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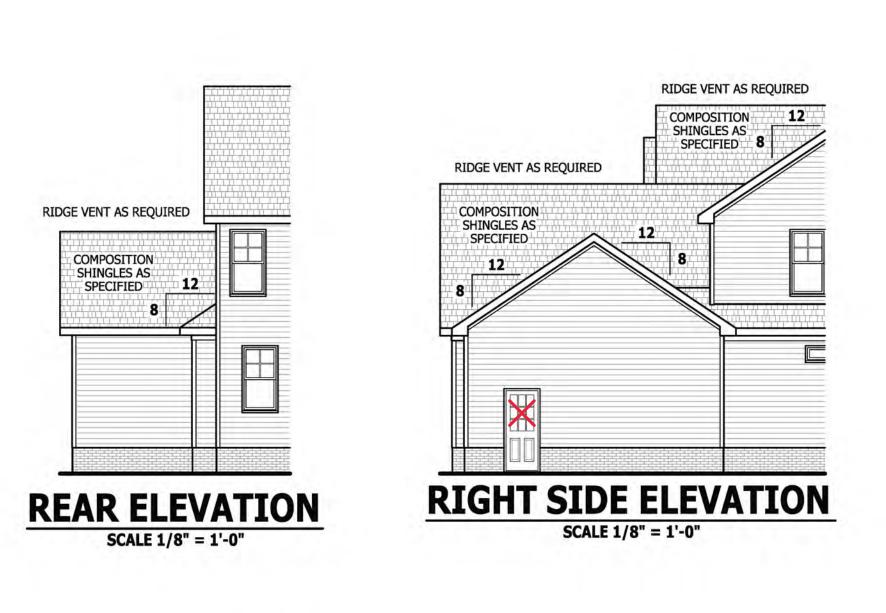
ELEVATIONS Mayview

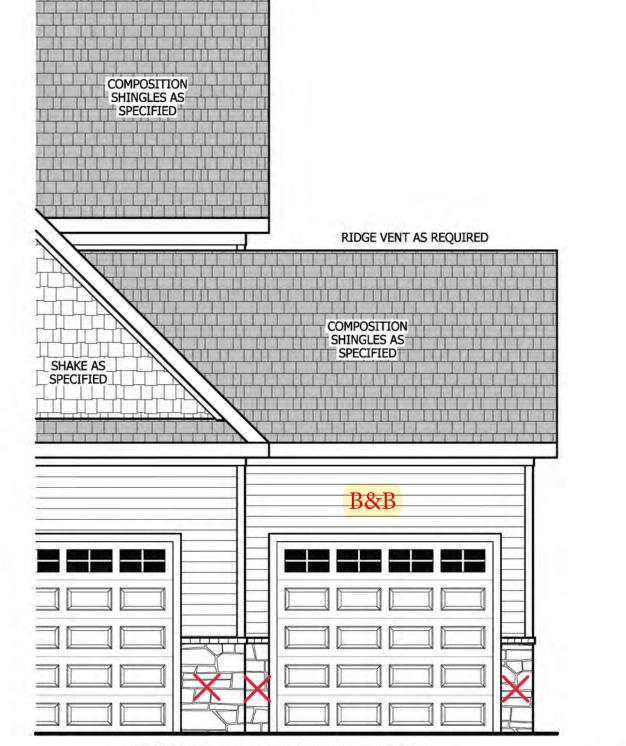
SQUARE FOOTAGE
HEATED
FIRST FLOOR 1395 SQ.FT.
SECOND FLOOR 1336 SQ.FT. UNHEATED GARAGE FRONT PORCH COVERED PORCH DECK/PATIO **UNHEATED OPTIONAL**

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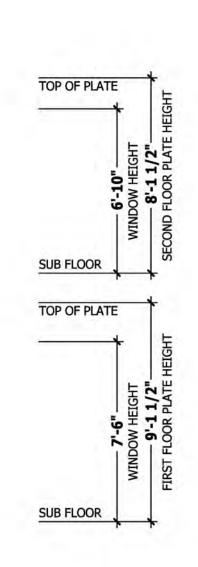
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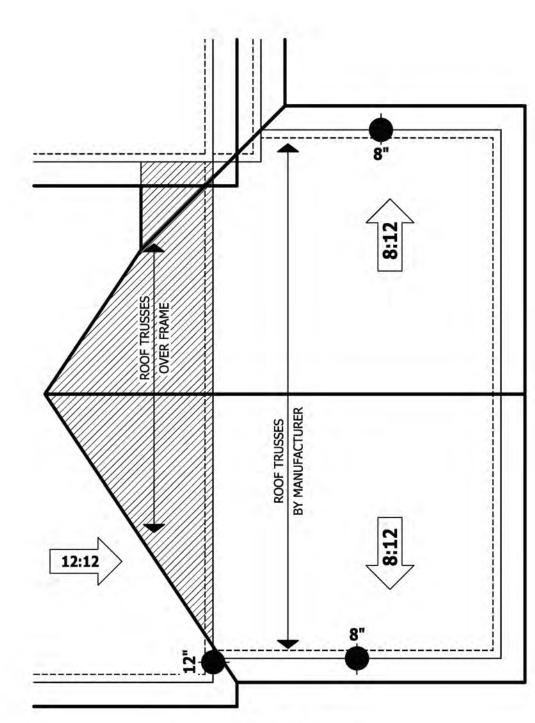
RIDGE VENT AS REQUIRED



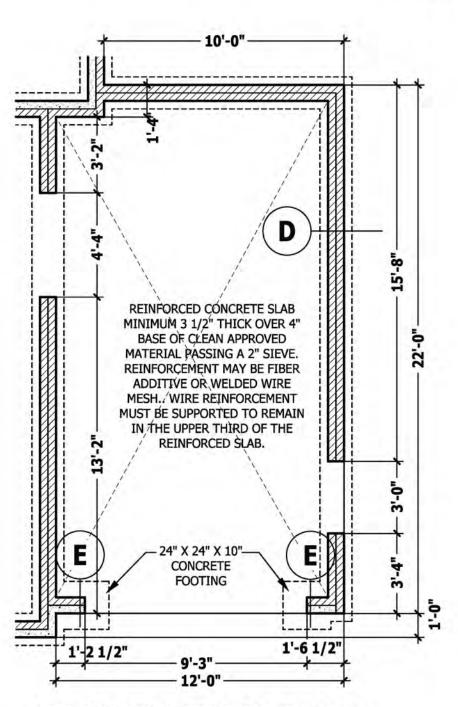
FRONT ELEVATION

SCALE 1/4" = 1'-0"

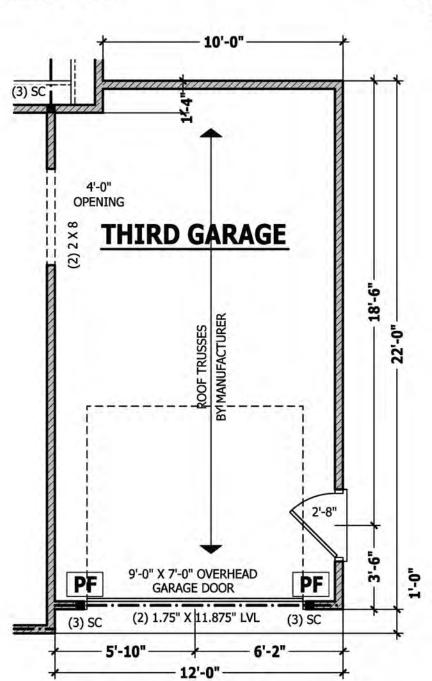
SEE BASE PLAN FOR STRUCTURAL NOTES **AND DETAILS**



ROOF PLAN SCALE 1/4" = 1'-0"



FOUNDATION PLAN SCALE 1/4" = 1'-0"



FIRST FLOOR PLAN SCALE 1/4" = 1'-0"

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GABLE

GARAGE

THIRD

Mayview

 SQUARE FOOTAGE

 HEATED
 1395 SQ.FT.

 FIRST FLOOR
 1395 SQ.FT.

 SECOND FLOOR
 1336 SQ.FT.

 TOTAL
 2731 SQ.FT.

 UNHEATED
 GARAGE
 547 SQ.FT.

 FRONT PORCH
 185 SQ.FT.

 COVERED PORCH
 185 SQ.FT.

 DECK/PATIO
 120 SQ.FT.

 STORAGE
 205 SQ.FT.

 TOTAL
 117; SQ.FT.
 GARAGE 547 SQ.FT.
FRONT PORCH 114 SQ.FT.
COVERED PORCH 185 SQ.FT.
DECK/PATIO 120 SQ.FT.
STORAGE 205 SQ.FT.
TOTAL 1171 SQ.FT.
UNHEATED OPTIONAL
THIRD GARAGE 261 SQ.FT.
TOTAL 261 SQ.FT.

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ADDENDUM

PURCHASER MUST VERIFY ALL
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AS SUCH SHALL REMAII PROPERTY OF THE DESIGN

EM WALL SLAB PLAI Mayview

SIGNATURE HOME BUILDERS, INC.

SQUARE FOOTAGE
HEATED
FIRST FLOOR 1395 SQ.FT.
SECOND FLOOR 1336 SQ.FT.
TOTAL 2731 SQ.FT.
UNHEATED
GARAGE 547 SQ.FT.
FRONT PORCH 114 SQ.FT.
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WALL THICKNESSES

stud face.

Exterior walls and walls adjacent to a garage area are drawn as 4" or as noted 2 X 6 are drawn as 6" to include 1/2" sheathing or gypsum. Subtract 1/2" for

Interior walls are drawn as 3 1/2" or as noted 2 X 6 are drawn as 5 1/2", and do not include gypsum.

DWELLING / GARAGE SEPARATION

WALLS. A minimum 1/2" gypsum board must be installed on all walls supporting

STAIRS. A minimum of 1/2" gypsum board must be installed on the underside and

a minimum of 5/8" type X gypsum board must be installed on the garage ceiling. **OPENING PENETRATIONS.** Openings between the garage and residence shall be equipped with solid wood doors not less than 1 3/8 inches (35 mm) in thickness, solid or honeycomb core steel doors not less than 1 3/8 inches (35 mm) thick, or 20-minute

DUCT PENETRATIONS. Ducts in the garage and ducts penetrating the walls or ceilings separating the *dwelling* from the garage shall be constructed of a minimum No. 26 gage (0.48 mm) sheet steel or other *approved* material and shall have no openings

OTHER PENETRATIONS. Penetrations through the separation required in Section

SQUARE FOOTAGE HEATED

UNHEATED OPTIONAL

FIRST FLOOR PLAN

SCALE 1/4" = 1'-0"

1395 SQ.FT. 1336 SQ.FT. 2731 SQ.FT.

547 SQ.FT. 114 SQ.FT. 185 SQ.FT. 120 SQ.FT. 205 SQ.FT. 1171 SQ.FT.

261 SQ.FT. 261 SQ.FT.

FIRST FLOOR SECOND FLOOR

UNHEATED

GARAGE FRONT PORCH COVERED PORCH

DECK/PATIO STORAGE

THIRD GARAGE

TOTAL

R302.6 shall be protected as required by Section R302.11, Item 4.

