Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 57911 JOB: 25-2454-R01 JOB NAME: LOT 0.0021 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 29 Truss Design(s)

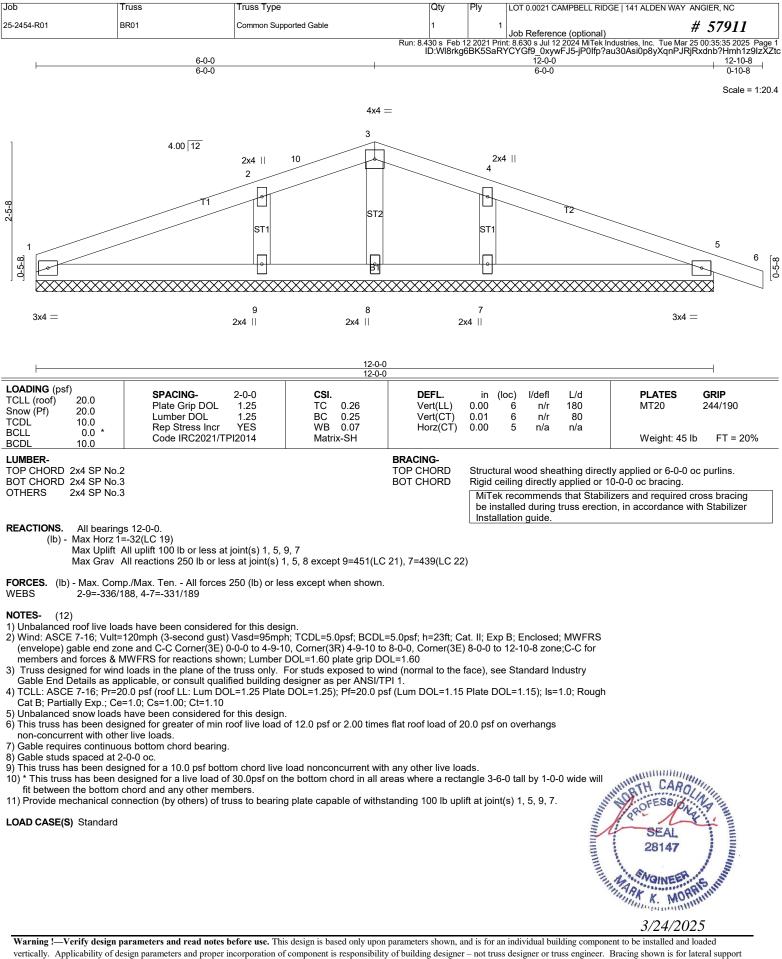
Trusses:

BR01, BR02, R01, R02, R02A, R02B, R03, R03A, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, SP01, SP02, SP03, SV01, SV02, VT01, VT02, VT03, VT04

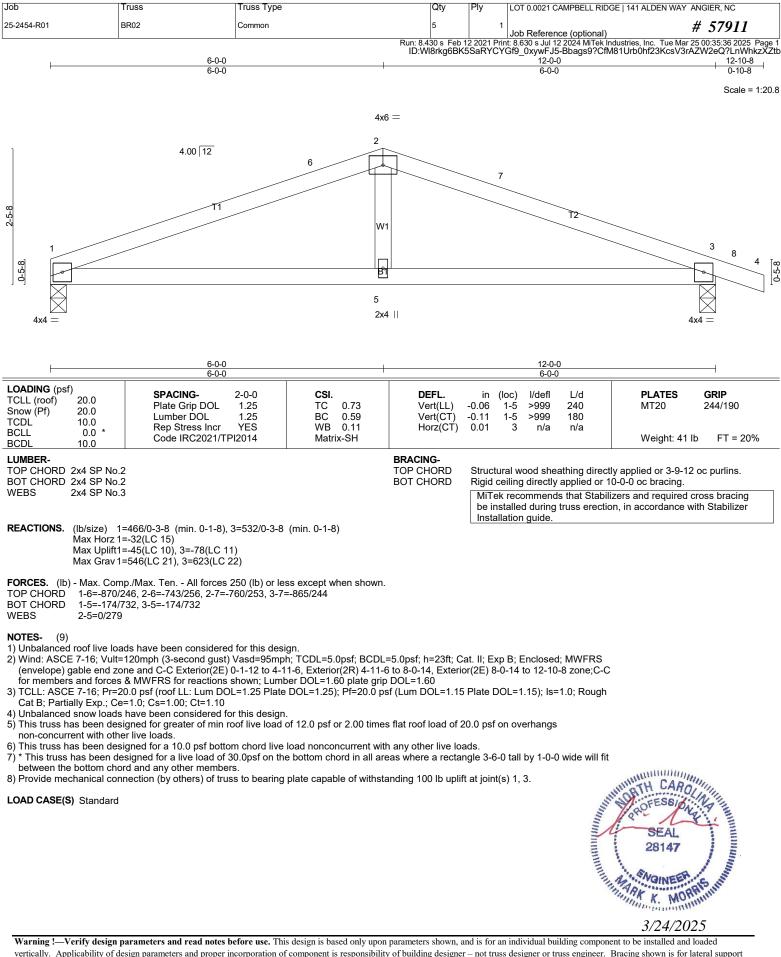


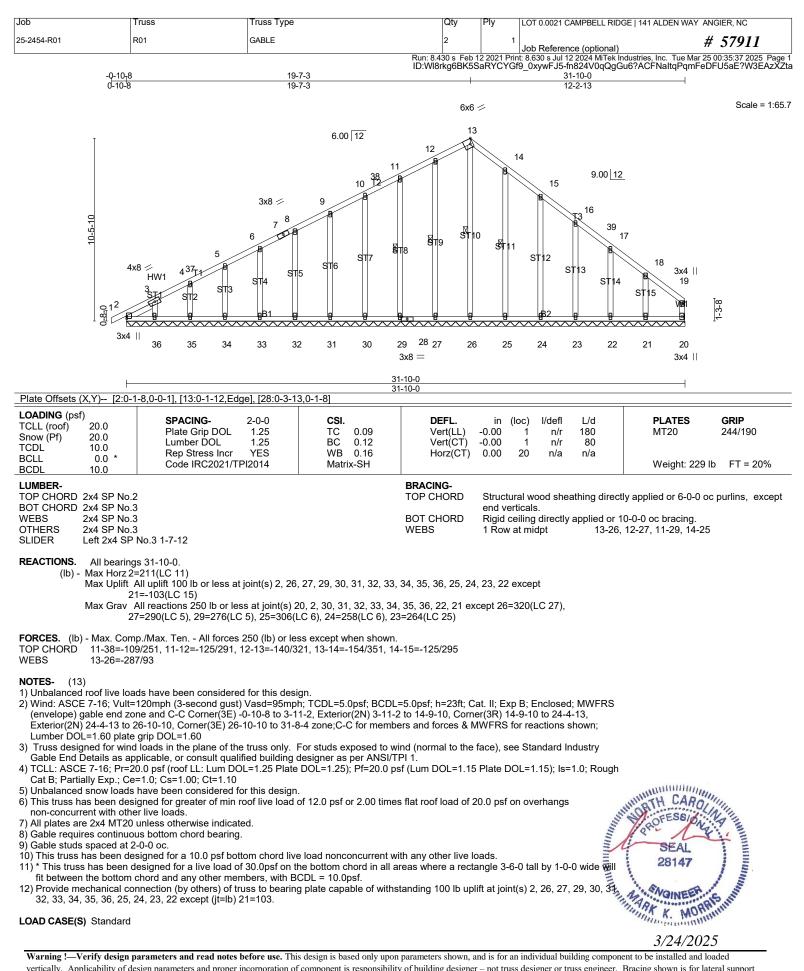
My license renewal date for the state of North Carolina is 12/31/2025

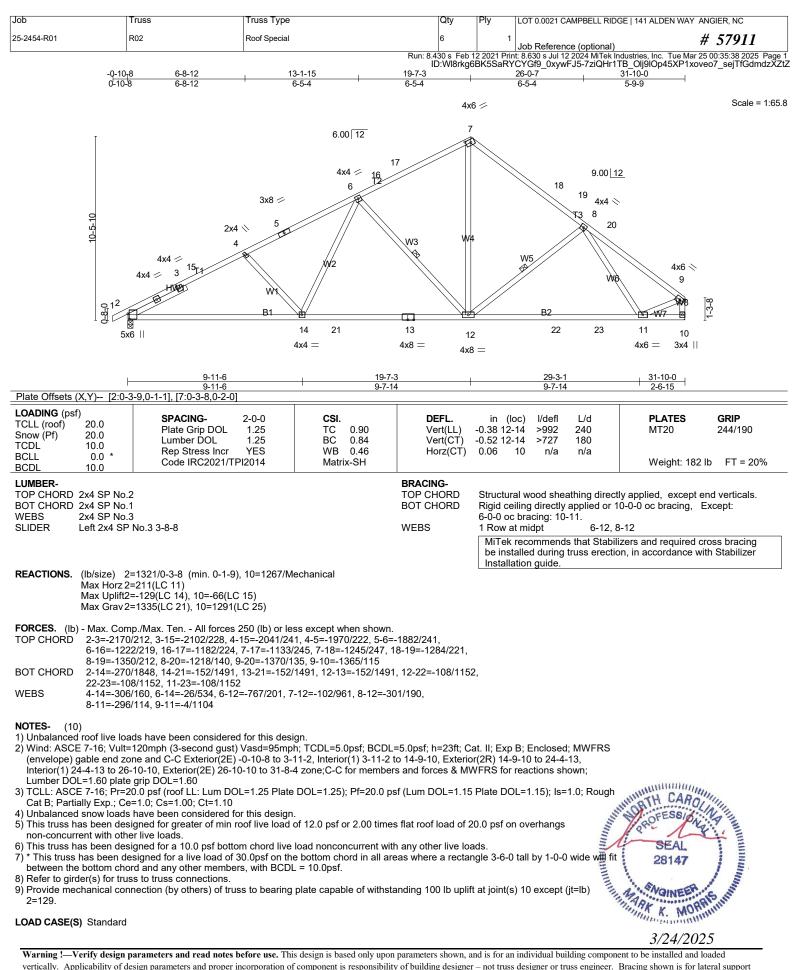
Warning !--- Verify design parameters and read notes before use.

