# MIDDLETON-RALE

RALEIGH - LOT 00.0107 BLAKE POND SF (MODEL# 2183)

**ELEVATION 2- GL** 

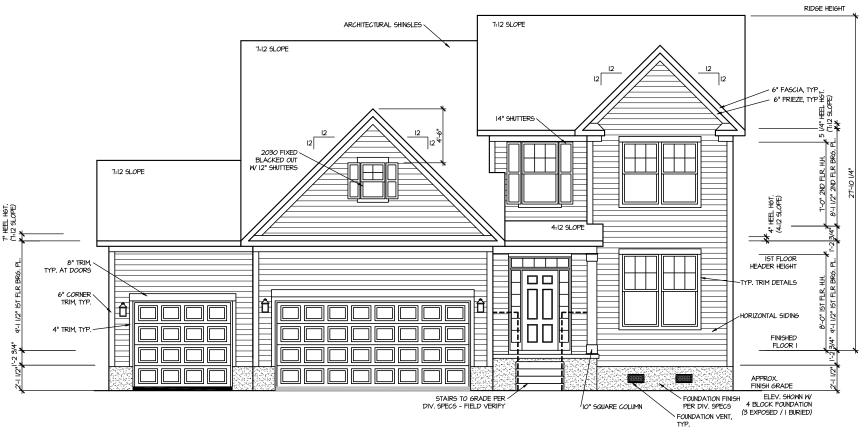


AREA CALCULATIONS  ELEVATION 2 FIRST FLOOR GARAGE FRONT PORCH - ELEVATION 2 SECOND FLOOR  OPTIONS BONUS ROOM COVERED PORCH 3RD CAR GARAGE		HEATED 1495 SF 692 SF	COVERED / UNHEATED  417 SF 45 SF  124 SF 252 SF	UNCOVERED
	TOTAL	2497 SF	838 SF	
<u> </u>				

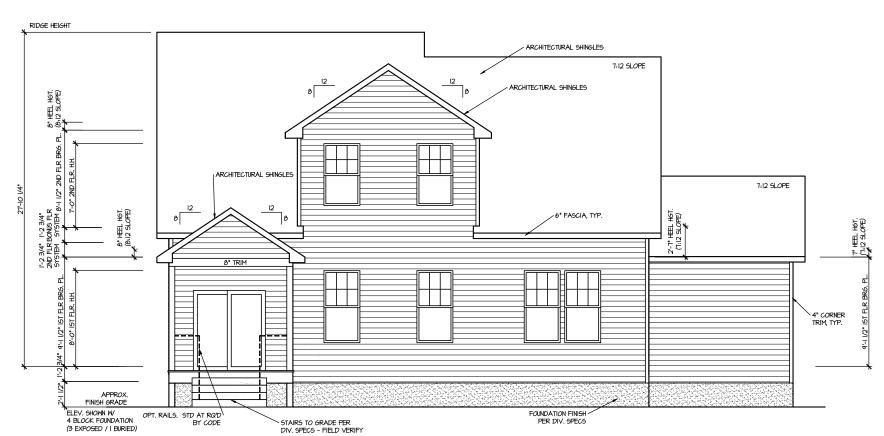
## 114 Frost Meadow Way

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<b>■</b> L() [	<u>SPECIFIC</u>	
1	LOT 00.0107	BLAKE POND SF
		MIDDLETON DEV. DALE O ELEVATION O
		MIDDLETON REV. RALE 2 ELEVATION 2
2	ADDRESS	114 FROST MEADOW WAY LILLINGTON, NC 27546
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<u>INDEX</u>	



FRONT ELEVATION 2



REAR ELEVATION 2

SCALE: 1/8" = 1'-0"

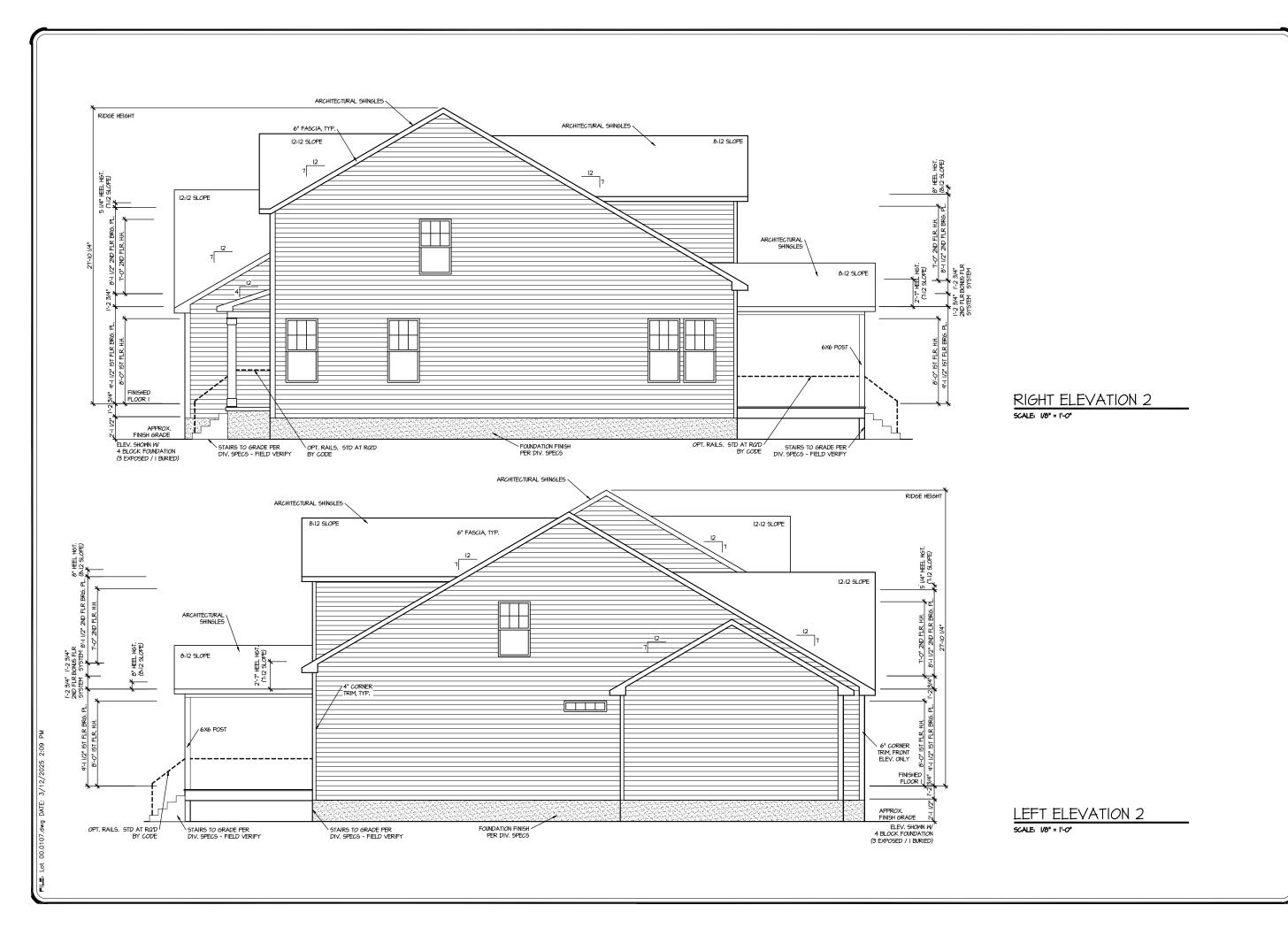
DRAWN BY: ITS DATE: 03/12/2025 PLAN NO. 2183



HOUSE NAME:
MIDDLETON
DRAWING TITLE
FRONT & REAR ELEVATIONS

SHEET No.

