

## **Trenco**

818 Soundside Rd Edenton, NC 27932

Re: J1024-5875

Lot 15 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I72421075 thru I72421092

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



April 2,2025

Galinski, John

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421075 **GABLE** J1024-5875 A1-GE Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:42 2025 Page 1 Comtech, Inc. ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 31-0-0 15-6-0 15-6-0 15-6-0 Scale = 1:66.7 4x6 = 8.00 12 11 12 13 6x6 6x6 × 8 14 15 16 6 17 18 19 20 3x4 = 3x4 36 35 33 32 31 29 28 27 26 25 23 22 30 24 4x6 =

Plate Offsets (X,Y)	[8:0-3-0,0-4-4], [11:0-3-0,Edge], [14:0-3-	-0,0-4-4]

LOADIN	\( \ \ \ \ \	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.07	Vert(LL)	-0.00	20	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	20	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.01	20	n/a	n/a		
BCDL	10.0	Code IRC2021/TF	PI2014	Matri	x-S						Weight: 276 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No 1 **BOT CHORD** 2x6 SP No.1 2x4 SP No.2 **OTHERS** 

**BRACING-**

TOP CHORD **BOT CHORD** WFBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. T-Brace: 2x4 SPF No.2 - 10-30, 12-28 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

REACTIONS. All bearings 31-0-0.

Max Horz 2=-331(LC 10) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 20, 32, 33, 34, 35, 26, 25, 24, 23 except 31=-104(LC 12),

36=-103(LC 12), 27=-109(LC 13), 22=-101(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 20, 31, 32, 33, 34, 35, 36, 28, 27, 26, 25, 24, 23, 22 except 30=257(LC 22)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-350/234, 3-4=-262/198, 19-20=-286/143

**BOT CHORD** 2-36=-130/294, 35-36=-130/294, 34-35=-130/294, 33-34=-130/294, 32-33=-130/294, 31-32=-130/294, 30-31=-130/294, 28-30=-130/294, 27-28=-130/294, 26-27=-130/294,

25-26=-130/294, 24-25=-130/294, 23-24=-130/294, 22-23=-130/294, 20-22=-130/294

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-0-15 to 3-3-14, Exterior(2N) 3-3-14 to 15-6-0, Corner(3R) 15-6-0 to 19-10-13, Exterior(2N) 19-10-13 to 32-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 20, 32, 33, 34, 35, 26, 25, 24, 23 except (jt=lb) 31=104, 36=103, 27=109, 22=101.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



April 2,2025

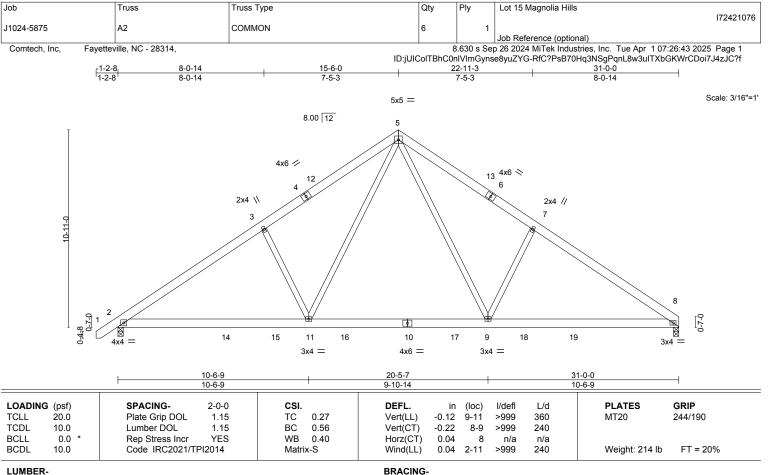


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WFBS

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=260(LC 9)

Max Uplift 2=-82(LC 12), 8=-65(LC 13) Max Grav 2=1616(LC 19), 8=1545(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2171/361, 3-5=-2041/454, 5-7=-2047/460, 7-8=-2176/366 TOP CHORD

2-11=-201/1896, 9-11=0/1233, 8-9=-190/1724 **BOT CHORD** 

5-9=-172/1082, 7-9=-471/311, 5-11=-170/1073, 3-11=-467/305 WEBS

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 15-6-0, Exterior(2R) 15-6-0 to 19-10-13, Interior(1) 19-10-13 to 30-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



Structural wood sheathing directly applied or 5-0-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

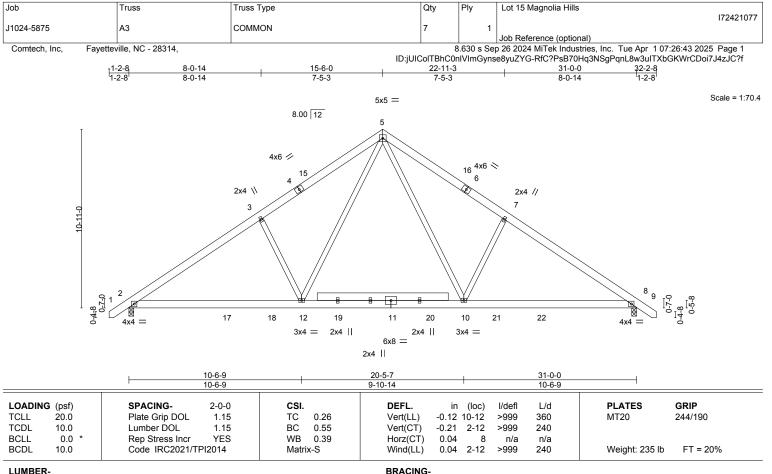
April 2,2025



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**BOT CHORD** 

LUMBER-

TOP CHORD 2x6 SP No.1 2x6 SP No.1 BOT CHORD 2x4 SP No.2 **WEBS** 

REACTIONS. (size) 2=0-3-8, 8=0-3-8

Max Horz 2=-265(LC 10)

Max Uplift 2=-82(LC 12), 8=-82(LC 13) Max Grav 2=1608(LC 19), 8=1608(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2156/360, 3-5=-2026/452, 5-7=-2027/452, 7-8=-2157/360 TOP CHORD **BOT CHORD** 2-12=-160/1892, 10-12=0/1232, 8-10=-158/1717

5-10=-169/1067, 7-10=-468/305, 5-12=-169/1066, 3-12=-467/305 WEBS

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 15-6-0, Exterior(2R) 15-6-0 to 19-10-13, Interior(1) 19-10-13 to 32-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



Structural wood sheathing directly applied or 5-0-15 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421078 **GABLE** J1024-5875 A4-GE Job Reference (optional) Comtech, Inc.

Fayetteville, NC - 28314,

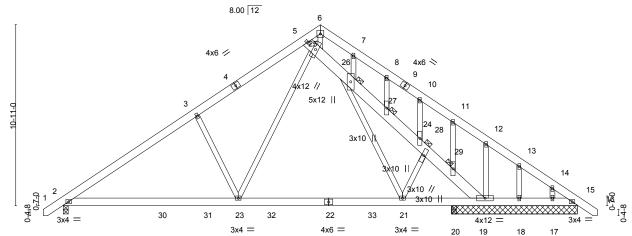
8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:44 2025 Page 1 ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 8-0-14 8-0-14 14-5-11 15-6-0 1-0-5 22-11-3 7-5-3 31-0-0 6-4-14 8-0-14

> Scale = 1:69.5 5x5 =

Structural wood sheathing directly applied or 5-4-7 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 24, 26, 27, 29



	10-6-9 10-6-9	+	20-5-7 9-10-14	23-5-0 2-11-9	31-0-0 7-7-0		
LOADING (psf)	SPACING- 2-0-0	CSI.	I .	(loc) I/defl	L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.29 BC 0.57	Vert(LL) -0.16 2 Vert(CT) -0.24 2			MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2021/TPI2014	WB 0.43 Matrix-S	Horz(CT) 0.03 Wind(LL) 0.05	15 n/a 2-23 >999	-	Weight: 275 lb	FT = 20%

BOT CHORD

**JOINTS** 

LUMBER-TOP CHORD

BRACING-2x6 SP No.1 TOP CHORD

2x6 SP No.1 BOT CHORD 2x4 SP No.2 \*Except\* WEBS

5-19: 2x8 SP No.1 **OTHERS** 2x4 SP No.2

All bearings 7-7-0 except (jt=length) 2=0-3-8, 20=0-3-8.

REACTIONS. (lb) -Max Horz 2=-331(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 18, 17, 20 except 2=-228(LC 12), 19=-150(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 18, 17 except 2=1417(LC 19), 15=626(LC 20), 19=530(LC 19),

20=336(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1800/262, 3-5=-1659/361, 5-6=-1138/406, 6-7=-871/344, 7-8=-932/289, TOP CHORD

8-10=-964/241, 10-11=-891/144, 11-12=-908/82, 12-13=-822/4, 13-14=-869/0,

14-15=-884/0

**BOT CHORD** 2-23=-273/1632, 21-23=-3/956, 20-21=-40/1254, 19-20=-40/1254, 18-19=0/720,

17-18=0/720, 15-17=0/720

**WEBS** 21-26=-86/642, 21-24=-392/232, 23-25=-238/1080, 3-23=-392/367, 5-25=-794/284,

25-26=-267/208, 26-27=-701/158, 27-28=-701/174, 24-28=-796/223, 24-29=-719/190,

19-29=-731/217, 6-25=-358/1167, 12-19=-287/131

## NOTES-

- Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 15-6-0, Exterior(2R) 15-6-0 to 19-10-13, Interior(1) 19-10-13 to 32-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 17, 20 except (jt=lb) 2=228, 19=150.



April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421079 **GABLE** J1024-5875 B1-GE Job Reference (optional)

14-5-11 14-5-11

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:44 2025 Page 1 ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 23-10-8

Scale = 1:60.2

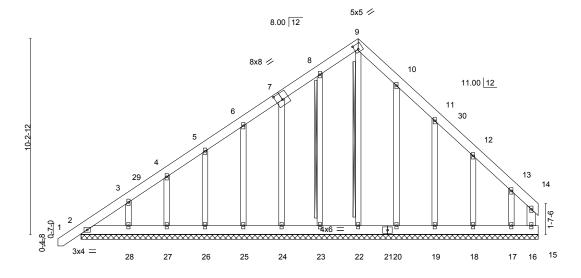


Plate Offsets (X,Y)--[7:0-4-0,0-4-8], [9:0-2-3,0-3-4] LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defl L/d **PLATES** GRIP **TCLL** 20.0 Plate Grip DOL 1.15 TC 0.07 Vert(LL) 0.00 120 MT20 244/190 n/r **TCDL** 10.0 Lumber DOL 1.15 BC 0.01 Vert(CT) 0.00 120 n/r **BCLL** 0.0 Rep Stress Incr YES WB 0.21 Horz(CT) 0.00 15 n/a n/a **BCDL** 10.0 Code IRC2021/TPI2014 Matrix-S Weight: 217 lb FT = 20%

LUMBER-TOP CHORD 2x6 SP No 1 **BOT CHORD** 2x6 SP No.1 2x6 SP No.1 WFBS **OTHERS** 2x4 SP No.2 **BRACING-**TOP CHORD **BOT CHORD** WEBS

23-10-8 23-10-8

> Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing, Except: 10-0-0 oc bracing: 16-17,15-16. 2x4 SPF No.2 - 9-22, 8-23

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 23-10-8

Max Horz 2=299(LC 9) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 22, 23, 25, 26, 27, 20, 16 except

2=-149(LC 8), 24=-102(LC 12), 28=-104(LC 12), 19=-152(LC 13), 18=-126(LC 13),

17=-104(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 2, 15, 23, 24, 25, 26, 27, 28, 20,

19, 18, 17, 16 except 22=436(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-327/327, 3-4=-295/300, 4-5=-271/296, 5-6=-252/325, 6-7=-231/367, 7-8=-212/420,

8-9=-227/421, 9-10=-249/461, 10-11=-213/397, 11-12=-140/265

**WEBS** 9-22=-412/160

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-0-15 to 3-3-14, Exterior(2N) 3-3-14 to 14-5-11, Corner(3R) 14-5-11 to 18-10-8, Exterior(2N) 18-10-8 to 23-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 22, 23, 25, 26, 27, 20, 16 except (jt=lb) 2=149, 24=102, 28=104, 19=152, 18=126, 17=104.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



April 2,2025

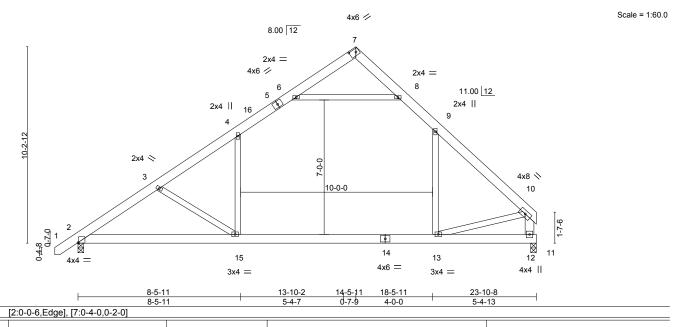
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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421080 J1024-5875 B2 ROOF SPECIAL 10 Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:45 2025 Page 1 Comtech, Inc. ID:jUIColTBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 14-5-11 18-5-11 23-10-8 6-0-0 4-0-0 5-4-13



in (loc)

0.02

0.19

-0.32 13-15

-0.50 13-15

12

15 >999

I/defl

>881

>563

except end verticals.