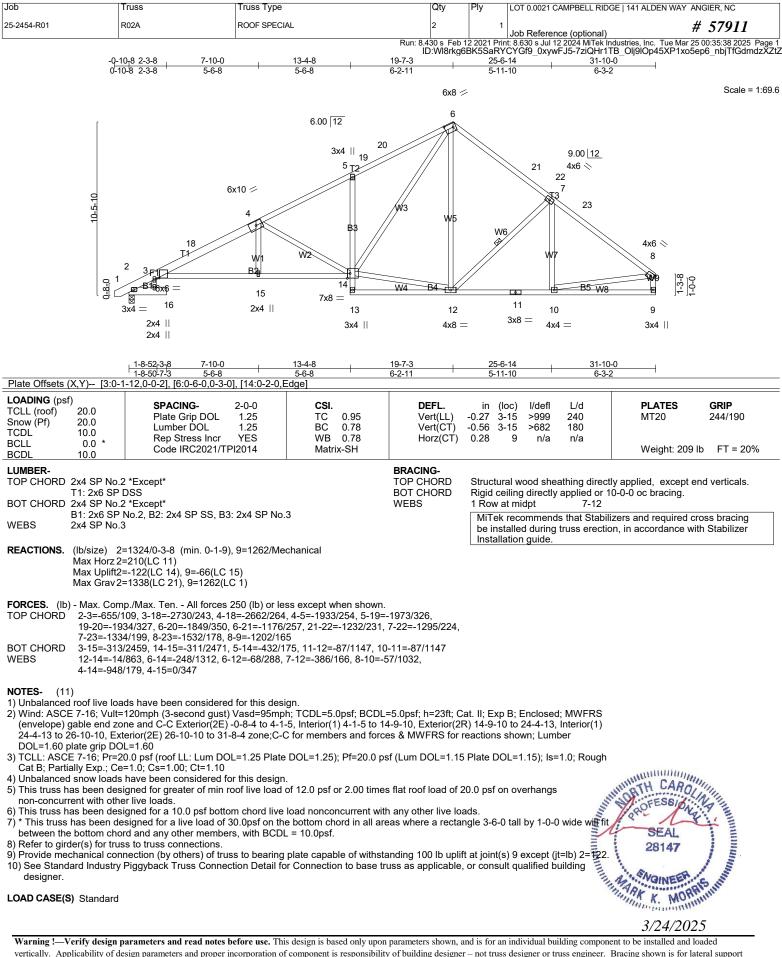


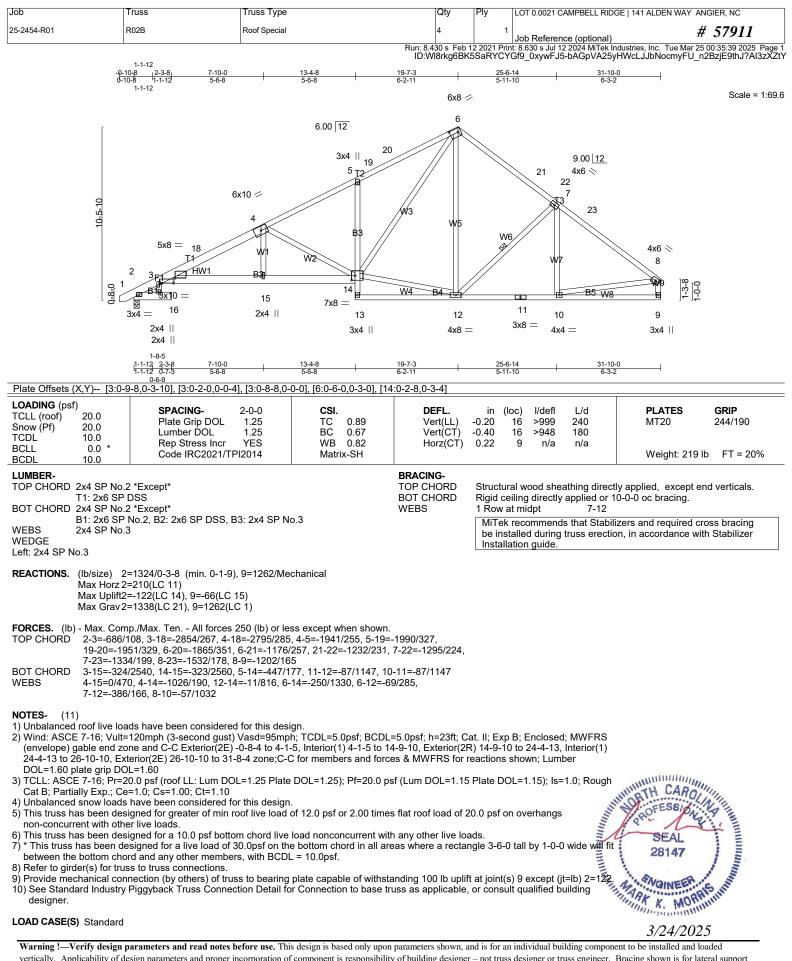
vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and loaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