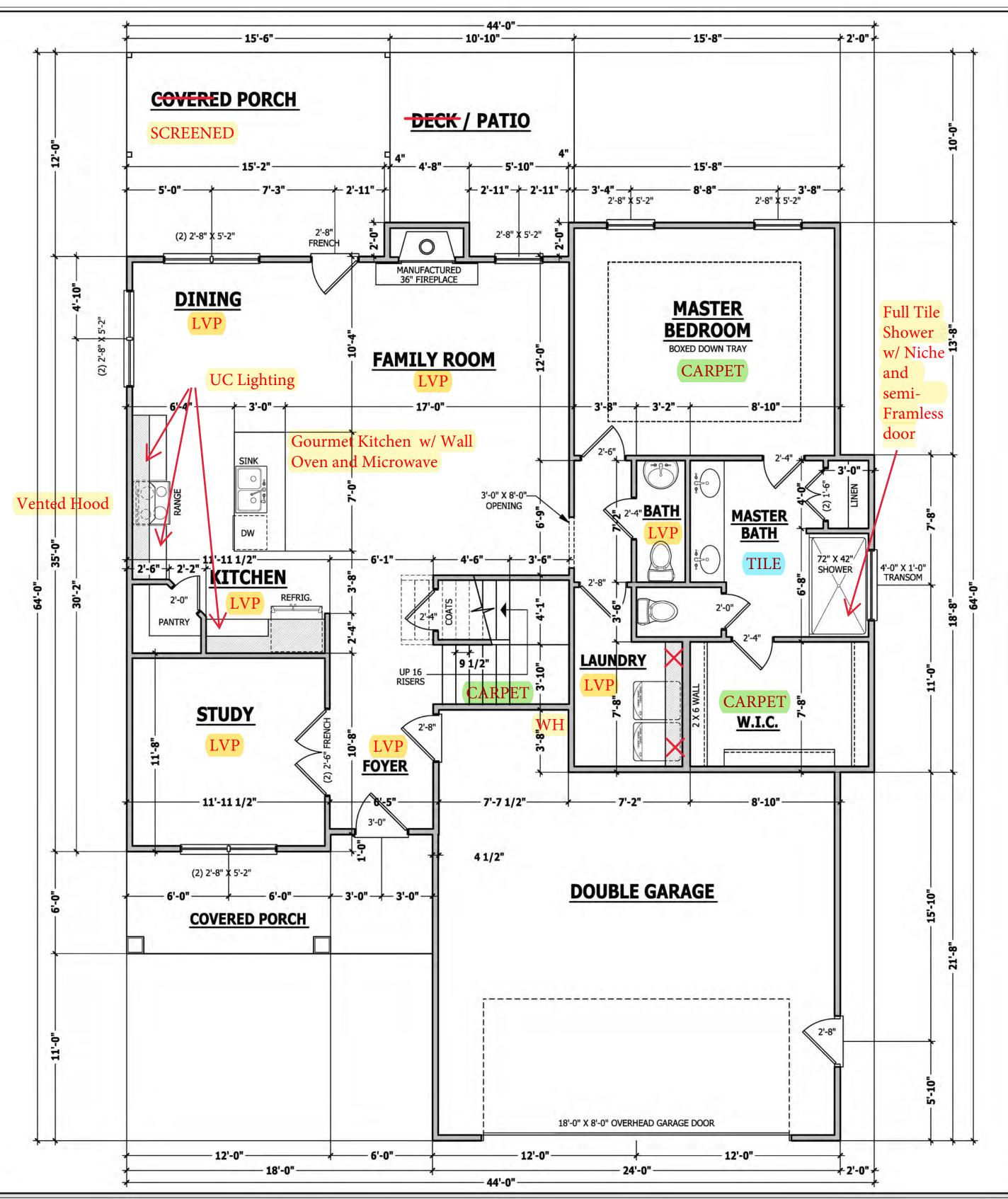
CEILINGS. A minimum of 1/2" gypsum must be installed on the garage ceiling if there are no habitable room above the garage. If there are habitable room above the garage

floor/ceiling assemblies used for separation required by this section.

REFER TO SECTIONS R302.5, R302.6, AND R302.7

exposed sides of all stairways.

fire-rated doors.



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ST FLOOR PLAN
Mayview

FIRST

SIGNATURE HOME BUILDERS, INC.

SQUARE FOOTAGE
HEATED
FIRST FLOOR 1395 SQ.FT.
SECOND FLOOR 1336 SQ.FT.
TOTAL 2731 SQ.FT.
UNHEATED
GARAGE 547 SQ.FT.
FRONT PORCH 114 SQ.FT.
COVERED PORCH 185 SQ.FT.
STORAGE 205 SQ.FT.
STORAGE 205 SQ.FT.
TOTAL 1171 SQ.FT.
UNHEATED OPTIONAL
THIRD GARAGE 261 SQ.FT.
TOTAL 261 SQ.FT.

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STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good

construction practic	e and	the	building (.oue.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	(242)	
Guardrail in-fill components	50	1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -	
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	1-12	L/360
Snow	20	1-24-1	

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

ROOF TRUSS REQUIREMENTS

CONCRETE AND SOILS: See foundation notes.

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

BRACE WALL PANEL

NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1. REQUIRED LENGTH OF BRACING: Required brace wall

length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

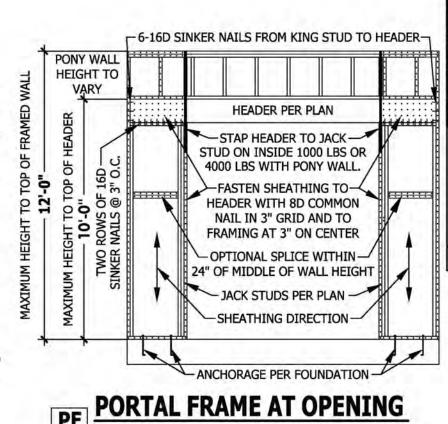
HD: 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

Methods Per Table R602.10.1

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter).

CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing nails.

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. **PF**: Portal fame per figure R602.10.1

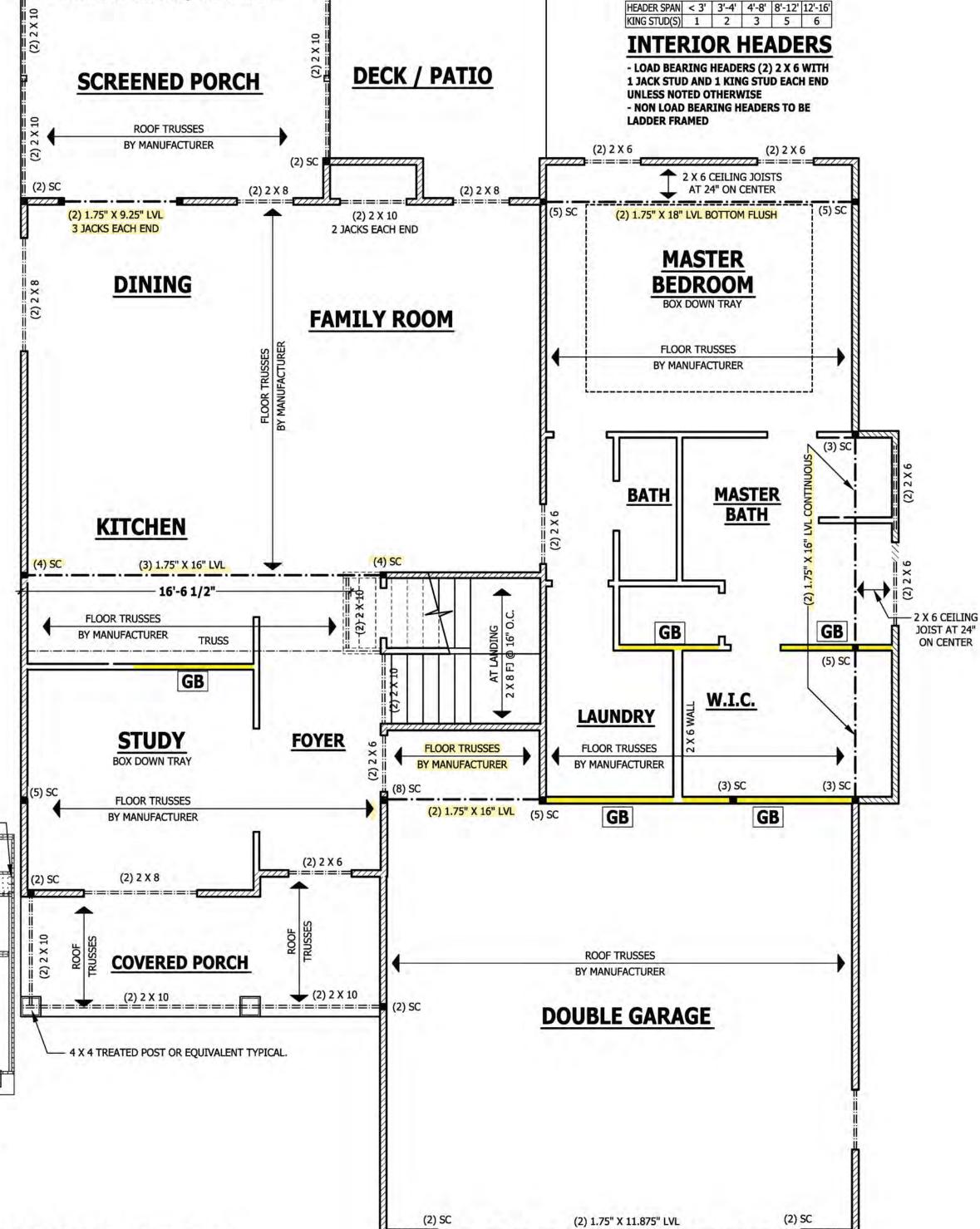


(METHOD PF PER FIGURE AND SECTION R602.10.1) SCALE 1/4" = 1'-0"

> FIRST FLOOR STRUCTURAL SCALE 1/4" = 1'-0"

(2) 2 X 10

4 X 4 TREATED POST OR EQUIVALENT TYPICAL.



PF

EXTERIOR HEADERS

- KING STUDS EACH END PER TABLE BELOW

- (2) 2 X 6 WITH 1 JACK STUD EACH END

UNLESS NOTED OTHERWISE

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STRUCTURAL

Mayview **FIRST**

FLOOR

SQUARE FOOTAGE HEATED FIRST FLOOR SECOND FLOOR TOTAL UNHEATED GARAGE FRONT PORCH COVERED PORCH DECK/PATIO STORAGE UNHEATED OPTIONAL

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PF

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LIVE LOAD	DEAD LOAD	DEFLECTION
(PSF)	(PSF)	(LL)
10		L/240
20	10	L/360
40	10	L/360
40	10	L/360
40	10	L/360
200		
50	94	
50	10	L/360
40	10	L/360
30	10	L/360
40		L/360
20		
	(PSF) 10 20 40 40 40 200 50 50 40 30 40	(PSF) (PSF) 10 10 20 10 40 10 40 10 200 50 50 10 40 10 30 10 40

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc.