6-0-0 oc bracing: 12-13.

n/a

L/d

360

240

n/a

240

**PLATES** 

Weight: 172 lb

MT20

Structural wood sheathing directly applied or 4-4-9 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing, Except:

GRIP

244/190

FT = 20%

DEFL.

Vert(LL)

Vert(CT)

Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

REACTIONS.

Plate Offsets (X,Y)--

20.0

10.0

0.0

10.0

LOADING (psf)

**TCLL** 

TCDL

**BCLL** 

BCDL

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x6 SP 2400F 2.0E 2x4 SP No.2 \*Except\* **WEBS** 10-12: 2x6 SP No.1

(size) 2=0-3-8, 12=0-3-8

Max Horz 2=242(LC 9)

Max Uplift 2=-72(LC 12), 12=-33(LC 12) Max Grav 2=1257(LC 19), 12=1213(LC 19)

SPACING-

Plate Grip DOL

Rep Stress Incr

Code IRC2021/TPI2014

Lumber DOL

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1716/283, 3-4=-1406/236, 4-6=-939/278, 6-7=-30/309, 8-9=-977/286, TOP CHORD

2-0-0

1.15

1.15

YES

CSI.

0.76

0.50

0.68

TC

BC

WB

Matrix-S

9-10=-1493/223, 10-12=-1396/208 **BOT CHORD** 2-15=-236/1568, 13-15=-65/1049

**WEBS** 4-15=0/685, 9-13=0/577, 6-8=-1254/334, 10-13=-56/1200, 3-15=-617/203

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 14-5-11, Exterior(2R) 14-5-11 to 18-7-9, Interior(1) 18-7-9 to 23-6-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 12.



April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421081 J1024-5875 **GABLE** C1-GE Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:45 2025 Page 1 ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 22-0-0

11-0-0 11-0-0 11-0-0

5x5 =

Scale = 1:76.6

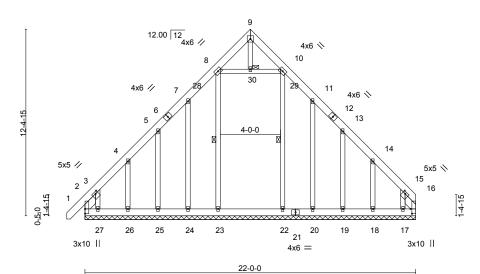


Plate Offsets (X,Y)-- [2:0-3-12,0-0-1], [3:0-2-0,0-1-8], [15:0-2-0,0-1-8], [16:0-3-12,0-1-1]

LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	<b>CSI.</b> TC 0.03	DEFL. Vert(LL) -0.	in (loc)	l/defl n/r	L/d 120	PLATES MT20	<b>GRIP</b> 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.07	\ '	.00 1	n/r	120		
BCLL 0.0 *	Rep Stress Incr YES	WB 0.15	Horz(CT) 0	.01 16	n/a	n/a		
BCDL 10.0	Code IRC2021/TPI2014	Matrix-S					Weight: 223 lb	FT = 20%

LUMBER-

TOP CHORD 2x6 SP No 1 BOT CHORD 2x6 SP No.1 2x4 SP No.2

WFBS **OTHERS** 2x4 SP No.2 **SLIDER** 

Left 2x6 SP No.1 1-2-10, Right 2x6 SP No.1 1-2-10

BRACING-

TOP CHORD BOT CHORD WERS JOINTS

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 8-23. 10-22

1 Brace at Jt(s): 30

REACTIONS. All bearings 22-0-0.

Max Horz 2=360(LC 11) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 23, 22 except 2=-274(LC 10),

16=-266(LC 11), 24=-152(LC 12), 25=-136(LC 12), 26=-157(LC 12), 27=-439(LC

12), 20=-153(LC 13), 19=-137(LC 13), 18=-157(LC 13), 17=-438(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 24, 25, 26, 20, 19, 18 except 2=653(LC 12), 16=662(LC 13), 23=388(LC 19), 27=264(LC 10), 22=362(LC 20),

17=256(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-713/338, 3-4=-406/218, 4-5=-258/162, 14-15=-375/172, 15-16=-690/292

**BOT CHORD** 2-27=-155/376, 26-27=-155/377, 25-26=-156/378, 24-25=-156/378, 23-24=-156/378,

22-23=-155/375, 20-22=-156/378, 19-20=-156/377, 18-19=-156/377, 17-18=-155/376,

16-17=-153/373

**WEBS** 3-27=-206/372. 15-17=-211/379

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) -1-1-2 to 3-3-11, Interior(1) 3-3-11 to 11-0-0, Exterior(2R) 11-0-0 to 15-1-15, Interior(1) 15-1-15 to 22-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 22 except (jt=lb) 2=274, 16=266, 24=152, 25=136, 26=157, 27=439, 20=153, 19=137, 18=157, 17=438.



April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421082 J1024-5875 C2 COMMON 2 Job Reference (optional)

5x5 =

Fayetteville, NC - 28314, Comtech, Inc.

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ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 11-0-0 15-0-0 22-0-0 7-0-0 4-0-0 4-0-0

Scale = 1:75.0

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 12

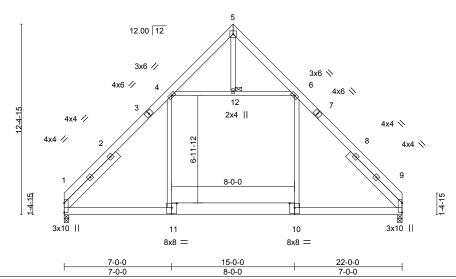


Plate Offsets (X,Y)-- [1:0-7-7,0-0-1], [9:0-7-7,0-0-1], [10:0-4-0,0-3-8], [11:0-4-0,0-3-8]

LOADIN	\( \( \)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.19	Vert(LL)	0.13	1-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.34	Vert(CT)	-0.14	1-11	>999	180		
BCLL	0.0 *	Rep Stress Incr YES	WB 0.12	Horz(CT)	0.01	9	n/a	n/a		
BCDL	10.0	Code IRC2021/TPI2014	Matrix-S						Weight: 201 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

JOINTS

LUMBER-

TOP CHORD 2x6 SP No 1

BOT CHORD 2x6 SP No.1 \*Except\* 10-11: 2x10 SP No.1

**WEBS** 2x4 SP No.2

**SLIDER** Left 2x6 SP No.1 4-11-8, Right 2x6 SP No.1 4-11-8

REACTIONS. (size) 1=0-3-8, 9=0-3-8

Max Horz 1=-284(LC 8) Max Uplift 1=-34(LC 13), 9=-34(LC 12)

Max Grav 1=1085(LC 20), 9=1085(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-4=-1321/236, 4-5=-262/83, 5-6=-262/83, 6-9=-1324/237 **BOT CHORD** 

1-11=-16/837, 10-11=-11/843, 9-10=-11/834 WEBS 6-10=-35/511, 4-11=-33/508, 4-12=-648/336, 6-12=-648/336

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 11-0-0, Exterior(2R) 11-0-0 to 15-1-15, Interior(1) 15-1-15 to 22-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421083 J1024-5875 С3 COMMON 4 Job Reference (optional)

5x5 =

Fayetteville, NC - 28314, Comtech, Inc.

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ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 7-0-0 7-0-0 11-0-0 15-0-0 4-0-0 4-0-0 7-0-0

Scale = 1:75.0

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

1 Brace at Jt(s): 12

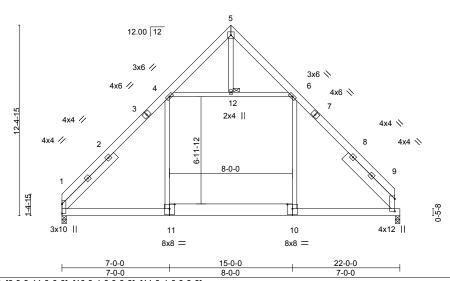


Plate Offsets (X,Y)-- [1:0-7-7,0-0-1], [9:0-9-11,0-0-0], [10:0-4-0,0-3-8], [11:0-4-0,0-3-8]

LOADIN	G (psf)	SPACING- 2-0-	0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.1	5	TC	0.27	Vert(LL)	0.14	1-11	>999	240	MT20	244/190
TCDL	10.0	Lumber DOL 1.1	5	BC	0.37	Vert(CT)	-0.16	1-11	>999	180		
BCLL	0.0 *	Rep Stress Incr YE	S	WB	0.12	Horz(CT)	0.01	9	n/a	n/a		
BCDL	10.0	Code IRC2021/TPI2014	1	Matri	x-S						Weight: 203 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

JOINTS

LUMBER-

TOP CHORD 2x6 SP No 1

BOT CHORD 2x6 SP No.1 \*Except\* 10-11: 2x10 SP No.1

**WEBS** 2x4 SP No.2

**SLIDER** Left 2x6 SP No.1 4-11-8, Right 2x8 SP No.1 4-8-11

REACTIONS. (size) 1=0-3-8, 9=0-3-8

Max Horz 1=284(LC 9)

Max Uplift 1=-34(LC 13), 9=-36(LC 12) Max Grav 1=1068(LC 20), 9=1075(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-4=-1282/231, 4-5=-266/85, 5-6=-269/84, 6-9=-1307/239**BOT CHORD** 1-11=-19/809, 10-11=-15/814, 9-10=-15/805

WEBS 6-10=-38/528, 4-11=-36/482, 4-12=-611/330, 6-12=-611/330

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 11-0-0, Exterior(2R) 11-0-0 to 15-1-15, Interior(1) 15-1-15 to 21-8-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9.



April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421084 J1024-5875 C4 COMMON GIRDER Job Reference (optional)

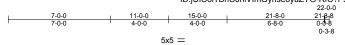
Comtech, Inc, Fayetteville, NC - 28314,

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:47 2025 Page 1 ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.

1 Brace at Jt(s): 12



Scale = 1:79.0

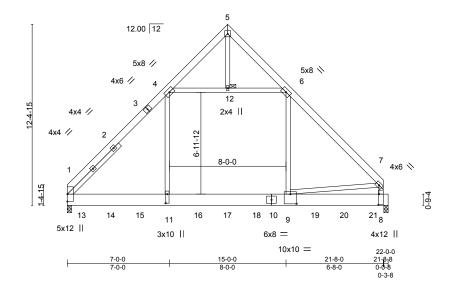


Plate Off	Plate Offsets (X,Y) [8:0-8-12,0-0-8], [9:0-5-0,0-8-0], [11:0-7-8,0-1-8]											
LOADIN	G (psf)	SPACING- 2	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.68	Vert(LL)	-0.12	9-11	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	-0.16	9-11	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.58	Horz(CT)	0.01	8	n/a	n/a		
BCDL	10.0	Code IRC2021/TPI20	014	Matri	x-S	Wind(LL)	0.01	9	>999	240	Weight: 647 lb	FT = 20%

**BRACING-**

TOP CHORD

**BOT CHORD** 

**JOINTS** 

LUMBER-

TOP CHORD 2x6 SP No.1 **BOT CHORD** 2x10 SP 2400F 2.0E

WEBS 2x4 SP No.2 SLIDER Left 2x4 SP No.2 4-11-8

REACTIONS. (size) 1=0-3-8, 8=0-3-8

Max Horz 1=279(LC 5)

Max Uplift 1=-32(LC 9), 8=-36(LC 8)

Max Grav 1=10803(LC 14), 8=10855(LC 14)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

1-4=-10373/143, 4-5=-310/51, 5-6=-439/54, 6-7=-9815/138 TOP CHORD

**BOT CHORD** 1-11=-25/6906, 9-11=-24/6922

6-9=-69/6670, 4-11=-60/7650, 4-12=-6645/189, 6-12=-6645/189, 7-9=-26/7139, WEBS

7-8=-8266/100

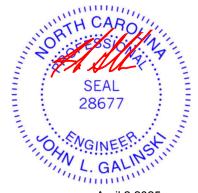
## NOTES-

1) 3-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-4-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, Except member 8-7 2x4 - 1 row at 0-7-0 oc.