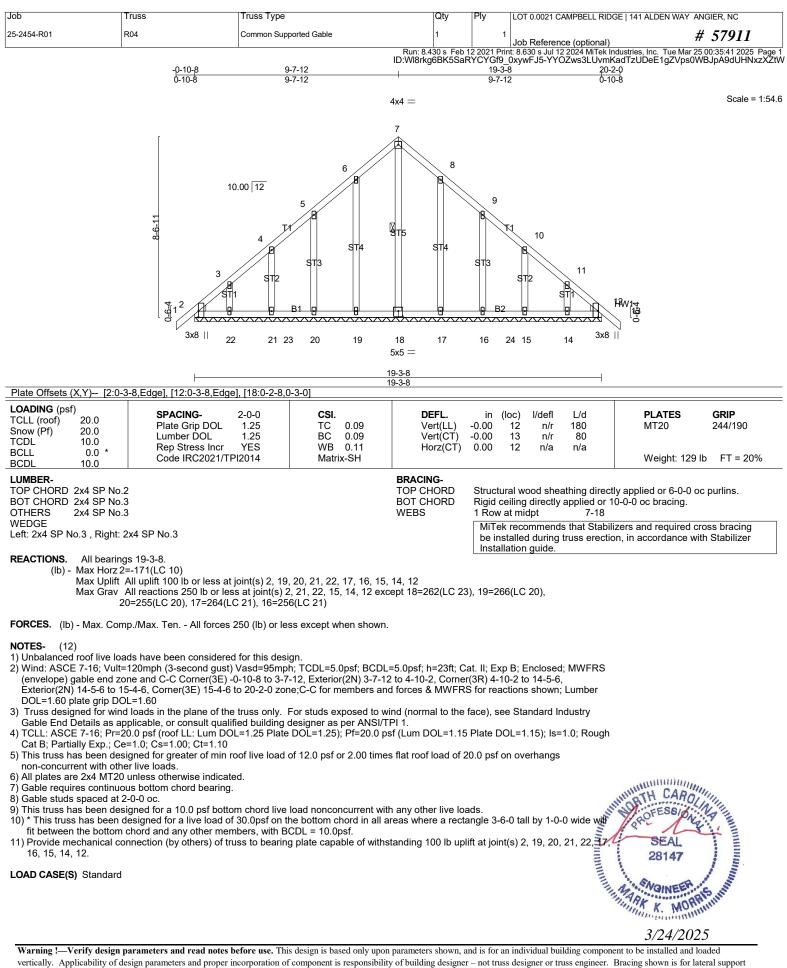


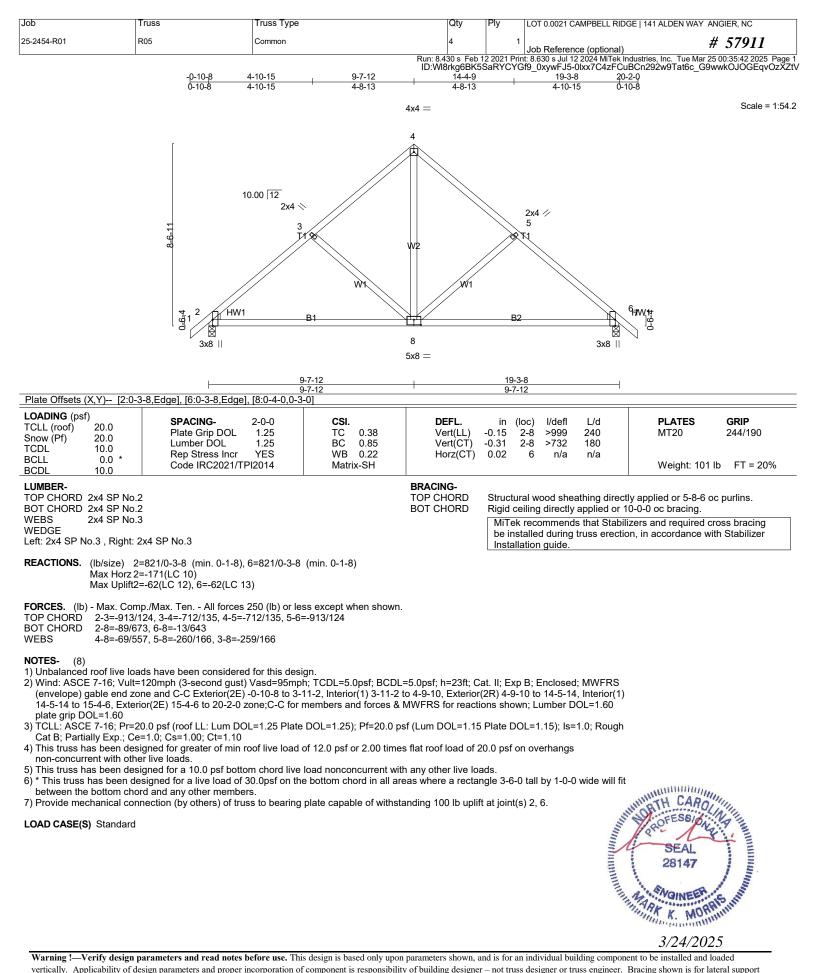


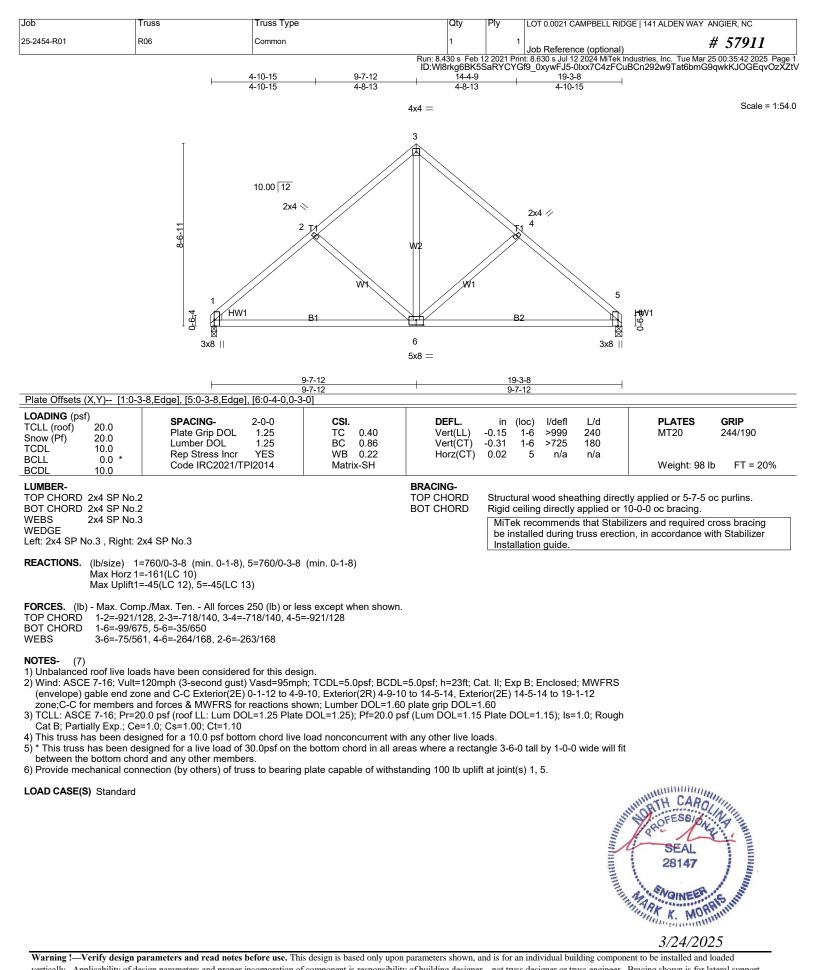


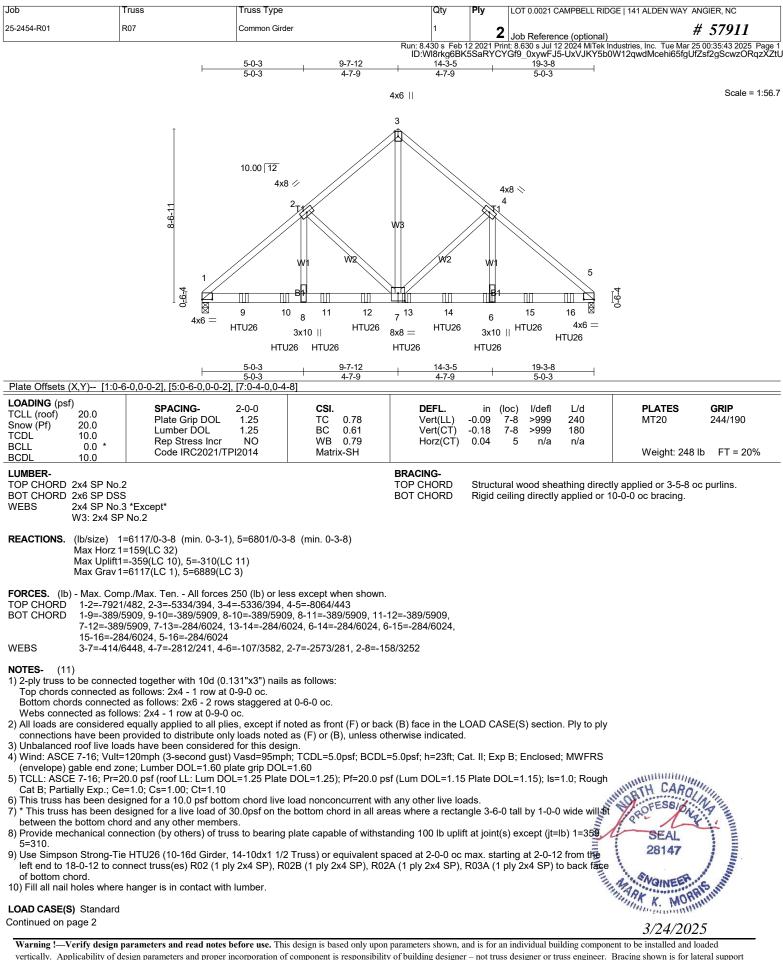
Job		Truss	Truss Type		Qty	Ply LOT 0.0	021 CAMPBELL RIDO	GE 141 ALDEN WAY AN	IGIER, NC
25-2454-R01	F	R03	Roof Special		3	1			57911
					Run: 8.430 s Feb 7	Job Re 12 2021 Print: 8.630 s	ference (optional) Jul 12 2024 MiTek In		
	-()-10 _r 8 6-8-12	13-1-1	5 .	ID:WI8rkg6E 19-7-3	3K5SaRYCYGf9_0 25-6-14	xywFJ5-3MqBiW3j 31-10-0	idustries, Inc. Tue Mar 25 jbeTzTunwV7?US18eS	SV7Sjl0wzlkqVzXZtX
	C	-10-8 6-8-12	6-5-4		6-5-4	5-11-10	6-3-2		
					6x8 ⋍				Scale = 1:72.6
				6.00 12	7				
	Ī				- FA				
				21 4x4 ≈ 20 <			9.00 12		
				6 12	// \/	22	 23 ^{4x6 ⊗}		
			3x8 ⋍	Ŧ		$\parallel \qquad \nearrow$. 8		
	10-5-10	2x	4 \\ 5				24		
	10-		4		XV4	VXA			
		4x4 = 4x4 = 3	1 W	2 W3		\\ %	7 108	3x4 ∥	
		4x4 = 3 14 2 Hight	W1			\\ //		9	
	o.				// Bs2 ₱		·		
	0-8-0		 18	25 26 17	15 ¹⁶ 27 ³² 12	11	29 _{30 31}	에긔 (1)	
		5x5	4x4 =		1v8 —	4×4 —	2030 31	10	
					$\begin{array}{ccc} 4x0 &=& 2x4 \\ \downarrow &=& 2$	2×4 -		4x4 =	
				2:	x4 =				
		7-8-6		15-1-3	19-7-3	24-1-3	31-10-0		
Plate Offsets	(X Y) [2·0-3	<u>7-8-6</u> 3-1,0-0-4], [7:0-6-0,0-3-0]	1	7-4-14	4-6-0	4-6-0	7-8-13		
LOADING (ps				001	DEEL	in (1) 1(1.4		
TCLL (roof)	20.0 20.0	SPACING- Plate Grip DOL	2-0-0 1.25	CSI. TC 0.89	DEFL. Vert(LL)		defl L/d 914 240	PLATES MT20	GRIP 244/190
Snow (Pf) TCDL	10.0	Lumber DOL Rep Stress Incr	1.25 YES	BC 0.81 WB 0.70	Vert(CT) Horz(CT)		598 180 n/a n/a		
BCLL BCDL	0.0 * 10.0	Code IRC2021/TP		Matrix-SH	1012(01)	0.07 10	n/a n/a	Weight: 204 lt	o FT = 20%
LUMBER-			i		BRACING-			1	
TOP CHORD BOT CHORD					TOP CHORD BOT CHORD			ly applied, except en 0-0-0 oc bracing. Ex	
	B2: 2x4 SP	No.2				6-0-0 oc bracin	g: 13-16	0-0-0 00 bracing. Ex	
WEBS SLIDER	2x4 SP No.3 Left 2x4 SP				WEBS	1 Row at midpt		zers and required cro	oss bracing
						be installed d	uring truss erectio	on, in accordance with	
REACTIONS.	(lb/size) 2:	=1383/0-3-8 (min. 0-1-12), 10=1368/0-3-8	(min. 0-1-14)		Installation gu	ide.		
	Max Horz 2:	=211(LC 11)		, , , , , , , , , , , , , , , , , , ,					
		=1488(LC 5), 10=1611(LC							
FORCES. (1)) - Max. Com	p./Max. Ten All forces 2	250 (lb) or less ex	cept when shown.					
	2-3=-2574	, /149, 3-19=-2501/151, 4- 5/199, 20-21=-2004/203,	19=-24 [́] 85/165, 4-		-2379/215,				
	8-23=-201	7/213, 9-24=-309/107, 9-1	10=-297/117						
BOT CHORD		/2197, 18-25=-88/1895, 2 241, 12-27=0/1241, 12-28				537.			
WEBS	30-31=-35	/1537, 10-31=-35/1537 /164, 6-18=-118/514, 6-13				,			
WEDS		/773, 11-13=-136/669, 8-1			102/1247,				
NOTES- (9)								
1) Unbalance	d roof live loa	ds have been considered 120mph (3-second gust)							
(envelope)	gable end zo	ne and C-C Exterior(2E) -	0-10-8 to 3-11-2,	Interior(1) 3-11-2	to 14-9-10, Exterio	r(2R) 14-9-10 to	24-4-13,		
Interior(1) 2	24-4-13 to 26-)I =1 60 plate	10-10, Exterior(2E) 26-10 grip DOI =1 60	I-10 to 31-8-4 zor	ne;C-C for member	rs and forces & MV	VFRS for reactio	ns shown;	MUMMINIA	
3) TCLL: ASC	E 7-16; Pr=2	0.0 psf (roof LL: Lum DOI	_=1.25 Plate DOI	_=1.25); Pf=20.0 p	sf (Lum DOL=1.15	Plate DOL=1.15); Is=1.0; Rough	INNIGTH CAROL	Alle
4) Unbalance	d snow loads	have been considered fo	r this design.				Mult	ROFESSIDA	Aller
5) This truss I	has been designed	igned for greater of min ro er live loads	oof live load of 12	.0 psf or 2.00 time	s flat roof load of 2	0.0 psf on overh	angs	SFAL	
6) This truss I	nas been des	10-10, Exterior(2E) 26-10 grip DOL=1.60 0.0 psf (roof LL: Lum DOI =1.0; Cs=1.00; Ct=1.10 have been considered fo igned for greater of min ro er live loads. igned for a 10.0 psf botto signed for a live load of 3 of and any other member	n chord live load	nonconcurrent wit	h any other live loa	ads.	100.00	SEAL 28147 3/24/202	11114
/) " This truss between th	e nas been de e bottom cho	signed for a live load of 3 rd and any other member	u.upsi on the bot s, with BCDL = 1	iom cnord in all an 0.0psf.	eas wnere a rectar	igle 3-6-0 tall by	1-U-U wide with fit	1	
		nection (by others) of trus			anding 100 lb uplif	t at joint(s) 2, 10.	1 million	1 NOINEER	Summer
LOAD CASE(S) Standard							MAK K. MORK	mm
								2/24/202	5
								5/24/2023	2