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FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

CONCRETE AND SOILS: See foundation notes.

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

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ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

and floor system thicknesses.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

 - KING STUDS EACH END PER TABLE BELOW

 HEADER SPAN
 < 3'</td>
 3'-4'
 4'-8'
 8'-12'
 12'-16'

 KING STUD(S)
 1
 2
 3
 5
 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE
- NON LOAD BEARING HEADERS TO BE LADDER FRAMED

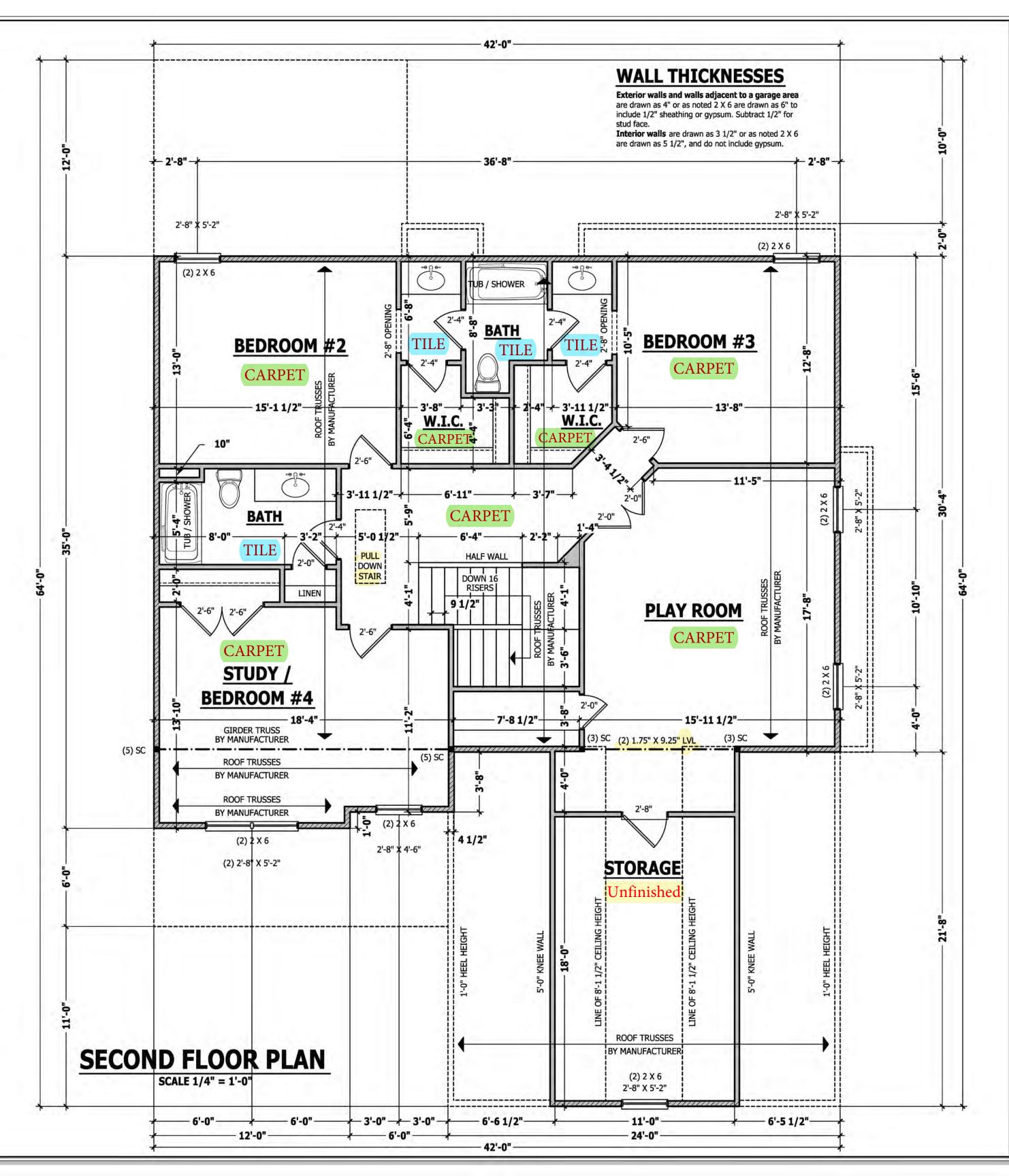
ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

- Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
- 2. Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.



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PLAN

Mayview

FLOOR

ECOND

SIGNATURE HOME BUILDERS, INC.

 SQUARE FOOTAGE

 HEATED
 1395 SQ.FT.

 FIRST FLOOR
 1395 SQ.FT.

 SECOND FLOOR
 1336 SQ.FT.

 TOTAL
 2731 SQ.FT.

 UNHEATED
 GARAGE

 GARAGE
 547 SQ.FT.

 FRONT PORCH
 114 SQ.FT.

 OECK/PATIO
 120 SQ.FT.

 STORAGE
 205 SQ.FT.

 TOTAL
 1171 SQ.FT.

 UNHEATED OPTIONAL
 THIRD GARAGE

 TOTAL
 261 SQ.FT.

 TOTAL
 261 SQ.FT.

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ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

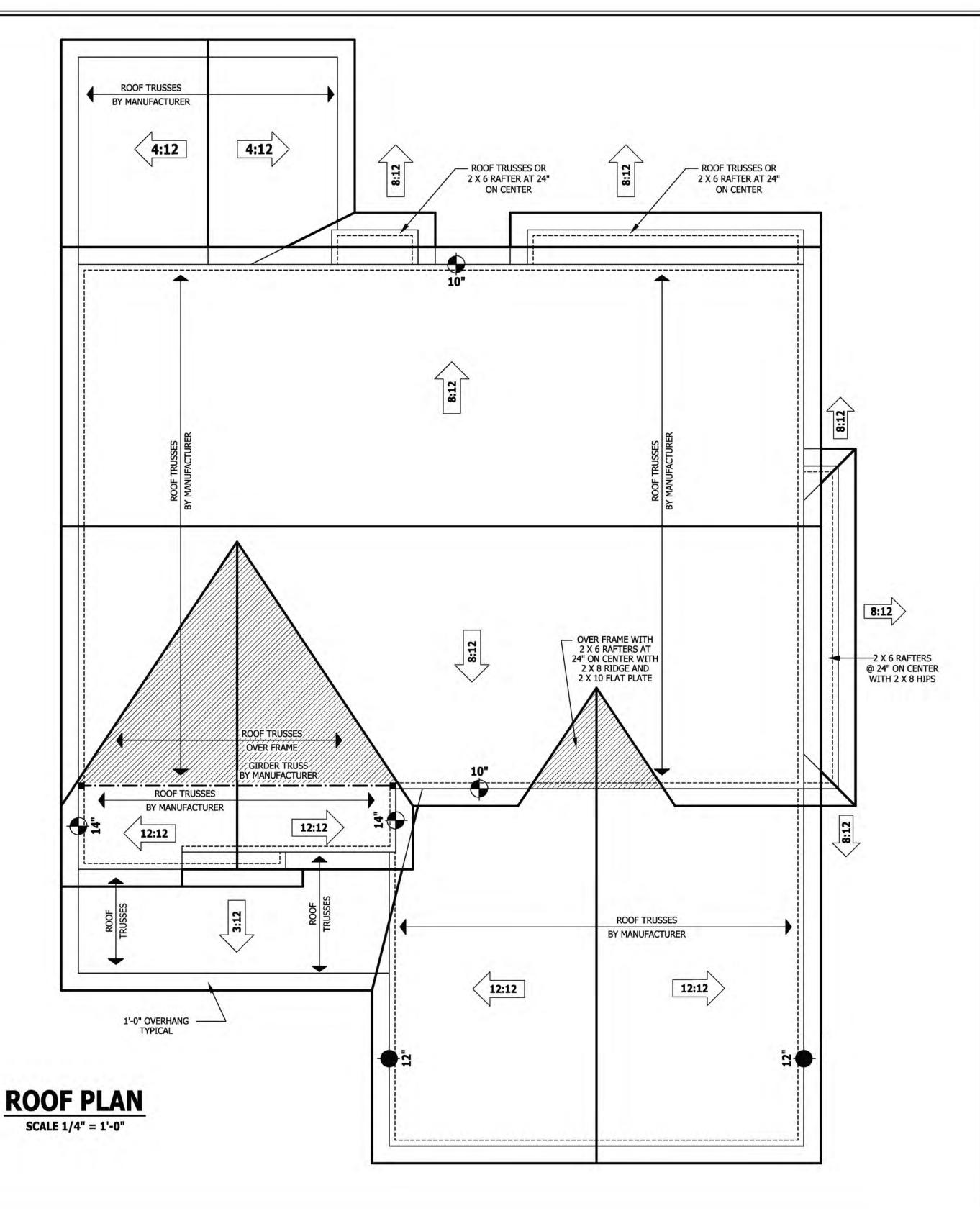
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Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE



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ROOF PLAN
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SIGNATURE HOME BUILDERS, INC.