2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to

- ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60



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## Continued on page 2



building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills	
J1024-5875	C4	COMMON GIRDER	1	3	Job Reference (optional)	172421084

Fayetteville, NC - 28314, Comtech, Inc,

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## NOTES-

5) N/A

- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1829 lb down at 0-10-4, 1826 lb down at 2-10-4, 1826 lb down at 4-10-4, 1826 lb down at 6-10-4, 1826 lb down at 8-10-4, 1826 lb down at 10-10-4, 1826 lb down at 12-10-4, 1826 lb down at 14-10-4, 1826 lb down at 1826 lb down at 18-10-4, and 1783 lb down at 20-10-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-5=-60, 5-7=-60, 1-8=-20

Concentrated Loads (lb)

Vert: 9=-459(F) 11=-459(F) 13=-462(F) 14=-459(F) 15=-459(F) 16=-459(F) 17=-459(F) 18=-459(F) 19=-459(F) 20=-459(F) 21=-448(F)



Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421085 COMMON SUPPORTED GAB J1024-5875 D1-GE Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:47 2025 Page 1 Comtech, Inc. ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 11-8-8 10-6-0 5-3-0 1-2-8 Scale = 1:30.7 5x5 = 5 10.00 12 6 0-9-10 0-9-10 0-4-12 3x4 = 3x4 =13 12 11 10 14 10-6-0 10-6-0 Plate Offsets (X,Y)--[8:0-1-13, Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defl L/d **PLATES GRIP TCLL** 20.0 Plate Grip DOL 1.15 TC 0.04 Vert(LL) -0.00 8 120 MT20 244/190 n/r

10.0 **BCLL** 0.0 BCDL 10.0

2x6 SP No.1

TOP CHORD BOT CHORD 2x6 SP No.1 2x4 SP No.2 **OTHERS** 

**BRACING-**

Vert(CT)

Horz(CT)

-0.00

0.00

8

8

n/r

n/a

TOP CHORD **BOT CHORD**  Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 82 lb

FT = 20%

120

n/a

REACTIONS. All bearings 10-6-0.

(lb) -Max Horz 2=-158(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 8 except 13=-112(LC 12), 14=-106(LC 12), 11=-109(LC 13), 10=-103(LC 13)

ВС

WB

Matrix-S

0.03

0.04

Max Grav All reactions 250 lb or less at joint(s) 2, 8, 12, 13, 14, 11, 10

1.15

YES

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

TCDL

LUMBER-

1) Unbalanced roof live loads have been considered for this design.

Lumber DOL

Rep Stress Incr

Code IRC2021/TPI2014

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-1-1 to 3-3-0, Exterior(2N) 3-3-0 to 5-3-0 Corner(3R) 5-3-0 to 9-7-13, Exterior(2N) 9-7-13 to 11-7-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8 except (jt=lb) 13=112, 14=106, 11=109, 10=103.
- 10) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



April 2,2025



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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421086 J1024-5875 D2 COMMON GIRDER 2 Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:48 2025 Page 1 Comtech, Inc. ID:jUICoITBhC0nlVImGynse8yuZYG-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 10-6-0 Scale = 1:30.7 5x5 || 2 10.00 12 3 0-9-10 5 6 8 7 4 3x10 || 10-6-0 5-3-0

LOADING	G (psf)	SPACING- 2-0-	CSI		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.1	5 TC	0.17	Vert(LL)	-0.04	1-4	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL 1.1	5 BC	0.52	Vert(CT)	-0.08	1-4	>999	240		
BCLL	0.0 *	Rep Stress Incr NO	) WB	0.53	Horz(CT)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2021/TPI2014	Mat	rix-S	Wind(LL)	0.02	1-4	>999	240	Weight: 129 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP 2400F 2.0E 2x4 SP No.2 WFBS

REACTIONS. (size) 1=0-3-8, 3=0-3-8

Max Horz 1=112(LC 28)

Max Uplift 1=-175(LC 8), 3=-236(LC 9) Max Grav 1=3379(LC 2), 3=4630(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-3547/224, 2-3=-3553/224 **BOT CHORD** 1-4=-115/2610, 3-4=-115/2610

WFBS 2-4=-168/4354

## NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows: Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb)
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1457 lb down and 85 lb up at 2-0-12, 1457 lb down and 85 lb up at 4-0-12, 1457 lb down and 85 lb up at 6-0-12, and 1457 lb down and 85 lb up at 8-0-12, and 1464 lb down and 78 lb up at 10-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of

## LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 1-3=-20

Vert: 3=-1214(B) 5=-1207(B) 6=-1207(B) 7=-1207(B) 8=-1207(B)



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

April 2,2025

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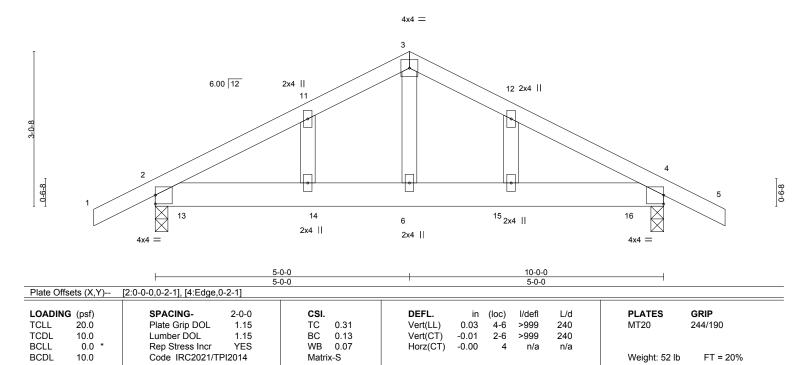
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421087 J1024-5875 G1-SG **GABLE** Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:48 2025 Page 1 Comtech, Inc. ID:a?v8YwYZEPnU3AGkeBHSMyz37aZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 5-0-0 , 10-0-0 11-2-8 5-0-0 5-0-0 1-2-8

Scale = 1:22.7



BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1

2x4 SP No.2 WERS **OTHERS** 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-3-0

Max Horz 2=-63(LC 17) Max Uplift 2=-121(LC 9), 4=-121(LC 8)

Max Grav 2=470(LC 1), 4=470(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-515/944. 3-4=-515/943 **BOT CHORD** 2-6=-676/387, 4-6=-676/387

**WEBS** 3-6=-572/240

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -1-2-8 to 3-2-5, Exterior(2N) 3-2-5 to 5-0-0, Corner(3R) 5-0-0 to 9-4-13, Exterior(2N) 9-4-13 to 11-2-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=121, 4=121.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-1-14 oc bracing.

April 2,2025



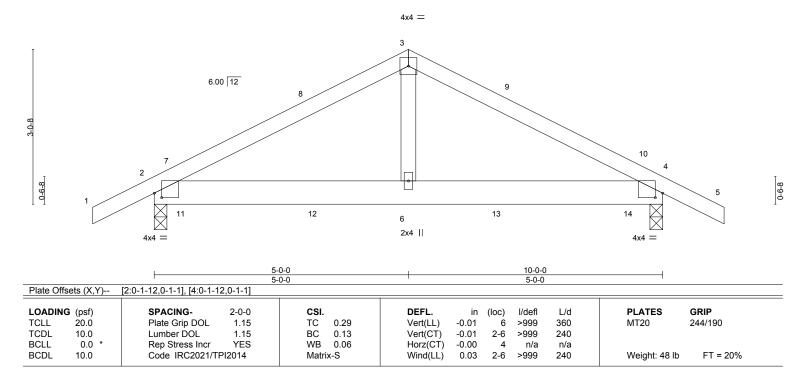
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Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421088 J1024-5875 G2 COMMON Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:26:49 2025 Page 1 Comtech, Inc. ID:a?v8YwYZEPnU3AGkeBHSMyz37aZ-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 5-0-0 , 1<u>0-0-0</u> 11-2-8 5-0-0 5-0-0 1-2-8

Scale = 1:22.7



BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1 2x4 SP No.2 WFBS

REACTIONS. (size) 2=0-3-0, 4=0-3-0

Max Horz 2=-40(LC 10)

Max Uplift 2=-91(LC 9), 4=-91(LC 8) Max Grav 2=470(LC 1), 4=470(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-515/826 3-4=-515/826

**BOT CHORD** 2-6=-585/387, 4-6=-585/387

WFBS 3-6=-495/240

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-0-0, Exterior(2R) 5-0-0 to 9-4-13, Interior(1) 9-4-13 to 11-2-8 zone; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 9-8-1 oc bracing.

April 2,2025

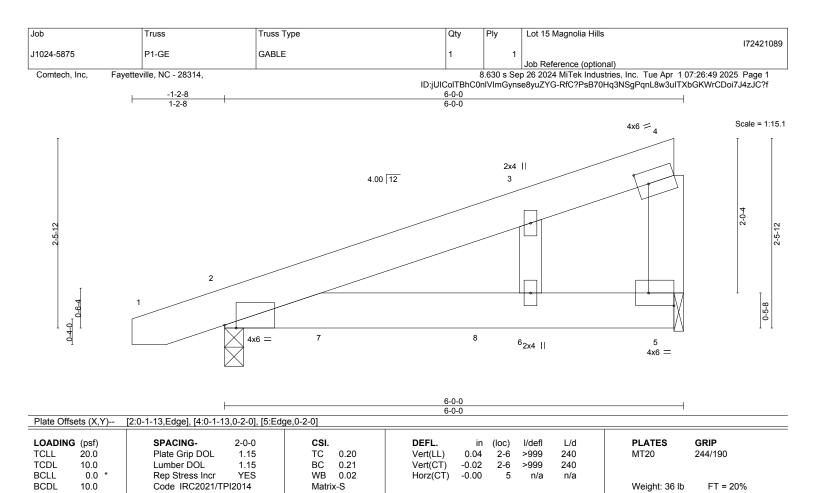


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BRACING-

TOP CHORD

**BOT CHORD** 

LUMBER-TOP CHORD

2x6 SP No.1 2x6 SP No.1

**BOT CHORD** 2x6 SP No.1 WFBS **OTHERS** 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 5=0-1-8 Max Horz 2=110(LC 8)

Max Uplift 2=-173(LC 8), 5=-138(LC 8)

Max Grav 2=299(LC 1), 5=219(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-5=-99/291

**BOT CHORD** 2-6=-290/89. 5-6=-290/89

## NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; Gable Roof; Common Truss; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-11-13 to 3-5-0, Exterior(2N) 3-5-0 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable studs spaced at 2-0-0 oc.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=173, 5=138.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing

except end verticals.