AI.



| MASTER PLAN INFORMATION | REVISION | DATE | DATE

DRAWN BY: ITS DATE: 03/12/2025 PLAN NO.

PLAN NO. 2183



HOUSE NAME:
MIDDLETON
DRAWING TITLE
RIGHT & LEFT ELEVATIONS

SHEET No.

ROOF VENTILATION CALCULATIONS: ROOF APEA = 2/02 50. FT. OVERALL REGISTED VENTILATION: 11 50 60 14/01 50. FT. 50-60% IN TOP THIRD = 350 - 561 50. FT. (1 70 300) NET FREE AREA OF VENTED SOFFIT = 5.7 SQ. IN / LINEAR FT. NET FREE AREA OF RIDGE VENT = 10 SQ. IN/ LINEAR FT.

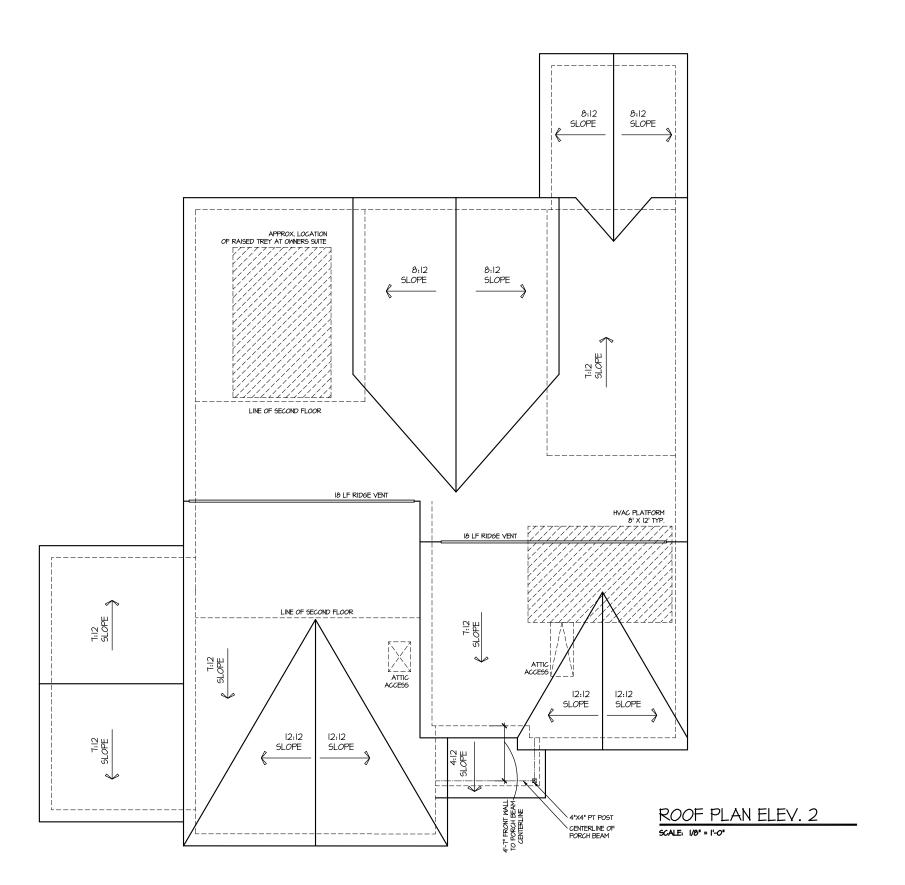
LOWER VENTING. (BOTTOM 2/3 RDS.)

14 LINEAR PIET OF SOFFIT X 3,1 50, IN = 243 50, FT.

14 LINEAR PIET OF SOFFIT X 3,1 50, IN = 45 50, FT.

4/00 50, FT. ESTIVES 50% - 20%

10 75 500 ALLOWED 13 50, FT. > 10,1 50, FT. (R07b)



DRAWN BY:

DATE: 03/12/2025 PLAN NO. 2183



HOUSE NAME:
MIDDLETON
DRAWING TITLE
ROOF PLAN

SHEET No.