Job	ر ا	russ	Truss Type		Qty	Ply L	OT 0.0021 CAMPBELL RID	GE 141 ALDEN WAY AN	GIER, NC
25-2454-R01	F	RO3A	ROOF SPECIAL		4	1 J	ob Reference (optional)		57911
					Run: 8.430 s Feb ID:Wl8rkg6Bl	12 2021 Print: 8 K5SaRYCYG	3.630 s Jul 12 2024 MiTek l f9_0xywFJ5-3MqBiW3jj	ndustries, Inc. Tue Mar 25 beTzTunwV7?US18tSU	00:35:40 2025 Page JWSjY0wzlkqVzXZ
	-u 0	-10-8 6-8-12 -10-8 6-8-12	13-1-15 6-5-4		19-7-3 6-5-4	26-0-7 6-5-4	31-10- 5-9-9		
					6x8 🚧				Scale = 1:72.
	т			6.00 12	7				
			3x8 ⋍	4x4 = 20 6 12			9.00 12 22 23 4x6 ×		
	10-5-10	4x4 = 4x4 = 3 191	2x4 \\ 5 4 W2	VY3	1 14	we	T3 8 24	3x4	
	0-8-0	2 High	W1		B2 V154			6 0 0 1-3 -3 -3 -3 -3 -4 -4 -4 -4 -4 -4 -4 -4 -4 -4 -4 -4 -4	
	- 0	∽ ⊠ 5x5	18 29 4x4 =	4x4	$\begin{array}{cccc} & 15 & 16 & 27 & 31 \\ & 4x8 = & & 2x4 \\ & 4 = & & 2x4 \\ & x4 = & & & \\ \end{array}$	2	1113 29 30 4x4 = 4 =	10 4x6 =	
Diata Offecto	(X X) [2:0.2	7-8-6 7-8-6	7.	5-1-3 -4-14	19-7-3 4-6-0	24-1-3 4-6-0	31-10-0 7-8-13		
-DADING (ps		-1,0-0-4], [7:0-6-0,0-3-0	-	001	DEEL	in (las)			CDID
TCLL (roof) Snow (Pf) TCDL 3CLL 3CDL	20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/7	1.25 1.25 YES	CSI. TC 0.87 BC 0.85 WB 0.69 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc -0.43 14 -0.66 14 0.07 10	>879 240 >578 180	PLATES MT20 Weight: 203 Ib	GRIP 244/190 p FT = 20%
	2x4 SP No.2 T3: 2x4 SP I	No.1			BRACING- TOP CHORD BOT CHORD	end vertica	als.	tly applied or 1-7-8 oc	
WEBS SLIDER	2x4 SP SS * B2: 2x4 SP 2x4 SP No.3 Left 2x4 SP	No.2			WEBS	6-0-0 oc b 1 Row at r	racing: 13-16 nidpt 8-10	10-0-0 oc bracing. Ex lizers and required cro	·
REACTIONS.	Max Horz 2= Max Uplift2=	=1384/0-3-8 (min. 0-1- =211(LC 11) =-97(LC 14), 10=-14(LC =1489(LC 5), 10=1612(15)	nical		be install Installatio		on, in accordance with	h Stabilizer
F ORCES. (IL TOP CHORD	2-3=-2575/ 6-20=-204	p./Max. Ten All force 148, 3-19=-2502/151, 4 7/197, 20-21=-2006/202 2/197, 9-24=-269/97, 9-	l-19=-2486/165, 4-5= 2, 7-21=-1941/223, 7	=-2459/189, 5-6=	-2380/215,				
BOT CHORD	2-18=-205/ 15-27=0/12 29-30=-44/	/2198, 18-25=-87/1897, 274, 12-27=0/1274, 12- /1531, 10-30=-44/1531 /164, 6-18=-119/513, 6-	25-26=-87/1897, 17 28=0/1274, 11-28=0	/1274, 11-29=-44	4/1531,				
		45, 11-13=-117/617, 8-		,	3				
2) Wind ASC	d [´] roof live loa F 7-16 [.] Vult=	ds have been consider 120mph (3-second gus ne and C-C Exterior(2E 10-10, Exterior(2E) 26- grip DOL=1.60) Vasd=95mph [·] TCF	DL=5.0psf; BCDL hterior(1) 3-11-2 ;C-C for membe	=5.0psf; h=23ft; C to 14-9-10, Exterio rs and forces & MV	at. II; Exp B; or(2R) 14-9- WFRS for re	10 to 24-4-13, actions shown;	WINNING CAROL	9999
) TCLL: ASC Cat B; Part) Unbalance) This truss I	E 7-16; Pr=2 ially Exp.; Ce d snow loads nas been desi	0.0 psf (roof LL: Lum D =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of min	OL=1.25 Plate DOL= for this design. roof live load of 12.0	1.25); Pf=20.0 p psf or 2.00 time	sf (Lum DOL=1.15 s flat roof load of 2	Plate DOL=	-1.15); ls=1.0; Rough	SEAL	11.11.11.11.11.11.11.11.11.11.11.11.11.
non-concu) This truss I) * This truss between th	rent with othe nas been desi has been de e bottom cho	ne and C-C Exterior(2E) 10-10, Exterior(2E) 26- grip DOL=1.60 0.0 psf (roof LL: Lum D =1.0; Cs=1.00; Ct=1.10 have been considered gned for greater of min er live loads. gned for a 10.0 psf bott signed for a live load of rd and any other memb is to truss connections.	om chord live load n 30.0psf on the botto ers, with BCDL = 10.	onconcurrent wit m chord in all ar 0psf.	h any other live loa eas where a rectar	ads. ngle 3-6-0 ta	Il by 1-0-0 wide wilfit	SEAL 28147 3/24/2022	S
		is to truss connections. nection (by others) of tr					2, 10.	MARK K. MORR	mint
OAD CASE(S) Standard							3/24/202	5
Warning !	Varify dasign n	arameters and read notes	hefere use This design	is based only unor	noromotors shown	nd is for an in	dividual building compor	J/L F/L UL	adad









Job	Truss	Truss Type	Qty	Ply	LOT 0.0021 CAMPBELL RIDGE 141 ALDEN WAY	' ANGIER, NC
25-2454-R01	R07	Common Girder	1	2	Job Reference (optional)	# 57911
Run: 8 430 s Feb 12 2021 Print: 8 630 s Jul 12 2024 MiTek Industries Inc. Tue Mar 25 00 35 43 2025 Page 2						

un: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Mar 25 00:35:43 2025 Page 2 ID:WI8rkg6BK5SaRYCYGf9_0xywFJ5-UxVJKY5b0W12qwdMcehi65fgUfZsf2gScwzORqzXZtU