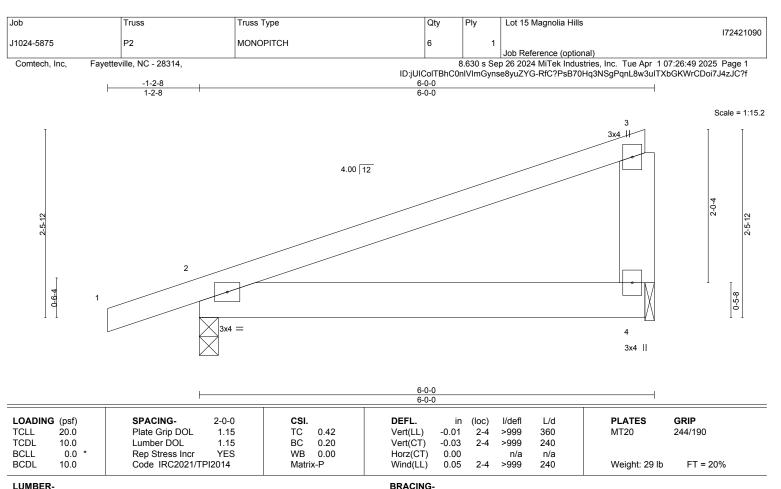
April 2,2025



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**BOT CHORD** 

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 2x6 SP No.1 BOT CHORD

2x6 SP No.1 WFBS

> (size) 2=0-3-0, 4=0-1-8 Max Horz 2=80(LC 8)

Max Uplift 2=-129(LC 8), 4=-93(LC 8) Max Grav 2=315(LC 1), 4=216(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 5-9-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60  $\,$
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=129.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

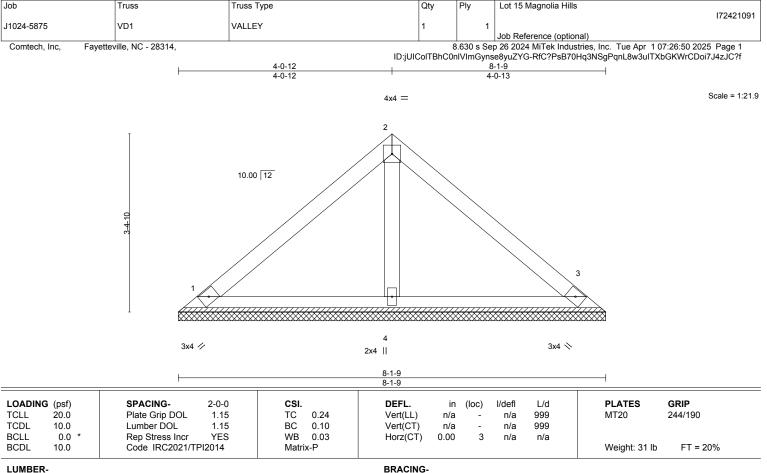
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BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS** 

(size) 1=8-1-9, 3=8-1-9, 4=8-1-9 Max Horz 1=-73(LC 8)

Max Uplift 1=-26(LC 13), 3=-32(LC 13)

Max Grav 1=169(LC 1), 3=169(LC 1), 4=247(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

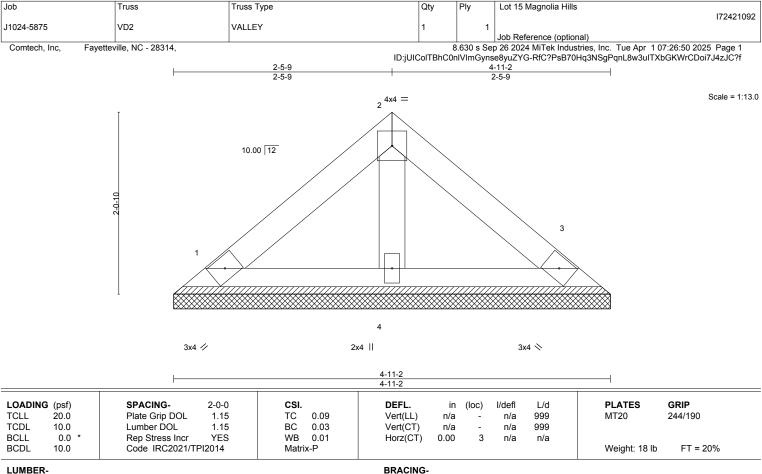
Rigid ceiling directly applied or 10-0-0 oc bracing.



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BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 2x4 SP No.2 **OTHERS** 

REACTIONS.

(size) 1=4-11-2, 3=4-11-2, 4=4-11-2

Max Horz 1=-41(LC 10)

Max Uplift 1=-14(LC 13), 3=-18(LC 13) Max Grav 1=95(LC 1), 3=95(LC 1), 4=139(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



Structural wood sheathing directly applied or 4-11-2 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

April 2,2025



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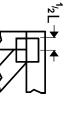
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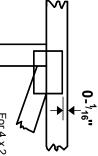


## Symbols

## PLATE LOCATION AND ORIENTATION



offsets are indicated and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths Center plate on joint unless x, y



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

₹

connector plates required direction of slots in This symbol indicates the

\* Plate location details available in MiTek software or upon request

## **PLATE SIZE**

4 × 4

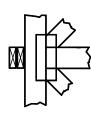
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

## LATERAL BRACING LOCATION



output. Use T or I bracing if indicated by text in the bracing section of the ndicated by symbol shown and/or

## **BEARING**



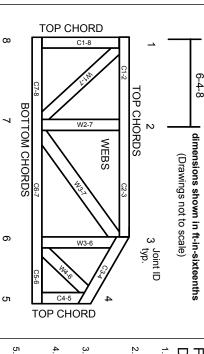
Min size shown is for crushing only number/letter where bearings occur reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

## Industry Standards:

DSB-22:

ANSI/TPI1: National Design Specification for Metal Installing, Restraining & Bracing of Metal Guide to Good Practice for Handling, Building Component Safety Information, Design Standard for Bracing. Plate Connected Wood Trusses Plate Connected Wood Truss Construction.

## Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

# Design General Notes

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

established by others section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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## MiTek



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

# General Safety Notes

## Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5 Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

œ

- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated



## **Trenco**

818 Soundside Rd Edenton, NC 27932

Re: J1024-5876

Lot 15 Magnolia Hills

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I72421093 thru I72421108

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



April 2,2025

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

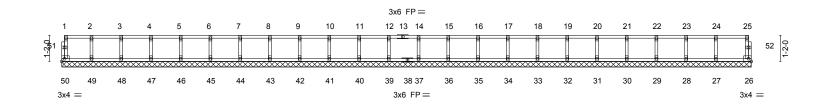
Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
	ET4	0.515			172421093
J1024-5876	EI1	GABLE	1	1	
					Job Reference (optional)

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:10 2025 Page 1

0-<u>1</u>-8

ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Scale = 1:51.7



 $+\frac{1.4-0}{1.4-0}+\frac{2.8-0}{1.4-0}+\frac{4.0-0}{1.4-0}+\frac{5.4-0}{1.4-0}+\frac{6.8-0}{1.4-0}+\frac{8.0-0}{1.4-0}+\frac{9.4-0}{1.4-0}+\frac{12-0-0}{1.4-0}+\frac{13-4-0}{1.4-0}+\frac{14-8-0}{1.4-0}+\frac{16-0-0}{1.4-0}+\frac{17-4-0}{1.4-0}+\frac{18-8-0}{1.4-0}+\frac{20-0-0}{1.4-0}+\frac{21-4-0}{1.4-0}+\frac{22-8-0}{1.4-0}+\frac{26-8-0}{1.4-0}+\frac{28-0-0}{$ 

LOADING	G (psf)	SPACING- 2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL 1.00	TC 0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL 1.00	BC 0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr YES	WB 0.03	Horz(CT)	0.00	26	n/a	n/a		
BCDL	5.0	Code IRC2021/TPI2014	Matrix-R						Weight: 127 lb	FT = 20%F, 11%E

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.3(flat) WFBS **OTHERS** 2x4 SP No.3(flat)

except end verticals.

Structural wood sheathing directly applied or 6-0-0 oc purlins,

TOP CHORD **BOT CHORD** 

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 30-11-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 50, 26, 49, 48, 47, 46, 45, 44, 43, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 31, 30, 29, 28, 27

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
					172421094
J1024-5876	ET2	GABLE	1	1	
					Joh Reference (ontional)

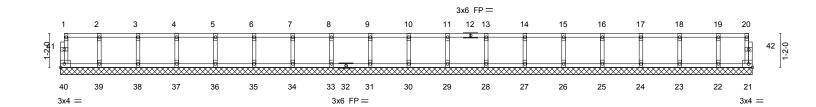
8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:11 2025 Page 1

0-11-8

ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Scale = 1:39.7

0-11-8



L	1-4-0	2-8-0	J <sub>1</sub> 4-0-0 <sub>1</sub>	5-4-0	6-8-0	8-0-0	9-4-0	10-8-0	12-0-0	13-4-0	14-8-0	16-0-0	17-4-0	18-8-0	20-0-0	21-4-0	22-8-0 23-10-0
	1-4-0	1-4-0	0 1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0 1-2-0
LOADI	NG (psf)		SPAC	ING-	2-0-0	)	CSI.			DEFL.	in	(loc)	l/defl	L/d	PL	ATES	GRIP
TCLL	40.0		Plate (	Grip DOL	1.00	)	TC	0.06		Vert(LL)	n/a	-	n/a	999	MT	20	244/190
TCDL	10.0		Lumbe	er DOL	1.00	)	BC	0.01		Vert(CT)	n/a	-	n/a	999			
BCLL	0.0		Rep S	tress Inc	YES	;	WB	0.03		Horz(CT)	0.00	21	n/a	n/a			
BCDL	5.0		Code	IRC2021	/TPI2014		Matri	x-R							We	ight: 99 lb	FT = 20%F, 11%E

LUMBER-BRACING-

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.3(flat) WFBS **OTHERS** 2x4 SP No.3(flat)

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 23-10-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 40, 21, 39, 38, 37, 36, 35, 34, 33, 31, 30, 29, 28, 27, 26, 25, 24, 23, 22

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



April 2,2025



Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421095 J1024-5876 ЕТ3 **GABLE** Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

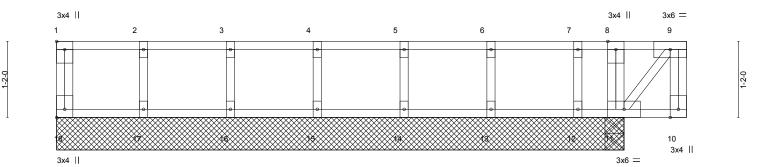
8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:11 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

8-0-0

8-7-0 8<sub>7</sub>8<sub>7</sub>8 9-4-0 9-8-0

8-2-8 0-8-8

Scale = 1:17.7



	-	1-4-0	-4-0	1-4-0	1	1-4-0	1-4	1-0	1	1-4-0	0-7-0 0-1-8 0-7	-8 <sup>1</sup> 0-4-0 <sup>1</sup>
Plate Offsets (X,Y) [1:Edge,0-1-8], [18:Edge,0-1-8]												
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.26	Vert(LL)	0.00	12	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.22	Vert(CT)	0.00	12	>999	360		
BCLL	0.0	Rep Stress Incr	NO	WB	0.05	Horz(CT)	-0.00	11	n/a	n/a		
BCDL	5.0	Code IRC2021/T	PI2014	Matrix	-S						Weight: 47 lb	FT = 20%F, 11%E

5-4-0

6-8-0

LUMBER-BRACING-

2-8-0

TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly applied or 9-8-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals.

2x4 SP No.3(flat) BOT CHORD WFBS

4-0-0

Rigid ceiling directly applied or 10-0-0 oc bracing. **OTHERS** 2x4 SP No.3(flat)

REACTIONS. All bearings 8-8-8.

(lb) - Max Uplift All uplift 100 lb or less at joint(s) except 12=-190(LC 4)

Max Grav All reactions 250 lb or less at joint(s) 18, 17, 16, 15, 14, 13 except 11=441(LC 1), 11=441(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

**WEBS** 8-11=-294/0

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 1.5x3 MT20 unless otherwise indicated.

1-4-0

- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 190 lb uplift at joint 12.
- 7) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 8) CAUTION, Do not erect truss backwards.

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 10-18=-10, 1-8=-100, 8-9=-220



April 2,2025



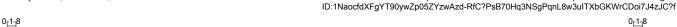
Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
					172421096
J1024-5876	ET4	GABLE	1	1	
					Joh Reference (ontional)

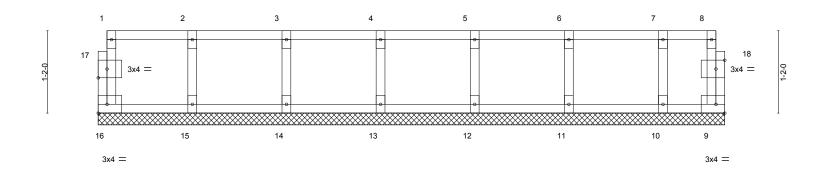
Comtech, Inc,

Fayetteville, NC - 28314,

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:11 2025 Page 1

Scale = 1:16.3





		1-4-0	2-8-0	4-0-0	5-4-0			5-8-0	1	8-0-0	8-10-8
	1	1-4-0	1-4-0	1-4-0	1-4-0	- '	1	-4-0	1	1-4-0	0-10-8
Plate Offset	s (X,Y)	[17:0-1-8,0-1-8], [18	:0-1-8,0-1-8]								
LOADING	(pst)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DC	OL 1.00	TC 0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC 0.01	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress In	ncr YES	WB 0.03	Horz(CT)	0.00	9	n/a	n/a		
BCDL	5.0	Code IRC202	21/TPI2014	Matrix-R						Weight: 39 I	b FT = 20%F, 11%E

LUMBER-TOP CHORD

2x4 SP No 1(flat) 2x4 SP No.1(flat)

**BOT CHORD** WFBS

2x4 SP No.3(flat)

**OTHERS** 2x4 SP No.3(flat) **BRACING-**

BOT CHORD

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 8-10-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 16, 9, 15, 14, 13, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
J1024-5876	F1	Floor	6	1	172421097
					Job Reference (optional)

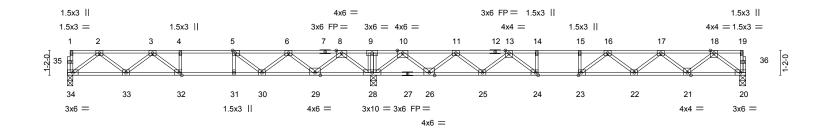
8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:12 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-1-8

HI 1-3-0 2-4-12

1-10-12

0-1-8 Scale = 1:54.2



	14-4-12	'	17-6-4	
Plate Offsets (X,Y)	[5:0-1-8,Edge], [23:0-1-8,Edge], [24:0-1	-8,Edge], [32:0-1-8,Edge]		
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.80	Vert(LL) -0.24 22-23 >858 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.90	Vert(CT) -0.33 22-23 >632 360	
BCLL 0.0	Rep Stress Incr YES	WB 0.63	Horz(CT) 0.05 20 n/a n/a	
BCDL 5.0	Code IRC2021/TPI2014	Matrix-S		Weight: 157 lb FT = 20%F, 11%E

LUMBER-TOP CHORD

2x4 SP No.1(flat) 2x4 SP No.1(flat)

BOT CHORD WFBS 2x4 SP No.3(flat) **BRACING-**

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

31-11-0

REACTIONS.