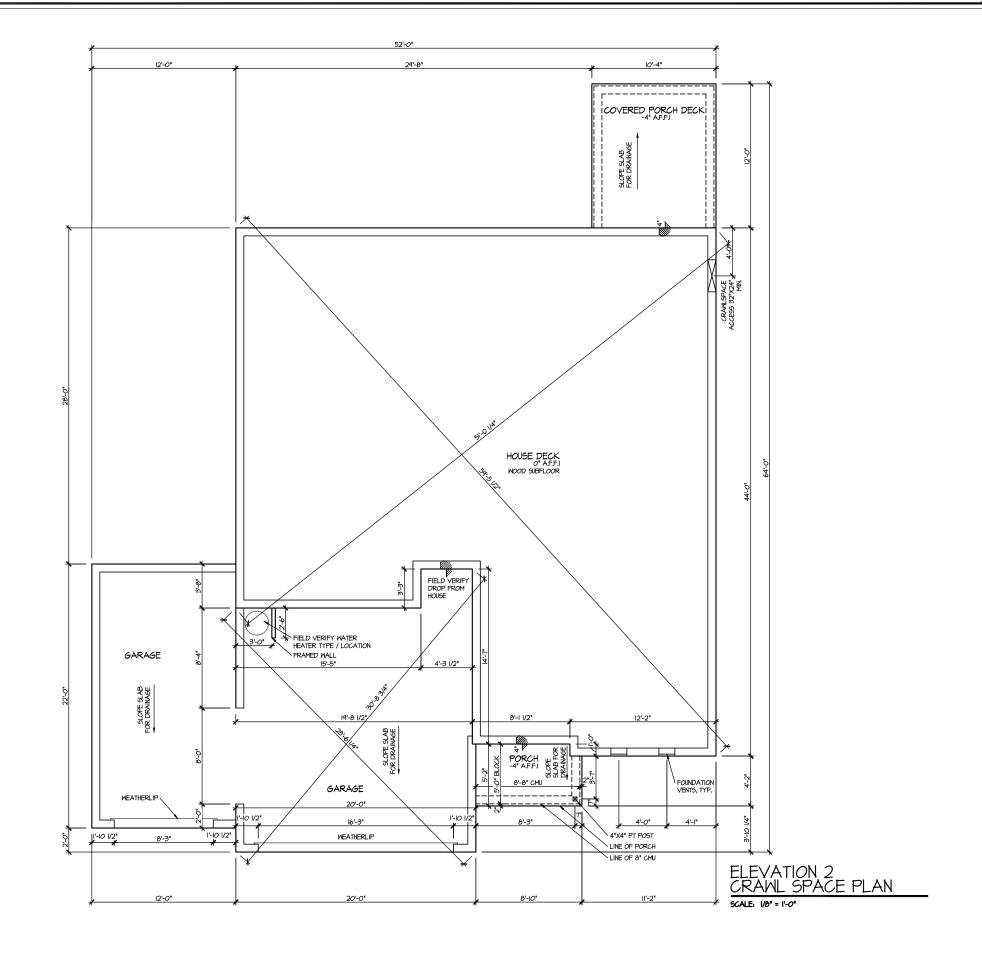
AI.3

CRAMLSPACE VENT CALCULATIONS ALL ELEVATIONS
CRAML AREA = 1445 50, FT.
OVERAL REQUIRED VENTILATION.
I SO, IN PER I SO, FT. = 1444 50, IN.

NET FREE AREA OF VENT = 12 SQ. IN. PER VENT WITTEN AUTOMATIC VENT OAL-I OR EQUAL

<u>VENTING REQUIREMENT:</u> 1495 SQ. IN. / 72 SQ. IN = 20.7 VENTS = 21 VENTS

ONLY VENTS ON THE FRONT ELEVATION ARE SHOWN. ALL OTHERS TO BE FIELD LOCATED. VENTS SHALL BE INSTALLED PER R322.22 - R322.22.1



SHEET No.

HOUSE NAME:
MIDDLETON
DRAWING TITLE

ACIII

O)

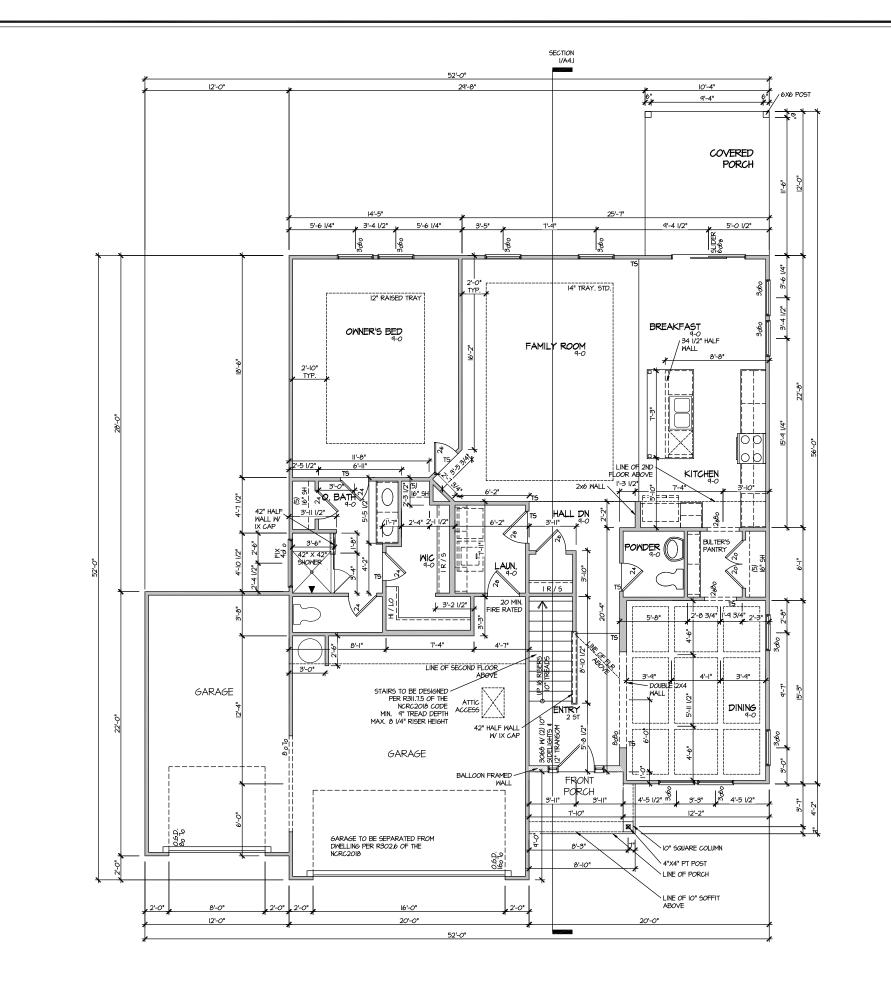
DATE

03-20-2024

DRAWN BY:

DATE: 03/12/2025 PLAN NO. 2183

A2.



DRAWN BY: ITS DATE: 03/12/2025 PLAN NO. 2183

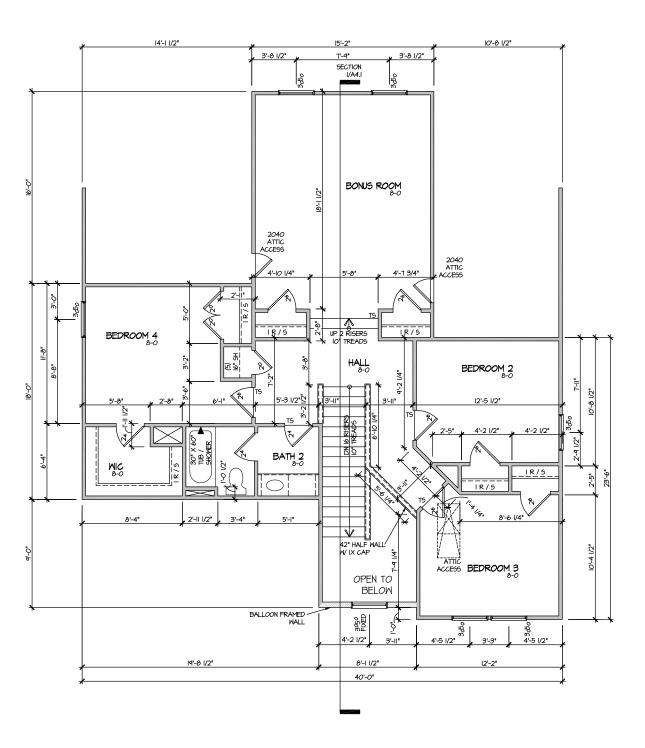


HOUSE NAME:
MIDDLETON
DRAWING TITLE
FIRST FLOOR P

ELEVATION 2 FIRST FLOOR PLAN

SCALE: 1/8" = 1'-0"

SHEET No.



