\* "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION \*\* INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL

DESIGNED FOR WIND SPEED OF 120 MPH 3 SECOND CHST (03 EASTEST MILE) EXPOSURE "R"

DESIGNED LOK MIN	D SPLLD	OI 120 I'II	11, 3 3LCC	וכטט עוול	(3) I ASI	LOT PILLE	LAFUSUR	LD
COMPONENT	% CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING I	OADS
MEAN ROOF	UP T	O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
70NF 5	15.5	-20.0	16.3	-21 0	16 9	-21 8	17 4	-22 4

DESIGNED FOR WIND SPEED OF 130 MPH, 3 SECOND GUST (101 FASTEST MILE) EXPOSURE "B MEAN ROOF UP TO 30' 30'-1" TO 35' 35'-1" TO 40' 40'-1" TO 45' ZONE 1 16.7 -18.0 17.5 -18.9 18.2 -19.6 18.7 -20.2 ZONE 2 16.7 -21.0 17.5 -22.1 18.2 -22.9 18.7 -23.5 ZONE 3 16.7 -21.0 17.5 -22.1 18.2 -22.9 18.7 -23.5 ZONE 4 18.2 -19.0 19.1 -20.0 19.8 -20.7 20.4 -21.. ZONE 5 18.2 -24.0 19.1 -25.2 19.8 -26.2 20.4 -26.0

## **ROOF VENTILATION**

### **SECTION R806**

**R806.1 Ventilation required.** Enclosed *attics* and enclosed rafter spaces formed where ceilings are applied directly to the underside of roof rafters shall have cross ventilation for each separate space by ventilating openings protected against the entrance of rain or snow. Ventilation openings shall have a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Ventilation openings having a least dimension larger than 1/4 inch (6.4 mm) shall be provided with corrosion-resistant wire cloth screening, hardware cloth, or similar material with openings having a least dimension of 1/16 inch (1.6 mm) minimum and 1/4 inch (6.4 mm) maximum. Openings in roof framing members shall conform to the requirements of Section R802.7.

**R806.2 Minimum area.** The total net free ventilating area shall not be less than 1/150 of the area of the space ventilated except that reduction of the total area to 1/300 is permitted provided that at least 50 percent and not more than 80 percent of the required ventilating area is provided by ventilators located in the upper portion of the space to be ventilated at least 3 feet (914 mm) above the eave or cornice vents with the balance of the required ventilation provided by eave or cornice vents. As an alternative, the net free cross-ventilation area may be reduced to 1/300 when a Class I or II

1. Enclosed attic/rafter spaces requiring less than 1 square foot (0.0929 m2) of ventilation may be vented with continuous soffit ventilation only. 2. Enclosed attic/rafter spaces over unconditioned space may be vented with continuous soffit vent only.

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,619 SQ.FT.

NET FREE CROSS VENTILATION NEEDED:

WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 17.46 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.73 SO.FT.

## **GUARD RAIL NOTES**

**R312.1 Where required.** *Guards* shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a guard.

R312.2 Height. Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

1. *Guards* on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads.

2. Where the top of the *guard* also serves as a handrail on the open sides of stairs, the top of the *guard* shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

**R312.3 Opening limitations.** Required *guards* shall not have openings from the walking surface to the required *guard* height which allow passage of a sphere 4 inches (102 mm)in diameter.

## **Exceptions:**

1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a *guard*, shall not allow passage of a sphere 6 inches (153

2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 43/8 inches (111 mm) in diameter.

Lot 1 Mabry Ridge- 29 Croatoke Court Angier NC

Front Door- 3/4 View-Stained

Gutters- Bronze

Columns- 8" Square Cedar- Stained

Garage Door- Recessed Panel w/ Glass- Stained

\_ SIDING AS-

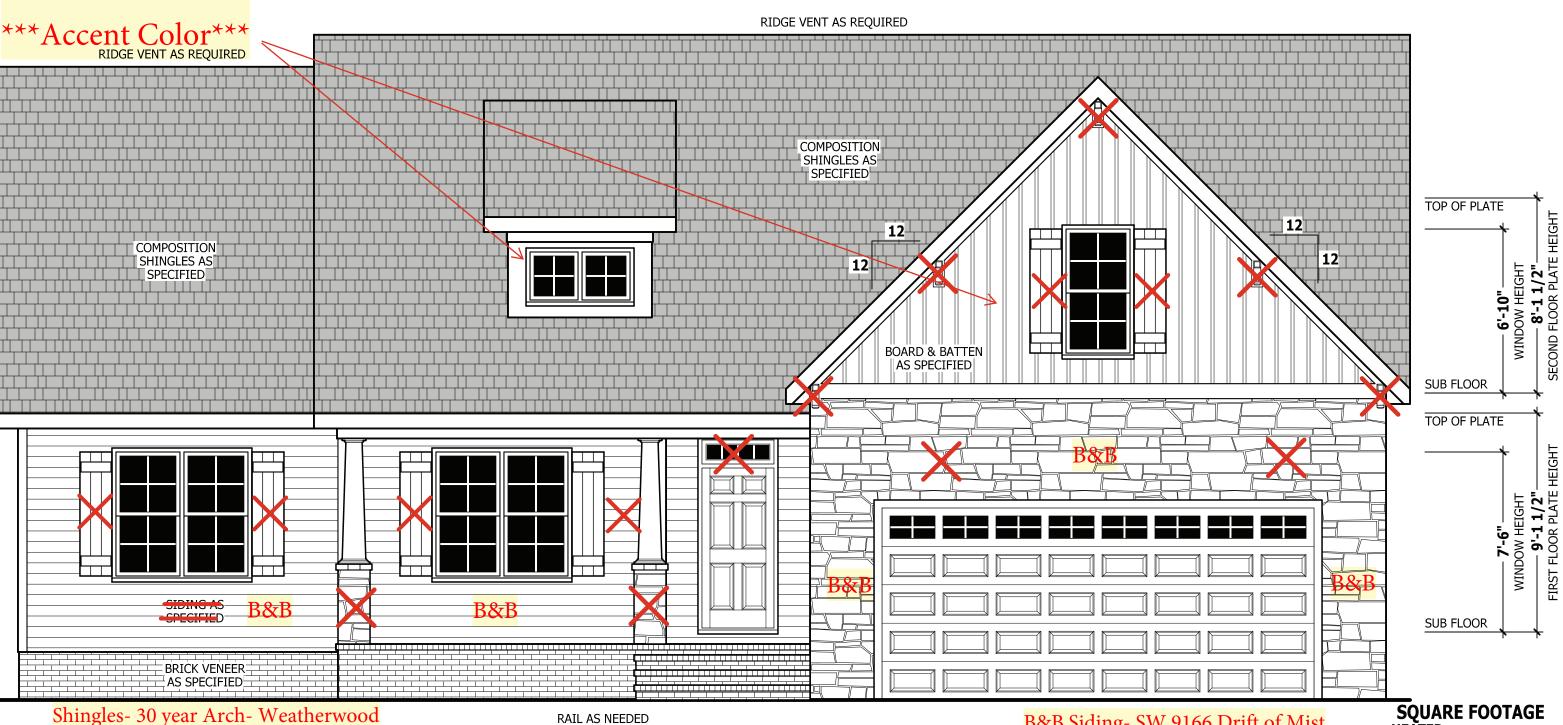
BRICK VENEER

AS SPECIFIED

\*\*\*Screened Rear Porch\*\*\*

SPECIFIED-

\*\*\*Brick Stemwall- Bessemer Gray\*\*\*



\*\*\*No Shutters Front FRONT ELEVATION

B&B Siding- SW 7100 Pine

Horizontal Siding- SW 9166 Drift of Mist Accent Color- SW 7043 Worldly Gray

get 1""x 4" MiraTEC

SCALE 1/4" = 1'-0"

