDL 10.0 Code IRC2021/TPI2014 Mark.P Wind(LL) 0.05 2.4 >999 240 BCDL 10.0 Code Iteracity and plate capable of withistanding 114 lb uplit at joint 2. 2.3 2.4 2.999 240 Weight: 33 lb FT = 20% UNBER 2.6 S P No.1 2.4 and 910 uplit at joint 4. LOAD CASE(S) Standard Barbarbarbarbarbarbarbarbarbarbarbarbarba	Loading	(1	psf)	Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCDL 10.0 Lumber DOL 1.15 BC 0.20 Virial (CT) 0.03 2.4 >999 240 BCDL 10.0 Code IRC2021/TPI2014 Matrix-P 0.00 Wind(LL) 0.05 2.4 >999 240 LUMBER 10.0 Code IRC2021/TPI2014 Matrix-P 0.00 Wind(LL) 0.05 2.4 >999 240 LUMBER 10.0 Provide mechanical connection (by others) of trues to bearing plate capable of withstanding 114 b uplit at joint 4. 2 and 91 b uplit at joint 4. 10.00 2.00 Q PC VEES 2.05 SP No.1 LOAD CASE(S) Standard DAT CASE(S) Standard BRACINOS Gite 2.0-3.0.4-0-1-8 Max Horiz 2.2-56 (LC 8) Max Horiz 2.2-56 (LC 8) Max Mark 2-2-56 (LC 8) Max Horiz 2.2-56 (LC 8) M	TCLL (roof)	2	20.0	Plate Grip DOL	1.15	тс	0.19	Vert(LL)	-0.01	2-4	>999	360	MT20	244/190
BCLL 0.0° Rep Stress Incr YES WB 0.00 Wind(L) 0.05 2.4 >999 240 LUMBER 10.0 Code IRC2021/TPI2014 Matrix-P Matrix-P Weight: 33 lb FT = 20% LUMBER 20.5 SP No.1 FT Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 114 lb uplift at joint 2 and 91 ho uplift at joint 2 and 91	TCDL	1	0.0	Lumber DOL	1.15	BC	0.20	Vert(CT)	-0.03	2-4	>999	240		
BCDL 10.0 Code IRC2021/TPI2014 Matrix-P Weight: 33 lb FT = 20% LUMBER TOP CHORD 2x6 SP No.1 7) Provide mechanical connection (by others) of truss to bearing place capable of withstanding 114 lb uplit at joint 2 and 91 lb uplit at joint 4. 7) Provide mechanical connection (by others) of truss to bearing place capable of withstanding 114 lb uplit at joint 2 and 91 lb uplit at joint 4. COP CHORD Structural wood sheathing directly applied or 6-00 oc purlins, except end verticals. CAD CASE(S) Standard BRACINOS TOP CHORD Structural wood sheathing directly applied or 6-00 oc purlins, except end verticals. CAD CASE(S) Standard BRACINOS (Strice) 2e-0-30, 4=0-1-8 Max Horiz 2=65 (LC 8) Max Grav 2=280 (LC 1), 4=222 (LC 1) Koat Standard FORCES (b) - Maximum Compression/Maximum Tension Tension For eactors shown; Lumber DOL = 160 (PTRS), creaced: c-C for methers and forces & MWFRS for reactors shown; Lumber DOL = 160 plate grip DOL= 60 at load nonconcurrent with any other live loads. SEAL 28677 1) This truss has been designed for a 100 pal bottom chord and nothord rul all areas where a rectangle 3-06-00 tall by 2-000 wide will fit between the bottom chord and nothord rul all areas where a rectangle 3-06-00 tall by 2-000 wide will fit between the bottom chord and any other members. SEAL 28677 2) This truss has been designed for a 100 pal bottom chord and nothord rul all areas where a rectangle 3-06-00 tall by 2-000 wide will fit between the bottom chord and any other members. SEAL 28677	BCLL		0.0*	Rep Stress Incr	YES	WB	0.00	Wind(LL)	0.05	2-4	>999	240		
LUMBER TOP CHORD DOT CHORD BCT CHORD BENCING BENCING TOP CHORD SHOULD a word sheathing directly applied or top-or or braining. Keepel and verticals. BOT CHORD TOP CHORD SHOULD a word sheathing directly applied or top-or or braining. 1. Provide mechanical connection (by others) of truss to beating plate capable of withstanding 114 tb uplift at joint 2 and 91 tb uplift at jo	BCDL	1	0.0	Code	IRC2021/TPI2014	Matrix-P							Weight: 33 lb	FT = 20%
Top CHORD 2x6 SP No.1 2 and S1 hought at joint 2 and S1 hought at joint 2 and S1 hought at joint 4. WEBS 2x6 SP No.1 LOAD CASE(S) Standard BRACINS TOP CHORD Structural wood sheathing directly applied or 6-0-0 or purine, except end verticals. BCT CHORD Structural wood sheathing directly applied or 0-0-0 or bracing. REACTIONS (size) 2-0-3-0, 4=0-1-8 Max Horiz 2-55 (LC 8) Max Korz 2-250 (LC 1), 4=222 (LC 1) FORCES IN (D) - Maximum compression/Maximum Tension TOP CHORD 2-4-00 NOTES OT CHORD 2-4-00 NOTES OT CHORD 2-4-00 NOTES 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd-103mph; TCDL=60pst; BCDL=60psf; h=15ft; Cast. II; Exp (2-a-06); BCDL=60psf; h=15ft; Cast. II; Exp					 Provide mec 	hanical connection	(bv oth	ers) of truss t	O					
BOT CHORD 2x6 SP No.1 2 and 91 lb uplift at joint 4. WEBS 2x6 SP No.1 LOAD CASE(S) Standard BRACING TOP CHORD Structural wood sheathing directly applied or 6-00 oc purifies, except end verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS (size) 2-0-30, 4-0-1-8 Max Uplift 251 (L6.8), 491 (LC.8) Max Uplift 251 (L6.8), 491 (LC.8) Max Grav 2-2280 (LC.1), 4-222 (LC.1) FORCES (b). Maximum Compression/Maximum TOP CHORD 1-2-008, 2-3-68/36, 3-4=-166/208 BOT CHORD 2-4-00 TOP CHORD 1-2-008, 2-3-68/36, 3-4=-166/208 BOT CHORD 2-4-00 TOP CHORD 1-2-0.00, 2-4-00 TOP CHORD 1-2-0.00, 2-4-00 Structural WWFRS for reactions show: Lumber DOL-1.60 pilete grip DOL-1.60 0. This truss has been designed for a 10.0 pd bottom chord live load nonconcurrent with any other live loads. 0. This truss has been designed for a 10.0 pd bottom chord live load nonconcurrent with any other live loads. 0. This truss has been designed for a 10.0 pd bottom chord live load nonconcurrent with any other live loads. 0. This truss has been designed for a 10.0 pd bottom chord and any other members. 4) All barings are assumed to be SP No.1 crushing capacity of 565 psi. 0. Baring at joint(s) 4 considers parallel to grain value using ANSITP1 + angle to grain formula. Building designer should vertify capacity of bearing surface. 0 Provide mechanical concurrent with capacity of bearing surface. 0 Provide mechanical concurrent with capacity of bearing surface. 0 Provide mechanical concurrent with capacity of bearing surface.	TOP CHORD	2x6 SP No.1			bearing plate	capable of withsta	nding 1	14 lb uplift at	joint					
WEBS 2x6 SP No.1 LOAD CASE(S) Standard BRACING Structural wood sheathing directly applied or 6-0-0 oc purifies, except end verticals. BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purifies, except end verticals. BRACING Structural wood sheathing directly applied or 10-0-0 oc bracing. REACTIONS (size) 2-0-3-0, 4-0-1-8 Max Horiz 2-55 (LC 8) Max Korz 2-280 (LC 1), 4-221 (LC 1) FORCES (b)-Maximum Compression/Maximum Torp CHORD 1-2-048, 2-3=-68/26, 3-4=-166/208 BOT CHORD 1-2-048, 2-3=-68/26, 3-4=-166/208 BOT CHORD 1-2-0-24, -1016 NUMCA SCE 7-16: VUIE130mph (3-second gust) Vasd=103mph; TODL=6.0pd; BCDL=6.0pd; h=15f; Cat. II, Exp C; -Botolosed; MWFRS (revelope) and C-C Exterior(22, -Bot-110, 3-24, Intero (1), 3-24, De 5-94 zone; porch left and right exposed/C-C for members and forces & MWFRS for reactions show; Lumber DOL=1.60 plate grip DOL=1.60 10 This truss has been designed for a 10.0 pd botom chord live load nonconcurrent with any other live loads. 10 ************************************	BOT CHORD	2x6 SP No.1			2 and 91 lb ι	plift at joint 4.	0	·						
BRACING TOP CHORD Structural wood sheating directly applied or 10-0-0 oc bracing. BOT CHORD Regid cealing directly applied or 10-0-0 oc bracing. Free Comment of the c	WEBS	2x6 SP No.1			LOAD CASE(S)	Standard								
TOP CHORD Structural wood sheathing directly applied or 6-0-00 cp utrives, sexcept and verticals. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS (size) 2=0-30, 4=0-1-8 Max Horiz 2=55 (LC 8) Max Horiz 2=55 (LC 8) Max Grav 2=280 (LC 1), 4=21 (LC 8), 4=91 (LC 8) Max Grav 2=280 (LC 1), 4=222 (LC 1) FORCES (b) - Maximum Compression/Maximum TOP CHORD 1-2=008, 2-3=-80/36, 3-4=-166/208 BOT CHORD 1-2=048, 2-3-80/36, 3-4=-166/208 BOT CHORD 2-4=00' NOTES 1 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6 (psf; BOL)=6, 0psf; b=15ft; Cat. II; Ery C; Enclosed; WHYRS (or reactions show), Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) ** This truss has been designed for a 10.0 psf bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing designer should weilt (f) to bearing structure. 5) Bearing at pint(s) 4 considers parallel to grain value designer should verify capacity of bearing value designer should verify capacity of bearing structe.	BRACING				.,									
 BOT CHORD Rigid celling directly applied or 10-0 oc bracing. REACTION (size) 2-0-3-0, 4=0-1-8 Wark Horiz 2-256 (LC 8) Max Upit 2=-114 (LC 8), 4=-91 (LC 8) Max Grav 2=2280 (LC 1), 4=-222 (LC 1) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2-0/8, 2-3a-68/36, 3-4=-166/208 BOT CHORD 2-4-00 NOTES NOTES 1) Wind: ASCE 7-16; Vull=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; BCDL=6.0psf;	TOP CHORD	Structural woo	od shea	athing directly applie	d or									
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. REACTIONS (size) 2=0-3-0, 4=0-1-8 Max Horiz 2=56 (LC 8) Max Uplit 2=-514 (LC 8), 4==91 (LC 8) Max Grav 2=280 (LC 1), 4=222 (LC 1) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1=2-0/8, 2-3=-68/36, 3-4=-166/208 BOT CHORD 1=2-0/8, 2-3=-68/36, 3-4=-166/208 BOIC 1=160 0-2-4-0/0 BOIC 1=160 0-2-6-69/5, BCDL=6.095, 1=-156; concentremt with any other live loads. This truss has been designed for a 10.0 pf bottom chord live load nonconcurrent with any other live loads. This truss has been designed tor a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. Bearing at live load on 30.0psf Bearing at live load on 30.0psf Bearing at live load on 30.0psf Bearing at live load to 7 allive load 30.0psf Bearing at live load on 30.0psf <		6-0-0 oc purlir	ns, exc	ept end verticals.										