LOAD CASE(S) Standard

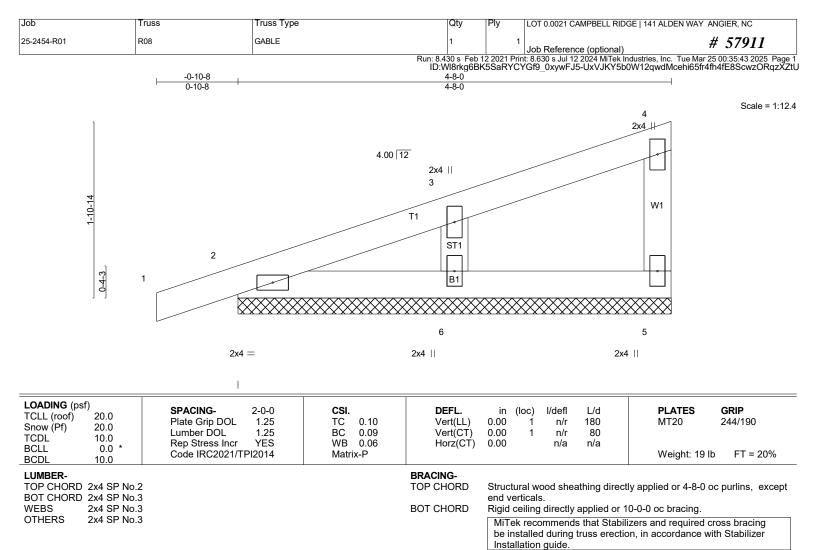
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 6=-1242(B) 9=-1247(B) 10=-1242(B) 11=-1242(B) 12=-1242(B) 13=-1242(B) 14=-1242(B) 15=-1350(B) 16=-1350(B)





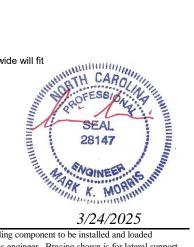
REACTIONS. (Ib/size) 5=67/4-8-0 (min. 0-1-8), 2=136/4-8-0 (min. 0-1-8), 6=211/4-8-0 (min. 0-1-8) Max Horz 2=57(LC 10) Max Uplift5=-13(LC 10), 2=-30(LC 10), 6=-36(LC 14) Max Grav 5=90(LC 21), 2=190(LC 21), 6=281(LC 21)

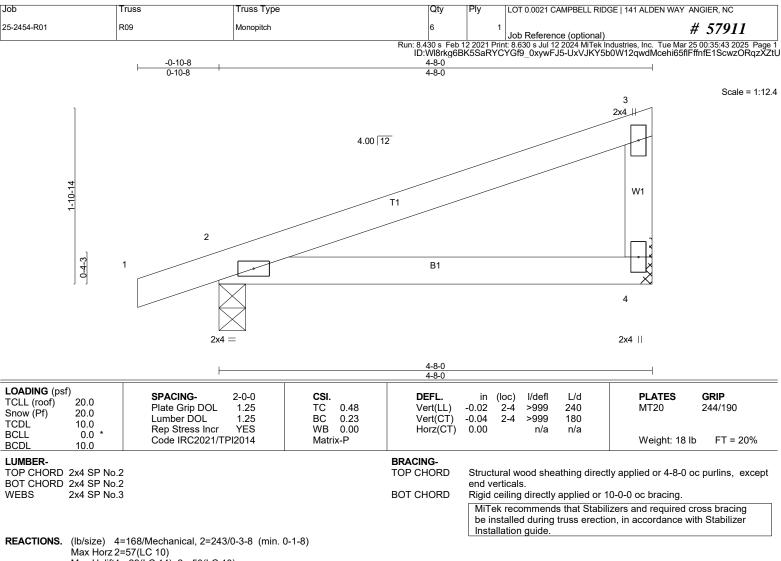
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.

LOAD CASE(S) Standard





Max Uplift4=-29(LC 14), 2=-50(LC 10) Max Grav 4=224(LC 21), 2=334(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

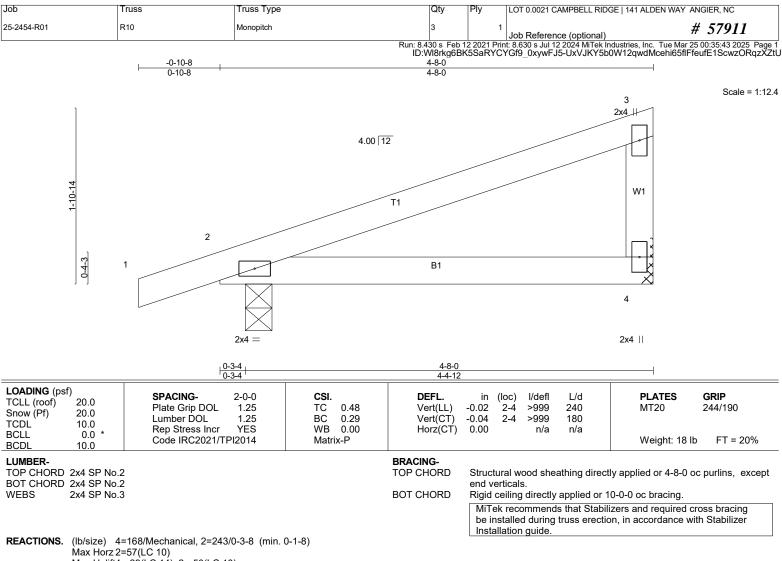
between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard





Max Uplift4=-29(LC 14), 2=-50(LC 10) Max Grav 4=224(LC 21), 2=334(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (9)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

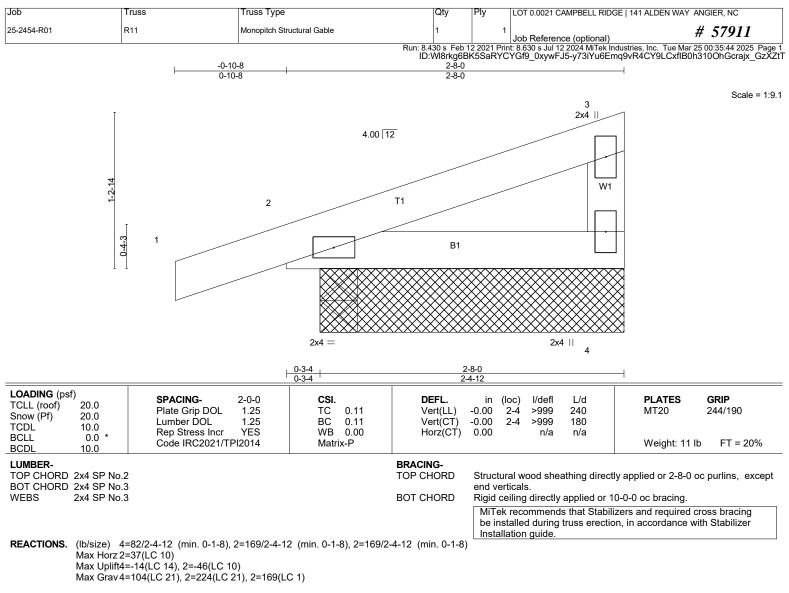
between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc.

LOAD CASE(S) Standard

- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.



Job	Truss	Truss Type		Qty	Ply	LOT 0.0021 CAMPBELL RI	DGE 141 ALDEN WAY ANGIER, NC
25-2454-R01	R12	GABLE		1		1 Job Reference (optional) # 57911
		I		Run: 8.630 s Jul ID:Wl8rkg6	12 2024 F BK5SaF	Print: 8.630 s Jul 12 2024 MiTek	, Industries, Inc. Tue Mar 25 00:35:44 2025 Page Emq9vR4CY9LCxfIBuz3ufOeXcrajx_GzX2
		-0-10 ₇ 8 0-10-8	<u>6-8-5</u> 6-8-5	13-6 6-9-1			
					3:	x4	Scale = 1:63
						5	
				9.00 12		BE	
			5.0	T2	B		
			5x6	P			
		10-7-12		4		wvз	
		-01	3x4 1/2	STI3	ST5	ST7	
		3x	4 // 3	W2			
			HUNT ST2	W1 ST4			
		2	B ST1		ST6		
		1-1 -0-0			Ð		
		0 / X 4x4	$\equiv 21$	8 22			
						3x4 =	
		<u> </u>	6-8-5	12-7-8		<u>13-6-</u> 0	
Plate Offsets (X,	Y) [2:0-0-5,0-1-1	2], [4:0-3-0,0-3-0]	6-8-5	5-11-3		0-10-8	
LOADING (psf)	so o s	PACING- 2-0-0	CSI.	DEFL.	in	(loc) l/defl L/d	PLATES GRIP
Snow (Pf) 2	20.0	Plate Grip DOL 1.25 umber DOL 1.25	TC 0.61 BC 0.64	Vert(LL) Vert(CT)	-0.04 -0.09	2-8 >999 240 2-8 >999 180	MT20 244/190
	10.0 00* R	Rep Stress Incr YES Code IRC2021/TPI2014	WB 0.24 Matrix-SH	Horz(CT)	0.01	6 n/a n/a	Weight: 130 lb FT = 20%
	10.0		Matrix-SIT				
LUMBER- TOP CHORD 2x				BRACING- TOP CHORD			ctly applied or 6-0-0 oc purlins, except
BOT CHORD 2x WEBS 2x	<4 SP No.3 <4 SP No.3			BOT CHORD		erticals. ceiling directly applied or	10-0-0 oc bracing.
	<4 SP No.3 eft 2x4 SP No.3 3-1	11-12		WEBS		v at midpt 5-6, 4	I-6 ilizers and required cross bracing
					be ir	nstalled during truss erect	ion, in accordance with Stabilizer
		2-0 (min. 0-1-8), 2=582/2-5-8	(min. 0-1-8), 7=92/0-3	8-8 (min. 0-1-8)	Insta	allation guide.	
М	1ax Horz 2=324(LC 1ax Uplift6=-258(LC	C 12)					
М	lax Grav6=532(LC	20), 2=615(LC 20), 7=219(LC	C 5)				
	Max. Comp./Max. 2-3=-633/0, 3-4=-5	Ten All forces 250 (lb) or les	ss except when shown	l.			
BOT CHORD 2	2-21=-157/515, 8-2	21=-157/515, 8-22=-158/511,	7-22=-158/511, 6-7=-1	58/511			
	4-8=0/338, 4-6=-64	+0/197					
	7-16; Vult=120mph	(3-second gust) Vasd=95mp					
		C-C Exterior(2E) -0-10-8 to 3- RS for reactions shown; Lumb			(2E) 8-6	6-10 to 13-4-4 zone;C-C	
		n the plane of the truss only. I , or consult qualified building (face), s	see Standard Industry	
3) TCLL: ASCE 7		roof LL: Lum DOL=1.25 Plate			Plate [DOL=1.15); ls=1.0; Rough	1
4) This truss has	been designed for	r greater of min roof live load	of 12.0 psf or 2.00 time	es flat roof load of 2	20.0 psf	f on overhangs	
5) All plates are 2	nt with other live loa 2x4 MT20 unless o	otherwise indicated.					MUMMINIA CAR
 Gable studs sp This truss has 	paced at 2-0-0 oc. been designed for	r a 10.0 psf bottom chord live	load nonconcurrent wi	th any other live loa	ads.		TESSIA NULL
8) * This truss ha	as been designed f	ads. otherwise indicated. r a 10.0 psf bottom chord live for a live load of 30.0psf on the ny other members, with BCDL (by others) of truss to bearing	e bottom chord in all a = 10.0psf.	reas where a rectar	ngle 3-6	6-0 tall by 1-0-0 wide will	it offer the state
	anical connection ((by others) of truss to bearing	plate capable of withs	tanding 100 lb uplif	t at join	t(s) except (jt=lb) 6=25	SEAL
9) Provide mecha						1000	28147
9) Provide mecha							
9) Provide mecha						III III	A NOINEER
9) Provide mecha							A ANGINEER BUILT
9) Provide mecha							SEAL 28147 3/24/2025