Windows- MGM SH 4 over 1- White

Trim- White

RIDGE VENT AS REQUIRED

HEĂTED

FIRST FLOOR PLAYROOM 1880 SQ.FT. 307 SQ.FT. 2187 SQ.FT. **UNHEATED** SCREENED PORCH

TOP OF PLATE

SUB FLOOR

SUB FLOOR

Harnett

TOP OF PLATE

**6'-** WINDOW

153 SQ.FT. 501 SQ.FT. 218 SQ.FT. 872 SQ.FT. UNHEATED OPTIONAL

264 SQ.FT. 264 SQ.FT.

SQUARE FOOTAGE 1880 SQ.FT. 307 SQ.FT. 2187 SQ.FT. TOTAL UNHEATED

UNHEATED OPTIONAL

191021B

Rear Door- Full View Clear- White

RAIL AS NEEDED

**SCALE 1/4" = 1'-0"** 

**REAR ELEVATION** 

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**ELEVATIONS** 

SINCLAIR REAR **∞ FRONT** 

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**ELEVATIONS** 

SINCLAIR **RIGHT** 

8 LEFT

 SQUARE FOOTAGE

 HEATED
 1880 SQ.FT.

 FIRST FLOOR
 1880 SQ.FT.

 PLAYROOM
 307 SQ.FT.

 TOTAL
 2187 SQ.FT.

 UNHEATED
 FRONT PORCH
 153 SQ.FT.

 GARAGE
 501 SQ.FT.

 SCREENED PORCH
 218 SQ.FT.

 TOTAL
 872 SQ.FT.

 UNHEATED OPTIONAL

 THIRD GARAGE
 264 SQ.FT.

 TOTAL
 264 SQ.FT.

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**SLAB** SINCL WALL EΜ

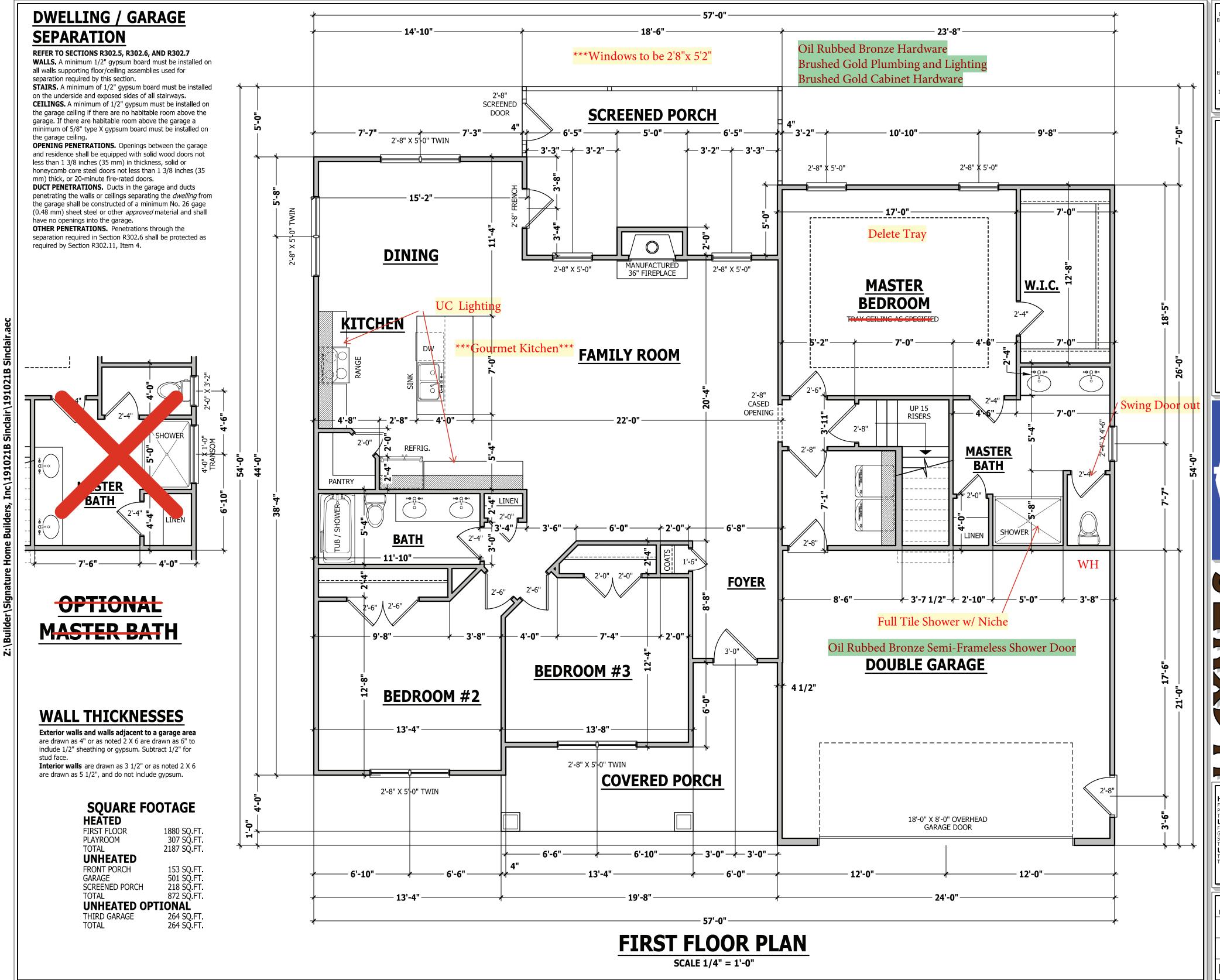
**SQUARE FOOTAGE** 1880 SQ.FT. 307 SQ.FT. 2187 SQ.FT. TOTAL UNHEATED UNHEATED OPTIONAL

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FIRST FLOOR PLAN
SINCLAIR

SIGNATURE HOME BUILDERS, INC.

HOWE PLANS, INC. 7588 919-478-6180

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PAGE 4 OF 8

out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good

construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		
Guardrail in-fill components	50		
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

**FRAMING LUMBER:** All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise. **ENGINEERED WOOD BEAMS:** 

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. **LINTELS:** Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for

16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing. **ROOF SHEATHING:** OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. **CONCRETE AND SOILS:** See foundation notes.

## **BRACE WALL PANEL NOTES**

**EXTERIOR WALLS:** All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

**GYPSUM:** All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. Winden not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. actual length

Method PF contributes 1.5 times its actual length. **HD:** 800 lbs hold down hold down device fastened to the edge of the brace wall panel closets to the corner.