(size) 34=0-3-0, 28=0-3-8, 20=0-3-8

Max Grav 34=667(LC 3), 28=2085(LC 1), 20=846(LC 4)

14-4-12

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD

2-3=-1302/0, 3-4=-1928/111, 4-5=-1928/111, 5-6=-1510/467, 6-8=-413/1022, 8-9=0/2607, 9-10=0/2607, 10-11=-539/669, 11-13=-2039/130, 13-14=-3079/0,

14-15=-3079/0, 15-16=-3079/0, 16-17=-2782/0, 17-18=-1749/0

33-34=0/824, 32-33=0/1736, 31-32=-111/1928, 30-31=-111/1928, 29-30=-737/1111,

**BOT CHORD** 28-29=-1475/0, 26-28=-1179/0, 25-26=-363/1455, 24-25=0/2624, 23-24=0/3079,

22-23=0/3080, 21-22=0/2421, 20-21=0/1051

2-34=-1031/0, 2-33=0/622, 3-33=-565/55, 8-28=-1524/0, 8-29=0/1065, 10-28=-1764/0,

10-26=0/1316, 11-26=-1268/0, 11-25=0/830, 13-25=-850/0, 13-24=0/928, 14-24=-405/0, 18-20=-1315/0, 6-29=-1019/0, 6-30=0/675, 5-30=-864/0, 18-21=0/908, 17-21=-876/0,

17-22=0/469, 16-22=-389/12, 16-23=-394/235

WEBS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



April 2,2025



J	ob	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
J	1024-5876	F2	Floor	3	1	172421098
						Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

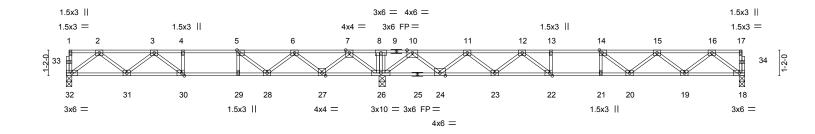
8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:13 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

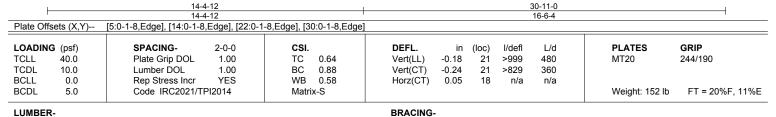
0-1-8

HI 1-3-0 2-4-12

2-1-12

0-1-8 Scale = 1:52.5





LUMBER-TOP CHORD

2x4 SP No.1(flat) 2x4 SP No.1(flat)

BOT CHORD WFBS

2x4 SP No.3(flat)

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing

REACTIONS.

**BOT CHORD** 

(size) 32=0-3-0, 26=0-3-8, 18=0-3-0

Max Grav 32=672(LC 3), 26=2015(LC 1), 18=795(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1313/0, 3-4=-1956/36, 4-5=-1956/36, 5-6=-1548/363, 6-7=-462/889, 7-8=0/2427, TOP CHORD

8-10=0/2427, 10-11=-551/643, 11-12=-1918/136, 12-13=-2754/0, 13-14=-2754/0,

14-15=-2516/0 15-16=-1624/0

31-32=0/830, 30-31=0/1753, 29-30=-36/1956, 28-29=-36/1956, 27-28=-616/1155, 26-27=-1313/0, 24-26=-1115/0, 23-24=-353/1398, 22-23=0/2425, 21-22=0/2754,

20-21=0/2754, 19-20=0/2235, 18-19=0/983 WEBS 2-32=-1039/0, 2-31=0/629, 3-31=-573/33, 3-30=-205/259, 7-26=-1501/0, 7-27=0/1046,

 $6-27 = -1001/0, \ 6-28 = 0/650, \ 5-28 = -826/0, \ 10-26 = -1668/0, \ 10-24 = 0/1226, \ 11-24 = -1181/0, \ 11-24 = -1181/0, \ 11-2$ 11-23=0/749, 12-23=-756/0, 12-22=0/799, 13-22=-330/0, 16-18=-1230/0, 16-19=0/835,

15-19=-795/0, 15-20=-1/365, 14-20=-336/121

1) Unbalanced floor live loads have been considered for this design.

2) All plates are 3x4 MT20 unless otherwise indicated.

3) Plates checked for a plus or minus 1 degree rotation about its center.

4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

5) CAUTION, Do not erect truss backwards.



April 2,2025



Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
J1024-5876	F3	Floor	10	1	172421099
0.02.00.0		1.00.			Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:13 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

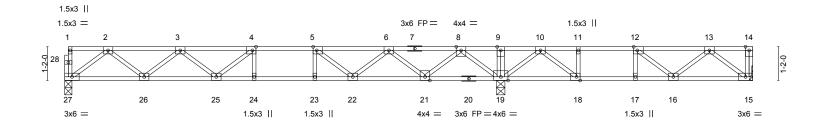
Structural wood sheathing directly applied or 6-0-0 oc purlins,

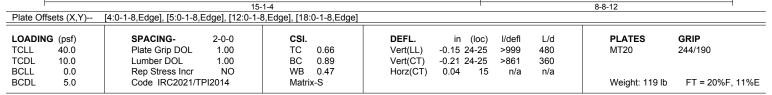
Rigid ceiling directly applied or 6-0-0 oc bracing

except end verticals.

0-1-8

H 1-3-0 1-11-12 1-10-4 Scale = 1:39.9





BOT CHORD

LUMBER-BRACING-TOP CHORD

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

WFBS 2x4 SP No.3(flat)

REACTIONS. (size) 15=Mechanical, 27=0-3-0, 19=0-3-8

Max Grav 15=1846(LC 4), 27=762(LC 10), 19=1476(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

14-15=-1426/0, 2-3=-1542/0, 3-4=-2351/0, 4-5=-2532/0, 5-6=-2102/0, 6-8=-1024/0, TOP CHORD

8-9=0/1105, 9-10=0/1105, 10-11=-805/300, 11-12=-805/300, 12-13=-721/79

BOT CHORD 26-27=0/938, 25-26=0/2116, 24-25=0/2532, 23-24=0/2532, 22-23=0/2532, 21-22=0/1715,

19-21=-8/283, 18-19=-670/345, 17-18=-300/805, 16-17=-300/805, 15-16=0/544 WFBS 2-27=-1174/0, 2-26=0/786, 3-26=-747/0, 3-25=0/351, 4-25=-392/14, 8-19=-1376/0,

8-21=0/991, 6-21=-934/0, 6-22=0/546, 5-22=-652/0, 10-19=-841/0, 10-18=0/828,

13-15=-682/0, 12-16=-107/282, 11-18=-358/0

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 15-27=-10, 1-14=-100

Concentrated Loads (lb) Vert: 14=-1400

April 2,2025



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
					172421100
J1024-5876	F4	Floor	1	1	
					Job Reference (optional)

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:13 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1-3-0

Scale = 1:26.2

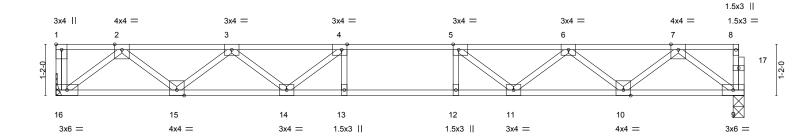


Plate Offse	Plate Offsets (X,Y) [1:Edge,0-1-8], [4:0-1-8,Edge], [5:0-1-8,Edge]										
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.46	Vert(LL)	-0.19 13-14	>956	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.85	Vert(CT)	-0.25 13-14	>728	360		
BCLL	0.0	Rep Stress Incr	YES	WB	0.43	Horz(CT)	0.05 9	n/a	n/a		
BCDL	5.0	Code IRC2021/TPI	2014	Matrix	:-S	, ,				Weight: 77 lb	FT = 20%F, 11%E

**BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat)

**WEBS** 2x4 SP No.3(flat)

REACTIONS. (size) 16=Mechanical, 9=0-3-0 Max Grav 16=848(LC 1), 9=842(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1741/0, 3-4=-2748/0, 4-5=-3091/0, 5-6=-2748/0, 6-7=-1741/0 TOP CHORD

**BOT CHORD** 15-16=0/1047, 14-15=0/2401, 13-14=0/3091, 12-13=0/3091, 11-12=0/3091, 10-11=0/2401,

9-10=0/1047

WEBS 2-16=-1314/0, 2-15=0/903, 3-15=-858/0, 3-14=0/513, 7-9=-1310/0, 7-10=0/904,

6-10=-859/0, 6-11=0/513, 5-11=-640/0, 4-14=-640/0

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION. Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
J1024-5876	EF	Floor	2	1	172421101
31024-3070	13	1 1001		!	Job Reference (optional)

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:14 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

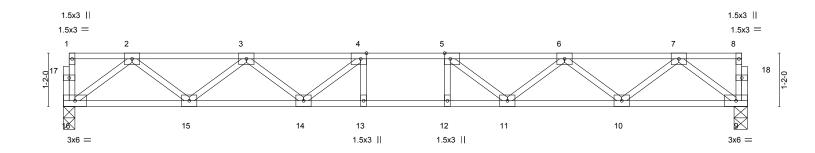
Structural wood sheathing directly applied or 6-0-0 oc purlins,

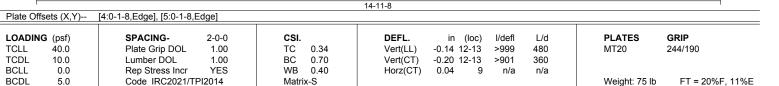
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

 $0_{1}$  8 Scale = 1:25.2

1-3-0 1-8-8  $H \vdash$ 





BOT CHORD

LUMBER-BRACING-TOP CHORD

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat)

2x4 SP No.3(flat) WFBS

REACTIONS. (size) 16=0-3-0, 9=0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1644/0, 3-4=-2552/0, 4-5=-2836/0, 5-6=-2552/0, 6-7=-1644/0 TOP CHORD

**BOT CHORD** 15-16=0/994, 14-15=0/2260, 13-14=0/2836, 12-13=0/2836, 11-12=0/2836, 10-11=0/2260,

9-10=0/994 WEBS 2-16=-1244/0, 2-15=0/847, 3-15=-802/0, 3-14=0/439, 7-9=-1244/0, 7-10=0/847,

6-10=-802/0, 6-11=0/439, 5-11=-533/0, 4-14=-533/0

## NOTES-

1) Unbalanced floor live loads have been considered for this design.