 bracing. REACTIONS (size) 2=0-3-0, 4=0-1.8 Max Horiz 2=55 (LC 8) Max Upit 2=-114 (LC 8), 4==91 (LC 8) Max Grav 2=280 (LC 1), 4=222 (LC 1) FORCES (b)-Maximum Compression/Maximum Tension TOP CHORD 1:2=0/8, 2:3=-68/36, 3:4=-166/208 BOT CHORD 2:4=0.0 NOTES 1) Wind: ASCE 7-16; Vull=130mph (3-second gust) Vasd=105mph; TOLL=6, 0pst; EDL=6, 0pst; En-15t; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0.8-11 to 3:8-2, Interior (1) 3:8-2 to 5:9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DDL=1:60 (path grip DDL=1:60) 2) This truss has been designed for a 10.0 psf bottom chord if we load nonconcurrent with any other live loads. 3) * This truss has been designed for a inve load of 30.0psf on the bottom chord in all areas where a rectangle 3:0-60: 011 by 2:0-00: 04e will fit between the bottom chord and any other members. 3) * This truss has been designed for a inve load of 30.0psf on the bottom chord in all areas where a rectangle 3:0-60: 011 by 2:0-00: 04e will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of S65 psi. 5) Bearing at joint(3) 4: considers parallel to grain value using ANS//TP11 angle to grain formula. Building designet mechanicae using surface. 6) Provide mechanicae using contents of the spoel of the other spoel o	BOT CHORD	Rigid ceiling d	lirectly	applied or 10-0-0 oc										
REACTIONS (size) 2=0-3-0, 4=0-1-8 Max Horiz 2=55 (LC 8) Max Uplift 2=-114 (LC 8), 4=-91 (LC 8) Max Orav 2=280 (LC 1), 4=222 (LC 1) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/8, 2-3=-68/36, 3-4=-166/208 BOT CHORD 2=4=00 NOTES 1) Wind: ASCE 7-16; Vull=130mph (3-second gust) Vasd=103mph; TCDL=6.0pst; BCDL=6.0pst; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2, Interior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces 8. MWFRS for reactions show; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord late lead nonconcernet with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at point(3) 4 considers parallel to grain value using ANSI/TP1 4 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to		bracing.												
Max Horiz 2=55 (LC 8) Max Grav 2=280 (LC 1), 4=222 (LC 1) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/8, 2-3=-68/36, 3-4=-166/208 BOT CHORD 2-4=0/0 NOTES 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd= 103mph; TCDL=6.0pst; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (or reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord and any other members. -Chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a 10.0 psf bottom chord and any other members. -Chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 6) Provide mechanical connection (by others) of truss to	REACTIONS	(size) 2=0	0-3-0, 4	=0-1-8										
Max Grav 2=280 (LC 1), 4=221 (LC 8) Max Grav 2=280 (LC 1), 4=222 (LC 1) FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1:2=0/8, 2:3=-68/36, 3:4=-166/208 BOT CHORD 2:4=0/0 NOTES 1) Wind: ASCE 7-16; Vull=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2; Interior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a 10.0 psf bottom chord any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psl. 5) Bearing at joint(s) 4 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. (b) Provide mechanical connection (by others) of truss to		Max Horiz 2=5	55 (LC 8	8)										
Max Grav 2=280 (LC 1), 4=222 (LC 1) FORCES (lb) - Maximum Compression/Maximum Tension TOP CHORD 1.2=0/l8, 2.3=-68/36, 3-4=-166/208 BOT CHORD 2.4=0/0 NOTES 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0ps; BCDL=6.0ps; I=-15t; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0.8-11 to 3-8-2, Interior (1) 3-8-2 to 5-9-4 zone; porch left and right expose(2-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This trues has been designed for a 10.0 psf bottom chord and on concurrent with any other live loads. 3) * This trues has been designed for a 10.0 psf bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANS/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of trues to		Max Uplift 2=-	114 (LC	C 8), 4=-91 (LC 8)										
 FORCES (b) - Maximum Compression/Maximum Tension TOP CHORD 1-2=0/8, 2-3=-68/36, 3-4=-166/208 BOT CHORD 2-4=0/0 NOTES 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2, Interior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to 		Max Grav 2=2	280 (LC	51), 4=222 (LC 1)										
 Inersion This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3.06.00 wide will fit between the bottom chord and any other members. All bearings are assumed to be SP No.1 crushing capacity of 565 psi. Berning at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to 	FORCES	(lb) - Maximun	n Com	pression/Maximum										
 IOP CHORD 1-2=08/36, 3-4=-168/208 BOT CHORD 2-4=0/0 NOTES 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; b=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2, Interior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load noncourternt with any other live loads. 3) * This truss has been designed for a 10.0 psf bottom chord inve load on concurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to 			co/20	2 4 400/200										
 Notes 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2, Interior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 otll by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to 	POT CHORD	1-2=0/8, 2-3=-	-08/30,	3-4=-100/208										
 NOTES 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2; Netrior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 566 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hereintered the defined to the spending surface. 	BOT CHORD	2-4=0/0												
 1) Wind: ASUE 7-16; Vull=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; h=15tr; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-8-11 to 3-8-2, Interior (1) 3-8-2 to 5-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to herefore the existing the function of the store the bottom 	NUIES			(0										
 Vasd-Todad-Todage (WFRS) (enclosed; MWFRS) (enclosed; MWF	1) Wind: AS	CE 7-16; $Vuil=133mph: TCDI = 6.0$	bornpn Inef: BC	(3-second gust)										
 Exterior(2E) -0.8-11 to 3.8-2, Interior (1) 3.8-2 to 5-9-4 zone; porch left and right exposed; C- C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to bearing surface. 	Cat II: Fx	o C: Enclosed: N	/WFRS	S (envelope) and C-(0									
 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 Plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to the provide mechanical connection (by others) o	Exterior(2	2E) -0-8-11 to 3-8	3-2. Inte	rior (1) 3-8-2 to 5-9-	4								minin	1111.
 forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hencine lotte at initiat 0.4 (d) 	zone; por	ch left and right e	expose	d;C-C for members a	and								I'''H CA	ROUL
 DOL=1.60 plate grip DOL=1.60 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hencine live (b) there in the bottom. 	forces & I	MWFRS for react	tions sh	nown; Lumber								N	A de	Plin
 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hypothemical the trut in the function. 	DOL=1.6	0 plate grip DOL=	=1.60									S.	O' SS	S: NE
 chord live load nonconcurrent with any other live loads. 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hereing whether in the formula is to provide the provide mechanical connection (by others) of truss to 	2) This truss	s has been desigr	ned for	a 10.0 psf bottom								: 2	<u> </u>	MA. Y S
 3) * This truss has been designed for a live load of 30.0psr on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP1 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hereing with interval. 	chord live	load nonconcurr	rent wit	h any other live load	ls.						1		Nh/V	VA: 3
 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members. 4) All bearings are assumed to be SP No.1 crushing capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to the provid	3) ^ I his tru	ss has been desig	gned fo	or a live load of 30.0	pst						=		SEA	r 1 2 .
 28677 All bearings are assumed to be SP No.1 crushing capacity of 565 psi. Bearing at joint(s) 4 considers parallel to grain value using ANSI/TP11 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to here in the formula. 	3-06-00 t	all by 2-00-00 wic	areas v do will f	it between the botton	m						=	:	SLA	- : :
 All bearings are assumed to be SP No.1 crushing capacity of 565 psi. Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. Provide mechanical connection (by others) of truss to hereing rechanged and the statements of the statement o	chord and	any other memb	bers	it between the botto							=		2867	7 : 5
 capacity of 565 psi. 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to be an inclusion of the structure of the struct	 All bearin 	as are assumed	to be S	P No.1 crushina										1 E
 5) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to have rechanical to the structure of the st	capacity	of 565 psi.		9								1	N	A 1. 8
using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to hydrographical triate (A)	5) Bearing a	at joint(s) 4 consid	ders pa	rallel to grain value								20	O SNGINI	Ent
 designer should verify capacity of bearing surface. 6) Provide mechanical connection (by others) of truss to bearing latest initial (b) (b) (b) (b) (b) (b) (b) (b) (b) (b)	using AN	SI/TPI 1 angle to	grain f	ormula. Building								11	YA.	
6) Provide mechanical connection (by others) of truss to	designer	should verify cap	acity of	f bearing surface.									L G	ALILIN
	6) Provide n	nechanical conne	ection (I	by others) of truss to)								1111111	11111