Job	Truss	Truss Type	Qty	Ply	LOT 0.0021 CAMPBELL RIDGE 141 ALDEN W/	AY ANGIER, NC
25-2454-R01	R12	GABLE	1	1	Job Reference (optional)	# 57911
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Mar 25 00:35:44 2025 Page 2						

Run: 8.630 s_Jul 12 2024 Print: 8.630 s_Jul 12 2024 Mi Tek Industries, Inc. Tue Mar 25 00:35:44 2025 Page 2 ID:WI8rkg6BK5SaRYCYGf9_0xywFJ5-y73iYu6Emq9vR4CY9LCxflBuz3ufOeXcrajx_GzXZtT

Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

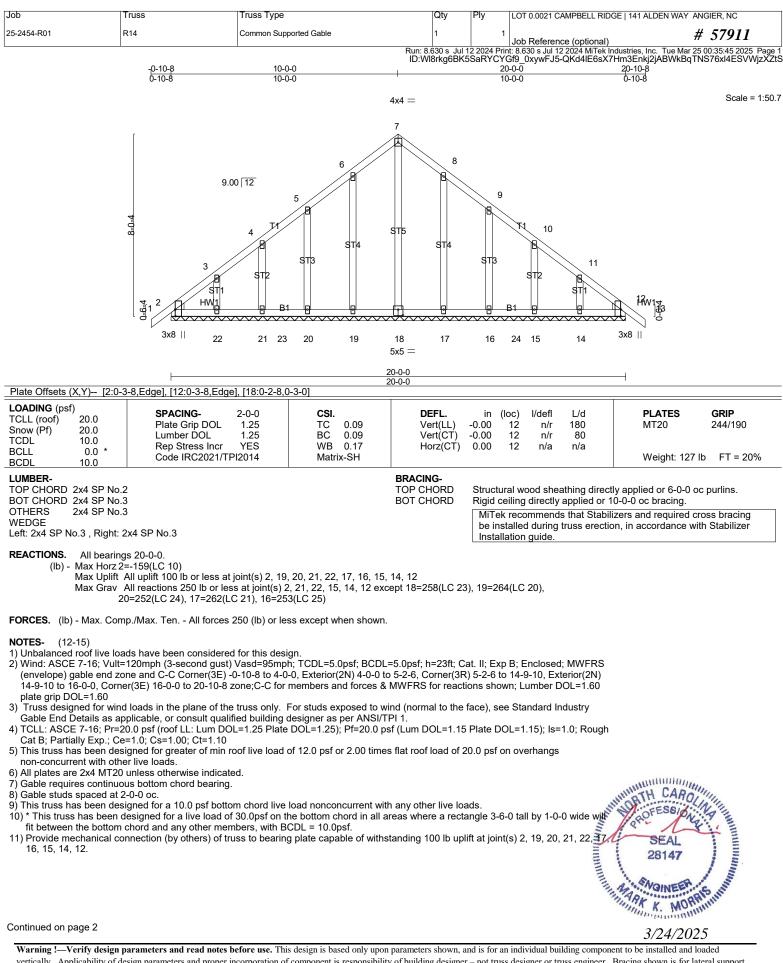
13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply LOT 0.0021 CAMPBELL	RIDGE 141 ALDEN WAY ANGIER, NC
25-2454-R01	R13	Monopitch	10	1	# 57911
			Run: 8.430 s Feb	Job Reference (option 12 2021 Print: 8.630 s Jul 12 2024 MiTe RK5SaRXCXCf0, 0xawE 15 x72iX	aı) ek Industries, Inc. Tue Mar 25 00:35:44 2025 Page u6Emq9vR4CY9LCxflBus3xTOeTcrajx_GzX2
		-0 <u>-10-8 6-8-5</u> 0-10-8 6-8-5	13-0	6-0	
		d-10-8 6-8-5	6-9-		
				3x4	Scale: 3/16"=
		I	9.00 12	4	
			Ţ	2/	
			5x6 ⁄/		
		2	3		
		0-7-12	3 A	₩B	
		=			
		и		2 ≹	
			W1		
		₹1 ² Dr HW1			
		3x8	7 6 8	5 3x4 =	
			2x4	0.4	
		6-8-5	13-0		
Plate Offsets (X,Y) [2	:0-3-8,Edge], [3:0-3-0,0-3		0-9-	-11	
_OADING (psf) FCLL (roof) 20.0	SPACING-	2-0-0 CSI .	DEFL.	in (loc) l/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL Lumber DOL	. 1.25 TC 0.61 1.25 BC 0.46	Vert(LL) Vert(CT)	-0.05 5-6 >999 240 -0.10 5-6 >999 180	MT20 244/190
TCDL 10.0 BCLL 0.0 *	Rep Stress Inc	r YES WB 0.24	Horz(CT)	0.01 5 n/a n/a	Weight: 82 lb $FT = 20\%$
<u>BCDL 10.0</u>	Code IRC2021	/TPI2014 Matrix-SH			Weight: 82 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP N	No.2		BRACING- TOP CHORD	Structural wood sheathing dir	rectly applied or 6-0-0 oc purlins, except
BOT CHORD 2x4 SP N WEBS 2x4 SP N			BOT CHORD	end verticals. Rigid ceiling directly applied o	or 10-0-0 oc bracing.
WEDGE Left: 2x4 SP No.3			WEBS	1 Row at midpt 4-5	, 3-5
Len. 274 OF 110.0					abilizers and required cross bracing action, in accordance with Stabilizer
REACTIONS. (Ib/size)	5=526/Mechanical, 2=5	92/0-3-8 (min 0-1-8)		Installation guide.	
Max Ho	z 2=324(LC 12) ift5=-189(LC 12)				
	iv 5=629(LC 20), 2=626(L	C 20)			
FORCES. (Ib) - Max. C	omp./Max. Ten All forc	es 250 (lb) or less except when sh	iown.		
OP CHORD 2-3=-6 30T CHORD 2-7=-1		=-151/521, 5-8=-151/521			
	370, 3-5=-656/190	101/021,000 101/021			
NOTES- (8)					_
		st) Vasd=95mph; TCDL=5.0psf; E E) -0-10-8 to 3-11-2, Interior(1) 3-			
		ns shown; Lumber DOL=1.60 plat DOL=1.25 Plate DOL=1.25); Pf=2		5 Plate DOI =1 15): Is=1 0: Rou	ah
Cat B; Partially Exp.;	Ce=1.0; Cs=1.00; Ct=1.1	0		,	9.1
non-concurrent with	other live loads.	n roof live load of 12.0 psf or 2.00			
 This truss has been This truss has beer 	designed for a 10.0 psf bo designed for a live load	ottom chord live load nonconcurre of 30.0psf on the bottom chord in	nt with any other live loa all areas where a recta	ads. ngle 3-6-0 tall by 1-0-0 wide wil	l fit
between the bottom	chord and any other mem	bers, with BCDL = 10.0psf.		0	WINNINGTH CARO
) Provide mechanical	connection (by others) of	,. truss to bearing plate capable of v	vithstanding 100 lb uplit	ft at joint(s) except (jt=lb) 5=189	SEAL 28147
LOAD CASE(S) Standa	ırd				and and the second
					SEAL
					4014/
					A MOINEER &
					ARK K MORRIGHT
					Man Mannah Man
					2/2//2025

3/24/2025



vertically. Applicability of design parameters and proze the responsibility of building designer – not truss designer or truss engines. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0021 CAMPBELL RIDGE 141 ALDEN WAY ANGIE	R, NC
25-2454-R01	R14	Common Supported Gable	1	1	Job Reference (optional) # 5	57911
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Mar 25 00:35:45 2025 Page 2 ID:WI8rkg6BK5SaRYCYGf9_0xywFJ5-QKd4IE6sX7Hm3Enkj2jABWkBqTNS76xI4ESVWjzXZtS						