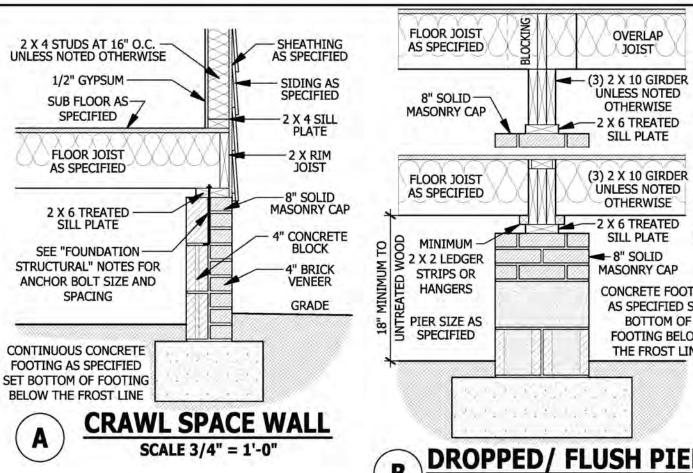


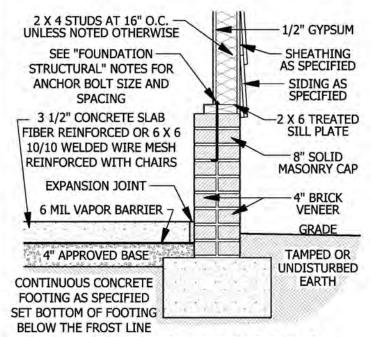
| SQUARE FOOTAGE | HEATED | FIRST FLOOR | 1395 SQ.FT. | SECOND FLOOR | 1336 SQ.FT. | TOTAL | 2731 SQ.FT. | UNHEATED | GARAGE | 547 SQ.FT. | FRONT PORCH | 114 SQ.FT. | COVERED PORCH | 185 SQ.FT. | STORAGE | 205 SQ.FT. | STORAGE | 205 SQ.FT. | TOTAL | 1171 SQ.FT. | UNHEATED OPTIONAL | THIRD GARAGE | 261 SQ.FT. | TOTAL | 261

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SCALE 3/4" = 1'-0"

GARAGE STEM WALL

DECK STAIR NOTES

SECTION AM110

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer. If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

DECK BRACING

see Chapter 45.

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to provide lateral stability.

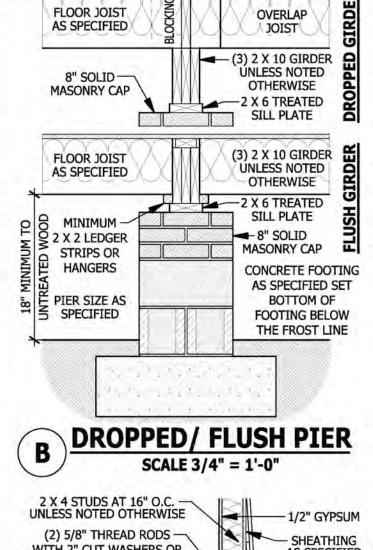
AM109.1.1. When the deck floor height is less than 4'-0" above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

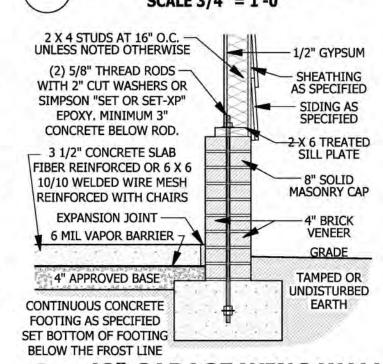
AM109.1.2. 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

AM109.1.3. For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2 and the following

POST SIZE	TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 X 4	48 SF	4'-0"	2'-6"	1'-0"
6 X 6	120 SF	6'-0"	3'-6"	1'-8"

AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109.3. AM109.1.5. For embedment of piles in Coastal Regions,





<48" GARAGE WING WALL E SCALE 3/4" = 1'-0"

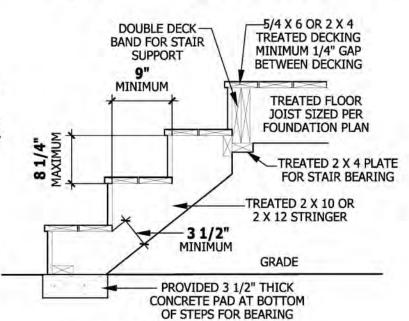


FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

WEEP SCREEDS

SHEATHING-STONE VEENER AS SPECIFIED AS SPECIFIED LATH-VAPOR BARRIER WEEP SCREED MINIMUM 4" TO **GROUND OR 2"** -TO PAVEMENT SEE FOUNDATION FOR FOUNDATION GRADE DETAILS

> **WEEP SCREED** SCALE 3/4" = 1'-0"

All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential Building code.

R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic weep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shall shall cover and terminate on the

attachment flange of the weep screed.

locations: 1. In each sleeping room. 2. Outside each separate sleeping area in the immediate vicinity of the bedrooms. On each additional story of the dwelling, including basements and habitable attics (finished) but not including crawl spaces,

2 X 4 STUDS AT 16" O.C.

UNLESS NOTED OTHERWISE

SUB FLOOR AS-

SPECIFIED

FLOOR JOIST

AS SPECIFIED

2 X 6 TREATED SILL PLATE

SEE "FOUNDATION

STRUCTURAL" NOTES FOR

ANCHOR BOLT SIZE AND

SPACING

CONTINUOUS CONCRETE

FOOTING AS SPECIFIED

SET BOTTOM OF FOOTING

BELOW THE FROST LINE

2 X TREATED-

HOUSE BAND

SUB FLOOR AS

SPECIFIED

AS SPECIFIED

2 X 6 TREATED-SILL PLATE

8" CONCRETE BLOCK

TAMPED OR UNDISTURBED

CRAWL SPACE AT GARGE

SCALE 3/4" = 1'-0"

-2 X 4 SOLE PLATE

FLASHING MINIMUM 16" WIDE

3 1/2" CONCRETE SLAB

SEE "FOUNDATION

STRUCTURAL" NOTES FOR

ANCHOR BOLT SIZE AND

(2) 4" CORRUGATED PIPES

8" X 16" CONCRETE BLOCKS

APPROVED FILL

CONTINUOUS CONCRETE.

FOOTING AS SPECIFIED

SET BOTTOM OF FOOTING

FILLED PORCH SECTION WITH VENT

WITH (2) 1/2" HOT-DIPPED GALVANIZED BOLTS

5/4 X 6 OR 2 X 4 TREATED-

GAP BETWEEN DECKING

FLASHING

FOUNDATION PLAN

ATTACH JOIST WITH HANGERS -

OR TREATED 2 X 2 LEDGER

5/8" HOT-DIPPED GALVANIZED

1/2" FROM EDGE WITH (3) 12c

GALVANIZED NAILS AT 6" O.O.

FOOTING SIZED PER-

FOUNDATION PLAN

SET BOTTOM OF FOOTING BELOW FROST LINE

SMOKE ALARMS

equipment provisions of NFPA 72.

requirements of Section R314.4.

NFPA 72.

DECK ATTACHMENT

SCALE 1/2" = 1'-0"

R314.1 Smoke detection and notification. All smoke alarms shall be

listed in accordance with UL 217 and installed in accordance with

R314.2 Smoke detection systems. Household fire alarm systems

a combination of smoke detector and audible notification device

installed as required by this section for smoke alarms, shall be

installed in accordance with NFPA 72 that include smoke alarms, or

permitted. The household fire alarm system shall provide the same

level of smoke detection and alarm as required by this section for

smoke alarms. Where a household fire warning system is installed

using a combination of smoke detector and audible notification

owned by the homeowner. The system shall be monitored by an

R314.3 Location. Smoke alarms shall be installed in the following

uninhabitable (unfinished) attics and uninhabitable (unfinished)

attic-stories. In dwellings or dwelling units with split levels and

overcurrent protection. Smoke alarms shall be interconnected.

Exception: Where smoke alarms are provided meeting the

approved supervising station and be maintained in accordance with

the provisions of this code and the household fire warning

- COBBLED BRICK FOR SLAB SUPPORT

TREATED GIRDER

AS SPECIFIED

GRADE

ROWLOCK

8 X 16 VENT

GRADE

without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level. When more than one smoke alarm is required to be installed within in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit. R314.4 Power source. Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall lap the attachment flange. The exterior lath receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for

—1/2" GYPSUM SEE ROOF EDGED OR PORCH FLOOR PLAN OR **ELEVATION** SHINGLES AS SPECIFIED PLATE FOR PITCH SHEATHING AS SPECIFIED -2 X RIM JOIST - 15# BUILDING FELT -8" SOLID MASONRY CAP **ROOF TRUSSES BY MANUFACTURER** 4" CONCRETE BLOCK PORCH HEADER PER 4" BRICK VENEER PLAN INSTALLED OVER EXPANSION JOINT CENTER OF COLUMN BASE - VINYL OR HARDIE SOFFIT -6 MIL VAPOR INSTALLED PER MANUFACTURERS **BLOCKING INSTALLED-**BARRIER INSTRUCTIONS ON BOTH SIDES & UNDER 3 1/2" SLAB HEADER AS DESIRED TAPERED COLUMN OVER 4" BASE MASONRY BASE 1 X MATERIAL -ATTACHED TO HEADER TAMPED OR CENTER LINE OF HEADER UNDISTURBED WITH POST CAP AND COLUMN

PORCH HEADER WITH TAPERED COLUMN

SCALE 3/4" = 1'-0"

CARBON MONOXIDE ALARMS

R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed by the alarm manufacturer

R315.2 Where required in existing dwellings. In existing dwellings, where interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

R315.3 Alarm requirements. The required carbon monoxide alarms shall be audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer's installation instructions.

STAIRWAY NOTES

R311.7.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stairway.

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of the adiacent treads.

R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater device(s), it shall become a permanent fixture of the occupancy and than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

> R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers. R311.7.7.1 Height. Handrail height, measured vertically from the sloped

plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm). Exceptions

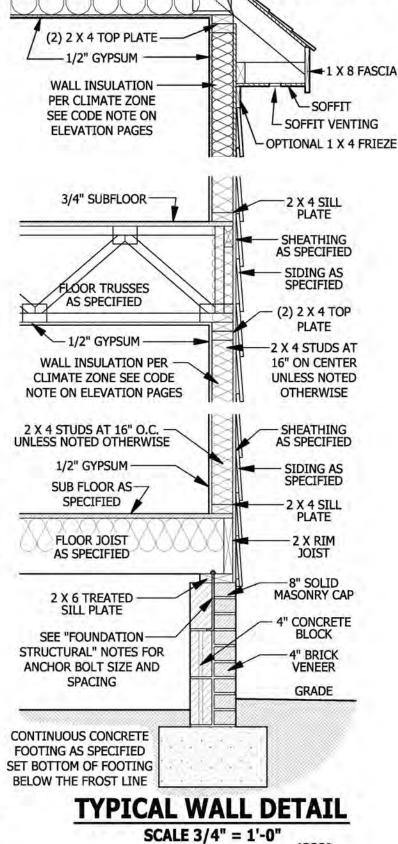
1. The use of a volute, turnout or starting easing shall be allowed over the lowest tread.