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

March 13,2025

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Job	Truss	Truss Type		Ply	Lot 3 Mabry Ridge				
J0325-1350	E3-GE	GABLE	1	1	Job Reference (optional)	171968861			

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f Page: 1



Scale = 1:26.6

Plate Offsets (X, Y): [2:0-2-12,Edge], [4:0-1-14,0-2-0], [5:Edge,0-2-0]

														•
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15		TC	0.20	Vert(LL)	0.04	2-6	>999	240	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15		BC	0.20	Vert(CT)	-0.02	2-6	>999	240			
BCLL	0.0*	Rep Stress Incr	YES		WB	0.02	Horz(CT)	0.00	5	n/a	n/a			
BCDL	10.0	Code	IRC2021/TPI	2014	Matrix-S							Weight: 34 lb	FT = 20%	
LUMBER TOP CHORD BOT CHORD WEBS OTHERS BRACING TOP CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Wind: ASC Vasd=103r Cat. II; Exp MWFRS fo grip DOL= 2) Truss desig only. For s see Standa or consult of 3) Gable stud 4) This truss I chord live I	2x6 SP No.1 2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 Structural wood sh 6-0-0 oc purlins, e Rigid ceiling direct bracing. (size) 2=0-3-0. Max Horiz 2=78 (LI Max Uplift 2=-164 (Max Grav 2=280 (I (Ib) - Maximum Co Tension 1-2=0/8, 2-3=-166/ 4-5=-100/285 2-6=-396/126, 5-6= 3-6=-82/86 SE 7-16; Vult=130mp mph; TCDL=6.0psf; I o C; Enclosed; Gable envelope) exterior zo 3-8-2, Exterior(2N) 3 ht exposed; C- C for ro re reactions shown; L 1.60 gned for wind loads studs exposed at 2-0-0 oc has been designed f oad nonconcurrent v	eathing directly applie xcept end verticals. y applied or 10-0-0 oc 5=0-1-8 C 8) LC 8), 5=-133 (LC 8) LC 1), 5=222 (LC 1) mpression/Maximum 316, 3-4=-124/315, -396/126 h (3-second gust) 3CDL=6.0psf; h=15ft; Roof; Common Trus; ne and C-C Corner(3I -8-2 to 5-9-4 zone; po nembers and forces & umber DOL=1.60 plat n the plane of the trus d (normal to the face) nd Details as applicat signer as per ANSI/TP 5. or a 10.0 psf bottom vith any other live load	5) * TI on 1 3-00 chc 6) All 1 cap d or 7) Bei d or 7) Bei s des 9) Pro bez 9) Pro bez join LOAD 0 5; 5) rch s, i.e s, i.e s, i.e s.	his truss h the bottorn 06-00 tall b ord and an bearings a poacity of 56 aring at joi ng ANSI/T signer shou vide mech aring plate vvide mech aring plate t 2 and 13 CASE(S)	as been designed f ochord in all areas y 2-00-00 wide will y other members. re assumed to be 5 55 psi. nt(s) 5 considers pa Pl 1 angle to grain ald verify capacity of anical connection of capable of withstar 3 lb uplift at joint 5. Standard	or a liv where fit betw SP No. arallel t (by othor (by othor ading 1	e load of 30.(a rectangle veen the bott 1 crushing o grain value a. Building ng surface. ers) of truss t 64 lb uplift at	Dpsf om o o			All	VVeignt: 34 ID OH H CA SEA 2867 OH L. GI	ROUTUNE ROUTUNE T	-

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



March 13,2025

Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	G1	Monopitch	9	1	Job Reference (optional)	171968862

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





4x6 🛚



Scale = 1:28.3

Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-P	0.02 0.04 0.00	DEFL Vert(LL) Vert(CT) Wind(LL)	in 0.00 0.00 0.00	(loc) 2-6 2-6 2-6	l/defl >999 >999 >999	L/d 360 240 240	PLATES MT20 Weight: 16 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD 2x6 BOT CHORD 2x6 WEBS 2x4 WEDGE Left BRACING TOP CHORD Stru 2-0- BOT CHORD Rig brav REACTIONS (size, Max Max	SP No.1 SP No.1 SP No.2 : 2x4 SP No.2 ictural wood she: 0 oc purlins, exi d ceiling directly cing. 2=0-3-8, 5 Horiz 2=59 (LC Uplift 5=-29 (LC Grav 2=137 (LC	athing directly applie cept end verticals. applied or 10-0-0 oc 5= Mechanical 12) 12) 5 1), 5=70 (LC 19)	d or									
 FORCES (Ib) Ten TOP CHORD 1-2: BOT CHORD 2-6: NOTES 1) Wind: ASCE 7-1 Vasd=103mph; Cat. II; Exp C; E Exterior(2E) zor MWFRS for rea grip DOL=1.60 2) This truss has on the bottor cl 3-06-00 tall by 2 chord and any c 4) Bearings are as capacity of 565 5) Refer to girder(s 6) Provide mechar bearing plate ca 5. LOAD CASE(S) S 	- Maximum Com sion =0/5, 2-3=-100/5! =0/0, 5-6=0/0 6; Vult=130mph TCDL=6.0psf; Bf nclosed; MWFR: e;C-C for memb ctions shown; Lu een designed for nonconcurrent wi been designed for nonconcurrent wi been designed for nord in all areas u -00-00 wide will ther members. sumed to be: Joi psi. con truss to trus ical connection (pable of withstar tandard	pression/Maximum 5, 3-4=-3/0, 3-6=-92/ (3-second gust) CDL=6.0psf; h=15ft; S (envelope) and C-1 ers and forces & mber DOL=1.60 plat r a 10.0 psf bottom th any other live load or a live load of 30.0 where a rectangle fit between the botto nt 2 SP No.1 crushin ss connections. by others) of truss to ding 29 lb uplift at jo	128 C e Is. osf m g							A A A A A A A A A A A A A A A A A A A	SEA 2867	ROULS ALMONTHING

March 13,2025



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	G2	Jack-Open	2	1	Job Reference (optional)	171968863

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1





	2-8-7	
Scale = 1:24.5		
Plate Offsets (X, Y): [2:Edge,0-1-13]		