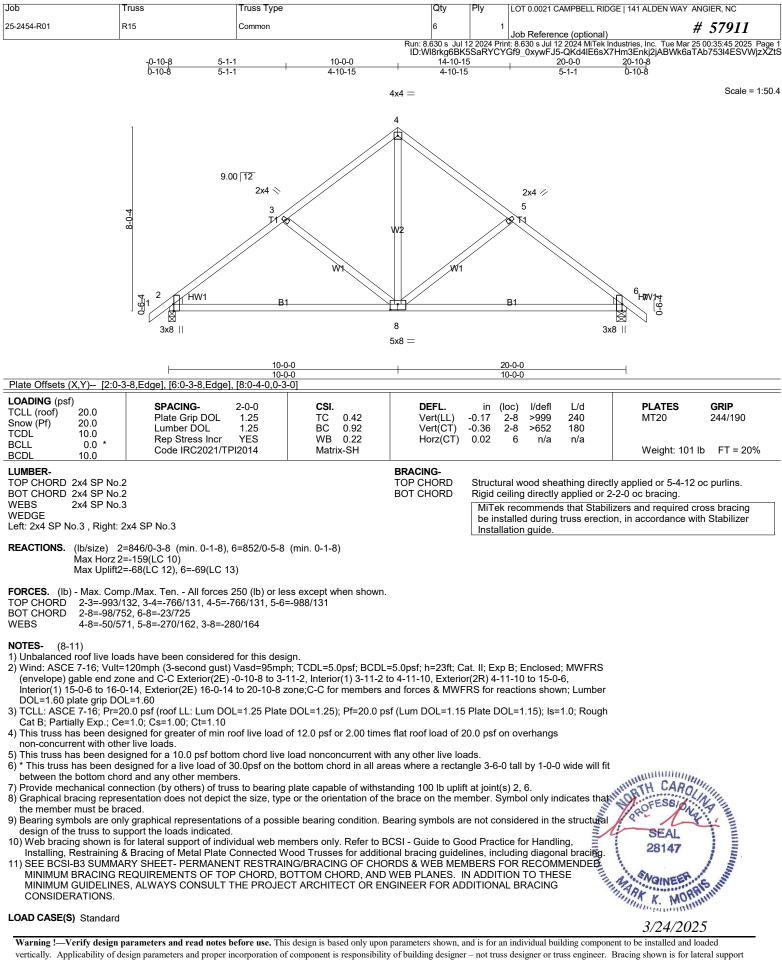
12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

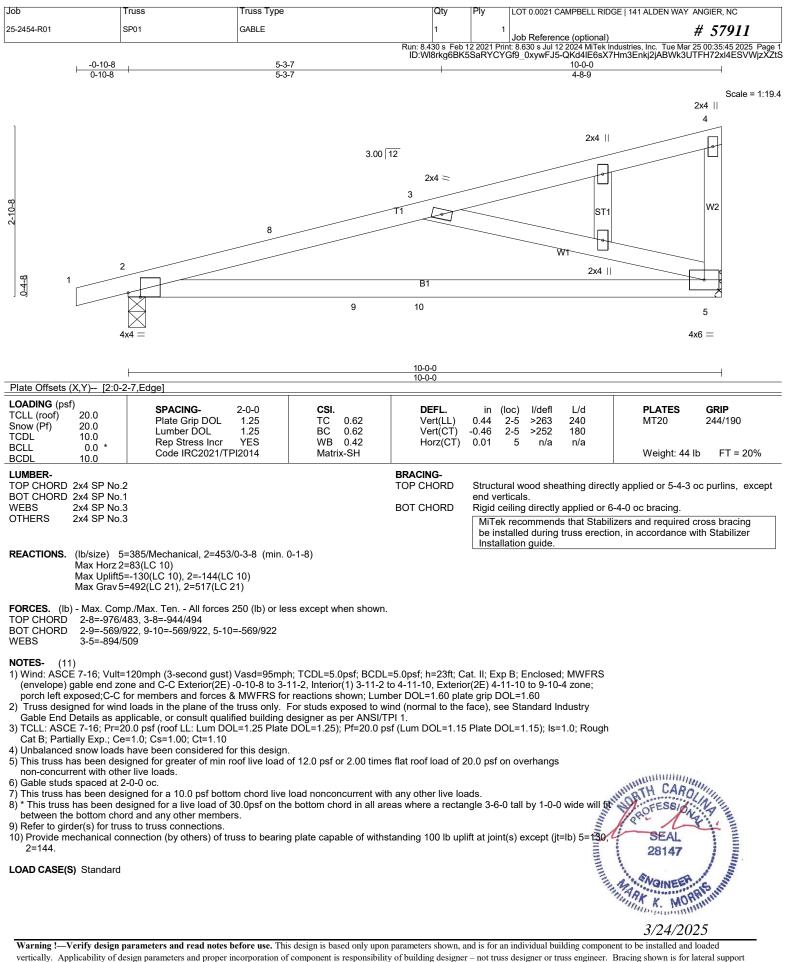
14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

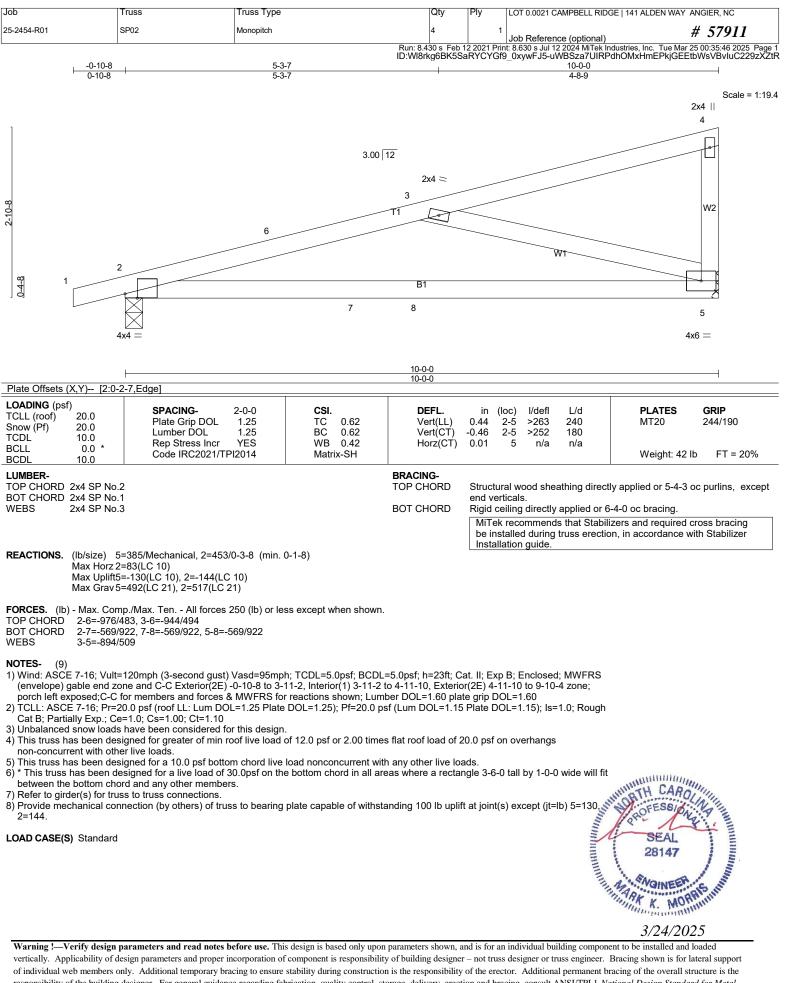
Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

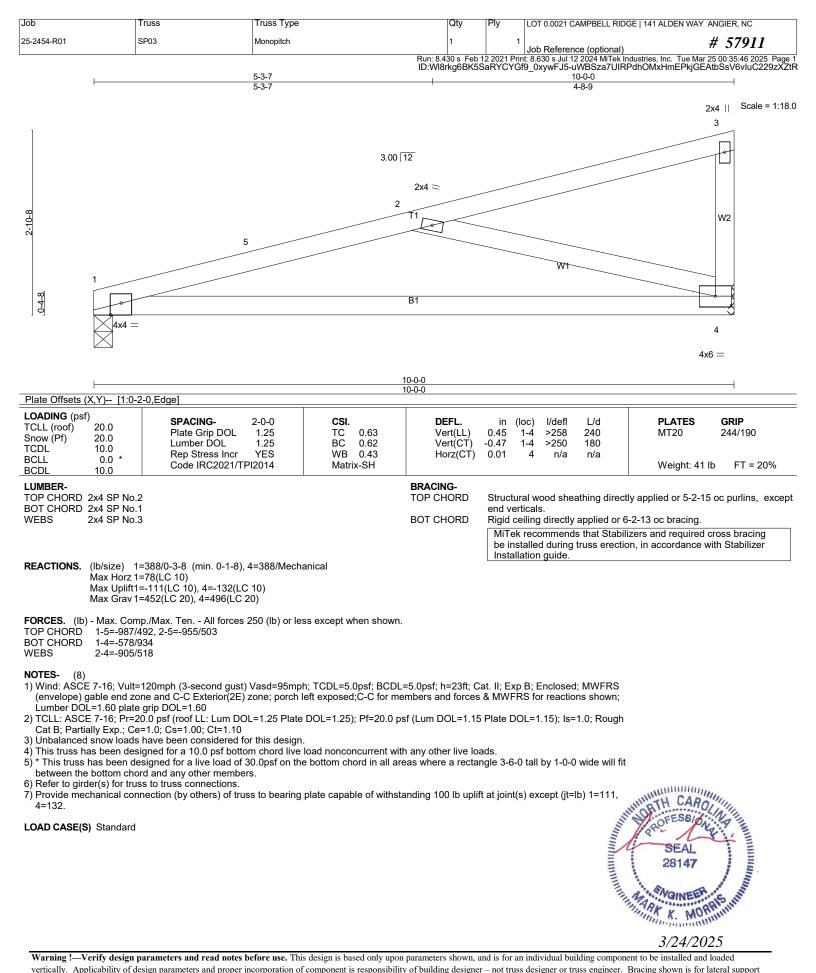


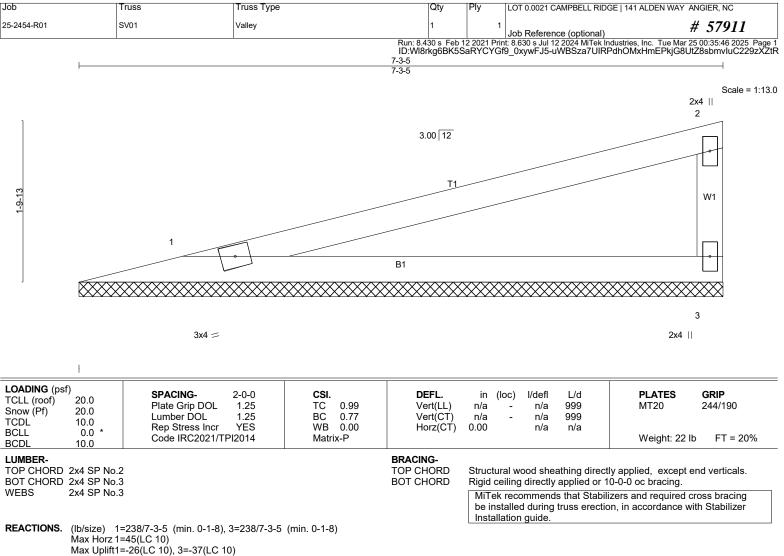






of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





Max Grav 1=308(LC 20), 3=308(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 3) Unbalanced snow loads have been considered for this design.

Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

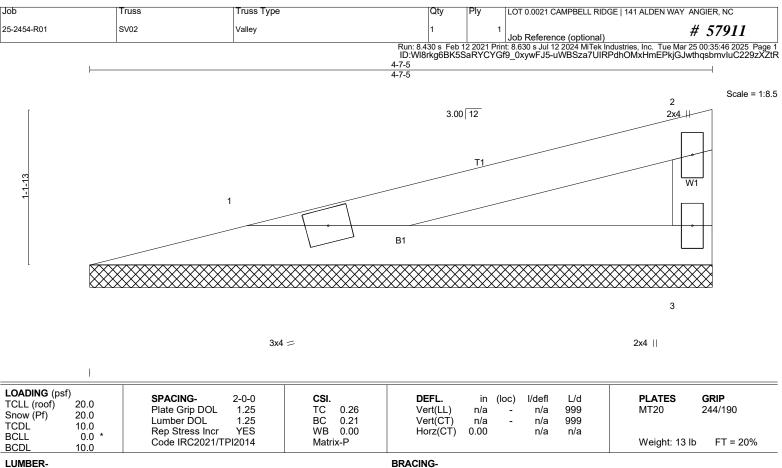
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





TOP CHORD

BOT CHORD

	8.4	Р	-	R-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No 3

Structural wood sheathing directly applied or 4-7-5 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Installation guide.

REACTIONS. (lb/size) 1=131/4-7-5 (min. 0-1-8), 3=131/4-7-5 (min. 0-1-8) Max Horz 1=25(LC 10) Max Uplift1=-14(LC 10), 3=-20(LC 10) Max Grav 1=163(LC 20), 3=163(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

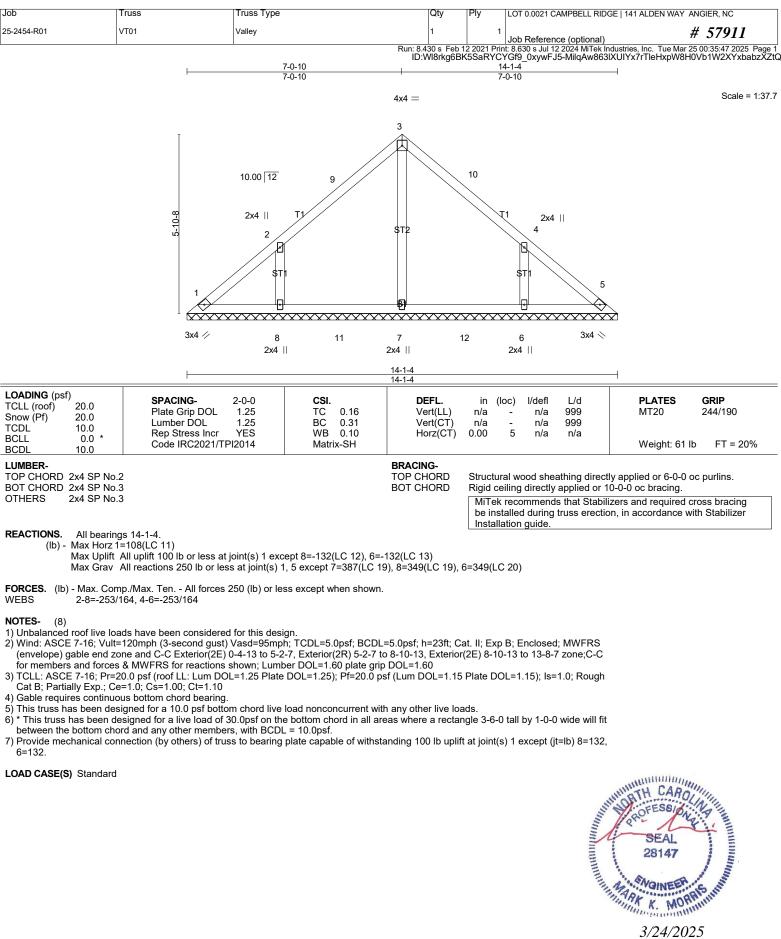
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

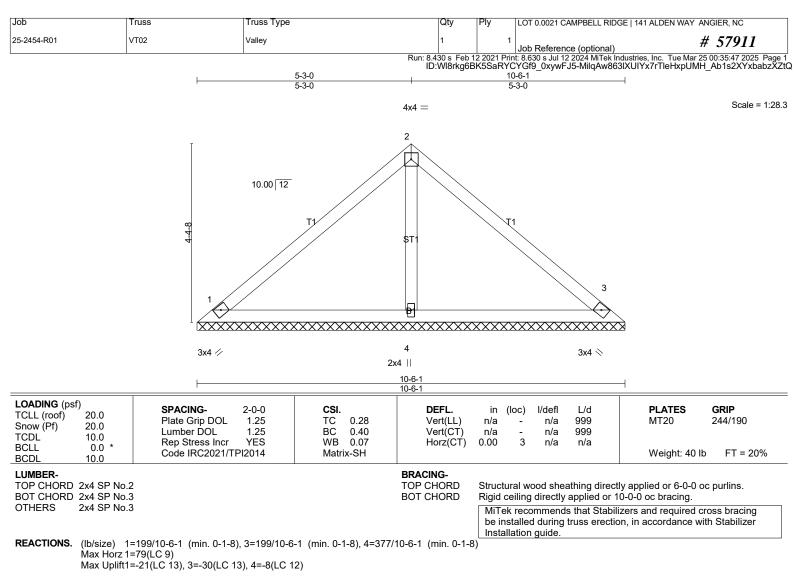
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard









FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

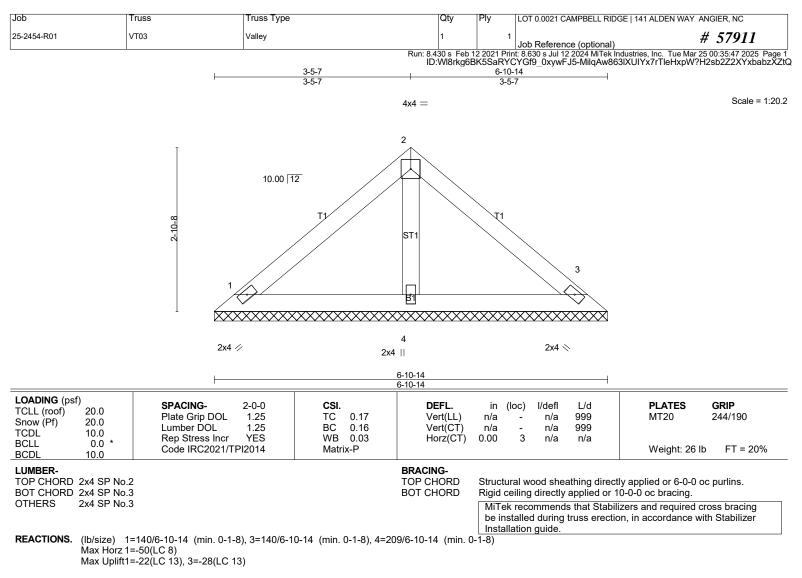
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

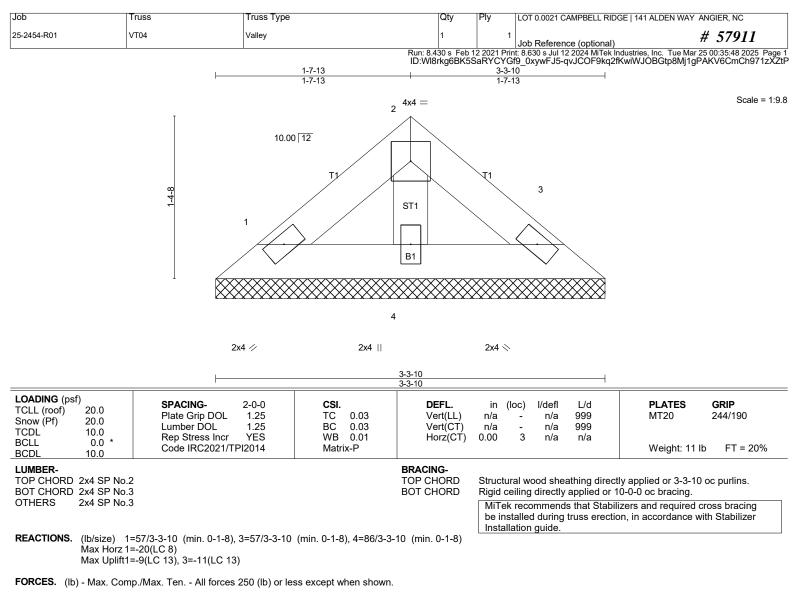
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





NOTES- (8)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