## **Methods** Per Table R602.10.1

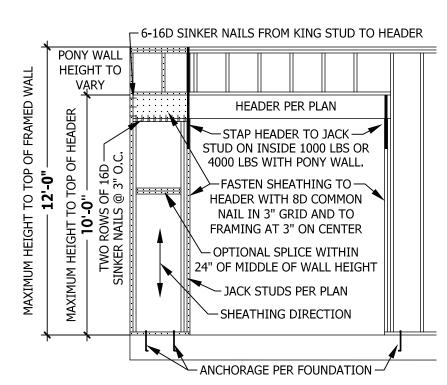
Inc\191021B Sinclair\191021B Sinclair.aec

Z:\Builder\Signature Home

CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). **CS-SFB:** Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

**GB:** Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws.

**PF**: Portal fame per figure R602.10.1



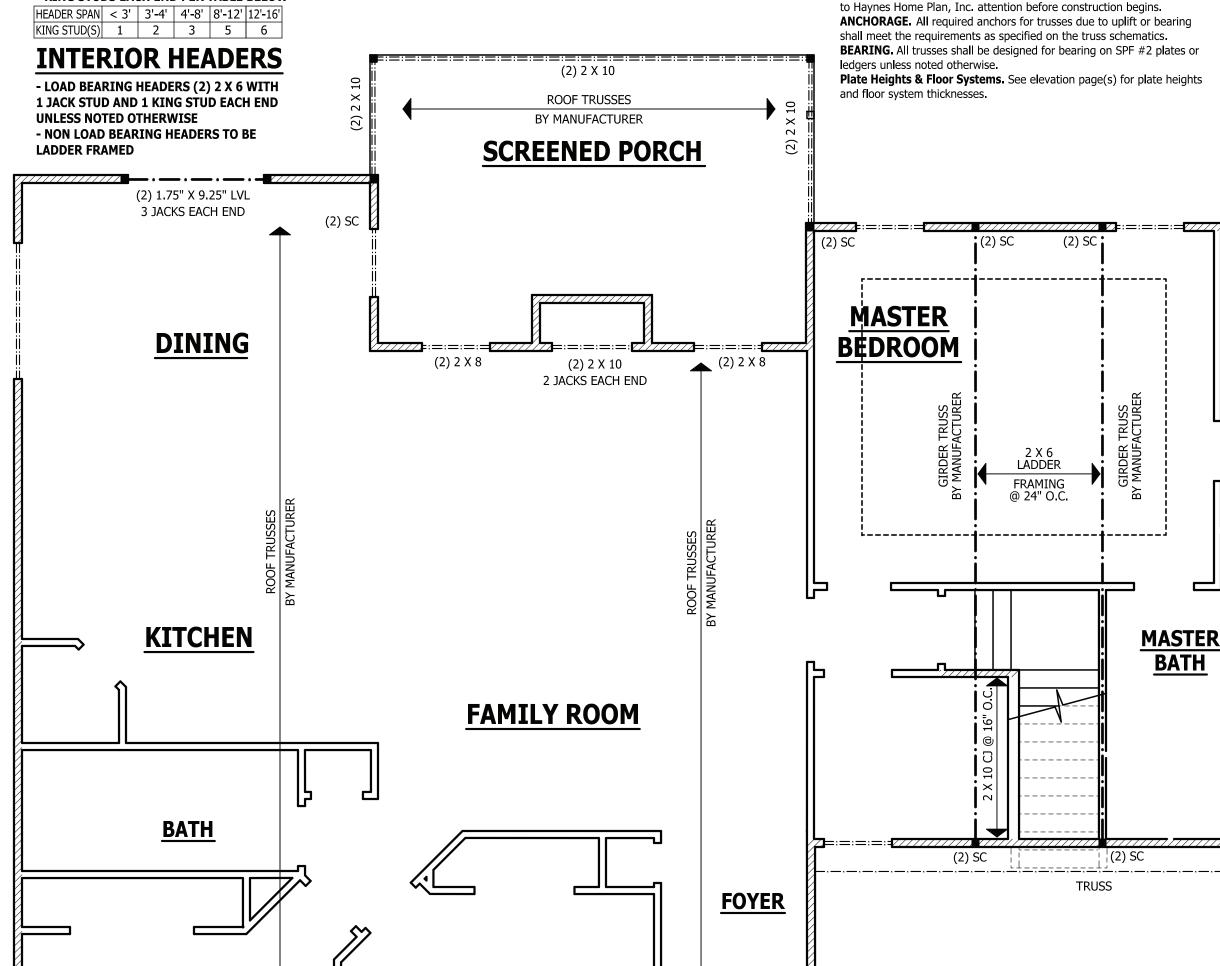
## PORTAL FRAME AT OPENING

( METHOD PF PER FIGURE AND SECTION R602.10.1 ) SCALE 1/4" = 1'-0"

## **EXTERIOR HEADERS ROOF TRUSS REQUIREMENTS**

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW

HEADER SPAN < 3' | 3'-4' | 4'-8' | 8'-12' | 12'-16'



**DOUBLE GARAGE** BEDROOM #3 (2) 2 X 8

(2) 1.75" X 9.25" LVL 3 JACKS EACH END (2) 1.75" X 9.25" LVL 3 JACKS EACH END

BEDROOM #2

(2) 2 X 10 PLACE BEAM OVER BEARING PROVIDED BY COLUMN(S) AND FURR BEAM AS DESIRED

**COVERED PORCH** 

FIRST FLOOR STRUCTURAL

SCALE 1/4" = 1'-0"

(2) SC

**ROOF TRUSSES** 

BY MANUFACTURER

(2) 1.75" X 11.875" LVL

**TRUSS DESIGN.** Trusses to be designed and engineered in accordance

with these drawings. Any variation with these drawings must be brought

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> STRUCTURAL FLOOR

SINCLAIR **FIRST** 

W.I.C.