Loading TCLL (roof) TCDL BCLL BCDL	(ps 20 10 0. 10) D D D * D	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-P	0.08 0.02 0.00	DEFL Vert(LL) Vert(CT) Horz(CT)	in 0.00 0.00 0.00	(loc) 2-4 2-4 3	l/defl >999 >999 n/a	L/d 360 240 n/a	PLATES MT20 Weight: 18 lb	GRIP 244/190 FT = 20%	
LUMBER TOP CHORD BOT CHORD WEDGE BRACING TOP CHORD BOT CHORD REACTIONS	2x6 SP No.1 2x6 SP No.1 Left: 2x4 SP No Structural wood 2-8-7 oc purlins Rigid ceiling dire bracing. (size) 2=0-4 Mech	2 shea ectly a -9, 3=	thing directly applie applied or 10-0-0 or = Mechanical, 4=	LOAD CASE(S) ed or c	Standard									
FORCES TOP CHORD BOT CHORD	Mechanical Max Horiz 2=58 (LC 12) Max Uplift 2=-15 (LC 12), 3=-39 (LC 12) Max Grav 2=201 (LC 1), 3=51 (LC 1), 4=49 (LC 3) (Ib) - Maximum Compression/Maximum Tension DRD 1-2=0/5, 2-3=-80/47 DRD 2-4=0/0													
 NOTES Wind: ASC Vasd=103i Cat. II; Exp Corner (3) for reactior DOL=1.60 This truss chord live I * This truss on the bott 3-06-00 tal chord and Bearings a capacity of Refer to gi Provide me bearing pla 3 and 15 lb 	CE 7-16; Vult=130 mph; TCDL=6.0ps o C; Enclosed; MV zone;C-C for men has been designe load nonconcurre s has been design tom chord in all ar Il by 2-00-00 wide any other member any other member f 565 psi. rder(s) for truss the echanical connect ate capable of with to uplift at joint 2.	nph (f; BC /FRS DOL d for tt with ed fo eas w will fi rs. ; , Joi o trus ion (b stand	(3-second gust) iDL=6.0psf; h=15ft; 5 (envelope) and C- 5 and forces & MWF =1.60 plate grip a 10.0 psf bottom h any other live load r a live load of 30.0 where a rectangle t between the bottoc int 2 SP No.1 crush s connections. by others) of truss to ding 39 lb uplift at jo	-C FRS ds.)psf om ning o						. antitution.	and a start of the	SEA 2867	RO 12 7 4 13,2025	

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	H1GE	GABLE	1	1	Job Reference (optional)	171968864

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



21-	11-0	
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Scale = 1:54.8 Plate Offsets (X, Y): [2:Edge,0-0-13], [18:Edge,0-0-13]

Loading		(psf)	Spacing	2-0-0			CSI		DEFL	in	(lo	c)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)		20.0	Plate Grip DOL	1.15		1	TC	0.04	Vert(LL)	n/a		-	n/a	999	MT20	244/190	
TCDL		10.0	Lumber DOL	1.15		11	BC	0.02	Vert(CT)	n/a		-	n/a	999			
BCLL		0.0*	Rep Stress Incr	YES		1	WB	0.13	Horz(CT)	0.00		18	n/a	n/a			
BCDL		10.0	Code	IRC20	21/TPI2014		Matrix-S								Weight: 208 lb	FT = 20%	
SULL SCDL LUMBER TOP CHORD SOT CHORD TOP CHORD SOT CHORD SOT CHORD REACTIONS	2x6 SP No 2x6 SP No 2x4 SP No Structural 6-0-0 oc p Rigid ceili bracing. (size) Max Horiz Max Uplift	0.0 1	athing directly applie applied or 10-0-0 oc 0, 18=21-11-0, 0, 21=21-11-0, 0, 23=21-11-0, 0, 32=21-11-0, 0, 32=21-11-0, 0, 32=21-11-0, 0, 32=21-11-0, 0, 32=21-11-0, 0, 32=21-11-0, 0, 34=21-11-0, 0, 34=21-11-0, 0, 34=21-11-0, 13), 21=-58 (LC 12) C 13), 21=-58 (LC 12) C 13), 23=-58 (LC 12) C 12), 31=-67 (LC 12) C 12), 31=-67 (LC 12) C 12), 31=-67 (LC 12) C 12), 31=-67 (LC 12) C 12), 33=-57 (LC 12) C 12), 33=-57 (LC 12) C 12), 33=-57 (LC 12) C 12), 33=-57 (LC 12) C 20), 18=139 (LC 22) C 20), 21=115 (LC 22) C 20), 26=122 (LC 22)	res IRC200 ed or ed or e till (13), (13), (13), (14) (14) (14) (14) (14) (14) (14) (14)	21/TPI2014 TOP CHORD BOT CHORD BOT CHORD WEBS NOTES 1) Unbalancenthis design. 2) Wind: ASC Vasd=103r Cat. II; Exp MWFRS (3R) 10-11. zone;C-C fr reactions s DOI -1 60	1-2 4-5 7-8 10 12 15 17 2-3 3 31 29 27 24 22 20 00 7-5 4-5 12 15 0 7-6 -8 to row how	Matrix-S Matrix-S Matrix-S Matrix-S 2=0/5, 2-3=-251/1 3=-101/160, 8-9 -11=-129/221, 11 -13=-90/148, 13- -16=-73/49, 16-17 -18=-192/20, 148, 13- -16=-73/49, 16-17 -18=-192/20, 148, 13- -32=-84/178, 30- -32=-84/178, 30- -32=-84/178, 30- -23=-84/178, 28- -26=-84/178, 28- -26=-84/178, 28- -26=-84/178, 28- -26=-84/178, 18- -28=-133/43, 9-22 -21=-84/178, 18- -28=-133/43, 9-22 -21=-84/178, 18- -22=-91/75, 16-2° of live loads have -16; Vult=130mph; TCDL=6.0psf; B Enclosed; Gable lope) exterior call performation of the second performation of the second -215-4-5, Exterior(18- -215-4-5, Exterior(18-	92, 3-4 1227/11 -120/20 -122-1: 14=-61, 7=-104, -19=0// 33=-84, 33=-84, 33=-84, 33=-84, 33=-84, 33=-84, 27=-84, 29=-84, 20=-84	=-169/147, 9, 6-7=-113/1 2, 9-10=-129/ 20/202, 95, 14-15=-63 68, 5 78, 178, 178, 178, 178, 178, 178, 178,	4, 31, 221, 3/50, 3/50, 4, 23, 74, 9/128 5; 5;	7) 8) 9) 10)	This chorc * This on th 3-06- chorc All bé capa Provi beari 2, 21 uplift 26, 6 uplift joint : AD C /	truss ha d live lo s truss e botto -00 tall d and a earings city of g ide mee ing plat lb uplit at joint 3 lb up at joint 20. ASE(S)	as bee ad non has be m cho by 2-0 ny oth are as 565 ps 565 ps 565 ps 565 ps 565 ps 563 ps 161 at ju 22, 56 Sta	Weight: 208 lb en designed for a nconcurrent with a er designed for a rd in all areas wh 0-00 wide will fit l er members. ssumed to be SP i. al connection (by able of withstandii int 18, 19 lb uplift 2 lb uplift at joint 27 oint 33, 59 lb uplif lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplif 8 lb uplift at joint 27 oint 24, 58 lb uplift at joint 26 ndard	FT = 20% 10.0 psf bottor any other live I a live load of 3 ere a rectangle between the be No.1 crushing others) of trus ng 80 lb uplift a at joint 29, 66 31, 58 lb uplift at t at joint 23, 5 21 and 109 lb of A Solution 100 lb of	n oads. 0.0psf a ottom s to at joint lb at joint 19 lb t joint 7 lb uplift at
FORCES	(lb) - Maxi Tension	27=105 (L 29=118 (L 31=118 (L 33=118 (L 35=157 (L mum Com	C 20), 28=136 (LC 1 C 19), 30=120 (LC 1 C 19), 32=117 (LC 1 C 19), 32=117 (LC 1 C 19), 34=116 (LC 1 C 19) pression/Maximum	13), 19), 19), 19), 19), 2	 DOL=1.60 Truss desig only. For s see Standa or consult c All plates a Gable requ Gable stud 	gneo ard I qual are 2 iires s sp	d for wind loads ir s exposed to wind ndustry Gable En ified building desi x4 MT20 unless of continuous botto baced at 1-4-0 oc.	n the pla d (norm d Deta gner as otherwi m chor	ane of the trus al to the face) ils as applicab per ANSI/TP se indicated. d bearing.	ss , ile, il 1.			LUR.		OK NGIN	ALINST	inner.