ELEVATION 2 SECOND FLOOR PLAN SCALE: 1/8" = 1'-0"

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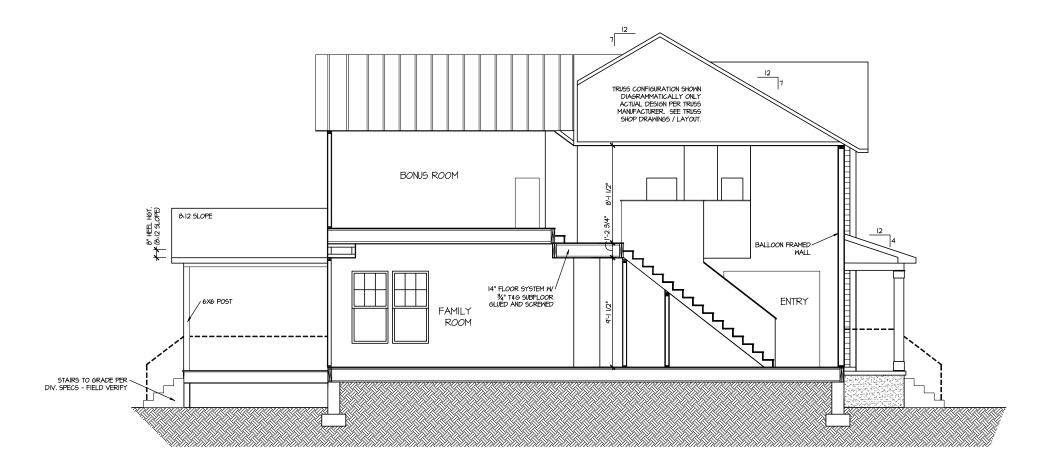
DATE: 03/12/2025 PLAN NO. 2183



HOUSE NAME:
MIDDLETON
DRAWING TITLE
SECOND FLOOR

SHEET No.

A3.2



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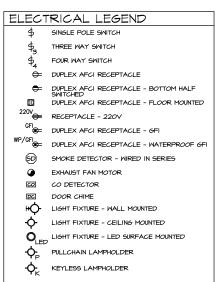
DATE: 03/12/2025 PLAN NO. 2183



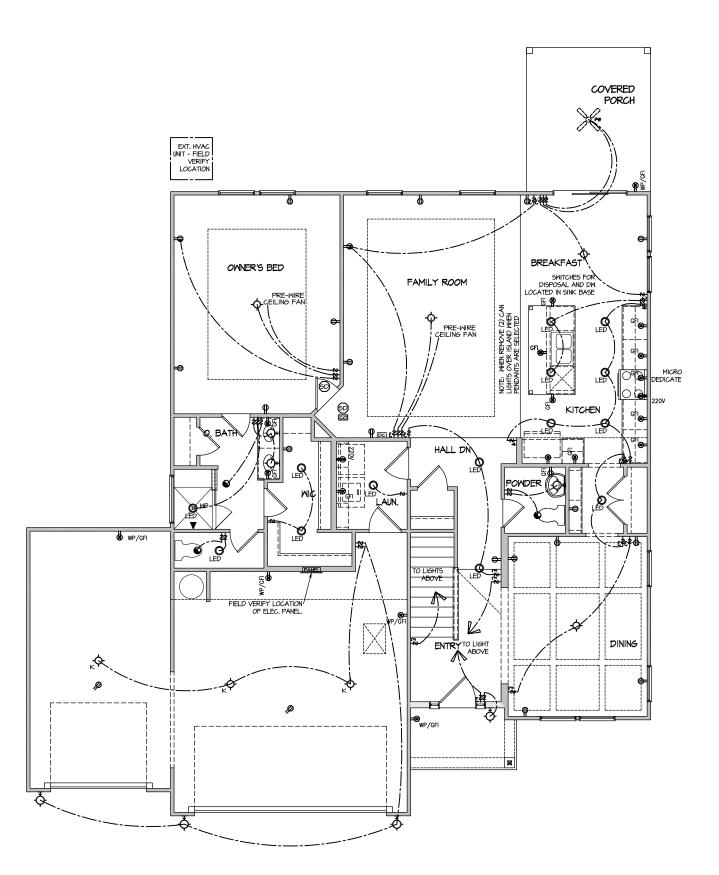
SECTION HOUSE NAME:
MIDDLETON
DRAWING TITLE
BUILDING SECTION

SHEET No. **A4**.

SECTION I SCALE: 1/8" = 1'-0"



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADDPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL POWER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



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ILOOR ELECTRICAL

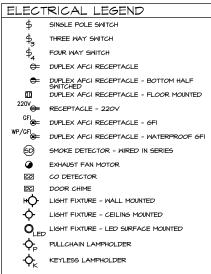
HOUSE NAME:
MIDDLETON
DRAWING TITLE

SHEET No.

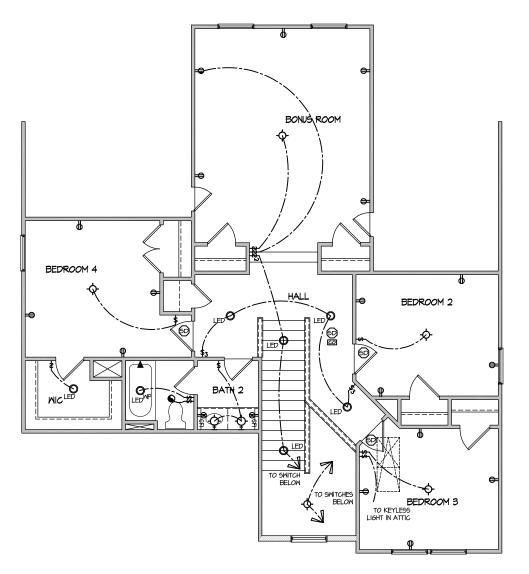
ELECTRICAL PLAN FIRST FLOOR - ELEV. 2

SCALE: 1/8" = 1'-0"

**=**|.|



NOTE: ALL ELECTRICAL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE ADOPTED VERSION OF THE NATIONAL ELECTRICAL CODE, THE LOCAL PONER COMPANY AND TO ALL APPLICABLE LOCAL REGULATIONS.