(2) SC

PF



1880 SQ.FT 307 SQ.FT 2187 SQ.FT UNHEATED GARAGE SCREENED PORCH UNHEATED OPTIONAL

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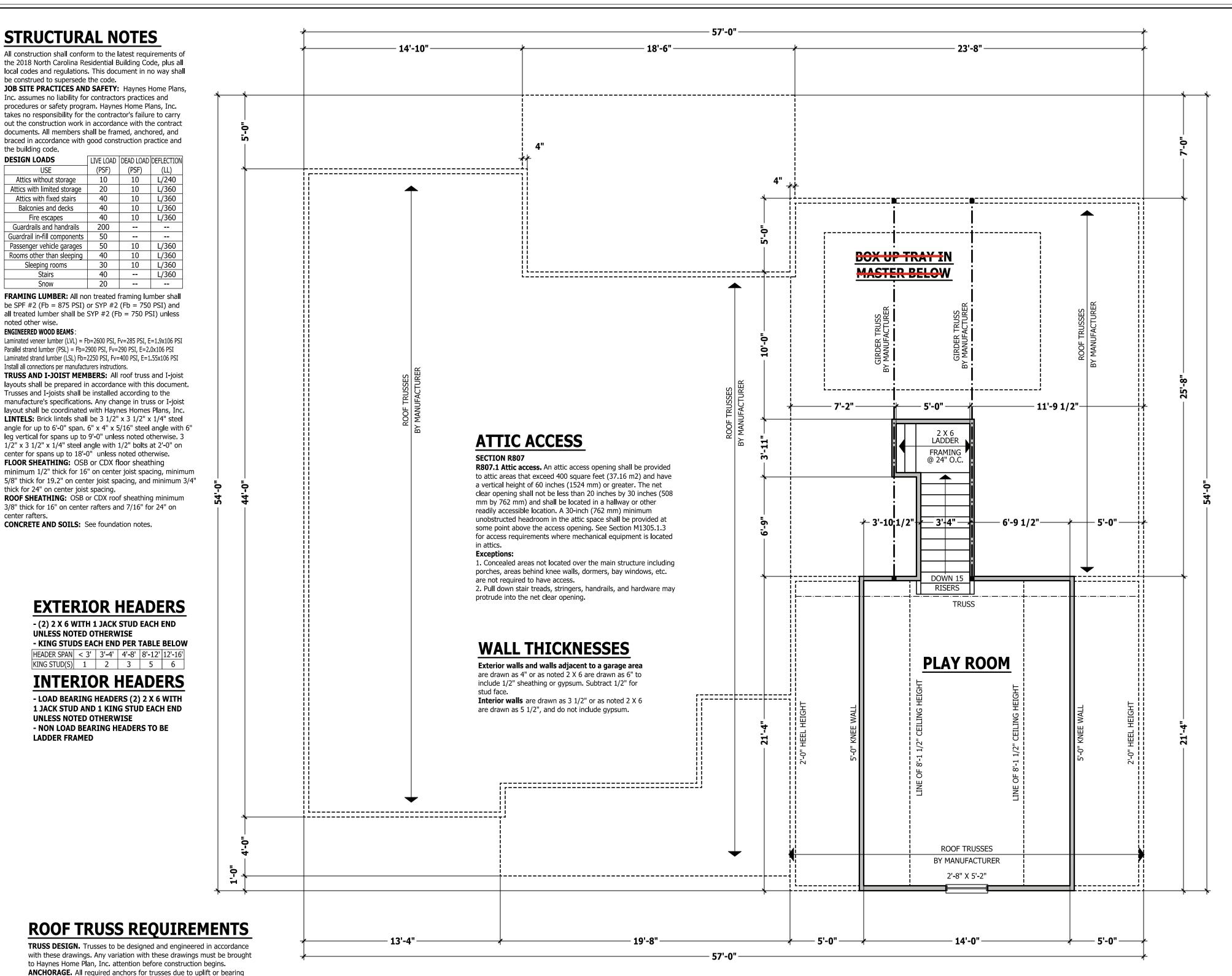
shall meet the requirements as specified on the truss schematics.

ledgers unless noted otherwise.

and floor system thicknesses.

**BEARING.** All trusses shall be designed for bearing on SPF #2 plates or

**Plate Heights & Floor Systems.** See elevation page(s) for plate heights



**SECOND FLOOR PLAN** 

SCALE 1/4" = 1'-0"

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PLAN

FLOOR

COND

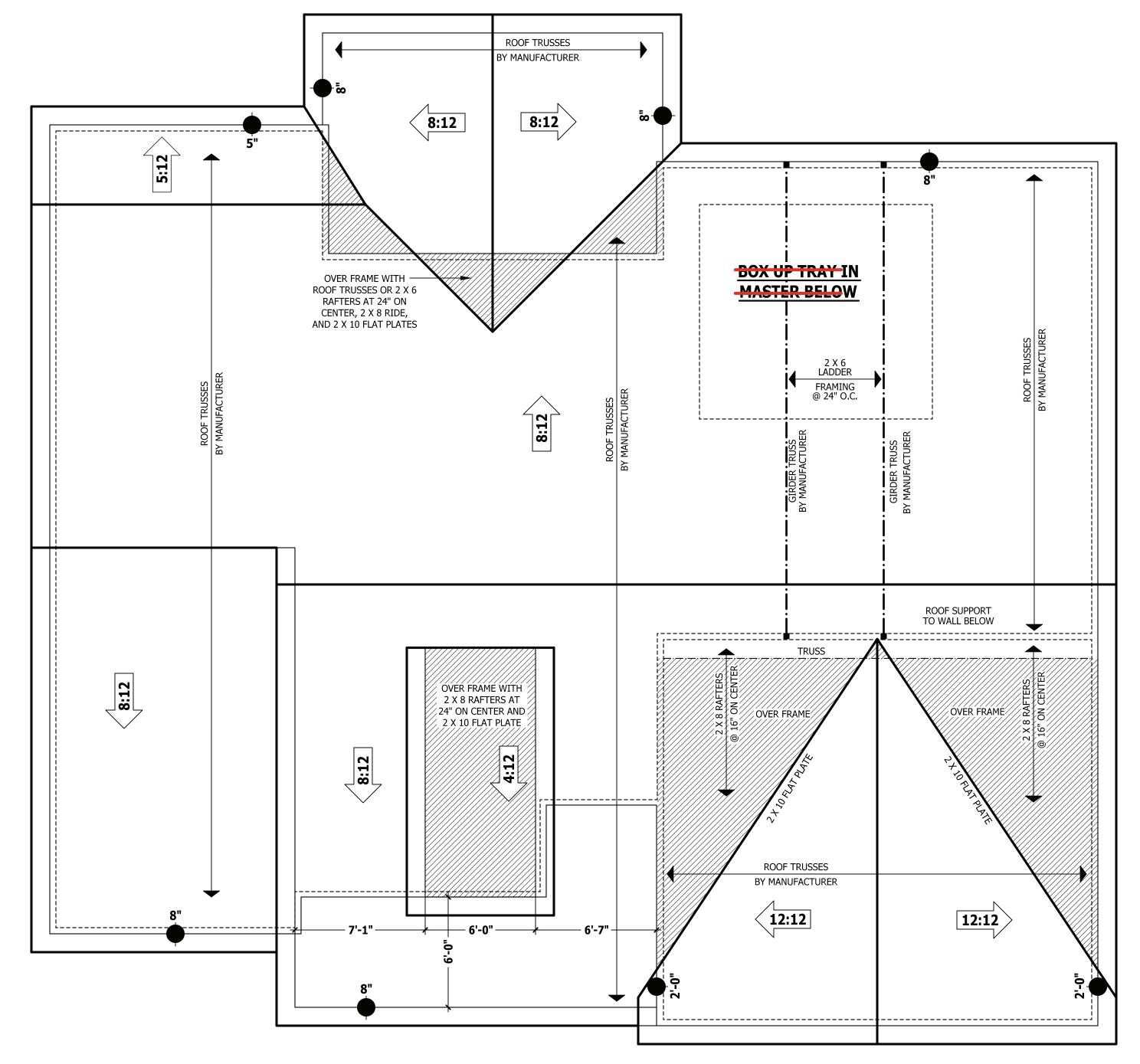
**SQUARE FOOTAGE** UNHEATED UNHEATED OPTIONAL

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## **ROOF TRUSS REQUIREMENTS**

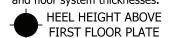
TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics.

BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

**Plate Heights & Floor Systems.** See elevation page(s) for plate heights and floor system thicknesses.





ROOF PLAN
SCALE 1/4" = 1'-0"

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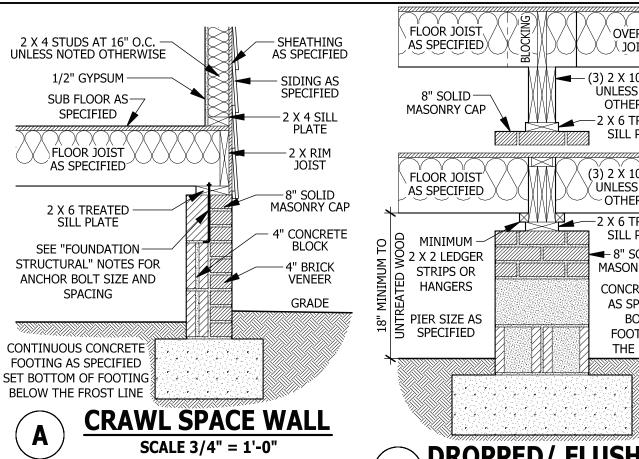
**ROOF PLAN** 

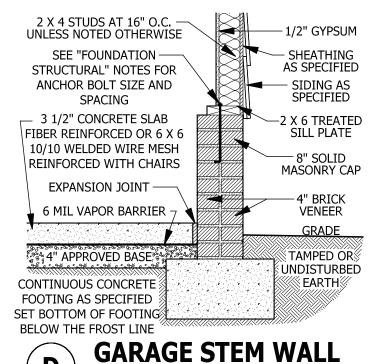


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## SCALE 3/4" = 1'-0"

**DECK STAIR NOTES SECTION AM110** 

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer. If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

## **DECK BRACING**

SECTION AM109

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to provide lateral stability.