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818 Soundside Road Edenton, NC 27932

4

Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	H2	Common	4	1	Job Reference (optional)	171968865

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:51

Comtech, Inc, Fayetteville, NC - 28314,



Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-S	0.20 0.26 0.15	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.05 -0.07 0.02 0.01	(loc) 8-10 8-10 6 2-10	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 156 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS WEDGE	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 Left: 2x4 SP No.2 Right: 2x4 SP No.2		 All bearings capacity of 5 Provide mec bearing plate 2 and 57 lb 0 LOAD CASE(S) 	are assumed to 65 psi. hanical connec capable of wit plift at joint 6. Standard	b be SP No. tion (by othe hstanding 5	1 crushing ers) of truss 7 lb uplift at	to joint					
BRACING												
TOP CHORD	Structural wood shea 6-0-0 oc purlins.	athing directly applie	d or									
BOT CHORD	Rigid ceiling directly bracing.	applied or 10-0-0 oc	:									
REACTIONS	(size) 2=0-3-8, 6	i=0-3-8										

- REACTIONS
 (size)
 2=0-3-8, 6=0-3-8

 Max Horiz
 2=-188 (LC 10)

 Max Uplift
 2=-57 (LC 12), 6=-57 (LC 13)

 Max Grav
 2=1068 (LC 19), 6=1068 (LC 20)

 FORCES
 (lb) Maximum Compression/Maximum Tension

 TOP CHORD
 1-2=0/5, 2-3=-1341/250, 3-4=-1242/321, 4-5=-1242/321, 5-6=-1341/250, 6-7=0/5
- BOT CHORD 2-10=-128/1145, 8-10=0/773, 6-8=-109/1023 WEBS 4-8=-119/622, 5-8=-255/216, 4-10=-119/622, 3-10=-255/216

NOTES

 Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -0-9-7 to 3-7-6, Interior (1) 3-7-6 to 10-11-8, Exterior(2R) 10-11-8 to 15-4-5, Interior (1) 15-4-5 to 22-8-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.



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March 13,2025





Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	H3	Common	1	1	Job Reference (optional)	171968866

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



	7-4-2	14-6-14	21-11-0
I	7-4-2	7-2-13	7-4-2
Scale = 1:59.1			

Loading TCLL (roof) TCDL BCLL BCDL	(psf 20.0 10.0 0.0 10.0) Spacir) Plate G) Lumbe)* Rep St) Code	ng Grip DOL er DOL tress Incr	2-0-0 1.15 1.15 YES IRC2021	/TPI2014	CSI TC BC WB Matrix-S	0.20 0.26 0.15	DEFL Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in -0.05 -0.07 0.02 0.01	(loc) 7-9 7-9 6 2-9	l/defl >999 >999 n/a >999	L/d 360 240 n/a 240	PLATES MT20 Weight: 154 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS WEDGE	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 Left: 2x4 SP No. Right: 2x4 SP No.	2		5) 6) LO	All bearings a capacity of 50 Provide mech bearing plate 2 and 44 lb u AD CASE(S)	are assumed to be 55 psi. nanical connection capable of withsta plift at joint 6. Standard	SP No. (by othe anding 5	1 crushing ers) of truss to 7 lb uplift at jo) bint					
BRACING TOP CHORD BOT CHORD	Structural wood a 6-0-0 oc purlins. Rigid ceiling dire	sheathing di	irectly applied or 10-0-0 oc	lor										
REACTIONS	(size) 2=0-3- Max Horiz 2=187 Max Uplift 2=-57 Max Gray 2=106	8, 6=0-3-8 (LC 11) (LC 12), 6=- 8 (LC 19) 6	-44 (LC 13) S=1014 (LC 20	n)										
FORCES	(lb) - Maximum C	compression	n/Maximum											
TOP CHORD	1 ension 1-2=0/5, 2-3=-13	42/250, 3-4=	=-1243/322,											
BOT CHORD WEBS	4-5=-1247/325, 5 2-9=-135/1145, 7 4-7=-121/628, 5- 3-9=-255/216	5-6=-1345/28 7-9=0/773, 6 7=-259/220,	53 5-7=-118/1025 , 4-9=-119/62	5 :1,										
NOTES	3-3=-233/210													1111
 Unbalance this design 	ed roof live loads ha n.	ave been co	onsidered for										"TH CA	Roite
2) Wind: ASC Vasd=103 Cat. II; Ex Exterior(2 21-9-4 zor reactions DOL=1.60	CE 7-16; Vult=130n smph; TCDL=6.0ps p C; Enclosed; MW E) -0-9-7 to 3-7-6, I R) 10-11-8 to 15-4- ne;C-C for member shown; Lumber DC	hph (3-secon ; BCDL=6.0 FRS (envelor nterior (1) 3- 5, Interior (1) s and forces L=1.60 plate	nd gust) Dpsf; h=15ft; lope) and C-C I-7-6 to 10-11- 1) 15-4-5 to s & MWFRS fi e grip	; -8, or							annun.	N. N.	SEA 2867	7
 This truss chord live 	has been designed load nonconcurren	l for a 10.0 p t with any ot	psf bottom ther live loads	6.								2.	. A.	AINE
 4) * This trus on the bot 3-06-00 ta chord and 	tom chord in all are all by 2-00-00 wide any other member	ed for a live as where a will fit betwe s, with BCD	load of $30.0p$ rectangle en the bottom DL = $10.0psf$.	sf									CHAN L. GI	ALIN Strin

March 13,2025

Page: 1

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	J1	Monopitch	9	1	Job Reference (optional)	171968867

-0-11-0

0-11-0

Comtech, Inc, Fayetteville, NC - 28314,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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4x6 🛚

2-0-0

2-0-0



Scale = 1:28.3

Loading TCLL (roof) TCDL BCLL BCDL	((psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-P	0.02 0.04 0.00	DEFL Vert(LL) Vert(CT) Wind(LL)	in 0.00 0.00 0.00	(loc) 2-6 2-6 2-6	l/defl >999 >999 >999	L/d 360 240 240	PLATES MT20 Weight: 16 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD WEBS WEDGE BRACING TOP CHORD BOT CHORD REACTIONS	2x6 SP No.1 2x6 SP No.1 2x4 SP No.2 Left: 2x4 SP No.2 Left: 2x4 SP No.2 Structural wo 2-0-0 oc purli Rigid ceiling of bracing. (size) 2=I Max Horiz 2=I Max Uplift 5=: Max Grav 2=	No.2 od shea ns, exc directly a 0-3-8, 5 59 (LC -29 (LC 137 (LC	thing directly applie ept end verticals. applied or 10-0-0 oc = Mechanical 12) 12) 1), 5=70 (LC 19)	d or									
FORCES TOP CHORD BOT CHORD	(lb) - Maximu Tension 1-2=0/5, 2-3= 2-6=0/0, 5-6=	m Comp ⊧-100/55 ⊧0/0	oression/Maximum 5, 3-4=-3/0, 3-6=-92/	128									
 NOTES 1) Wind: ASC Vasd=103n Cat. II; Exp Exterior(2E MWFRS for grip DOL=1 2) This truss h chord live k 3) * This truss on the bottor 3-06-00 tall chord and a 4) Bearings au capacity of 5) Refer to gir 6) Provide me bearing pla 5. LOAD CASE(S) 	E 7-16; Vult=1 nph; TCDL=6.(C; Enclosed; I) zone;C-C for r reactions sho l.60 has been desig bad nonconcur that been desig that been design that	30mph (Dpsf; BC MWFRS membeown; Lur gned for rrent wit igned for areas v de will f bbers. be: Joir s to trus ection (t withstan	(3-second gust) DL=6.0psf; h=15ft; 5 (envelope) and C-0 rs and forces & nber DOL=1.60 plat a 10.0 psf bottom h any other live load or a live load of 30.0 where a rectangle it between the botto at 2 SP No.1 crushin s connections. by others) of truss to ding 29 lb uplift at jo	C e Is. Osf g int						"THEFT	A MARTINE AND A	SEA 2867	ROTAR STATES

March 13,2025



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	P1-GE	GABLE	1	1	Job Reference (optional)	171968868

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:33

3-3-0

Plate Offsets (X, Y): [2:0-3-0,Edge], [10:0-3-0,Edge]