Max Grav 16=803(LC 1), 9=803(LC 1)

- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



April 2,2025



Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
			_		172421102
J1024-5876	F6	Floor	2	1	
					Job Reference (optional)

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:14 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



Scale = 1:23.3

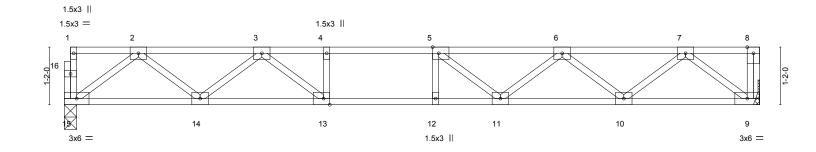


Plate Offsets (X,Y) [5:0-1-8,Edge], [13:0-1-8,Edge]										
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP						
TCLL 40.0 TCDL 10.0	Plate Grip DOL 1.00 Lumber DOL 1.00	TC 0.53 BC 0.81	Vert(LL) -0.16 11-12 >999 480 Vert(CT) -0.22 11-12 >772 360	MT20 244/190						
BCLL 0.0 BCDL 5.0	Rep Stress Incr YES Code IRC2021/TPI2014	WB 0.37 Matrix-S	Horz(CT) 0.03 9 n/a n/a	Weight: 70 lb FT = 20%F. 11%E						

**BRACING-**

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD 2x4 SP No.1(flat)

BOT CHORD 2x4 SP No.1(flat)

**WEBS** 2x4 SP No.3(flat)

REACTIONS. (size) 15=0-3-0, 9=Mechanical

Max Grav 15=755(LC 1), 9=761(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1505/0, 3-4=-2467/0, 4-5=-2467/0, 5-6=-2316/0, 6-7=-1525/0 TOP CHORD

**BOT CHORD** 14-15=0/935, 13-14=0/2068, 12-13=0/2467, 11-12=0/2467, 10-11=0/2092, 9-10=0/928 **WEBS** 2-15=-1171/0, 2-14=0/742, 3-14=-733/0, 3-13=0/690, 7-9=-1165/0, 7-10=0/776,

6-10=-739/0, 6-11=0/374, 5-11=-406/31, 4-13=-289/0

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION. Do not erect truss backwards.





Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421103 J1024-5876 F7 2 Floor Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:14 2025 Page 1 Comtech, Inc. ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 1-3-0 0-1-8 Scale = 1:10.6 3x4 =1 3x4 II 2 3 3x4 =4 1.5x3 II 3x4 =1.5x3 || 1.5x3 || 3x6 =6 3x6 = 5-8-0 Plate Offsets (X,Y)--[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,Edge], [9:0-1-8,0-1-8] LOADING (psf) SPACING-2-0-0 DEFL. (loc) I/defl L/d **PLATES GRIP TCLL** 40.0 Plate Grip DOL 1.00 TC 0.21 Vert(LL) -0.01 >999 480 MT20 244/190 TCDL 1.00 вс 0.12 Vert(CT) -0.01 10.0 Lumber DOL >999 360

Horz(CT)

**BRACING-**

TOP CHORD

**BOT CHORD** 

0.00

5

n/a

except end verticals.

n/a

Structural wood sheathing directly applied or 5-8-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

Weight: 29 lb

FT = 20%F, 11%E

LUMBER-

**BCLL** 

BCDL

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat)

**WEBS** 2x4 SP No.3(flat)

0.0

5.0

REACTIONS. (size) 8=Mechanical, 5=0-3-0 Max Grav 8=298(LC 1), 5=292(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

YES

TOP CHORD 2-3=-347/0

**BOT CHORD** 7-8=0/347, 6-7=0/347, 5-6=0/347

**WEBS** 2-8=-429/0, 3-5=-426/0

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.

Rep Stress Incr

Code IRC2021/TPI2014

- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

WB

Matrix-S

0.10

5) CAUTION, Do not erect truss backwards.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
J1024-5876	FG1	Floor	1	1	172421104
01024 0070		1 1001			Job Reference (optional)

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:15 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins,

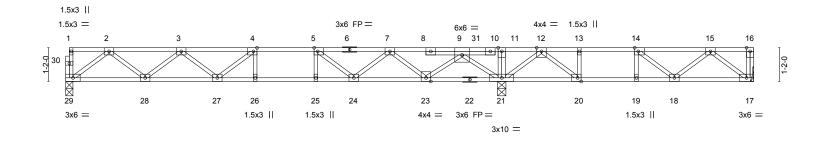
Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

0-1-8



1-10-4 Scale = 1:39.9



<b>—</b>	15-1-4 15-1-4									23-10-0 8-8-12				
late Offse	ets (X,Y)	[4:0-1-8,Edge], [5:0-1-8,E	Edge], [14:0-1	-8,Edge], [20:	0-1-8,Edge]									
OADING	i (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP			
CLL	40.0	Plate Grip DOL	1.00	TC	0.84	Vert(LL)	-0.15 26-27	>999	480	MT20	244/190			
CDL	10.0	Lumber DOL	1.00	BC	0.82	Vert(CT)	-0.21 26-27	>860	360					
CLL	0.0	Rep Stress Incr	NO	WB	0.52	Horz(CT)	0.04 21	n/a	n/a					
CDL	5.0	Code IRC2021/TI	PI2014	Matrix	x-S					Weight: 122 lb	FT = 20%F, 11%E			

**BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No 1(flat) BOT CHORD 2x4 SP No.1(flat)

WFBS 2x4 SP No.3(flat)

REACTIONS. (size) 17=Mechanical, 29=0-3-0, 21=0-3-8

Max Grav 17=1803(LC 4), 29=789(LC 10), 21=2349(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

16-17=-1421/0, 2-3=-1610/0, 3-4=-2485/0, 4-5=-2730/0, 5-7=-2350/0, 7-9=-1398/0, TOP CHORD

9-11=0/1413, 11-12=0/1393, 12-13=-607/486, 13-14=-607/486, 14-15=-613/181

BOT CHORD 28-29=0/975, 27-28=0/2213, 26-27=0/2730, 25-26=0/2730, 24-25=0/2730, 23-24=0/1988, 21-23=0/704, 20-21=-953/43, 19-20=-486/607, 18-19=-486/607, 17-18=-28/496

11-21=-594/0, 2-29=-1220/0, 2-28=0/827, 3-28=-784/0, 3-27=0/399, 4-27=-471/0, WFBS

9-21=-2210/0, 9-23=0/906, 7-23=-817/0, 7-24=0/514, 5-24=-577/0, 15-17=-622/36,

14-18=0/390, 12-21=-842/0, 12-20=0/952, 13-20=-410/0

## NOTES-

Pla LO TC TC BC ВС

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.
- 7) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 936 lb down at 14-2-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 8) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 17-29=-10, 1-16=-100

Concentrated Loads (lb)

Vert: 16=-1400 31=-861(F)



April 2,2025



Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
	F00	5,000			172421105
J1024-5876	FG2	FLOOR	1	1	
					Job Reference (optional)

1-3-0

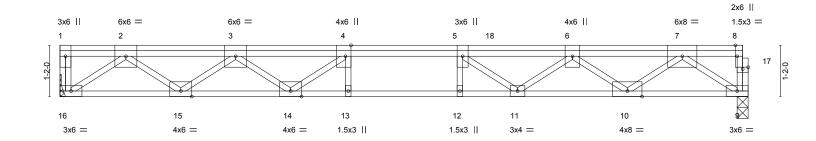
8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:15 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

Scale = 1:26.2



	15-8-0			
Plate Offsets (X,Y)				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 40.0	Plate Grip DOL 1.00	TC 0.71	Vert(LL) -0.24 12 >760 480	MT20 244/190
TCDL 10.0	Lumber DOL 1.00	BC 0.93	Vert(CT) -0.34 12 >545 360	
BCLL 0.0	Rep Stress Incr NO	WB 0.86	Horz(CT) 0.06 9 n/a n/a	
BCDL 5.0	Code IRC2021/TPI2014	Matrix-S		Weight: 99 lb FT = 20%F, 11%E

**BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP 2400F 2.0E(flat) WFBS

2x4 SP No.3(flat)

REACTIONS. (size) 16=Mechanical, 9=0-3-0 Max Grav 16=1114(LC 1), 9=1297(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-2517/0, 3-4=-4365/0, 4-5=-5379/0, 5-6=-5004/0, 6-7=-3095/0 TOP CHORD

**BOT CHORD** 15-16=0/1492, 14-15=0/3494, 13-14=0/5379, 12-13=0/5379, 11-12=0/5379, 10-11=0/4498,

9-10=0/1670

WFBS 2-16=-1832/0, 2-15=0/1302, 3-15=-1241/0, 3-14=0/1217, 4-14=-1435/0, 7-9=-2043/0,

7-10=0/1809, 6-10=-1782/0, 6-11=0/771, 5-11=-640/0

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION. Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 791 lb down at 9-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 9-16=-10, 1-8=-100 Concentrated Loads (lb) Vert: 18=-721(B)



April 2,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 15 Magnolia Hills
	F02				172421106
J1024-5876	FG3	Floor	1	1	Lete De Construction (Configuration)
			l		Job Reference (optional)

8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:16 2025 Page 1 ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1-3-0 1-8-8  $0_{1}$  8 Scale = 1:25.2  $H \vdash$ 

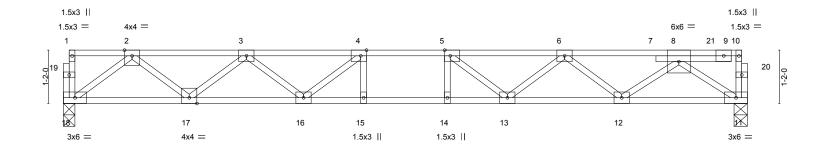


Plate Off	Plate Offsets (X,Y) [4:0-1-8,Edge], [5:0-1-8,Edge]											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.56	Vert(LL)	-0.16	14	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.94	Vert(CT)	-0.23	14	>779	360		
BCLL	0.0	Rep Stress Incr	NO	WB	0.43	Horz(CT)	0.05	11	n/a	n/a		
BCDL	5.0	Code IRC2021/T	PI2014	Matri	<-S						Weight: 77 lb	FT = 20%F, 11%E

BOT CHORD

LUMBER-**BRACING-**TOP CHORD

TOP CHORD 2x4 SP No 1(flat) BOT CHORD 2x4 SP No.1(flat)

WFBS 2x4 SP No.3(flat)

REACTIONS. (size) 18=0-3-0, 11=0-3-8

Max Grav 18=839(LC 1), 11=1650(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

10-11=-597/0, 2-3=-1735/0, 3-4=-2731/0, 4-5=-3093/0, 5-6=-2878/0, 6-8=-2095/0 TOP CHORD

**BOT CHORD** 17-18=0/1043, 16-17=0/2390, 15-16=0/3093, 14-15=0/3093, 13-14=0/3093, 12-13=0/2623,

11-12=0/1497

WFBS 2-18=-1306/0, 2-17=0/901, 3-17=-852/0, 3-16=0/502, 4-16=-633/0, 8-11=-1793/0,

8-12=0/757, 6-12=-690/0, 6-13=0/383, 5-13=-437/41

## NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are 3x4 MT20 unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 946 lb down at 14-2-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

## LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 11-18=-10, 1-10=-100

Concentrated Loads (lb) Vert: 21=-884(B)

April 2,2025



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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421107 J1024-5876 FLOOR GIRDER FG4 Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:16 2025 Page 1 Comtech, Inc. ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 1-3-0 Scale = 1:10.7 3x6 II 4x6 II 4x6 || 3x6 | | 2 3 4 1.5x3 || 3x6 =1.5x3 || 5 3x6 = 5-8-8 Plate Offsets (X,Y)--[2:0-3-0,Edge], [3:0-3-0,Edge] LOADING (psf) SPACING-2-0-0 CSI. DEFL. (loc) I/defl L/d **PLATES GRIP TCLL** 40.0 Plate Grip DOL 1.00 TC 0.13 Vert(LL) -0.02 6-7 >999 480 MT20 244/190 TCDL 0.30 Vert(CT) -0.03 10.0 Lumber DOL 1.00 ВС 6-7 >999 360 WB 0.37 **BCLL** 0.0 Rep Stress Incr NO Horz(CT) 0.01 n/a n/a **BCDL** 5.0 Code IRC2021/TPI2014 Matrix-S Weight: 37 lb FT = 20%F, 11%E

**BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1(flat) TOP CHORD 2x4 SP No.1(flat) BOT CHORD

WFBS 2x4 SP No.3(flat)

REACTIONS. (size) 8=Mechanical, 5=Mechanical

Max Grav 8=961(LC 1), 5=961(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-1310/0

**BOT CHORD** 7-8=0/1310, 6-7=0/1310, 5-6=0/1310

2-8=-1585/0, 3-5=-1585/0 **WEBS** 

# NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 689 lb down at 1-10-4, and 689 lb down at 3-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf) Vert: 5-8=-10, 1-4=-100