AM109.1.1. When the deck floor height is less than 4'-0" above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

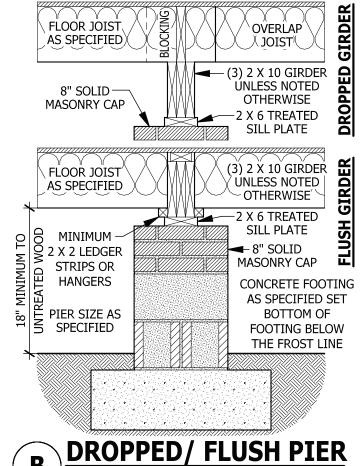
**AM109.1.2.** 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be bolted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the brace per Figure AM109.1

**AM109.1.3.** For freestanding decks without knee braces or diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2 and the following:

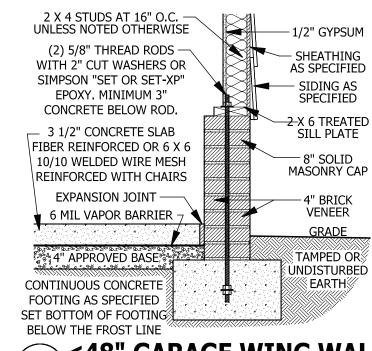
POST SIZE	MAX TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 X 4	48 SF	4'-0"	2'-6"	1'-0"
6 X 6	120 SF	6'-0"	3'-6"	1'-8"

**AM109.1.4.** 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109.3.

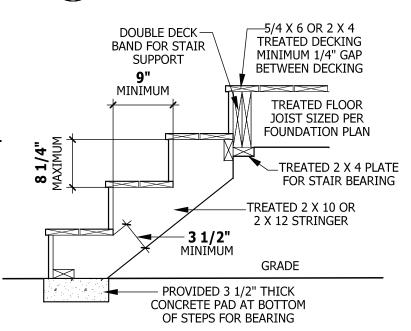
AM109.1.5. For embedment of piles in Coastal Regions, see Chapter 45.



# SCALE 3/4'' = 1'-0''



## <48" GARAGE WING WALL E SCALE 3/4" = 1'-0"



## FIGURE AM110 TYPICAL DECK STAIR DETAIL

SCALE 3/4" = 1'-0"

STONE VEENER

AS SPECIFIED

VAPOR BARRIER

-WEEP SCREED

MINIMUM 4" TO

GROUND OR 2"

-TO PAVEMENT

GRADE

SHEATHING +

AS SPECIFIED

LATH-

SEE FOUNDATION

FOR FOUNDATION

**DETAILS** 

**WEEP SCREED** 

**SCALE 3/4" = 1'-0"** 

# **WEEP SCREEDS**

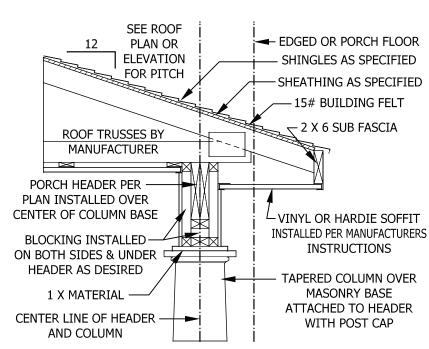
All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential Building code.

**R703.6.2.1 -** A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic weep screed, with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above paved areas and shall be of a type that will allow trapped water to drain to the exterior of the

shall cover and terminate on the

attachment flange of the weep screed.

**R314.4 Power source.** Smoke alarms shall receive their primary building. The weather-resistant barrier shall commercial source, and when primary power is interrupted, shall lap the attachment flange. The exterior lath receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.



## **PORCH HEADER WITH TAPERED COLUMN SCALE 3/4" = 1'-0"**

FOR SLAB SUPPORT 3 1/2" CONCRETE SLAB ROWLOCK TRUCTURAL" NOTES FOR - 8 X 16 VEN GRADE

## FILLED PORCH SECTION WITH VENT

**CRAWL SPACE AT GARAGE** 

SCALE 3/4" = 1'-0"

-2 X 4 SOLE PLATE

FLASHING MINIMUM 16" WIDE

CONTINUOUS CONCRETE

SET BOTTOM OF FOOTING

2 X 4 STUDS AT 16" O.C.

**UNLESS NOTED OTHERWISE** 

SUB FLOOR AS-7

SPECIFIED

FLOOR JOIST

AS SPECIFIED

2 X 6 TREATED SILL PLATE

SEE "FOUNDATION

STRUCTURAL" NOTES FOR

ANCHOR BOLT SIZE AND

SPACING

CONTINUOUS CONCRETE<sup>®</sup>

FOOTING AS SPECIFIED

SET BOTTOM OF FOOTING

BELOW THE FROST LINE

2 X TREATED— HOUSE BAND

SUB FLOOR AS -

SPECIFIED

FLOOR JOIST AS SPECIFIED

8" CONCRETE BLOCK

TAMPED OR

—1/2" GYPSUM

- 2 X 4 SILL

- 2 X RIM

JOIST

-8" SOLID

MASONRY CAP

4" CONCRETE

BLOCK

-4" BRICK VENEER

- EXPANSION JOINT

-6 MIL VAPOR

BARRIER

3 1/2" SLAB

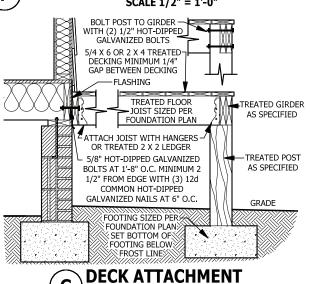
្ទ្រឹ 4" BASE

TAMPED OR

UNDISTURBED

COBBLED BRICK

PLATE



SCALE 1/2" = 1'-0"

## **SMOKE ALARMS**

R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning

equipment provisions of NFPA 72. **R314.2 Smoke detection systems.** Household fire alarm systems installed in accordance with NFPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with

NFPA 72. **Exception:** Where smoke alarms are provided meeting the requirements of Section R314.4.

**R314.3 Location.** Smoke alarms shall be installed in the following locations:

1. In each sleeping room.

2. Outside each separate sleeping area in the immediate vicinity of the bedrooms.

3. On each additional story of the dwelling, including basements and habitable attics (finished) but not including crawl spaces, uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In *dwellings* or *dwelling units* with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full story below the upper level.