When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall be permitted to exceed the maximum height.

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails an individual dwelling unit the alarm devices shall be interconnected adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

1. Handrails shall be permitted to be interrupted by a newel post. 2. The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread.

3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.



PITCH PER ROOF PLAN

OR ELEVATIONS

ROOF INSULATION

PER CLIMATE ZONE

SEE CODE NOTE ON

ELEVATION PAGES

SHINGLES AS SPECIFIED

-15# BUILDING FELT

-SHEATHING AS SPECIFIED

INSULATION BAFFLE

TYPICAL STAIR DETAIL SCALE 1/4" = 1'-0"

CONTINUOUS HANDRAIL

34 TO 38 INCHES

ABOVE TREAD NOSING

MAXIMUM 6" GAP

BETWEEN WALL

MOUNTED AND

OPEN RAIL

11/9/2021

PAGE 7 OF 7

COVERED PORCH DECK/PATIO STORAGE UNHEATED OPTIONAL © Copyright 2019

SQUARE FOOTAGE HEATED

FIRST FLOOR SECOND FLOOR TOTAL UNHEATED

GARAGE FRONT PORCH

PURCHASER MUST VERIFY ALL

BEFORE CONSTRUCTION BEGINS

HAYNES HOME PLANS, INC.

ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND

CODES AND CONDITIONS MAY

DESIGNER ARCHITECT OR

IGINEER SHOULD BE CONSULTE

BEFORE CONSTRUCTION.

THESE DRAWING ARE

NSTRUMENTS OF SERVICE AND

AS SUCH SHALL REMAIN

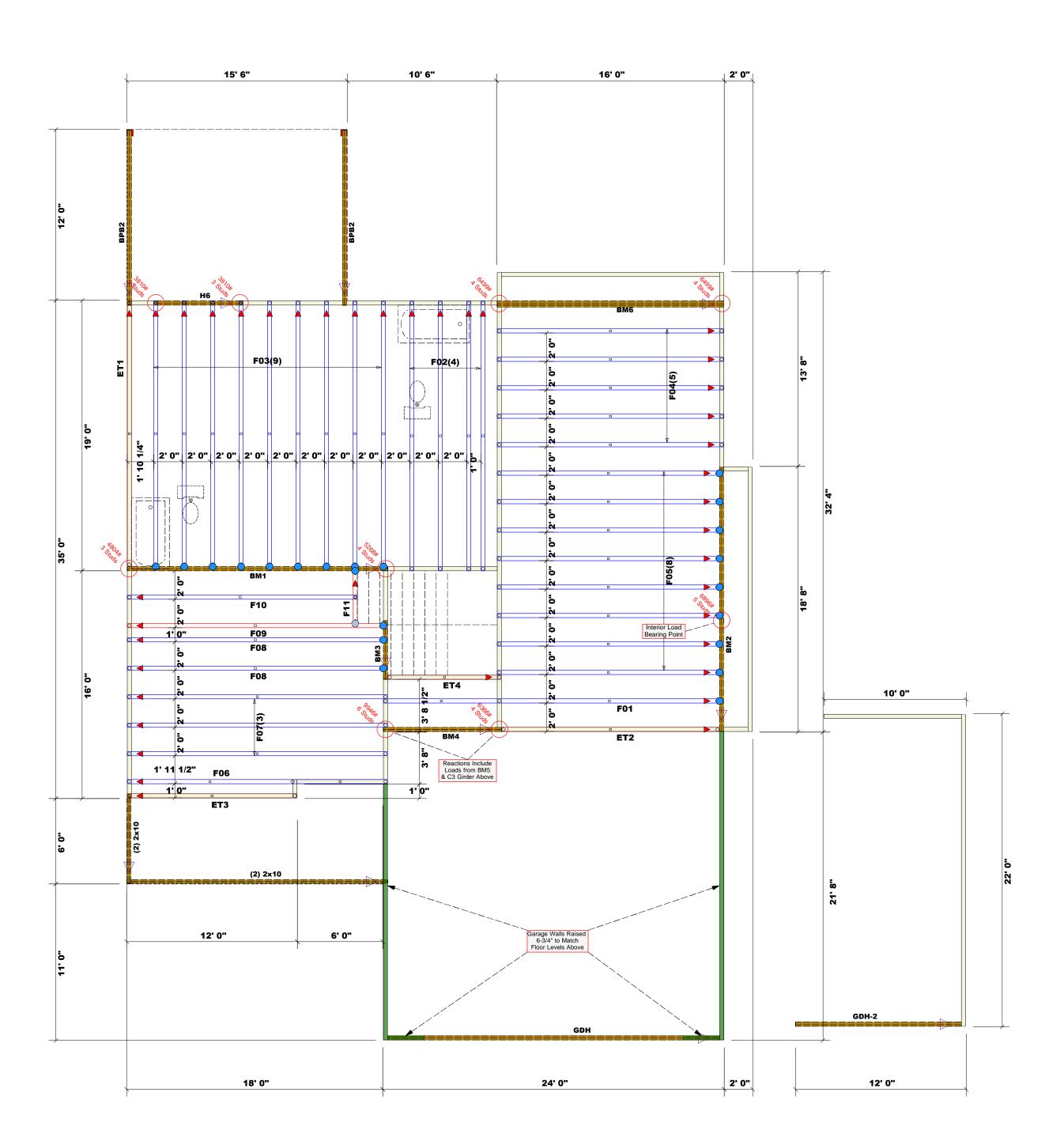
PROPERTY OF THE DESIGNER.

Mayview

DET

TYPICAL

Haynes Home Plans, Inc. 201222B





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

 Connector Information
 Nail Information

 Sym
 Product
 Manuf
 Qty
 Supported Member
 Header
 Truss

 HUS410
 USP
 22
 Varies
 16d/3-1/2"
 16d/3-1/2"

MSH422 USP 1 Varies 10d/3" 10d/3"

WALL SCHEDULE 1st Floor Brg. Wall 2nd Floor Brg. Wall Non-Bearing Walls

▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24°oc.

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of stud unless noted otherwise
3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Anthony Williams

LOAD CHART FOR JACK STUDS
(BASED ON TABLES R502.5(1) & (b))

	(R	ASED C	N LABLE	5 R502.	.5(1) & (I	((ر			
NU/	MBER C	OF JACI							
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR		
700	1		2550	1		3400	1		
3400	2		5100	2		6800	2		
5100	3		7650	3		10200	3		
6800	4		10200	4		13600	4		
3500	5		12750	5		17000	5		
0200	6		15300	6					
1900	7								
3600	8								
5300	9								
	NOTE OF THE PROPERTY OF THE PR	NUMBER C SUMBER C SUM	NUMBER OF JACK NUMBER OF JACK	NUMBER OF JACK STUDS F HEADER/I NUMBER OF JACK STUDS F HEADER/I	NUMBER OF JACK STUDS REQUIF HEADER/GIRDER NUMBER OF JACK STUDS REQU	NUMBER OF JACK STUDS REQUIRED @ E HEADER/GIRDER NUMBER OF JACK STUDS REQUIRED @ E HEA	Total Tota		

	CITY/County	City/County Angier / Harnett County
	ADDRESS	Lot 3 Mabry Ridge
_	MODEL	Floor
	DATE REV . 3/11/25	3/11/25
	DRAWN BY	DRAWN BY Anthony Williams
	SALESMAN	SALESMAN Anthony Williams

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

/201222B/

Mayview

PLAN

11/9/21

SEAL DATE

Z

QUOTE#

J0325-1351

JOB

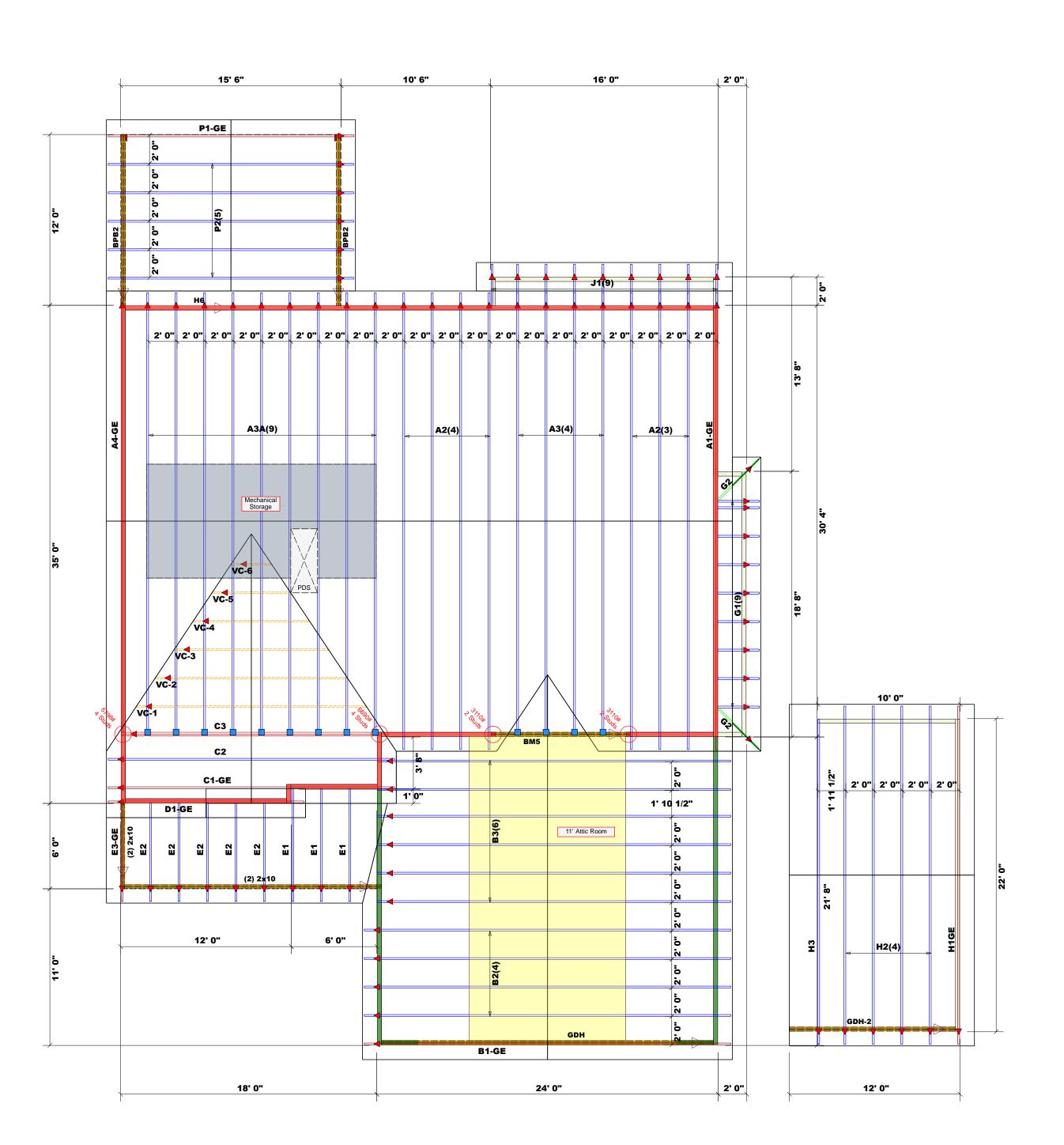
Signature Home Builders

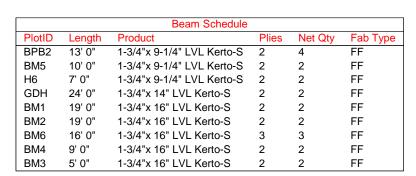
BUILDER

3 Mabry Ridge

JOB NAME

Truss Placement Plan SCALE: 3/16" = 1'-0"