Concentrated Loads (lb)

Vert: 2=-661(B) 3=-661(B)



Structural wood sheathing directly applied or 5-8-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

April 2,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Edenton, NC 27932

Job Truss Truss Type Qty Ply Lot 15 Magnolia Hills 172421108 J1024-5876 FLOOR GIRDER FG5 Job Reference (optional) Fayetteville, NC - 28314, 8.630 s Sep 26 2024 MiTek Industries, Inc. Tue Apr 1 07:27:16 2025 Page 1 Comtech, Inc. ID:1NaocfdXFgYT90ywZp05ZYzwAzd-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 1-3-0 Scale = 1:10.6 3x6 II 2x6 II 3x6 II 3x6 II 2 3 4 3x4 = 1.5x3 || 1.5x3 II 3x6 =6 3x6 = 5-8-0 Plate Offsets (X,Y)--[4:0-3-0,Edge], [9:0-1-8,0-0-8] LOADING (psf) 2-0-0 SPACING-CSI. DEFL. (loc) I/defl L/d **PLATES GRIP TCLL** 40.0 Plate Grip DOL 1.00 TC 0.19 Vert(LL) -0.01 6-7 >999 480 MT20 244/190 TCDL Vert(CT) 10.0 Lumber DOL 1.00 ВС 0.25 -0.02 6-7 >999 360 WB 0.29 **BCLL** 0.0 Rep Stress Incr NO Horz(CT) 0.01 n/a n/a **BCDL** 5.0 Code IRC2021/TPI2014 Matrix-S Weight: 36 lb FT = 20%F, 11%E

**BRACING-**

TOP CHORD

**BOT CHORD** 

LUMBER-

2x4 SP No.1(flat) TOP CHORD 2x4 SP No.1(flat)

BOT CHORD WFBS

2x4 SP No.3(flat)

REACTIONS. (size) 8=Mechanical, 5=0-3-8

Max Grav 8=821(LC 1), 5=807(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1040/0

**BOT CHORD** 7-8=0/1040, 6-7=0/1040, 5-6=0/1040

**WEBS** 2-8=-1258/0, 3-5=-1250/0

# NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 226 lb down at 1-10-4, and 228 lb down at 3-10-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 5-8=-10, 1-4=-220 Concentrated Loads (lb)

Vert: 2=-198(F) 3=-198(F)



Structural wood sheathing directly applied or 5-8-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

April 2,2025



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

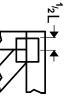
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



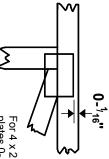
Edenton, NC 27932

# Symbols

# PLATE LOCATION AND ORIENTATION



offsets are indicated and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths Center plate on joint unless x, y



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates required direction of slots in This symbol indicates the

\* Plate location details available in MiTek software or upon request

# **PLATE SIZE**

4 × 4

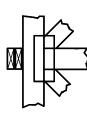
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

# LATERAL BRACING LOCATION



output. Use T or I bracing if indicated by text in the bracing section of the ndicated by symbol shown and/or

# **BEARING**



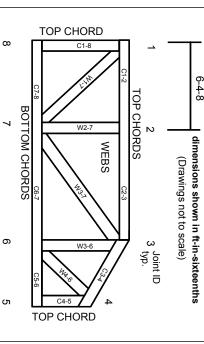
Min size shown is for crushing only number/letter where bearings occur reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

# Industry Standards:

ANSI/TPI1: National Design Specification for Metal Installing, Restraining & Bracing of Metal Guide to Good Practice for Handling, Building Component Safety Information, Design Standard for Bracing. Plate Connected Wood Trusses Plate Connected Wood Truss Construction.

DSB-22:

# Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

# Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

# Design General Notes

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

established by others section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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# MiTek



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

# General Safety Notes

# Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- ယ Never exceed the design loading shown and never stack materials on inadequately braced trusses
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other

5

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

œ

- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated

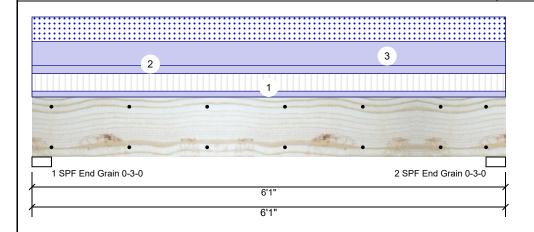


Date: 4/1/2025 Input by: Neal Baggett

Job Name: 15 MAGNOLIA HILLS Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM1

Level: Level



Application: Design Method:

**Building Code:** 

Load Sharing:

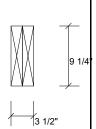
Deck:

ASD

No

**IBC/IRC 2015** 

Not Checked



Page 1 of 12

## Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal
	_

- II Temperature: Temp <= 100°F

# Reactions UNPATTERNED Ib (Uplift)

**Bearings** 

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	803	1771	1116	0	0
2	Vertical	803	1771	1116	0	0

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4299 ft-lb	3' 1/2"	14423 ft-lb	0.298 (30%)	D+0.75(L+S)	L
Unbraced	4299 ft-lb	3' 1/2"	10861 ft-lb	0.396 (40%)	D+0.75(L+S)	L
Shear	2138 lb	1' 1/4"	7943 lb	0.269 (27%)	D+0.75(L+S)	L
LL Defl inch	0.031 (L/2185)	3' 1/2"	0.143 (L/480)	0.220 (22%)	0.75(L+S)	L
TL Defl inch	0.070 (L/980)	3' 1/2"	0.190 (L/360)	0.367 (37%)	D+0.75(L+S)	L

# **Analysis Results**

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.

Self Weight

- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

 vertica	1 803	1771	1116	U	U

,						
Bearing Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 3.000" End Grain	Vert	36%	1771 / 1439	3210	L	D+0.75(L+S)
2 - SPF 3.000" End Grain	Vert	36%	1771 / 1439	3210	L	D+0.75(L+S)

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	88 PLF	264 PLF	0 PLF	0 PLF	0 PLF	F1
2	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
3	Uniform			Тор	367 PLF	0 PLF	367 PLF	0 PLF	0 PLF	
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

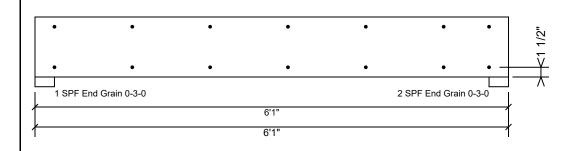
This design is valid until 6/28/2026

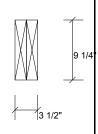
Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

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Version 23.40.705 Powered by iStruct™ Dataset: 24051401.1529

Client: Date: 4/1/2025 Page 2 of 12 Project: Input by: Neal Baggett isDesign Address: Job Name: 15 MAGNOLIA HILLS Project #: Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED Level: Level BM1





# Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	, , , , , , , , , , , , , , , , , , , ,
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

# Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

# Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us

Manufacturer Info





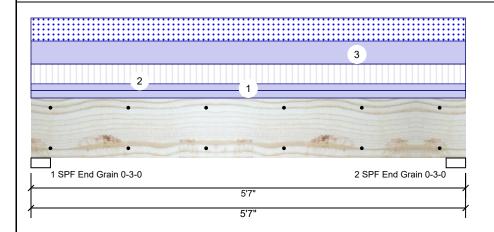
4/1/2025 Input by: Neal Baggett

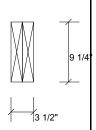
Job Name: 15 MAGNOLIA HILLS

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM<sub>2</sub>

Level: Level





Page 3 of 12

## **Member Information**

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Normal - II Temperature: Temp <= 100°F

Application: Design Method: ASD **Building Code: IBC/IRC 2015** 

Load Sharing: No Deck: Not Checked

# Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	877	1662	1016	0	0
2	Vertical	877	1662	1016	0	0

# **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3742 ft-lb	2'9 1/2"	14423 ft-lb	0.259 (26%)	D+0.75(L+S)	L
Unbraced	3742 ft-lb	2'9 1/2"	11402 ft-lb	0.328 (33%)	D+0.75(L+S)	L
Shear	1960 lb	4'6 3/4"	7943 lb	0.247 (25%)	D+0.75(L+S)	L
LL Defl inch	0.024 (L/2564)	2'9 1/2"	0.130 (L/480)	0.187 (19%)	0.75(L+S)	L
TL Defl inch	0.053 (L/1182)	2'9 1/2"	0.174 (L/360)	0.305 (30%)	D+0.75(L+S)	L

7 ti lai y 515	/ totaai	Location	/ IIIOWCG	Capacity	COITID.	Ouse
Moment	3742 ft-lb	2'9 1/2"	14423 ft-lb	0.259 (26%)	D+0.75(L+S)	L
Unbraced	3742 ft-lb	2'9 1/2"	11402 ft-lb	0.328 (33%)	D+0.75(L+S)	L
Shear	1960 lb	4'6 3/4"	7943 lb	0.247 (25%)	D+0.75(L+S)	L
LL Defl inch	0.024 (L/2564)	2'9 1/2"	0.130 (L/480)	0.187 (19%)	0.75(L+S)	L
TL Defl inch	0.053 (L/1182)	2'9 1/2"	0.174 (L/360)	0.305 (30%)	D+0.75(L+S)	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

Bearings	5						
Bearing	Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.000"	Vert	35%	1662 / 1420	3081	L	D+0.75(L+S)

End Grain						,
2 - SPF End Grain	3.000"	Vert	35%	1662 / 1420	3081 L	D+0.75(L+S)

I	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
l	1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
l	2	Uniform			Тор	104 PLF	314 PLF	0 PLF	0 PLF	0 PLF	FG3
l	3	Uniform			Тор	364 PLF	0 PLF	364 PLF	0 PLF	0 PLF	A3
١		Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intented

# Handling & Installation

- LVL beams must not be cut or drilled
   Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

application, and to verify the dimensions and loads.	approvals
Lumber  1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	Damaged Beams must not be used     Design assumes top edge is laterally restrained     Provide lateral support at bearing points to avoid lateral displacement and rotation

- This design is valid until 6/28/2026

isDesign

Client: Project: Address: Date: 4/1/2025 Input by:

Neal Baggett Job Name: 15 MAGNOLIA HILLS

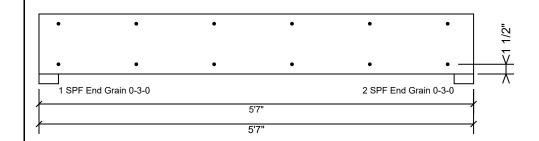
Project #:

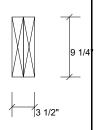
**Kerto-S LVL BM2** 

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 4 of 12

# Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	,	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
CM	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

# Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us



Date: 4/1/2025 Input by: Neal Baggett

Project #:

1-SPF 3.000"

2 - SPF 3.000"

End Grain

End Grain Vert

Vert

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM<sub>3</sub>

Level: Level

Job Name: 15 MAGNOLIA HILLS

Page 5 of 12

Wind

2267 / 1906

2267 / 1906

4172 L

4172 L

0

0

Const

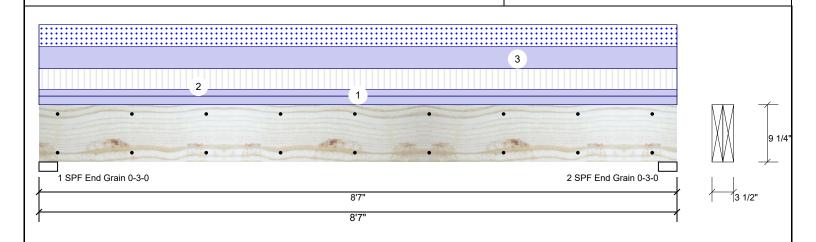
Ld. Comb.