ELECTRICAL PLAN SECOND FLOOR - ELEV. 2 SCALE: 1/8" = 1'-0"

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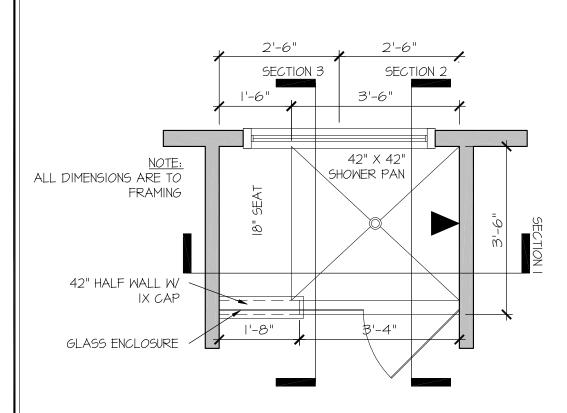
DATE: 03/12/2025 PLAN NO. 2183



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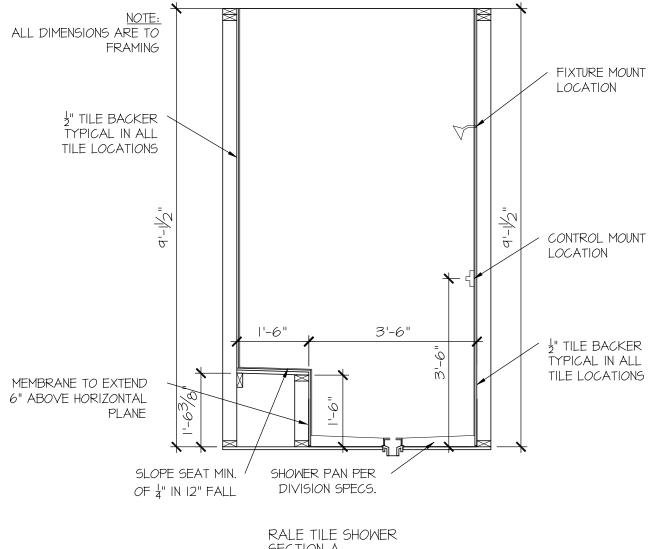
HOUSE NAME:
MIDDLETON
DRAWING TITLE
SECOND FLOOR

SHEET No.



RALE TILE SHOWER 42" X 42" W 18" SEAT

SCALE: 1/2" = 1'-0"



SECTION A

SCALE: 1/2" = 1'-0"

CONSULTANT LOGO

DRAWN BY: L. BEAVERS DATE: 9/1/22 PLAN NO.

11 X 17 SCALE

24 X 36 SCALE



DETAIL SHOWER RALE

SHEET No.



SEAL

DRAWN BY:
L. BEAVERS
DATE: 9/1/22

PLAN NO.

24 X 36 SCALE

~ "

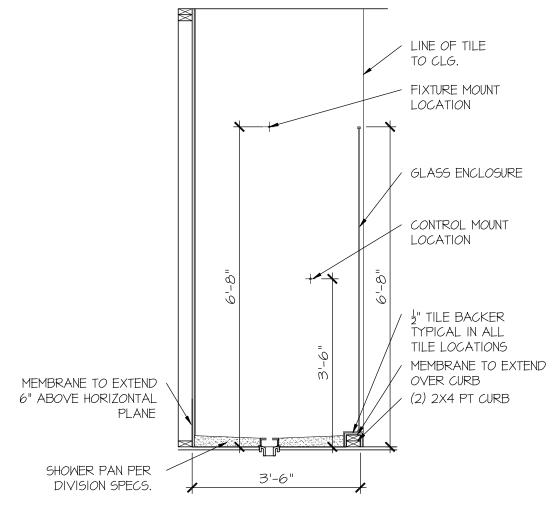


E ILE SHOWER DETAIL

OUSE NAME:

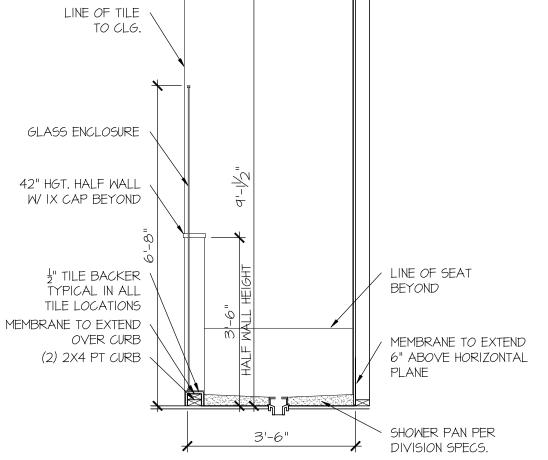
SHEET No.

P||.2



SCALE: 1/2" = 1'-0"





SCALE: 1/2" = 1'-0"

FOOTING DESIGN - 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED, BUILDER/CONTRACTOR MUST VERIEY.

FASTEN 2x4/6 SILL PLATES TO FND WITH A MINIMUM OF 2 ANCHORS

PER PLATE, 12" MAX. FROM PLATE ENDS - UTILIZING: . I/2" DIA. ANCHOR BOLTS @ 6'-0" O.C, 7" MIN. EMBEDMENT

(CONC), 15" MIN, EMBEDMENT (CMU)

SIMPSON MASA ANCHOR STRAPS @ 6'-0" O.C. (CONC) \* SIMPSON MAB23 ANCHOR STRAPS @ 2'-8" O.C. (CMU)

(REFER TO DETAILS FOR 10' TALL WALL ANCHOR REQUIREMENTS)

ALL LUMBER EXPOSED TO WEATHER OR IN CONTACT W CONCRETE OR CMU SHALL BE PRESERVATIVE TREATED SOUTHERN PINE #2.

BUILDER TO VERIFY CORROSION-RESISTANCE COMPATIBILITY OF HARDWARE & FASTENERS IN CONTACT w/ PRESERVATIVE-TREATED WOOD. CONTACT LUMBER & HARDWARE SUPPLIERS TO COORD.

BASEMENT INTERIOR BEARING WALLS & EXTERIOR WALK-OUT BASEMENT WALLS SHALL BE 2x6 @ 16" O.C. SPF OR SYP, "STUD" GRADE OR BETTER

CONCRETE DESIGN BASED ON ACI 318. CONCRETE SHALL ATTAIN THE FOLLOWING MIN. COMPRESSIVE STRENGTHS IN 28 DAYS, U.N.O.; 4,000 psi: ...... FOUNDATION WALLS

2500 psi: ..... FOOTINGS & INTERIOR SLABS ON GRADE ...... GARAGE & EXTERIOR SLABS ON GRADE 60,000 psi

BASEMENT FOUNDATION WALL DESIGN BASED ON:

. 9' OR 10' HEIGHT (AS NOTED ON PLANS) - TALLER WALLS MUST BE ENGINEERED

NOMINAL WIDTH (9 1/5" FOR 10" THICK WALL).

BASEMENT WALL DESIGN IS BASED ON 60 PCF BACKFILL SOIL TYPE CLASSIFICATIONS (SC, ML-CL, OR CL).

BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL IST FLOOR DECK.

PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT, FND, WALL MITH 2" CLEAR, REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS,

 FOR OPENINGS UP TO 36". PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 W (2)2x6 JACK STUDS, U.N.C

• LARGER OPENINGS SHALL BE PER PLAN.

ALL CONCRETE EXPOSED TO THE WEATHER SHALL NOT HAVE LESS THAN 5% OR MORE THAN 7% AIR ENTRAINMENT

ALL FOOTINGS SHALL BEAR AT LEAST 12" BELOW FINISH GRADE.

• FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR

PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY

• JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" O.C. (MAXIMUM)

 JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (I:I RATIO), WITH A MAXIMUM OF I:1.5 RATIO

· CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL

CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C90 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (F/m=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 \$ 530.I.

CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.

PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRAWL SPACE PIERS. ALL PIERS SHALL BE GROUTED SOLID.

PROVIDE 2x6 P.T. PLATE ON INTERIOR CRAWL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE.

DIMENSIONS BY OTHERS, BUILDER TO VERIFY.

DESCRIPTION OF BLDG, ELEMENT 3"x0.131" NAILS

IOIST TO SOLE PLATE

**COUBLE STUD** 

OUBLE TOP PLATE

NTERSECTING WALLS

SOLE PLATE TO JOIST/BLK'G STUD TO SOLE PLATE

TOP OR SOLE PLATE TO STUD

BLK'G. BTWN. JOISTS TO TOP PL.

OP PLATE LAP @ CORNERS &

(ONLY ACCEPTABLE WHERE \* ARE SHOWN)

BUILDER TO VERIFY THAT MODEL HAS BEEN ADEQUATELY TREATED BY A LICENSED AND BONDED PEST CONTROL COMPANY FOR SUBTERRANEAN TERMITES. METHOD AND TYPE OF TREATMENT TO BE DETERMINED BY PEST CONTROL COMPANY.

CONNECTION SPECIFICATIONS (TYP. U.N.O.)

OUBLE TOP PLATE LAP SPLICE (4) NAILS IN LAPPED AREA

3) TOENAILS (3) NAILS @ 4" 0.0 (2) TOENAILS

OFNAILS @ 8"

(3) TOENAILS

(2) NAILS

NAILS @ 24" 04

2½"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS.

#### GENERAL STRUCTURAL NOTES

DESIGN IS BASED ON 2018 NORTH CAROLINA STATE BUILDING CODI RESIDENTIAL CODE.

WOOD FRAME ENGINEERING IS BASED ON NDS. "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION" - LATEST EDITION.

DESIGN LOADS: DEAD = 7 PSF T.C., 10 PSF B.C.

LOAD DURATION FACTOR = 1.25

FLOOR LIVE = 40 PSF (30 PSF @ SLEEPING AREAS) DEAD = 10 PSF (I-JOISTS & SOLID SAWN) IO PSF T.C., 5 PSF B.C. (TRUSSES)

LATERAL 120 MPH, EXPOSURE B. SEISMIC A/B.

2,000 PSF ASSUMED ALLOWABLE BEARING PRESSURE (TO BE VERIFIED BY BUILDER)

(ADD'L IO PSF @ TILE)

#### GENERAL FRAMING

ALL TYP. NAIL FASTENER REQUIREMENTS ARE NOTED IN STANDARD CONNECTIONS TABLE OR ON PLANS. ALL NAILS SPECIFIED ARE MIN DIAMETER AND LENGTH REQUIRED FOR CONNECTION ALL HANGER NAILS SHALL BE INSTALLED PER MANUFACTURER'S REQUIREMENTS FOR MAX CHARTED CAPACITY. NOTE: HANGERS USE COMMON NAIL DIAMETERS NOT TYPICAL FRAMING GUN NAILS.

REFER TO FASTENING SCHEDULE TABLE R602.3(I) FOR ALL CONNECTIONS, TYP. U.N.O.

EXT. & INT. BRG WALLS SHALL BE 2x4 OR 2x6 (AS SHOWN ON PLANS) • 16" O.C. SPF OR SYP "STUD" GRADE LUMBER, OR BETTER, U.N.O. ♦ WALLS OVER 12' TALL SHALL BE PER PLAN.

ALL HEADERS BEAMS & OTHER STRUCTURAL MEMBERS SHALL BE SPRUCE-PINE-FIR #2 (SPF) OR SOUTHERN PINE #2 (SYP) LUMBER, OR BETTER (KII N-DRIED). ALL HEADERS HAVE BEEN DESIGNED BASED ON CALCULATED LOADS & SIZED ACCORDINGLY, CODE TABLES HAVE NOT BEEN USED.

ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED ● 16" O.C. (MAX., U.N.O.)

• HEADERS IN NON-LOAD BEARING WALLS SHALL BE:

(1)2x4/6 FLAT @ OPENINGS UP TO 4', (2)2x4/6 FLAT UP TO 8'.

ALL FRAMING LUMBER SHALL BE DRIED TO 15% MC (KD-15). ENGINEERED LUMBER BEAMS TO MEET OR EXCEED THE FOLLOWING:

 'LSL' - Fb=2325 psi; Fv=310 psi; E=1.55x10^6 psi 'LVL' - Fb=2600 psi: Fv=285 psi: E=2.0xl0^6 psi

- FB=2900 PSI; FV=290 PSI; E=2.0XI0^6 PSI M+K SHALL BE FULLY INDEMNIFIED FOR ANY AND ALL ISSUES RESULTING FROM OR RELATED TO ANY BUILDING COMPONENT IF THI OWNER DOES NOT SUBMIT THE COMPONENT SHOP DRAWINGS TO MH FOR STRUCTURAL REVIEW PRIOR TO FABRICATION DELIVERY, OR

INSTALL ATION FOR 2 & 3 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 3"x0.120" NAILS @ 6" O/C OR 2 ROWS ¼"x3½" SIMPSON SDS SCREWS (OR 31/3" TRUSSLOK SCREWS) @ 16" O/C. USE A MINIMUM OF 3 ROWS FOR BEAM DEPTHS OF 14" OR GREATER APPLY FASTENING AT BOTH FACES FOR 3-PLY CONDITION. LOCATE TOP & BOTTOM NAILS/SCREWS 2" FROM EDGE. SOLID 3½" OR 5½" BEAMS ARE ACCEPTABLE, USE 2 ROWS OF NAILS FOR 2x6 \$ 2x8

FOR 4 PLY BEAMS OF EQUAL WIDTH, FASTEN PLIES TOGETHER WITH 3 ROWS OF 1/4"x6" SIMPSON SDS SCREWS (OR 6 3/4" TRUSSLOK SCREWS) @ 16" O/C. LISE A MINIMUM OF 4 ROWS FOR BEAM DEPTHS OF 14" OR GREATER. APPLY FASTENING AT BOTH FACES (ONE SIDE ONLY FOR TRUSSLOK SCREWS). LOCATE TOP AND BOTTOM SCREWS 2" FROM EDGE, A SOLID 7" BEAM IS ACCEPTABLE.

ALL HEADERS SHALL BE SUPPORTED BY (1)2x JACK STUD & (1)2x

- THE NUMBER OF STUDS SPECIFIED AT A SUPPORT INDICATES THE NUMBER OF JACK STUDS REQUIRED, U.N.O., ALL MULTI-PLY STUDS TO BE FASTENED TOGETHER W/ 3"X0.131"

NAILS @ 24" O.C. (MIN.), EACH PLY. PROVIDE SOLID BLOCKING IN FLOOR SYSTEM UNDER ALL POSTS CONTINUOUS TO FND /BEARING. BLOCKING TO MATCH POST ABOVE

FASTEN 2x WOOD PLATES TO TOP FLANGE OF STEEL BEAMS WITH P.A.F.'s ('HILTI' X-CF PINS OR EQUAL) @ 16" O.C. STAGGERED, OR 1/2" DIA, BOLTS @ 48" O.C. STAGGERED.