All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Reaction / # of Studs

	Conne	ctor Info	rmati	ion	Nail Info	ormation
m	Product	Manuf	Qty	Supported Member	Header	Truss
7	HUS26	USP	13	Varies	16d/3-1/2"	16d/3-1/2"

WALL SCHEDULE

1st Floor Brg. Wall
2nd Floor Brg. Wall
Non-Bearing Walls

▲ = Indicates Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.
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3. All exterior wall to truss dimensions are to face of stud unless noted otherwise



Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Anthony Williams

LOAD CHART FOR JACK STUDS

	(B	ASED O	N TABLES	5 R502.	5(1) & (l	o))		
NUM	MBER C		STUDS REQUIRED @ EA END OF HEADER/GIRDER					
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER		END RE <i>AC</i> TION (UP TO)	REQ'D STUDS FOR (4) PLY HEADER	
1700	1		2550	1		3400	1	
3400	2		5100	2		6800	2	
5100	3		7650	3		10200	3	
6800	4		10200	4		13600	4	
8500	5		12750	5		17000	5	
10200	6		15300	6				
11900	7							
13600	8							
15300	9							

City/County	City/County Angier / Harnett County
ADDRESS	Lot 3 Mabry Ridge / Angier, NC
MODEL	Roof
DATE REV.	3/11/25
DRAWN BY	DRAWN BY Anthony Williams
SALESMAN	SALESMAN Anthony Williams

BUILDERSignature Home BuildersJOB NAMELot 3 Mabry RidgePLANMayview / 201222B / 3 CSEAL DATEPlan Date: 11/9/21QUOTE #NAJOB ##J0325-1350

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Truss Placement Plan SCALE: 3/16" = 1'-0"



Signature Home Builders

Project:

Address: Lot 3 Mabry Ridge / Angier, NC

Date: 3/11/2025

Input by: Anthony Williams

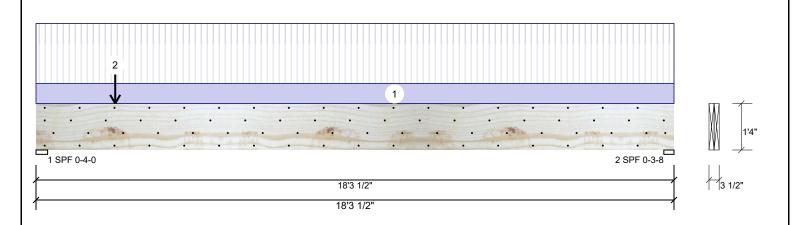
Job Name: Mayview Plan

Project #: J0325-1350 & 1351

Page 1 of 20

BM1 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Member Information				Rea	ctions UNP	ATTERN	NED lb (Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live	e Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	3863	1405	0	0	0
Moisture Condition	n: Dry	Building Code:	IBC 2012	2	Vertical	3516	1289	0	0	0
Deflection LL:	480	Load Sharing:	No							
Deflection TL:	360	Deck:	Not Checked							
Importance:	Normal - II									
Temperature:	Temp <= 100°F									
				Bea	rings					
				Bea	aring Length	Dir.	Cap. React D/L	lb Total	Ld. Case	Ld. Comb.
				1 -	SPF 4.000"	Vert	89% 1405 / 38	5268	L	D+L
				2 -	SPF 3.500"	Vert	92% 1289 / 35	16 4804	L	D+L

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	21179 ft-lb	9' 1/2"	34565 ft-lb	0.613 (61%)	D+L	L
Unbraced	21179 ft-lb	9' 1/2"	21265 ft-lb	0.996 (100%)	D+L	L
Shear	5095 lb	1'8"	11947 lb	0.426 (43%)	D+L	L
LL Defl inch	0.404 (L/529)	9'1 7/16"	0.445 (L/480)	0.908 (91%)	L	L
TL Defl inch	0.552 (L/387)	9'1 7/16"	0.594 (L/360)	0.930 (93%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at a maximum of 5'4 5/16" o.c.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width

/ Lateral diolide	inoco rado bacca en emgio	pry Widaii.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Far Face	127 PLF	380 PLF	0 PLF	0 PLF	0 PLF	F03	
2	Point	2-3-4		Near Face	143 lb	428 lb	0 lb	0 lb	0 lb	F11	
	Self Weight				12 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the responsibility of the customer and/or the contractor tensure the component suitability of the customer and/or the contractor tensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive 2. LVL not to be treated with fire retardant or corrosive 3. Design assumes top edge is laterally restrained 5. Provide lateral displacement and rotation 4. For flat roofs provide proper drainage to prevent ponding 4. Expert to menutacturer info Metsâ Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 Www.metsawood.com/us This design is valid until 6/28/2026



Signature Home Builders

Project: Address:

Lot 3 Mabry Ridge / Angier, NC

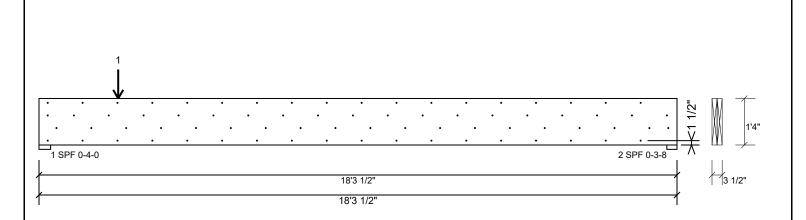
Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan

Page 2 of 20

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED BM₁

Project #: J0325-1350 & 1351 Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	77.4 %
Load	253.5 PLF
Yield Limit per Foot	327.4 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Infoculing & Installation

I. VIL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

(800) 622-5850 www.metsawood.com/us

Manufacturer Info



BM₂

Client:

Signature Home Builders

Project:

Address: Lot 3 Mabry Ridge / Angier, NC

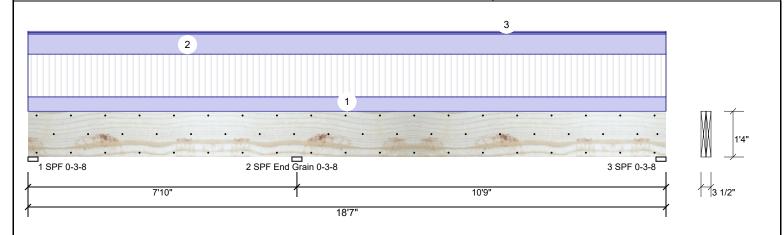
Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan

Page 3 of 20

Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Project #: J0325-1350 & 1351 Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Snow Wind Type: Floor Brg Direction Live Dead Const Plies: 2 Design Method: ASD 755 0 Vertical 859 27 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 3595 3158 113 0 0 Deflection LL: 480 Load Sharing: No 3 Vertical 1437 1262 45 0 0 Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1 - SPF 3.500" Vert 36% 716 / 1150 1865 L D+I 2 - SPF 3.500" Vert 61% 3225 / 3671 6896 LL D+I Analysis Results End Grain

3 - SPF 3.500"

ı	· ,						
	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Neg Moment	-6602 ft-lb	7'10"	34565 ft-lb	0.191 (19%)	D+L	LL
	Unbraced	-6602 ft-lb	7'10"	7396 ft-lb	0.893 (89%)	D+L	LL
	Pos Moment	5677 ft-lb	14'	34565 ft-lb	0.164 (16%)	D+L	_L
	Unbraced	5677 ft-lb	14'	7396 ft-lb	0.768 (77%)	D+L	_L
	Shear	2898 lh	9'3 3/4"	11947 lb	0.243 (24%)	D+I	11

LL Defl inch 0.034 (L/3685) 13'4 1/8" 0.263 (L/480) 0.130 (13%) L _L TL Defl inch 0.061 (L/2084) 13'4 13/16" 0.351 (L/360) 0.173 (17%) D+L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.

8 Lateral slenderness ratio based on single ply width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	106 PLF	317 PLF	0 PLF	0 PLF	0 PLF	F05	
2	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
3	Uniform			Тор	10 PLF	0 PLF	10 PLF	0 PLF	0 PLF	G1	
	Calf Maight				40 DLE						

1	Uniform	Тор	106 PLF	317 PLF	0 PLF	0 PLF	0 PLF	F05
2	Uniform	Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
3	Uniform	Тор	10 PLF	0 PLF	10 PLF	0 PLF	0 PLF	G1
	Self Weight		12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info

52%

Vert

1234 / 1496

2730 L

D+I

CSD DESIGN



Signature Home Builders

Project: Address:

Lot 3 Mabry Ridge / Angier, NC

Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan

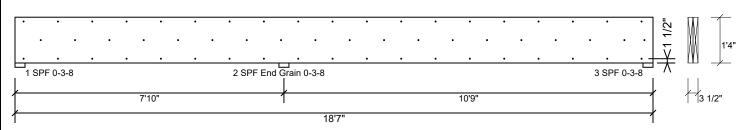
J0325-1350 & 1351

Project #: Level: Level

1.750" X 16.000" **BM2 Kerto-S LVL** 2-Ply - PASSED



Page 4 of 20



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1 00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Infoculing & Installation

I. VIL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

This design is valid until 6/28/2026

Manufacturer Info

www.metsawood.com/us



Signature Home Builders

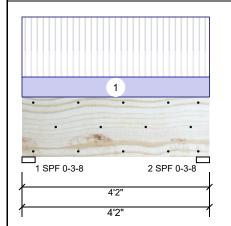
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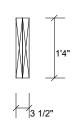
Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED BM₃

Level: Level





Wind

0

0

Const

0

0

Snow

n

0

Page 5 of 20

Member Information Reactions UNPATTERNED Ib (Uplift) Application: Direction Live Dead Type: Floor Brg Plies: Design Method: ASD 763 280 Vertical 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 763 280 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F