	(), [, - 3-]	1										-		
Loading	(psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (ro	of) 20.0	Plate Grip DOL	1.15		тс	0.32	Vert(LL)	-0.04	12-13	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15		BC	0.27	Vert(CT)	-0.07	12-13	>999	240			
BCLL	0.0*	Rep Stress Incr	YES		WB	0.09	Horz(CT)	-0.02	10	n/a	n/a			
BCDI	10.0	Code	IRC2021	/TPI2014	Matrix-S		Wind(LL)	0.09	17-18	>999	240	Weight [,] 95 lb	FT = 20%	
0002		0000			indian e		11110(22)	0.00		- 000	2.0	Trongilla do lo		
)RD 2x6 SP No 1		3)	Truss design only. For stu	ed for wind loads	in the pland (norm	ane of the tru al to the face	iss e).						
BOT CH	DRD 2x6 SP No 1			see Standard	Industry Gable E	nd Deta	ils as applica	ble.						
	2x4 SP No 2			or consult au	alified building de	signer as	s per ANSI/TI	PI 1.						
	224 01 110.2		4)	All plates are	2x4 MT20 unless	otherwi	se indicated.							
		منامه مانده منابر ممانه		Gable studs	spaced at 1-4-0 or	C.								
	C C C C C C C C C C C C C C C C C C C	atting directly applie	6)	This truss ha	s been designed f	or a 10.0) psf bottom							
	6-0-0 oc punins.	opplied or E 7 2 co	-,	chord live loa	ad nonconcurrent	with anv	other live loa	ads.						
	bracing.	applied of 5-7-2 oc	7)	* This truss h	has been designed	for a liv	e load of 30.0	0psf						
REACTIO	DNS (size) 2=0-3-8, 1	10=0-3-8		3-06-00 tall b	2 - 00 - 00 wide wi	S WIIEIE	a reclarigie	om						
	Max Horiz 2=57 (LC	12)		chord and ar	v other members		veen the bott	om						
	Max Uplift 2=-356 (L	C 8), 10=-356 (LC 9)) 8)	All bearings	are assumed to be	SP No	1 crushing							
	Max Grav 2=659 (L0	C 1), 10=659 (LC 1)	0)	capacity of 5	65 nei	501 100.	rcrusning							
FORCES	(lb) - Maximum Com	pression/Maximum	9)	Provide med	banical connection	hy oth	ers) of trues t	to						
	Tension		3)	hearing plate	canable of withst	anding 3	56 lb unlift at	t						
TOP CHO	ORD 1-2=0/8. 2-3=-1113/	1951. 3-4=-1049/194	10.	ioint 2 and 3	56 lb unlift at joint	10								
	4-5=-1032/1947. 5-6	6=-1018/1957.	 		Stondord	10.								
	6-7=-1018/1956, 7-8	3=-1032/1947.	LU	AD CASE(S)	Standard									
	8-9=-1049/1940, 9-1	0=-1113/1951.10-1	1=0/8											
ВОТ СН)RD 2-18=-1734/991 17	-18=-1734/991												
	16-17=-1734/991 1	5-16=-1734/991												
	14-15=-1734/991 1	3-14=-1734/991												
	12-13=-1734/991 1	0-12=-1734/991											in the	
WEBS	6-15733/345 5-16	S-42/42 4-17-18/1	7									N'TH CA	Roily	
WLD0	3-1854/79 7-14	42/42 8-13-18/17	7,								1	R	all have	
	9-12-54/79	42/42, 0 10= 10/17,										U. ESS	DKI V.	
	3-12-34/13										- <	PA 11	13:4	1
	law and work live to a date to aver	have a secold and for								1		T/ N	ny:	2
1) Unba	lanced roof live loads have	been considered for								-		OF A	n 1.	-
	esign.	(0 1 1)								=		SEA	L 🕴	
2) Wind	ASCE 7-16; Vult=130mph	(3-second gust)									- 1	2867	7	
Vaso	=103mph; TCDL=6.0psf; B	CDL=6.0pst; n=15ft;										2007	1 E	-
Cat.	I; Exp C; Enclosed; Gable I	Roof; Common Trus	s;									÷.	1	-
MWF	KS (envelope) exterior zon	e and C-C Corner(3)	=)								2.	·	ain	5
-0-9-	το 3-8-8, Exterior(2N) 3-8-	8 to 7-8-8, Corner(3	K)								1,0	O. VGINI	EENA	
7-8-6	to 12-1-5, Exterior(2N) 12-	1-5 to 16-2-1 zone;									11	'YAI	" Nº	
porch	ient and right exposed;C-C	for members and										11 L G	ALIN	
torce	s & IVIVVERS for reactions s	nown; Lumber										11111	and the second s	
DOL	1.60 plate grip DOL=1.60											- contra	(* * *	
												Manah	40.0005	

March 13,2025

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	17400000
J0325-1350	P2	Common	5	1	Job Reference (optional)	171968869

7-8-8

Comtech, Inc, Fayetteville, NC - 28314,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

15-5-0



Page: 1

16-4-0



-0-11-0



Scale = 1:33

Plate Offsets (X, Y): [2:0-0-1,0-1-1], [4:0-0-1,0-1-1]

Loading	(psf)	Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.27	Vert(LL)	-0.02	2-6	>999	360	MT20	244/190	
TCDL	10.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	-0.06	2-6	>999	240			
BCLL	0.0*	Rep Stress Incr	YES	WB	0.08	Horz(CT)	0.01	4	n/a	n/a			
BCDI	10.0	Code	IRC2021/TPI2014	Matrix-S		Wind(LL)	0.02	2-6	>999	240	Weight: 82 lb	FT = 20%	
DODE	10.0	0000		matrix 0		Wind(EE)	0.02	20	2000	210	Wolght. 02 lb	11 - 2070	
LUMBER			Provide med	chanical connect	ion (by othe	ers) of truss	to						
TOP CHORE	2x6 SP No.1		bearing plat	e capable of with	nstanding 8	1 lb uplift at	joint						
BOT CHORD	2x6 SP No.1		2 and 81 lb	uplift at joint 4.									
WEBS	2x4 SP No.2		LOAD CASE(S)	Standard									
BRACING													
TOP CHORE	Structural wood she	athing directly applie	ed or										
	6-0-0 oc purlins.	• • • •											
BOT CHORE	Rigid ceiling directly	applied or 10-0-0 o	C										
	bracing.												
REACTIONS	(size) 2=0-3-8, 4	4=0-3-8											
	Max Horiz 2=-34 (LC	C 17)											
	Max Uplift 2=-81 (LC	C 8), 4=-81 (LC 9)											
	Max Grav 2=659 (L0	C 1), 4=659 (LC 1)											
FORCES	(lb) - Maximum Corr	pression/Maximum											
	Tension	iprocolori, maximum											
TOP CHORE	0 1-2=0/8, 2-3=-1099/	443, 3-4=-1099/443	,										
	4-5=0/8												
BOT CHORE	2-6=-322/967, 4-6=-	322/967											
WEBS	3-6=0/360												
NOTES													
1) Unbaland	ced roof live loads have	been considered fo	r										
this desig	an.										annun.	1111	
2) Wind: AS	, CE 7-16: Vult=130mph	(3-second aust)									W'LL CA	Pall	
Vasd=10	3mph: TCDL=6.0psf: B	CDL=6.0psf: h=15ft:								~	all	"Quili,	
Cat. II; E	xp C; Enclosed; MWFR	S (envelope) and C	ъС							N.	0	id Init	
Exterior(2	2E) -0-9-1 to 3-7-12, Int	erior (1) 3-7-12 to 7-	·8-8,							5 2		1.73	
Exterior(2	2R) 7-8-8 to 12-1-5, Inte	erior (1) 12-1-5 to 16	-2-1							9	A M K	1 1 1	
zone;C-C	for members and force	es & MWFRS for							-		1114		-
reactions	shown; Lumber DOL=	1.60 plate grip								:	SEA	L :	=
DOL=1.6	0									:	2067	: ,	=
This trus:	s has been designed fo	r a 10.0 psf bottom							1		2007	/ ; ;	=
chord live	e load nonconcurrent wi	ith any other live loa	ds.										6
4) * This tru	ss has been designed f	for a live load of 30.0)psf							2	·	ains	
on the bo	ottom chord in all areas	where a rectangle								-,-	O. SNGINI	Ent	
3-06-00 t	all by 2-00-00 wide will	fit between the botto	om							11	YA,	NS.11	
chord an	d any other members.										VIG	ALILIN	
All bearir	ngs are assumed to be	SP No.1 crushing									111.0	unin,	
capacity	of 565 psi												

5) All bearings are assumed to be SP No.1 crushing capacity of 565 psi.