When more than one smoke alarm is required to be installed within in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit.

power from the building wiring when such wiring is served from a

## CARBON MONOXIDE ALARMS

R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed by the alarm manufacturer

**R315.2 Where required in existing dwellings.** In existing dwellings, where interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping rooms are added or created, carbon monoxide alarms shall be provided in accordance with Section

R315.3 Alarm requirements. The required carbon monoxide alarms shall be audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer's installation instructions.

## **STAIRWAY NOTES**

R311.7.2 Headroom. The minimum headroom in all parts of the stairway shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stairway.

**R311.7.4 Stair treads and risers.** Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of

the adjacent treads. R311.7.4.2 Tread depth. The minimum tread depth shall be 9 inches (229) mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a minimum tread depth of 4 inches (102 mm) at any point.

**R311.7.4.3 Profile.** The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

**R311.7.7 Handrails.** Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311.7.7.1 Height. Handrail height, measured vertically from the sloped plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm). **Exceptions:** 

1. The use of a volute, turnout or starting easing shall be allowed over the

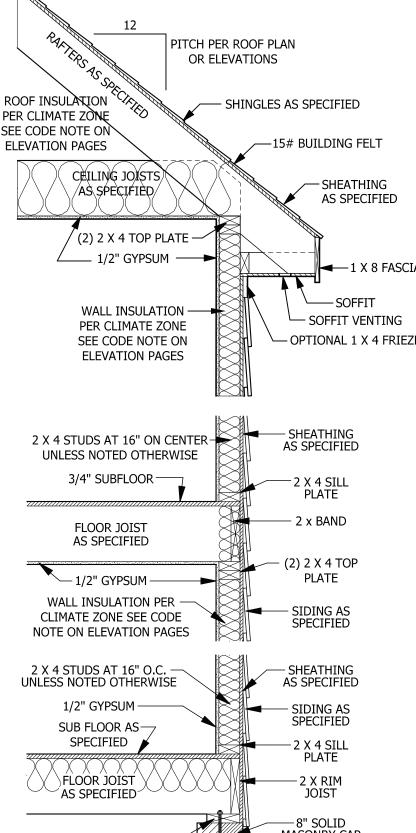
lowest tread. 2. When handrail fittings or bendings are used to provide continuous transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall be permitted to exceed the maximum height.

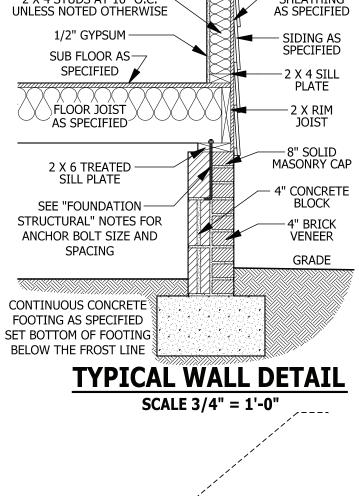
**R311.7.7.2 Continuity.** Handrails for stairways shall be continuous for the full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails an individual *dwelling* unit the alarm devices shall be interconnected adjacent to a wall shall have a space of not less than 11/2 inch (38 mm) between the wall and the handrails.

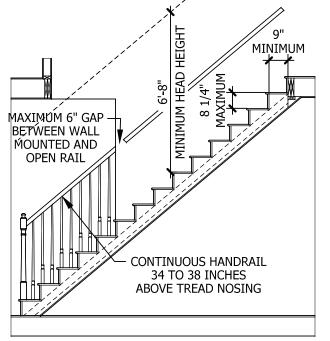
## Exceptions

1. Handrails shall be permitted to be interrupted by a newel post. 2. The use of a volute, turnout, starting easing or starting newel shall be allowed over the lowest tread.

3. Two or more separate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.







TYPICAL STAIR DETAIL

10/28/2019

191021B PAGE 8 OF 8

**SQUARE FOOTAGE** 1880 SQ.FT 307 SQ.FT 2187 SQ.FT UNHEATED

GARAGE SCREENED PORCH

PURCHASER MUST VERIFY ALL

EFORE CONSTRUCTION BEGINS

HAYNES HOME PLANS, INC.

ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND

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**DETAIL** 

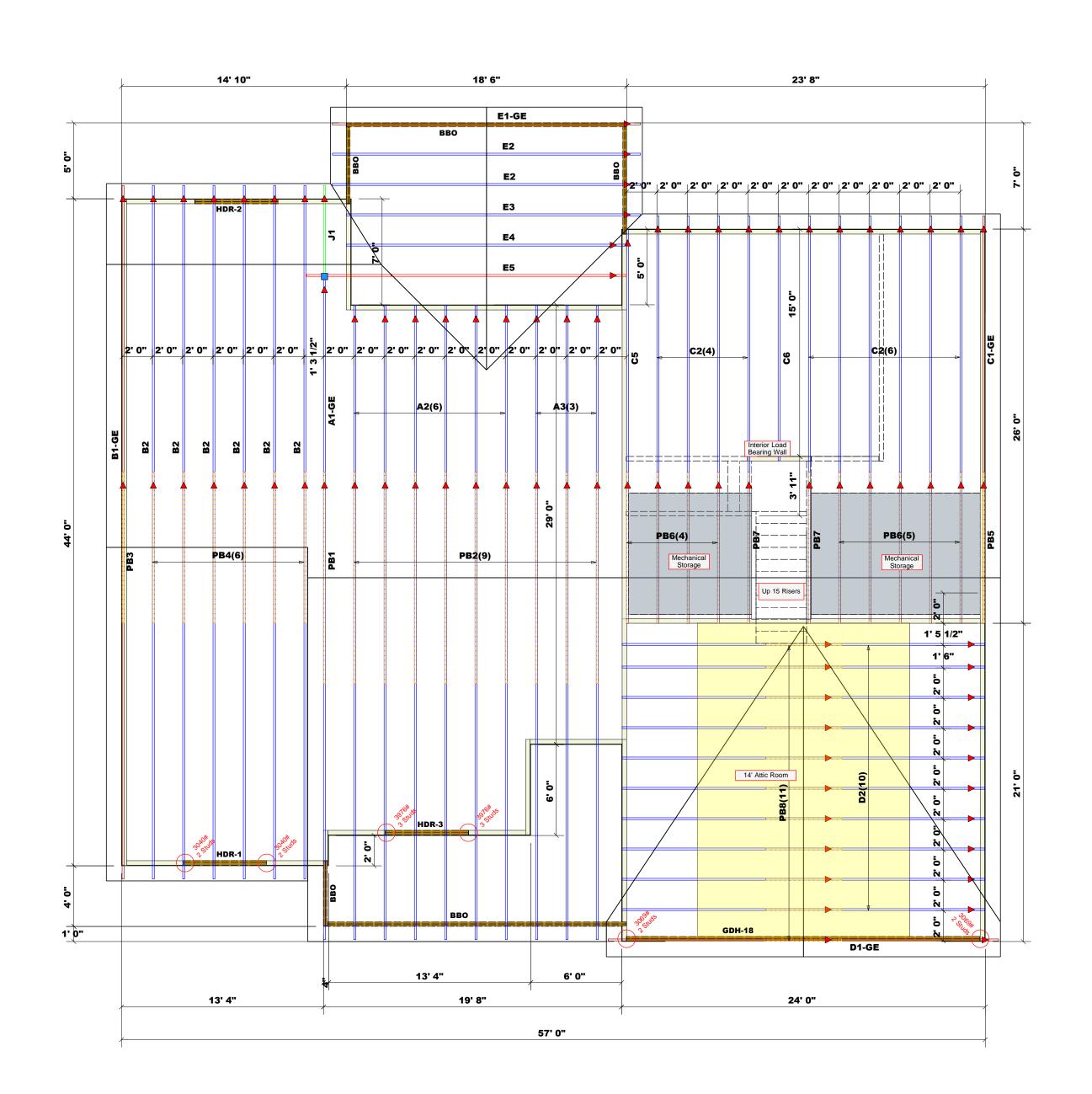
**TYPICAL** 

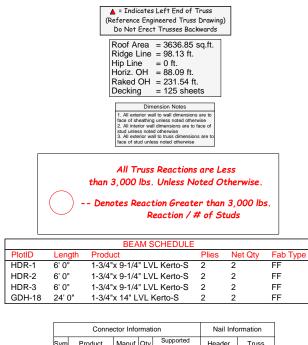
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ARY WITH LOCATION. A LOCAL