Bearings Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" D+L Vert 20% 280 / 763 1043 L 2 - SPF 3.500" Vert 20% 280 / 763 1043 L D+I

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	870 ft-lb	2'1"	34565 ft-lb	0.025 (3%)	D+L	L
Unbraced	870 ft-lb	2'1"	27947 ft-lb	0.031 (3%)	D+L	L
Shear	897 lb	2'6 1/2"	11947 lb	0.075 (8%)	D+L	L
LL Defl inch	0.002 (L/22654)	2'1 1/16"	0.093 (L/480)	0.021 (2%)	L	L
TL Defl inch	0.003 (L/16568)	2'1 1/16"	0.124 (L/360)	0.022 (2%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width

Lateral deliaciness ratio based on single ply water.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Near Face	122 PLF	366 PLF	0 PLF	0 PLF	0 PLF	F08	
	Self Weight				12 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us



Client: Project:

Signature Home Builders

Date: 3/11/2025 Input by:

Anthony Williams Job Name: Mayview Plan

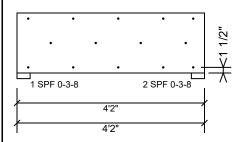
Address: Lot 3 Mabry Ridge / Angier, NC

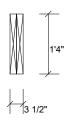
Project #: J0325-1350 & 1351 2-Ply - PASSED

Kerto-S LVL BM3

1.750" X 16.000"

Level: Level





Page 6 of 20

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

rasterial plies using 5 rows or roa box rialis (.120x5) a						
Capacity	99.4 %					
Load	244.0 PLF					
Yield Limit per Foot	245.6 PLF					
Yield Limit per Fastener	81.9 lb.					
См	1					
Yield Mode	IV					
Edge Distance	1 1/2"					
Min. End Distance	3"					
Load Combination	D+L					
Duration Factor	1.00					

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- - This design is valid until 6/28/2026

For flat roofs provide proper drainage to prevent ponding

(800) 622-5850 www.metsawood.com/us

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



BM4

Client:

Signature Home Builders

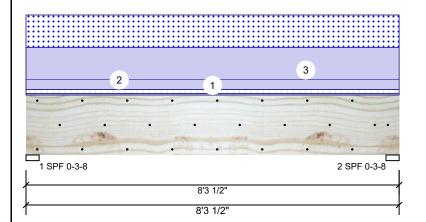
Project:

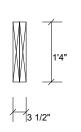
Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED

Level: Level





Page 7 of 20

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	166	1892	1364	0	0
2	Vertical	166	1892	1364	0	0

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	63%	1892 / 1364	3256	L	D+S
2 - SPF	3.500"	Vert	63%	1892 / 1364	3256	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6057 ft-lb	4'1 3/4"	39750 ft-lb	0.152 (15%)	D+S	L
Unbraced	6057 ft-lb	4'1 3/4"	15114 ft-lb	0.401 (40%)	D+S	L
Shear	1997 lb	1'7 1/2"	13739 lb	0.145 (15%)	D+S	L
LL Defl inch	0.017 (L/5541)	4'1 13/16"	0.196 (L/480)	0.087 (9%)	S	L
TL Defl inch	0.041 (L/2321)	4'1 13/16"	0.262 (L/360)	0.155 (16%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

-		3 1 7									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	15 PLF	40 PLF	0 PLF	0 PLF	0 PLF	FLOOR	
2	Uniform			Тор	100 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
3	Uniform			Тор	329 PLF	0 PLF	329 PLF	0 PLF	0 PLF	A2	
	Self Weight				12 PI F						

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us



BM4

Client:

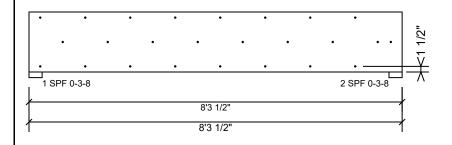
Signature Home Builders

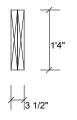
Project:

Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED Level: Level





Page 8 of 20

Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	/	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850

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www.metsawood.com/us

Manufacturer Info



BM₅

Client:

Signature Home Builders

Project:

Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Project #:

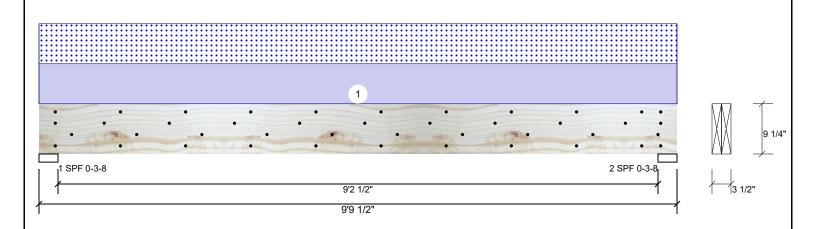
Input by: Anthony Williams Job Name: Mayview Plan

J0325-1350 & 1351

Page 9 of 20

Kerto-S LVL 2-Ply - PASSED 1.750" X 9.250"

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Direction Live Wind Type: Floor Brg Dead Snow Const Plies: 2 Design Method: ASD 0 1573 1537 0 Vertical 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 0 1573 1537 0 0 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. D+S 1 - SPF 3.500" Vert 1573 / 1537 3110 L 3.500" 2 - SPF Vert 60% 1573 / 1537 3110 L D+S

Analysis Results

Ī	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
	Moment	6917 ft-lb	4'10 3/4"	14423 ft-lb	0.480 (48%)	D+S	L
	Unbraced	6917 ft-lb	4'10 3/4"	7832 ft-lb	0.883 (88%)	D+S	L
	Shear	2925 lb	8'8 3/4"	7943 lb	0.368 (37%)	D+S	L
	LL Defl inch	0.128 (L/873)	4'10 3/4"	0.233 (L/480)	0.550 (55%)	S	L
	TL Defl inch	0.260 (L/432)	4'10 3/4"	0.311 (L/360)	0.834 (83%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	314 PLF	0 PLF	314 PLF	0 PLF	0 PLF	A3
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us



Signature Home Builders

Project:

Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

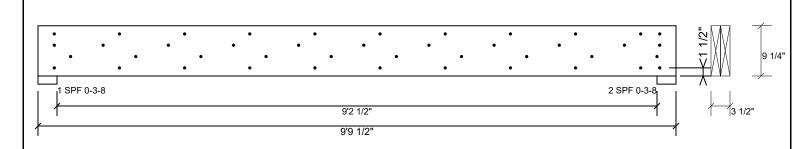
Input by: Anthony Williams Job Name: Mayview Plan

Page 10 of 20

Project #: J0325-1350 & 1351

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED BM₅

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	83.4 %
Load	314.0 PLF
Yield Limit per Foot	376.5 PLF
Yield Limit per Fastener	94.1 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+S
Duration Factor	1 15

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Infoculing & Installation

I. VIL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850 www.metsawood.com/us

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



Signature Home Builders

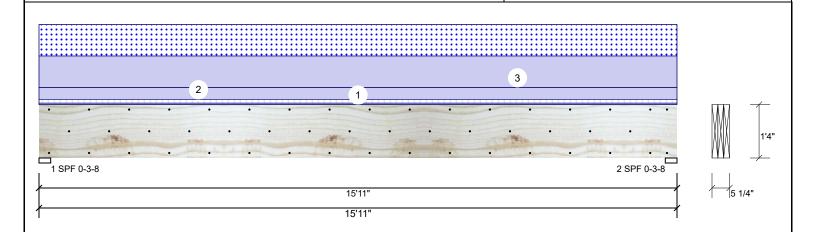
Project:

Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351 Page 11 of 20

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED BM6

Level: Level



Member Infor	mation			Rea	ctions UNP	ATTERI	NED Ib	(Uplift)			
Туре:	Girder	Application:	Floor	Brg	Direction	Live	е [Dead S	Snow	Wind	Const
Plies:	3	Design Method:	ASD	1	Vertical	318	В	3881	2618	0	0
Moisture Conditio	n: Dry	Building Code:	IBC 2012	2	Vertical	318	В	3881	2618	0	0
Deflection LL:	480	Load Sharing:	Yes								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal - II										
Temperature:	Temp <= 100°F			-							
				Bea	rings						
				Bea	aring Length	Dir.	Cap. R	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	SPF 3.500"	Vert	83%	3881 / 2618	6499	L	D+S
					SPF 3.500"	Vert	83%	3881 / 2618	6499	L	D+S

Analysis Results

ĺ	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
l	Moment	24460 ft-lb	7'11 1/2"	62010 ft-lb	0.394 (39%)	D+S	L
	Unbraced	24460 ft-lb	7'11 1/2"	24497 ft-lb	0.998 (100%)	D+S	L
l	Shear	5197 lb	1'7 1/2"	20608 lb	0.252 (25%)	D+S	L
l	LL Defl inch	0.132 (L/1406)	7'11 9/16"	0.387 (L/480)	0.341 (34%)	S	L
I	TL Defl inch	0.328 (L/567)	7'11 9/16"	0.516 (L/360)	0.635 (64%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". Nail from both sides.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 7'3 1/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

O Latera	al sicridefriess ratio based off	Single ply width.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	15 PLF	40 PLF	0 PLF	0 PLF	0 PLF	FLOOR	
2	Uniform			Тор	125 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
3	Uniform			Тор	329 PLF	0 PLF	329 PLF	0 PLF	0 PLF	A2	
	Self Weight				19 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

(800) 622-5850 www.metsawood.com/us



BM6

Client:

Signature Home Builders

Project: Address:

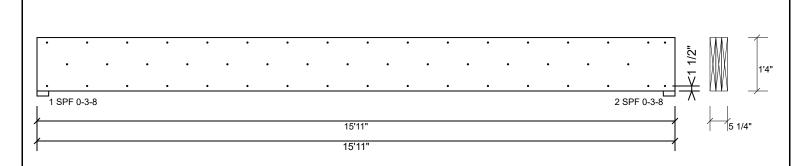
Lot 3 Mabry Ridge / Angier, NC

3/11/2025

Input by: Anthony Williams Page 12 of 20

Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 16.000" **Kerto-S LVL** 3-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Infoculing & Installation

I. VIL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

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GDH

Kerto-S LVL

Client:

Signature Home Builders

Project:

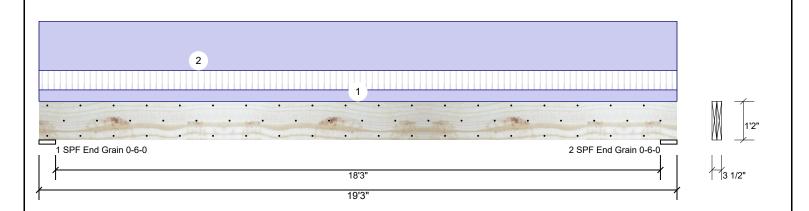
Address: Lot 3 Mabry Ridge / Angier, NC

Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan

Project #: 2-Ply - PASSED 1.750" X 14.000"

J0325-1350 & 1351 Level: Level



Member Information Application: Type: Plies: 2 Design Method: ASD Moisture Condition: Dry **Building Code:** IBC 2012 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	578	1885	0	0	0
2	Vertical	578	1885	0	0	0

Page 13 of 20

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10800 ft-lb	9'7 1/2"	26999 ft-lb	0.400 (40%)	D+L	L
Unbraced	10800 ft-lb	9'7 1/2"	10822 ft-lb	0.998 (100%)	D+L	L
Shear	2049 lb	1'8"	10453 lb	0.196 (20%)	D+L	L
LL Defl inch	0.102 (L/2160)	9'7 9/16"	0.459 (L/480)	0.222 (22%)	L	L
TL Defl inch	0.435 (L/506)	9'7 9/16"	0.612 (L/360)	0.711 (71%)	D+L	L

Bearings

Bearing	Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb
1 - SPF End Grain	6.000"	Vert	14%	1885 / 578	2463	L	D+L
2 - SPF End Grain	6.000"	Vert	14%	1885 / 577	2463	L	D+L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 9'11 5/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	35 PLF	60 PLF	0 PLF	0 PLF	0 PLF	F+4
2	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
	Self Weight				11 DI E					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code

approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Client: Project: Address:

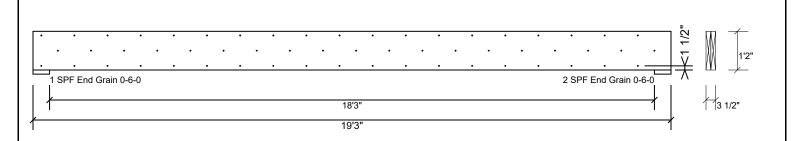
Signature Home Builders

Lot 3 Mabry Ridge / Angier, NC

3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351 Page 14 of 20

1.750" X 14.000" **GDH Kerto-S LVL** 2-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

, ,	•	•
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Infoculing & Installation

I. VIL beams must not be cut or drilled

Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

Damaged Beams must not be used

Design assumes top edge is laterally restrained

Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

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Manufacturer Info

CSD DESIGN



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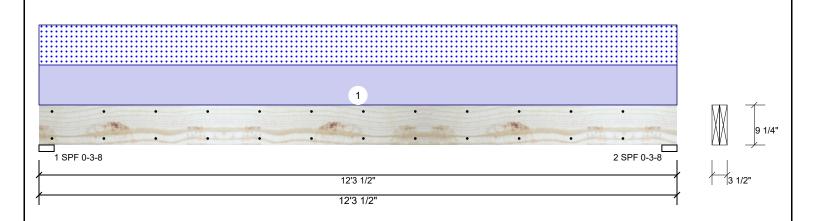
Project:

Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351 Page 15 of 20

1.750" X 9.250" **Kerto-S LVL** 2-Ply - PASSED BPB2

Level: Level



Member Info	rmation			Rea	ctions UNP	ATTER	NED Ib	(Uplift)			
Type:	Girder	Application:	Floor	Brg	Direction	Live	9	Dead	Snow	Wind	Const
Plies:	2	Design Method:	ASD	1	Vertical	(0	1058	1014	0	0
Moisture Condition	on: Dry	Building Code:	IBC 2012	2	Vertical		0	1058	1014	0	0
Deflection LL:	480	Load Sharing:	No								
Deflection TL:	360	Deck:	Not Checked								
Importance:	Normal - II										
Temperature:	Temp <= 100°F										
				Bea	rings						
				Bea	aring Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
				1 -	SPF 3.500"	Vert	40%	1058 / 1014	2072	L	D+S
					SPF 3.500"	Vert	40%	1058 / 1014	2072	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5902 ft-lb	6'1 3/4"	14423 ft-lb	0.409 (41%)	D+S	L
Unbraced	5902 ft-lb	6'1 3/4"	6421 ft-lb	0.919 (92%)	D+S	L
Shear	1720 lb	11'2 3/4"	7943 lb	0.217 (22%)	D+S	L
LL Defl inch	0.168 (L/845)	6'1 3/4"	0.296 (L/480)	0.568 (57%)	S	L
TL Defl inch	0.343 (L/414)	6'1 3/4"	0.394 (L/360)	0.870 (87%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	165 PLF	0 PLF	165 PLF	0 PLF	0 PLF	P2
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

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This design is valid until 6/28/2026



Signature Home Builders

Project:

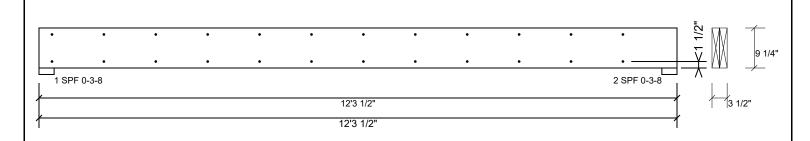
Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Anthony Williams

Page 16 of 20

Input by: Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 9.250" **Kerto-S LVL** 2-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	•	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Infoculing & Installation

 I. VIL beams must not be cut or drilled

 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used

 Design assumes top edge is laterally restrained

 Design assumes top edge is laterally restrained is provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

Manufacturer Info

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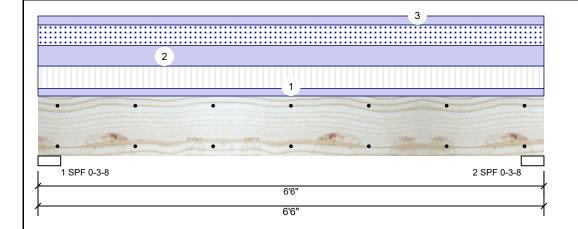
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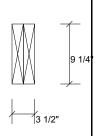
Address: Lot 3 Mabry Ridge / Angier, NC Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 9.250" Kerto-S LVL 2-Ply - PASSED **H6**

Level: Level





Page 17 of 20

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No

Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1229	2045	1125	0	0
2	Vertical	1229	2045	1125	0	0

Bearings

Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	73%	2045 / 1765	3810	L	D+0.75(L+S)
2 - SPF	3.500"	Vert	73%	2045 / 1765	3810	L	D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5348 ft-lb	3'3"	14423 ft-lb	0.371 (37%)	D+0.75(L+S)	L
Unbraced	5348 ft-lb	3'3"	10533 ft-lb	0.508 (51%)	D+0.75(L+S)	L
Shear	2570 lb	5'5 1/4"	7943 lb	0.324 (32%)	D+0.75(L+S)	L
LL Defl inch	0.044 (L/1645)	3'3"	0.151 (L/480)	0.292 (29%)	0.75(L+S)	L
TL Defl inch	0.095 (L/762)	3'3"	0.201 (L/360)	0.472 (47%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

ı	o Lateral Sieriae	These ratio based on single	pry wiatri.								
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
	1	Uniform			Тор	126 PLF	378 PLF	0 PLF	0 PLF	0 PLF	F03
	2	Uniform			Тор	346 PLF	0 PLF	346 PLF	0 PLF	0 PLF	A3A
	3	Uniform			Тор	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
		Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

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Client: Project:

Signature Home Builders

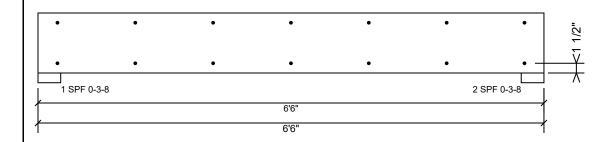
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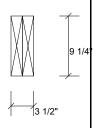
Input by: Anthony Williams

Address: Lot 3 Mabry Ridge / Angier, NC Job Name: Mayview Plan Project #: J0325-1350 & 1351

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED **H6**

Level: Level





Page 18 of 20

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



GDH-2

Client:

Signature Home Builders

Project: Address:

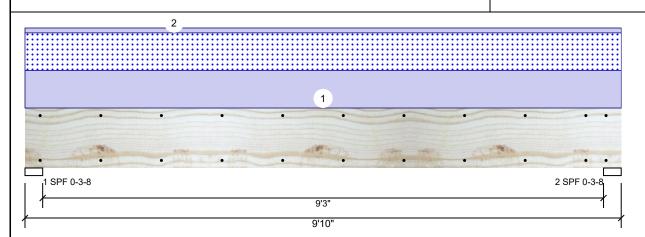
Lot 3 Mabry Ridge / Angier, NC

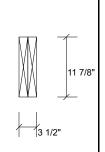
Date: 3/11/2025

Input by: Anthony Williams Job Name: Mayview Plan Project #: J0325-1350 & 1351

Level: Level

1.750" X 11.875" **Kerto-S LVL** 2-Ply - PASSED





Page 19 of 20

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012

Load Sharing: No

Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1338	1146	0	0
2	Vertical	0	1338	1146	0	0

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	48%	1338 / 1146	2484	L	D+S
2 - SPF	3.500"	Vert	48%	1338 / 1146	2484	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5551 ft-lb	4'11"	22897 ft-lb	0.242 (24%)	D+S	L
Unbraced	5551 ft-lb	4'11"	9857 ft-lb	0.563 (56%)	D+S	L
Shear	1846 lb	8'6 5/8"	10197 lb	0.181 (18%)	D+S	L
LL Defl inch	0.049 (L/2317)	4'11"	0.234 (L/480)	0.207 (21%)	S	L
TL Defl inch	0.105 (L/1069)	4'11"	0.312 (L/360)	0.337 (34%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	233 PLF	0 PLF	233 PLF	0 PLF	0 PLF	H2
2	Uniform			Тор	30 PLF	0 PLF	0 PLF	0 PLF	0 PLF	wall
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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Signature Home Builders

Project:

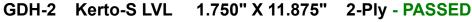
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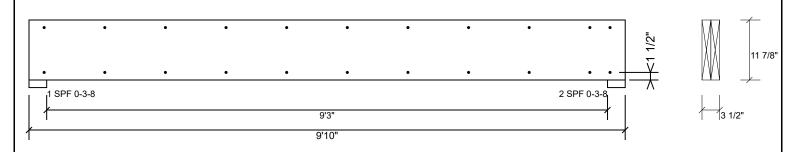
Input by: Anthony Williams Page 20 of 20

Job Name: Mayview Plan Project #: J0325-1350 & 1351

1.750" X 11.875" **Kerto-S LVL**

Level: Level





Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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