ALL EXTERIOR 4x4 WOOD POSTS SHALL HAVE SIMPSON BCS2-2/4 CAP & ABW44Z BASE, U.N.O.

3"x0120" NAII S

(3) TOENAILS\*

(3) TOENAILS\*

NAILS @ 16" O.C.

NAILS 0 16" 0.0

(3) NAILS @ 4" o.c.

OFNAILS @ 6" OC

(II) NAILS IN LAPPED AREA (2) NAILS

#### FLOOR FRAMING

I-JOISTS/TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA. (EXCLUDES MARBLE FLOORS - CONTACT MIKE FOR MARBLE FLOOR DESIGNS)

• AT I-JOIST FLOORS, PROVIDE I 1/8" MIN. OSB RIM BOARD. \* METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.

FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED 'STURD-I-FLOOR 24" O.C. EXPOSURE I (OR APPROVED EQUAL) WITH TONGLE AND

GROOVE EDGES. FASTEN TO FRAMING MEMBERS W GLUE AND

- 2 1 × 0.131 NAILS @ 6"0.c. @ PANEL EDGES \$ @ 12"0.c. FIELD

- 2 3 × 0.120" NAILS • 4" O.C. • PANEL EDGES \$ • 8" O.C. FIELD. - 2 3 × 0.113" NAILS • 3" O.C. • PANEL EDGES € • 6" O.C. IN FIELD #6 x 2" MIN. SCREMS @ 6" O.C. @ PANEL EDGES \$ @ 12" O.C. FIELD

#### ROOF FRAMING

\* BAY WINDOWS & SHED ROOFS (UP TO 6' SPAN) CAN BE 2x4 OR 2x6 RAFTERS & CEILING JOISTS @ 16/24" O.C.

FASTEN FACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5T CLIP (OR APPROVED EQUAL) • ALL BEARING POINTS. PROVIDE (2) H2.5T CLIPS AT 2-PLY GIRDER TRUSSES (3) H25T CLIPS AT 3-PLY GIRDER TRUSSES & ROOF BEAMS - AT ALL BEARING POINTS

METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.

\* ERECT AND INSTALL ROOF TRUSSES PER WTCA & TPI'S BCSI I-08 "GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."

SUPPORT PORCH & SHORT SPAN ROOF TRUSSES (MAX 7' SPAN) W 2x4 LEDGER FASTENED TO: - RIM BOARD w/ (2) 3"x0.131" NAILS @ 16" O.C. MAX. (1-JOISTS)

TRUSS VERTICALS w/ (3) 3"x0.131" NAILS ● 19.2" O.C. MAX. (FLOOR TRUSSES)

 ROOF SHEATHING SHALL BE 1/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE I (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS

- W/ 2 ½" x 0.131" NAILS • 6"O.C. • PANEL EDGES \$ • 12" O.C. FIELD. × 0.120" NAILS @ 4"o.c. @ PANEL EDGES \$ @ 8" O.C. FIELD.

- W 2 8 × 0.113" NAILS • 3"o.c. • PANEL EDGES \$ • 6" O.C. FIELD.

#### HOLD-DOWN SCHEDULE

5YMB0L	SPECIFICATION	
► HD-I	SIMPSON HTT4 HOLD-DOWN * (%" DIA. ANCHOR)	
HD-2	SIMPSON MSTC66 STRAP TIE (CENTER STRAP ON FLOOR SYSTEM UN.O.) -OR- MSTC66B3 ALTERNATE	
► HD-3	SIMPSON STHDI4/STHDI4RJ	

\* <u>UTILIZE THE 95TB24 ANCHOR BOLT</u> • ALL MONOSLAB & INTERIOR RAISED SLAB (I.E. THICKENED SLABS, FOOTINGS) CONDITIONS, MINIMUM 24\* MIN. FOOTING THICKNESS REQUIRED

EPOXY-SET ALTERNATE FOR MONOSLAB & INTERIOR RAISED SLAB CONDITIONS ONLY: UTILIZE SIMPSON 'SET' EPOXY SYSTEM TO FAS THREADED ROD INTO CONCRETE FOUNDATION, PROVIDE 10" (FOR 5/8" DIA.) OR <u>15" (FOR 1/8" DIA.) MIN. EMBEDMENT INTO CONCRETE.</u>

INSTALL PER MANUF, INSTRUCTIONS, MINIMUM 16" FOOTING THICKNESS REQ'D.

#### LEGEND

INTERIOR BEARING WALL

□===□ BEARING WALL ABOVE

• ---- BEAM / HEADER

• = INDICATES SHEAR WALL & EXTENT

EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

INDICATES HOLD-DOWN OR STRAP.

#### NON-BEARING HEADER SCHEDULE

SPAN	2x4 NON-BEARING	2x6 NON-BEARING
	PARTITION WALL	PARTITION WALL
UP TO 3'-0"	(I)2x4 FLAT	(I)2x6 FLAT
UP TO 6'-0"	(2)2x4	(3)2×4
UP TO 8'-0"	(2)2x6	(3)2x6
JP TO 12'-0"	(2)2x8	(3)2x8

NOTES:

\* ALL NON-BEARING INTERIOR STUD WALLS SHALL BE CONSTRUCTED WITH 2x 'STUD' GRADE MEMBERS SPACED a 24" O.C. (MAX.)

#### LATERAL BRACING & SHEAR MALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 20 MPH WIND IN 2018 NCSBC:RC

(120 MPH WIND SPEED IN ASCE 7-10 WIND MAP PER IRC R30(211) EXP. B. RISK CAT. 2 & SEISMIC CAT. A/B

THE DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R30113 OF THE 2018 NOSBORO OR THE SIMPLIFIED PRESCRIPTIVE PROCEDURE IN ACCORDANCE WITH THE 2015 IRC IF THE PARAMETERS OF SECTION R602.12 COMPLY CCORDINGLY, THIS MODEL, AS DOCUMENTED AND DETAILED HEREWITHIN, IS ADEQUATE TO RESIST THE CODE REQUIRED LATERAL FORCES.

DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NGSBG:RG SECTION R802.II.I. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602.3.5 R802.II.

#### EXT. WALL SHEATHING SPECIFICATION

7/16" OSB OR 15/32" PLYWOOD: FASTEN SHEATHING W/ 2 ¾"x0.II3" NAILS @ 6" O.C. AT EDGES \$ @ 12" O.C. IN THE PANEL FIELD. TYP, U.N.O.

HORIZONTAL BLOCKING OF EXT. WALL/SHEAR WALL PANEL EDGES IS NOT REQUIRED BY THIS DESIGN EXCEPT FOR THOSE AREAS SPECIFICALLY NOTED.