IGINEER SHOULD BE CONSULTED

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COMTECH **ROOF & FLOOR TRUSSES & BEAMS** 

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables ( derived from the prescriptive Code requirements ) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Anthony Williams

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))

					 -,,	
NUI	MBER C	F JACI	STUDS R		A END OF	=
END REACTION (UP TO)	REQ'D STUDS FOR (2) PLY HEADER		END REACTION (UP TO)	REQ'D STUDS FOR (3) PLY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR
1700	1		2550	1	3400	1
3400	2		5100	2	6800	2
5100	3		7650	3	10200	3
6800	4		10200	4	13600	4
8500	5		12750	5	17000	5
10200	6		15300	6		
11900	7					
13600	8					
15300	9					

Angier / Harnett County

COUNTY

Signature Home Builders

BUILDER

JOB NAME

Ridge /

1 Mabry

ADDRESS

HHP / The Sinclair (191021B)

Plan Date: 10/28/19

SEAL DATE

Williams

Anthony \

DRAWN BY

3/11/25

DATE REV.

Anthony Williams

SALESMAN

J0325-1347

JOB#

Ž

QUOTE#

		tuds			
		BEAM SCHEDULE			
tID	Length	Product	Plies	Net Qty	Fab Type
R-1	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
R-2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
R-3	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
H-18	24' 0"	1-3/4"x 14" LVL Kerto-S	2	2	FF

Connector Information Nail Information							
Product	Manuf	Qty	Supported Member	Header	Truss		
HUS26	USP	1	NA	16d/3-1/2"	16d/3-1/2"		

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.

These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

PLAN

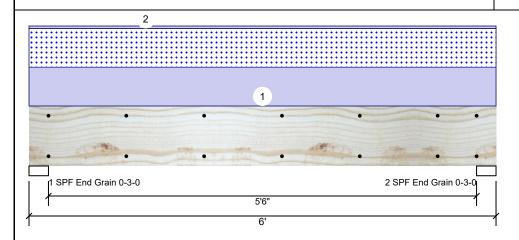


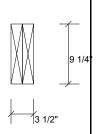
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

#### 1.750" X 9.250" 2-Ply - PASSED HDR-1 **Kerto-S LVL**

Level: Level





Page 1 of 10

#### Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F Application: Design Method: ASD

**Building Code: IBC/IRC 2015** Load Sharing: No

Deck: Not Checked

#### Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1576	1464	0	0
2	Vertical	0	1576	1464	0	0

## **Bearings**

Grain

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.000" 1576 / 1464 3040 L D+S Vert 34% End Grain 2 - SPF 3.000" 1576 / 1464 3040 L D+S Vert 34% End

#### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4007 ft-lb	3'	14423 ft-lb	0.278 (28%)	D+S	L
Unbraced	4007 ft-lb	3'	10944 ft-lb	0.366 (37%)	D+S	L
Shear	2011 lb	1' 1/4"	7943 lb	0.253 (25%)	D+S	L
LL Defl inch	0.031 (L/2200)	3'	0.141 (L/480)	0.218 (22%)	S	L
TL Defl inch	0.064 (L/1060)	3'	0.188 (L/360)	0.340 (34%)	D+S	L

### **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	488 PLF	0 PLF	488 PLF	0 PLF	0 PLF	B2 TRUSS
2	Uniform			Тор	30 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

  Damaged Beams must not be used

- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

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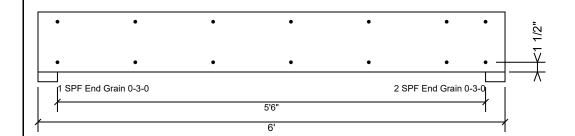
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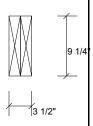
Anthony Williams Job Name: Lot 1 Mabry Ridge

J0325-1347

Project #: Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED HDR-1

Level: Level





Page 2 of 10

### Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

#### Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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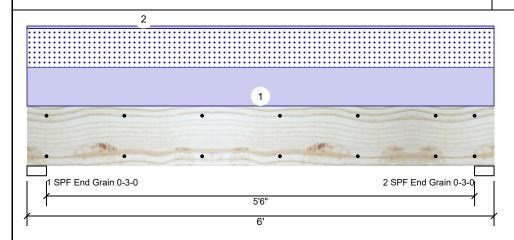


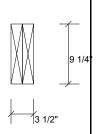
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

#### 1.750" X 9.250" 2-Ply - PASSED HDR-2 **Kerto-S LVL**

Level: Level





Page 3 of 10

#### Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** 

Load Sharing: No

Deck: Not Checked

#### Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1498	1386	0	0
2	Vertical	0	1498	1386	0	0

## **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3802 ft-lb	3'	14423 ft-lb	0.264 (26%)	D+S	L
Unbraced	3802 ft-lb	3'	10944 ft-lb	0.347 (35%)	D+S	L
Shear	1908 lb	1' 1/4"	7943 lb	0.240 (24%)	D+S	L
LL Defl inch	0.029 (L/2324)	3'	0.141 (L/480)	0.207 (21%)	S	L
TL Defl inch	0.060 (L/1117)	3'	0.188 (L/360)	0.322 (32%)	D+S	L

### **Bearings**

Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.000" 1498 / 1386 2884 L D+S Vert End Grain 2 - SPF 3.000" 1498 / 1386 2884 L D+S Vert End Grain

### **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	462 PLF	0 PLF	462 PLF	0 PLF	0 PLF	B2 TRUSS
2	Uniform			Тор	30 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
  - approvals

    Damaged Beams must not be used
  - Design assumes top edge is laterally restrained
    Provide lateral support at bearing points to avoid
    lateral displacement and rotation

Handling & Installation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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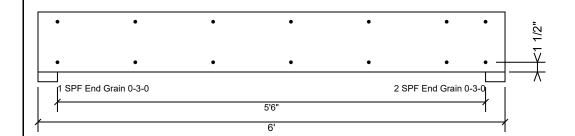
Client: Signature Homes

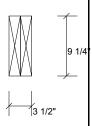
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** HDR-2

Level: Level





Page 4 of 10

### Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

#### Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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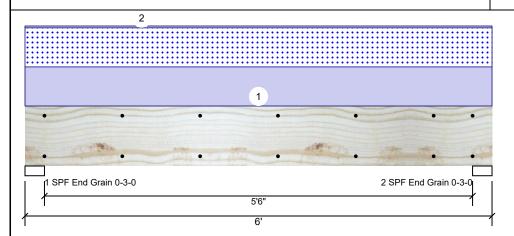