D+0.75(L+S)

D+0.75(L+S)

0

0



## Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Direction Live Type: Dead Snow Plies: 2 Design Method: ASD 1227 2267 1313 Vertical Moisture Condition: Dry **Building Code: IBC/IRC 2015** 2 Vertical 1227 2267 1313 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case

# **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Allalysis	Actual	Location	Allowed	Capacity	COITID.	Case
Moment	8188 ft-lb	4'3 1/2"	14423 ft-lb	0.568 (57%)	D+0.75(L+S)	L
Unbraced	8188 ft-lb	4'3 1/2"	8625 ft-lb	0.949 (95%)	D+0.75(L+S)	L
Shear	3185 lb	7'6 3/4"	7943 lb	0.401 (40%)	D+0.75(L+S)	L
LL Defl inch	0.112 (L/883)	4'3 9/16"	0.205 (L/480)	0.544 (54%)	0.75(L+S)	L
TL Defl inch	0.244 (L/403)	4'3 9/16"	0.274 (L/360)	0.893 (89%)	D+0.75(L+S)	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Lateral siende	erness ratio based on single									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Uniform			Тор	95 PLF	286 PLF	0 PLF	0 PLF	0 PLF	F3
3	Uniform			Тор	306 PLF	0 PLF	306 PLF	0 PLF	0 PLF	B2
	Self Weight				7 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

  Damaged Beams must not be used
- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturor Info

Manufacturer into	
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	

Client: Date: 4/1/2025 Page 6 of 12 Project: Input by: Neal Baggett isDesign Address: Job Name: 15 MAGNOLIA HILLS Project #: 1.750" X 9.250" 2-Ply - PASSED Level: Level **Kerto-S LVL** BM<sub>3</sub> 1 SPF End Grain 0-3-0 2 SPF End Grain 0-3-0 8'7' 8'7 Multi-Ply Analysis Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6". Capacity 0.0 PLF 163.7 PLF Yield Limit per Foot Yield Limit per Fastener 81.9 lb. См Yield Mode IV Edge Distance 1 1/2" Min. End Distance 3" Load Combination **Duration Factor** 1.00 For flat roofs provide proper drainage to prevent ponding Manufacturer Info Notes NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Handling & Installation Metsä Wood L. UV. beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

This design is valid until 6/28/2026

www.metsawood.com/us

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation



4/1/2025 Input by: Neal Baggett

Job Name: 15 MAGNOLIA HILLS

Project #:

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL** BM4

Application:

Design Method:

**Building Code:** 

Load Sharing:

Deck:

Floor

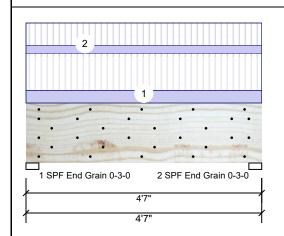
ASD

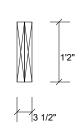
No

**IBC/IRC 2015** 

Not Checked

Level: Level





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# **Member Information**

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal

- II Temperature: Temp <= 100°F

# Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	2278	783	0	0	0
2	Vertical	2278	783	0	0	0

# **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2957 ft-lb	2'3 1/2"	26999 ft-lb	0.110 (11%)	D+L	L
Unbraced	2957 ft-lb	2'3 1/2"	21560 ft-lb	0.137 (14%)	D+L	L
Shear	2727 lb	1'5"	10453 lb	0.261 (26%)	D+L	L
LL Defl inch	0.010 (L/5285)	2'3 9/16"	0.105 (L/480)	0.091 (9%)	L	L
TL Defl inch	0.013 (L/3932)	2'3 9/16"	0.140 (L/360)	0.092 (9%)	D+L	L

# **Bearings**

Bearing	Length	Dir.	Cap. R	eact D/L lb	Total	Ld. Case	Ld. Com
1 - SPF End Grain	3.000"	Vert	35%	783 / 2278	3061	L	D+L
2 - SPF End Grain	3.000"	Vert	35%	783 / 2278	3061	L	D+L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 6 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ı	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
	1	Uniform			Far Face	206 PLF	618 PLF	0 PLF	0 PLF	0 PLF	FG3
2	2	Uniform			Near Face	125 PLF	376 PLF	0 PLF	0 PLF	0 PLF	FG2
		Self Weight				11 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

|--|

- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- This design is valid until 6/28/2026



Date: 4/1/2025 Input by:

Neal Baggett Job Name: 15 MAGNOLIA HILLS

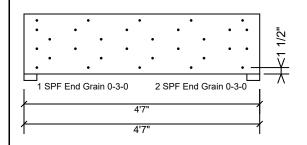
Project #:

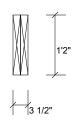
**Kerto-S LVL BM4** 

1.750" X 14.000"

2-Ply - PASSED

Level: Level





Page 8 of 12

# Multi-Ply Analysis

Fasten all plies using 6 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	· · · · · · · · · · · · · · · · · · ·	,
Capacity	83.9 %	
Load	412.0 PLF	
Yield Limit per Foot	491.1 PLF	
Yield Limit per Fastener	81.9 lb.	
CM	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination	D+L	
Duration Factor	1.00	

# Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

# Manufacturer Info

(800) 622-5850

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 www.metsawood.com/us



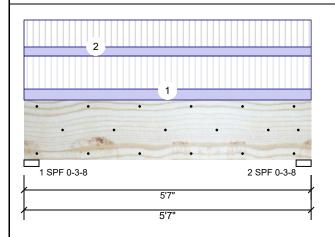
Date: 4/1/2025 Input by: Neal Baggett

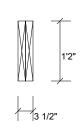
Job Name: 15 MAGNOLIA HILLS

Project #:

1.750" X 14.000" 2-Ply - PASSED **Kerto-S LVL** BM5

Level: Level





Wind

Const

Page 9 of 12

## **Member Information**

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance: Temperature:

Deck: Normal - II Temp <= 100°F

Application: Floor Design Method: ASD **Building Code: IBC/IRC 2015** Load Sharing: No Not Checked

## Reactions UNPATTERNED Ib (Uplift) Brg Direction Snow

1	Vertical	1789	625	0	0	0
2	Vertical	1789	625	0	0	0

# **Bearings**

Bearing Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF 3.500"	Vert	46%	625 / 1789	2414	L	D+L
2 - SPF 3.500"	Vert	46%	625 / 1789	2414	L	D+L

# **Analysis Results**

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2840 ft-lb	2'9 1/2"	26999 ft-lb	0.105 (11%)	D+L	L
Unbraced	2840 ft-lb	2'9 1/2"	18832 ft-lb	0.151 (15%)	D+L	L
Shear	2162 lb	1'5 1/2"	10453 lb	0.207 (21%)	D+L	L
LL Defl inch	0.011 (L/5509)	2'9 1/2"	0.128 (L/480)	0.087 (9%)	L	L
TL Defl inch	0.015 (L/4083)	2'9 1/2"	0.171 (L/360)	0.088 (9%)	D+L	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top must be laterally braced at end bearings.
- 6 Bottom must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Near Face	117 PLF	353 PLF	0 PLF	0 PLF	0 PLF	F8	
2	Uniform			Far Face	96 PLF	288 PLF	0 PLF	0 PLF	0 PLF	F1A	
	Self Weight				11 PI F						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive
- Handling & Installation andling & Installation.

  LVL beams must not be cut or drilled

  Refer to manufacturer's product information regarding installation requirements, multi-ply
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

	iastering details, bearn strength va
	approvals
3.	Damaged Beams must not be used
4	Docian accumes top adap is laterally

- Design assumes top edge is laterally restrained
   Provide lateral support at bearing points to avoid lateral displacement and rotation
- This design is valid until 6/28/2026



Date: 4/1/2025 Input by: Neal Baggett

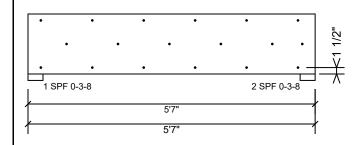
Job Name: 15 MAGNOLIA HILLS

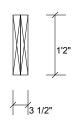
Project #:

**Kerto-S LVL BM5** 

1.750" X 14.000" 2-Ply - PASSED

Level: Level





Page 10 of 12

# Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	· · · · · · · ·	,
Capacity	95.7 %	
Load	235.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination	D+L	
Duration Factor	1.00	

# Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- This design is valid until 6/28/2026
- 6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850
(800) 622-5850
www.metsawood.com/us



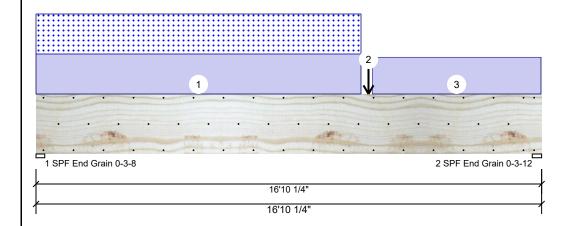
4/1/2025 Input by:

Neal Baggett Job Name: 15 MAGNOLIA HILLS

Project #:

**Kerto-S LVL GDH-SL** 

1.750" X 24.000" 2-Ply - PASSED Level: Level



ASD

No

**IBC/IRC 2015** 

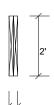
Not Checked

Application: Design Method:

**Building Code:** 

Load Sharing:

Deck:



Page 11 of 12

## Member Information

Туре:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal -
Temperature:	Tomn <-

Ш Temp <= 100°F

# Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	4214	3835	0	0
2	Vertical	0	5855	4514	0	0

# **Analysis Results**

Analysis         Actual         Location         Allowed         Capacity         Comb.         Case           Moment         52918 ft-lb         11'1"         84163 ft-lb         0.629 (63%)         D+S         L           Unbraced         52918 ft-lb         11'1"         53123 ft-lb         0.996 (100%)         D+S         L           Shear         9791 lb         14'6 1/2"         20608 lb         0.475 (48%)         D+S         L           LL Defl inch         0.163 (L/1210)         9'2 5/16"         0.410 (L/480)         0.397 (40%)         S         L           TL Defl inch         0.345 (L/571)         9'2 9/16"         0.547 (L/360)         0.631 (63%)         D+S         L	•						
Unbraced         52918 ft-lb         11'1"         53123 ft-lb         0.996 (100%)         D+S         L           Shear         9791 lb         14'6 1/2"         20608 lb         0.475 (48%)         D+S         L           LL Defl inch         0.163 (L/1210)         9'2 5/16"         0.410 (L/480)         0.397 (40%)         S         L	Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Shear 9791 lb 14'6 1/2" 20608 lb 0.475 (48%) D+S L LL Defl inch 0.163 (L/1210) 9'2 5/16" 0.410 (L/480) 0.397 (40%) S L	Moment	52918 ft-lb	11'1"	84163 ft-lb	0.629 (63%)	D+S	L
LL Defl inch 0.163 (L/1210) 9'2 5/16" 0.410 (L/480) 0.397 (40%) S L	Unbraced	52918 ft-lb	11'1"	53123 ft-lb		D+S	L
	Shear	9791 lb	14'6 1/2"	20608 lb	0.475 (48%)	D+S	L
TL Defl inch 0.345 (L/571) 9'2 9/16" 0.547 (L/360) 0.631 (63%) D+S L	LL Defl inch	0.163 (L/1210)	9'2 5/16"	0.410 (L/480)	0.397 (40%)	S	L
	TL Defl inch	0.345 (L/571)	9'2 9/16"	0.547 (L/360)	0.631 (63%)	D+S	L

# **Design Notes**

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 3'2 13/16" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

L	Bearings	5						
ſ	Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
	1 - SPF End Grain	3.500"	Vert	78%	4214 / 3835	8049	L	D+S
l	2 - SPF End	3.750"	Vert	94%	5855 / 4514	10369	L	D+S

e Eater a deriaci nece fatto bacca en emigio piy watar.											
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
	1	Part. Uniform	0-0-0 to 10-10-0		Тор	272 PLF	0 PLF	272 PLF	0 PLF	0 PLF	C2
	2	Point	11-1-0		Тор	5402 lb	0 lb	5402 lb	0 lb	0 lb	C4
		Bearing Length	0-3-11								
	3	Part. Uniform	11-2-8 to 16-10-0		Тор	250 PLF	0 PLF	0 PLF	0 PLF	0 PLF	FLOOR, WALL & B1-GE
		Self Weight				19 PLF					

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- LVL beams must not be cut or drilled
  Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
  2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
  Provide lateral support at bearing points to avoid
  lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Grain

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Manufacturer Info



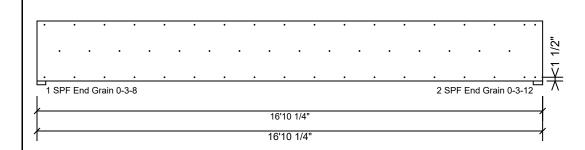
Date: 4/1/2025 Input by: Neal Baggett

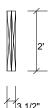
Job Name: 15 MAGNOLIA HILLS

Project #:

**Kerto-S LVL** 1.750" X 24.000" 2-Ply - PASSED **GDH-SL** 

Level: Level





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# Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	•	,
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

# Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
   LVL not to be treated with fire retardant or corrosive

# Handling & Installation

- Handling & Installation

  1. UVI beams must not be cut or drilled

  2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

  3. Damaged Beams must not be used

  4. Design assumes top edge is laterally restrained

  5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850 www.metsawood.com/us

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851