March 13,2025

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	VC-1	Valley	1	1	Job Reference (optional)	171968870

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:52.4

Loading TCLL (roof) TCDL BCLL BCDL	(ps 20 10 0 10	sf) Spacing 0 Plate Grip DOL 1 Lumber DOL 0* Rep Stress Incr 0 Code	2-0-0 1.15 1.15 YES IRC202 ⁻	1/TPI2014	CSI TC BC WB Matrix-S	0.18 0.17 0.14	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 78 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.1 2x4 SP No.1 2x4 SP No.2 Structural wood 6-0-0 oc purlins Rigid ceiling dir bracing. (size) 1=16 7=16 Max Horiz 1=18 Max Uplift 1=-2 9=-1 Max Grav 1=20 6=54 9=51	I sheathing directly appl ectly applied or 10-0-0 -1-6, 5=16-1-6, 6=16-1- -1-6, 9=16-1-6 5 (LC 9) 7 (LC 8), 6=-193 (LC 13 94 (LC 12) 1 (LC 20), 5=178 (LC 1 3 (LC 20), 7=407 (LC 2	5) 6) 7) lied or 8) -6, 9) -6, 9), 2), LC 9),	Gable studs This truss ha chord live loa * This truss f on the bottor 3-06-00 tall b chord and ar All bearings : capacity of 5 Provide mec bearing plate 1, 194 lb upli	spaced at 4-0- s been design ad nonconcurre has been design n chord in all a y 2-00-00 wide y other memb are assumed to 65 psi. hanical connec capable of wi ft at joint 9 and Standard	0 oc. ed for a 10.0 ent with any ined for a liv ireas where e will fit betw ers, with BC o be SP No. ction (by oth thstanding 2 d 193 lb uplif) psf bottom other live loa e load of 30. a rectangle veen the bott DL = 10.0ps 1 crushing ers) of truss 7 lb uplift at t at joint 6.	ads. Opsf fom f. to joint					
FORCES	(lb) - Maximum	Compression/Maximun	n										
TOP CHORD	1-2=-179/148, 2 4-5=-145/110	2-3=-192/159, 3-4=-192	/159,										
BOT CHORD	1-9=-113/147, 7	7-9=-113/147, 6-7=-113	/147,										

5-6=-113/147 WEBS 3-7=-137/21, 2-9=-427/314, 4-6=-427/314

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-4-4 to 4-9-0, Interior (1) 4-9-0 to 8-1-1, Exterior(2R) 8-1-1 to 12-5-14, Interior (1) 12-5-14 to 15-9-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.



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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	VC-2	Valley	1	1	Job Reference (optional)	171968871

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:46.6

Loading TCLL (roof) TCDL BCLL BCDL		(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202	1/TPI2014	CSI TC BC WB Matrix-S	0.16 0.16 0.10	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 63 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD	2x4 SP N 2x4 SP N 2x4 SP N Structural 6-0-0 oc p Rigid ceili bracing.	o.1 o.1 o.2 wood shea ourlins. ng directly	athing directly applie applied or 10-0-0 oc	5) 6) 7) ed or c 8)	Gable studs This truss ha chord live loa * This truss h on the botton 3-06-00 tall b chord and an All bearings a capacity of 5	spaced at 0-0-0 or s been designed f ad nonconcurrent in s been designed n chord in all area by 2-00-00 wide wi y other members, are assumed to be 65 psi. papical connection	c. for a 10.0 with any d for a liv s where ill fit betv , with BC e SP No.	D psf bottom other live loa e load of 30.0 a rectangle veen the botto DL = 10.0psf 1 crushing	ds. Opsf om					
REACTIONS	(size) Max Horiz Max Uplift Max Grav	1=13-5-6, 7=13-5-6, 1=-153 (Lt 1=-38 (LC (LC 13), 8 1=141 (LC 6=429 (LC	5=13-5-6, 6=13-5-6 8=13-5-6 C 8) : 8), 5=-13 (LC 9), 6= :=-166 (LC 12) C 20), 5=123 (LC 19) C 20), 7=398 (LC 19)	^{;, 9)} =-166 L(),),	bearing plate 1, 13 lb uplift uplift at joint DAD CASE(S)	capable of withst at joint 5, 166 lb o 6. Standard	anding 3 uplift at jo	18 lb uplift at j bint 8 and 16	oint 6 lb					
FORCES	(lb) - Max	imum Com	pression/Maximum											
TOP CHORD	Tension 1-2=-174/ 4-5=-163/ 1-8=-56/1	'127, 2-3=-' '91 40, 7-856	166/166, 3-4=-152/1 6/140, 6-7=-56/140	56,										
DOT CHOILD	5-6=-56/1	40	0/140, 0-7=-30/140,											
WEBS	3-7=-138/	0, 2-8=-296	6/359, 4-6=-296/359)									W''LL CA	Dalla
 Unbalance this design Wind: AS(Vasd=103) Cat. II; Ext Exterior(2) Exterior(2) 13-1-14 zr for reaction DOL=1.66 Truss dess only. For see Stand 	ed roof live I n. CE 7-16; Vu 3mph; TCDL p; C; Encloss E) 0-4-4 to 4 N; 0-9-1 to 1 one;C-C for ons shown; L) idgned for wi studs expos fard Industry	oads have It=130mph =6.0psf; BC ed; MWFR I-9-0, Interi 11-1-14, Int members a .umber DO nd loads in sed to wind (Gable End	been considered for (3-second gust) CDL=6.0psf; h=15ft; S (envelope) and C- ior (1) 4-9-0 to 6-9-1, terior (1) 11-14 to and forces & MWFR L=1.60 plate grip the plane of the trus (normal to the face) d Details as applicab	r -C -, S ss 1, Jole,								Sunday Street	OR FASS SEA 286	EER CLUM

Truss designed for wind loads in the plane of the truss 3) only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1. 4) Gable requires continuous bottom chord bearing.

818 Soundside Road Edenton, NC 27932

March 13,2025

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	VC-3	Valley	1	1	Job Reference (optional)	171968872

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f





Scale =	1:43.5
	1.40.0

Loading TCLL (roof) TCDL BCLL BCDL		(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC202 ⁻	1/TPI2014	CSI TC BC WB Matrix-S	0.18 0.09 0.08	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 5	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 47 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP N 2x4 SP N 2x4 SP N Structura 6-0-0 oc Rigid ceil bracing. (size) Max Horiz Max Uplift Max Grav	lo.1 lo.2 ll wood she: purlins. ling directly 1=10-9-6, 7=10-9-6, 1=-122 (L 1=-108 (L 6=-170 (L 1=121 (LC 6=355 (LC 8=356 (LC	athing directly applie applied or 10-0-0 oc 5=10-9-6, 6=10-9-6, 8=10-9-6 C 8) C 10), 5=-88 (LC 11) C 13), 8=-171 (LC 12 C 9), 5=-109 (LC 13), C 20), 7=221 (LC 1), C 19)	5) 6) 7) d or 8) 9) , 2) LC	Gable studs This truss ha chord live lo * This truss lo on the bottou 3-06-00 tall i chord and au All bearings capacity of 5 Provide mec bearing plate joint 1, 88 lb 170 lb uplift DAD CASE(S)	spaced at 0-0-0 as been designer ad nonconcurrer has been design m chord in all are by 2-00-00 wide ny other member are assumed to i65 psi. thanical connecti e capable of with uplift at joint 5, 1 at joint 6. Standard	oc. d for a 10. tt with any ed for a liv aas where will fit betw rs. be SP No. ton (by oth standing 1 171 lb uplif	0 psf bottom other live loa e load of 30. a rectangle veen the bott 1 crushing ers) of truss 08 lb uplift a t at joint 8 ar	ads. Opsf om to t					

10-9-6



WEBS 3-7=-135/13, 2-8=-332/471, 4-6=-332/471

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) 2) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-4-4 to 4-9-0, Interior (1) 4-9-0 to 5-5-1, Exterior(2R) 5-5-1 to 9-9-14, Interior (1) 9-9-14 to 10-5-14 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.



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Edenton, NC 27932

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	VC-4	Valley	1	1	Job Reference (optional)	171968873

4-1-1

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:33.2

Loading TCLL (roof) TCDL BCLL BCDL	(psf) 20.0 10.0 0.0* 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-0-0 1.15 1.15 YES IRC2021/TF	⊃I2014	CSI TC BC WB Matrix-P	0.34 0.10 0.03	DEFL Vert(LL) Vert(TL) Horiz(TL)	in n/a n/a 0.00	(loc) - - 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 33 lb	GRIP 244/190 FT = 20%
LUMBER TOP CHORD BOT CHORD OTHERS BRACING TOP CHORD BOT CHORD REACTIONS	2x4 SP No.1 2x4 SP No.1 2x4 SP No.2 Structural wood she 6-0-0 oc purlins. Rigid ceiling directly bracing. (size) 1=8-1-6, 3 Max Horiz 1=-90 (LC Max Uplift 1=-32 (LC Max Grav 1=182 (LC (LC 1)	athing directly applie applied or 10-0-0 or 3=8-1-6, 4=8-1-6 2 8) 2 13), 3=-32 (LC 13) C 1), 3=182 (LC 1), 4	7) * - or 3- ch 8) Al cc cc cc cc 9) Pr be 1 LOAD	This truss h n the bottom -06-00 tall b hord and an Il bearings a apacity of 56 rovide mech earing plate and 32 lb u D CASE(S)	as been designed o chord in all area y 2-00-00 wide w y other members are assumed to be 55 psi. nanical connection capable of withst plift at joint 3. Standard	d for a liv s where ill fit betv e SP No. n (by oth anding 3	e load of 30.0 a rectangle veen the botto 1 crushing ers) of truss t 12 lb uplift at j	Dpsf om o oint					
FORCES	(lb) - Maximum Com Tension	pression/Maximum											
TOP CHORD BOT CHORD WEBS	1-2=-137/88, 2-3=-1 1-4=-26/61, 3-4=-26 2-4=-140/100	23/112 /61											
NOTES 1) Unbalanc this desig 2) Wind: AS Vasd=100 Cat. II; Ex- Exterior(2 MWFRS i grip DOL=	ed roof live loads have n. CE 7-16; Vult=130mph 3mph; TCDL=6.0psf; B φ C; Enclosed; MWFR 2E) zone;C-C for memb for reactions shown; Lu =1.60	been considered for (3-second gust) CDL=6.0psf; h=15ft; S (envelope) and C- ers and forces & imber DOL=1.60 pla	C								New York	ORTH CA	ROUIN

- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 0-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

SEAL 28677

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	VC-5	Valley	1	1	Job Reference (optional)	171968874

2-8-11

Comtech, Inc, Fayetteville, NC - 28314,

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

5-2-0

Page: 1





Scale = 1:27.9

Loading		(psf)	Spacing	2-0-0	csi		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)		20.0	Plate Grip DOL	1.15	тс	0.16	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL		10.0	Lumber DOL	1.15	BC	0.04	Vert(TL)	n/a	-	n/a	999		
BCLL		0.0*	Rep Stress Incr	YES	WB	0.01	Horiz(TL)	0.00	3	n/a	n/a		
BCDL		10.0	Code	IRC2021/TPI2014	Matrix-P							Weight: 21 lb	FT = 20%
	-			7) * This tru	iss has been designed	ed for a liv	e load of 30 (Inef					
	2v4 SP N	0.1		on the h	ottom chord in all are	as where	a rectandle	ры					
BOT CHORD	2x4 SP N	0.1		3-06-00	tall by 2-00-00 wide	will fit betw	veen the bott	om					
OTHERS	2x4 GF No. 2 chord and any other members.												
BRACING				 All beari 	ngs are assumed to b	be SP No.	1 crushing						
TOP CHORD	Structural	l wood she	athing directly applied	d or capacity	of 565 psi.								
	5-6-2 oc p	purlins.		9) Provide	mechanical connection	on (by oth	ers) of truss t	0					
BOT CHORD	Rigid ceili bracing.	ing directly	applied or 10-0-0 oc	1 and 21	Ib uplift at joint 3.	standing 2	i ib uplit at j	oint					
REACTIONS	(size)	1=5-5-6, 3	3=5-5-6, 4=5-5-6	LOAD CASE	E(S) Standard								
	Max Horiz	1=58 (LC	9)										
	Max Uplift	1=-21 (LC	: 13), 3=-21 (LC 13)										
	Max Grav	1=117 (LC (LC 1)	C 1), 3=117 (LC 1), 4	=150									
FORCES	(lb) - Max	imum Com	pression/Maximum										
	Tension												
TOP CHORD	1-2=-88/6	58, 2-3=-79/	/86										
BOICHORD	1-4=-17/4	16, 3-4=-17/	/46										
WEB5	2-4=-90/8	50											
NOTES													
1) Unbalance	ed roof live I	loads have	been considered for										
2) Wind AS	∩. ≏E 7-16: \/u	lt_130mph	(3-second quet)										
Vasd=103	Smoh: TCDI	=6 0nsf B(CDI = 6 Onsf h = 15ft										1111
Cat. II: Ex	p C: Enclose	ed: MWFR	S (envelope) and C-0	0								"TH VA	ROUL
Exterior(2	E) zone;C-C	for memb	ers and forces &								N	A. tho	S. Alle
MWFRS f	or reactions	shown; Lu	mber DOL=1.60 plate	е							22	11	01. 51
grip DOL=	=1.60										2 >	ALL	14:1 -
3) Truss des	igned for wi	nd loads in	the plane of the trus	S						-	/		1 N N E
only. For	studs expos	sed to wind	(normal to the face),							=		SEA	L : =
see Stand	ard Industry	/ Gable End	Details as applicabl	le, I 1						=		2867	
4) Gable reg	uires contin	nous bottor	m chord bearing							=		2007	1 I E
5) Gable stu	ds spaced a	at 0-0-0 oc	n onora bearing.									N	1 8
6) This truss	has been d	lesigned for	a 10.0 psf bottom								2.0	EN	RINS
chord live	load noncor	ncurrent wi	th any other live load	s.							1	OKIGINI	EF. GT
	NY CONTRACTOR												
												11, L. G.	ALIN
													THE .

- see Standard Industry Gable End Details as application or consult qualified building designer as per ANSI/TPI 1. Gable requires continuous bottom chord bearing. 4)
- Gable studs spaced at 0-0-0 oc. 5)
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

March 13,2025

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Job	Truss	Truss Type	Qty	Ply	Lot 3 Mabry Ridge	
J0325-1350	VC-6	Valley	1	1	Job Reference (optional)	171968875

Run: 8.63 S Sep 26 2024 Print: 8.630 S Sep 26 2024 MiTek Industries, Inc. Tue Mar 11 18:26:52 ID:tLzISiCk4ttUXohUqmfgStyJZ5j-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

2-6-0

1-1-5

1-4-11

1-4-11

Page: 1



1-5-1



3x4 =

Scale = 1:24.7

Plate Offsets (X, Y): [2:0-2-0,Edge]

-				-							-			
Loading	((psf)	Spacing	2-0-0		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)		20.0	Plate Grip DOL	1.15		тс	0.04	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL		10.0	Lumber DOL	1.15		BC	0.04	Vert(TL)	n/a	-	n/a	999		
BCLL		0.0*	Rep Stress Incr	YES		WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCDL		10.0	Code	IRC2021	/TPI2014	Matrix-P		- ()					Weight: 9 lb	FT = 20%
_									-				- 5	
LUMBER				8)	All bearings a	are assumed to be	SP No.	1 crushing						
TOP CHORD	2x4 SP No.1				capacity of 5	65 psi.								
BOT CHORD	2x4 SP No.1			9)	Provide mech	nanical connection	(by oth	ers) of truss to	2					
BRACING					bearing plate	capable of withsta	nding 3	Ib uplift at joi	nt 1					
TOP CHORD	Structural wo	od shea	athing directly applied	lor	and 3 ib upin	at joint 3.								
	2-10-2 oc pu	rlins.		LO	AD CASE(S)	Standard								
BOT CHORD	Rigid ceiling	directly	applied or 10-0-0 oc											
	bracing.													
REACTIONS	(size) 1=	2-9-6, 3	=2-9-6											
	Max Horiz 1=	26 (LC	9)											
	Max Uplift 1=	-3 (LC 1	12), 3=-3 (LC 13)											
	Max Grav 1=	86 (LC	1), 3=86 (LC 1)											
FORCES	(lb) - Maximu	m Com	pression/Maximum											
	Tension													
TOP CHORD	1-2=-64/62, 2	2-3=-64/	74											
BOT CHORD	1-3=-6/36													
NOTES														
1) Unbalance	ed roof live load	ls have	been considered for											
this design	1. 	~~ I	(2)											
2) Wind: ASC	JE 7-16; Vult=1	30mph	(3-second gust)											
	n C: Enclosed:		SDL=0.0psi, n=15ii, S(0,0)											111 mars
Exterior/2	F = $C_{-}C_{-}C_{-}$ for	memb	ers and forces &	,									N' UCA	D
MWFRS fo	or reactions sho	wn I u	mber DOI =1 60 plate	Ż									THUT	NOI SI
arip DOL=	:1.60	, 20										5	Or Ass	in Alle
3) Truss desi	igned for wind le	oads in	the plane of the truss	3								32		· 7 ·
only. For:	studs exposed	to wind	(normal to the face),										1	North Internet
see Stand	ard Industry Ga	able End	Details as applicabl	e,									~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	. 1 1 - E
or consult	qualified buildir	ng desig	gner as per ANSI/TPI	1.							=		SEA	NL : =
Gable required	uires continuou	s bottor	n chord bearing.								=		286	77 : =
5) Gable stud	ds spaced at 0-	0-0 oc.									=		200	1 E E
6) This truss	has been desig	ned for	a 10.0 psf bottom										N	1 3
chord live	load nonconcul	rrent wit	in any other live load	5. 								2.0	. En	RINS
() This trus	tom chord in all		u a live load of 30.0p	51								2,0	O, GIN	EFICAT
3-06-00 ta	a ne boltom cindina in an aces while the a recidingle													
chord and	chord and any other members													
	,,												in an	mme.

- 4) 5) Gable studs spaced at 0-0-0 oc.
- This truss has been designed for a 10.0 psf bottom 6)
- chord live load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 30.0psf
- on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

818 Soundside Road Edenton, NC 27932

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mmm March 13,2025