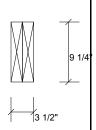
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

#### 1.750" X 9.250" HDR-3 **Kerto-S LVL** 2-Ply - PASSED

Level: Level





Page 5 of 10

#### Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD

**Building Code: IBC/IRC 2015** 

Load Sharing: No

Deck: Not Checked

#### Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2044	1932	0	0
2	Vertical	0	2044	1932	0	0

### **Bearings**

End Grain

Bearing Length	h Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.	
1 - SPF 3.000" End Grain	Vert	45%	2044 / 1932	3976	L	D+S	
2-SPE 3000"	Vert	45%	2044 / 1932	3976	1	D+S	

#### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5241 ft-lb	3'	14423 ft-lb	0.363 (36%)	D+S	L
Unbraced	5241 ft-lb	3'	10944 ft-lb	0.479 (48%)	D+S	L
Shear	2628 lb	1' 1/4"	7943 lb	0.331 (33%)	D+S	L
LL Defl inch	0.040 (L/1667)	3'	0.141 (L/480)	0.288 (29%)	S	L
TL Defl inch	0.083 (L/810)	3'	0.188 (L/360)	0.444 (44%)	D+S	L

### **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	644 PLF	0 PLF	644 PLF	0 PLF	0 PLF	A2 TRUSS
2	Uniform			Тор	30 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
  Refer to manufacturer's product information
  regarding installation requirements, multi-ply
  fastening details, beam strength values, and code
- approvals

  Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

	lateral displacement
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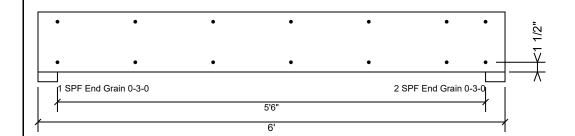
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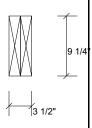
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** HDR-3

Level: Level





Page 6 of 10

### Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

#### Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

(800) 622-5850 www.metsawood.com/us

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

CSD DESIGN



Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

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Wind

0

0

Const

Ld. Comb.

D+0.75(L+S)

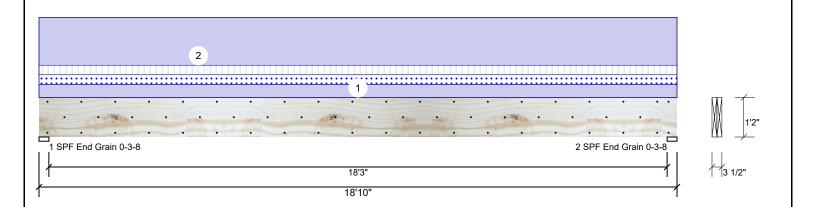
D+0.75(L+S)

0

0

**Kerto-S LVL** 1.750" X 14.000" 2-Ply - PASSED **GDH-18** 

Level: Level



#### Member Information Reactions UNPATTERNED Ib (Uplift) Application: Type: Floor Brg Direction Live Dead Snow Plies: 2 Design Method: ASD 377 2504 Vertical 377 1 Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 Vertical 377 2504 377 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case

### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	12910 ft-lb	9'5"	26999 ft-lb	0.478 (48%)	D+L	L
Unbraced	13754 ft-lb	9'5"	13784 ft-lb	0.998 (100%)	D+0.75(L+S)	L
Shear	2447 lb	1'5 1/2"	10453 lb	0.234 (23%)	D+L	L
LL Defl inch	0.102 (L/2160)	9'5 1/16"	0.459 (L/480)	0.222 (22%)	0.75(L+S)	L
TL Defl inch	0.555 (L/398)	9'5 1/16"	0.612 (L/360)	0.905 (91%)	D+0.75(L+S)	L

#### **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 7'5 9/16" o.c.
- 7 Bottom must be laterally braced at end bearings.

8 Lateral slend	lerness ratio based on single	e ply width.									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	55 PLF	40 PLF	40 PLF	0 PLF	0 PLF	R + F	
2	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
	Self Weight				11 DI E						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
  - Damaged Beams must not be used

  - Design assumes top edge is laterally restrained
    Provide lateral support at bearing points to avoid
    lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

1 - SPF 3.500"

2 - SPF 3.500"

End Grain

End Grain Vert

Vert

30%

30%

2504 / 565

2504 / 565

3069 L

3069 L

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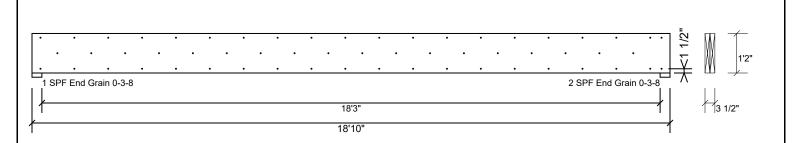
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

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Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED **GDH-18** 

Level: Level



## Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

aston an phos asing s	10115 01 100 2011 10115 (112016 ) 01
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

#### Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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This design is valid until 6/28/2026

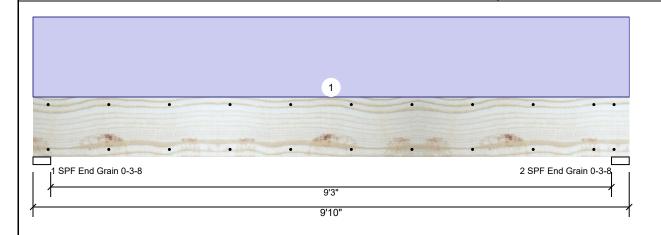


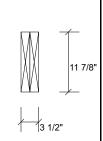
Project: Address: Date: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL** GDH-9

Level: Level





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#### Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Deck: Not Checked

#### Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	517	0	0	0
2	Vertical	0	517	0	0	0

#### **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1156 ft-lb	4'11"	17919 ft-lb	0.065 (6%)	D	Uniform
Unbraced	1156 ft-lb	4'11"	9664 ft-lb	0.120 (12%)	D	Uniform
Shear	392 lb	8'6 5/8"	7980 lb	0.049 (5%)	D	Uniform
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)		
TL Defl inch	0.022 (L/5130)	4'11"	0.312 (L/360)	0.070 (7%)	D	Uniform

## **Bearings**

Bearing Length	Dir.	Cap. Read	t D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 3.500" End Grain	Vert	5%	517 / 0	517	Uniform	D
2 - SPF 3.500" End Grain	Vert	5%	517 / 0	517	Uniform	D

### **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	96 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
	Self Weight				9 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  2 Damaged Beams must not be used
- - Design assumes top edge is laterally restrained
    Provide lateral support at bearing points to avoid
    lateral displacement and rotation This design is valid until 6/28/2026
- 6. For flat roofs provide proper drainage to prevent ponding

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Client: Signature Homes

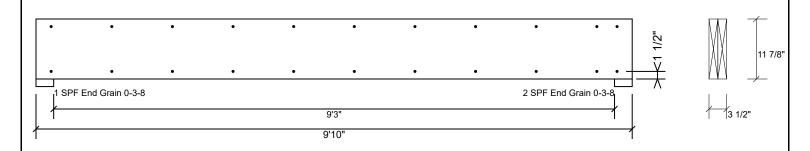
Project: Address: 3/11/2025

Input by: Anthony Williams Job Name: Lot 1 Mabry Ridge Project #: J0325-1347

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1.750" X 11.875" **Kerto-S LVL** 2-Ply - PASSED GDH-9

Level: Level



## Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

#### Notes

NOtes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

### Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

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