ALL EXT. WALLS SHALL BE CONTINUOUSLY SHEATHED AND ARE CONSIDERED SHEAR WALLS.

ALT. STAPLE CONNECTION SPEC: 1 1/2" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES \$ @ 6" O.C IN FIELD

#### BLOCKED PANEL EDGES

AT DESIGNATED AREAS - FASTEN SHEATHING w/ 2 36" × 0.113" NAILS @ 6" O.C. AT ALL PANEL EDGES AND 12" O.C. IN THE PANEL FIELD OR 13/4" 16 GA STAPLES (1/6" CROWN) @ 3" O.C. AT EDGES & @ 6 O.C IN FIELD. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT OF SHEAR WALL OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

#### 3" O.C. EDGE NAILING

AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W 8d NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED AND INSTALLED FULL HEIGHT SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING.

SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING, IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.

\* DESIGN ASSUMES 16" O.C MAX. STUD SPACING, U.N.O. \* ALL STRUCTURAL PANELS ARE TO BE DIRECTLY

PRE-MANUFACTURED PANELIZED WALLS:
FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD W/ 3" x 0.120" NAILS @ 4" O.C. (THRU ONE SIDE ONLY)

APPLIED TO STUD FRAMING.

INDICATES EXTENT OF INT. OSB SHEARWALL
OR 3" O.C. OSB SHEARWALL.

▶ INDICATES HOLDOWN BELOW

#### MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN. DETAIL. AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY DETERMINE THE ERECTION PROCEDURES SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS NCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING FLEMEN IN CONTACT WITH FLOOR FRAMING ARE LEVEL, INCLUDING, BUT NOT LIMITED TO: FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIEY LEVELNESS AND MAKE AD JUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTR OR WARRANTY TOLERANCES.

#### ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS NOTED OTHERWISE ON PLAN.

TRUSSES/ JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUS BEAMS DO NOT EXCEED THE FOLLOWING:

ROOF TRUSSES: I/4" DEAD LOAD

FLOOR TRUSSES, ATTIC TRUSSES, \$ 1-JOISTS: I/8" DEAD LOAD

FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAL LOAD. (NOT DIFFERENTIAL DEFLECTION)

YENEER LINTEL SCHEDULE STEEL ANGLE SIZE 3'-0" 20 FT, MAX L3"x3"x/4" 3 FT. MAX L3"x3"x/4" I2 FT. MAX L4"x3"x/4" 20 FT, MAX L5"x3½"x%" 3 FT, MAX L4"x4"x/4" \* 12 FT. MAX L5"x3K"x%"

L6"x31/2"x3/5"

L6"x3½"x%"

L7"x4"x/5" \*\*

SHALL SUPPORT 2 %" - 3 ½" VENEER w/ 40 psf MAXIMUM WEIGHT. 16' SHALL HAVE 4" MIN. BEARING

I6 FT. MAX

I2 FT, MAX

3 FT, MAX

16' SHALL HAVE 8" MIN. BEARING

2 FT. MAX

9'-6"

16'-0"

16' SHALL NOT BE FASTENED BACK TO HEADER, 16' SHALL BE FASTENED BACK TO WOOD HEADER IN WALL \$48"0.0 w/½" DIA. x 3½" LONG LAG SCREMS IN 2" LONG VERTICALLY SLOTTED HOLES.

AX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE

OPENING.
ALL LINTELS SHALL BE LONG LEG VERTICAL.
WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 1/2" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT

FINGHING.
SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPASSED BY THE ABOVE PARAMETERS, FOR ANY LINTEL FASTINERS SHALL MAINTAIN A 3/2\* WINNIAM) CLEAR DISTANCE FROM BOTTOM OF BEAM.

' FOR QUEEN VENEER USE L4x3が。' '\* FOR 3½' VENEER ONLY, SEE PLAN FOR VENEER SUPPORT IF VENEER < 3½' THICK.

SD2, REFERS TO SD2, A FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### ENGINEERED BEAM MATERIAL SCHEDULE

BEAM NUMBER	LVL OPTION	PSL OPTION	LSL OPTION	FLITCH OPTION	STEEL OPTION
001	(2)13/4"×117/6" - H	3½"xII%" - H	(2)134"x1136" - H	(2)2xi2 + (i) 片xik	N/A
OOIA	(2)13/4"×117/6" - H	3½"×11½" - H	(2)134"x1136" - H	(2)2xi2 + (i)火"xi以" STEEL FLITCH PLATE - H	N/A
002	(2)13/4"×18" - FT	5¼"xl8" - FT	N/A	(3)2xi2 + (2)片"xiK" STEEL FLITCH PLATES - FB	WI2xI9 - F
003	(2)134"×14" - F	3½"x 4" - F	(2)13/4"x14" - F	(2)2xl2 + (I)¼"xl¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
004	(2)134"×14" - F	3½"x14" - F	(2)i³¼"xi4" - F	(2)2xi2 + (i) ¼"xil¼" Steel Flitch Plate - FB	WI2xI4 - F
005	(2)13/4"×14" - F	3½"x 4" - F	(2)1 <sup>3</sup> / <sub>4</sub> "x14" - F	(2)2xi2 + (i)¼"xi¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
006	(2)134"×14" - F	3½"x 4" - F	(2)13/4"×14" - F	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
001	(2)13/4"x117/6" - F	3½"xli%" - F	(2)1¾"x11%" - F	(2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - F	MI0x12 - F
008	(2)13/4"×14" - F	3½"x 4" - F	(2)1 <sup>3</sup> / <sub>4</sub> "x14" - F	(2)2xi2 + (i) ¼"xil¼" STEEL FLITCH PLATE - FB	WI2xI4 - F
004	(3)13/4"x18" - FT	5¼"xl6" - FT	N/A	(4)2xi2 + (3) ½"xi以" Steel Flitch Plates - FB	WI2x26 - F
010	(3)1¾"x20" - FT	5¼"x20" - FT	N/A	(4)2xl2 + (3) %"xl以" STEEL FLITCH PLATES - FB	WI2x35 - F
OII	(2)1¾"x11%" - FB	3½"xII%" - FB	(2)13/4"x14" - FB	(2)2xl2 + (I) ¼"xl¼" STEEL FLITCH PLATE - FB	WIOxI2 - FB

BEAM NOTATION: - "F" INDICATES FLUSH BEAM

- "FT" INDICATES FLUSH TOP BEAM - "FB" INDICATES FLUSH BOTTOM BEAM

- "D" INDICATES DROPPED BEAM

- "H" INDICATES DROPPED OPENING HEADER

REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS

FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES BENEATH BEAM AS REQ'D. FASTEN ATES IN SUCCESSION W/ (2) 3"XO.120" NAILS @ 8" O.C.

FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN PLATES IN SUCCESSION W/ (2) 3"x0.120" NAILS @ 8" O.C.

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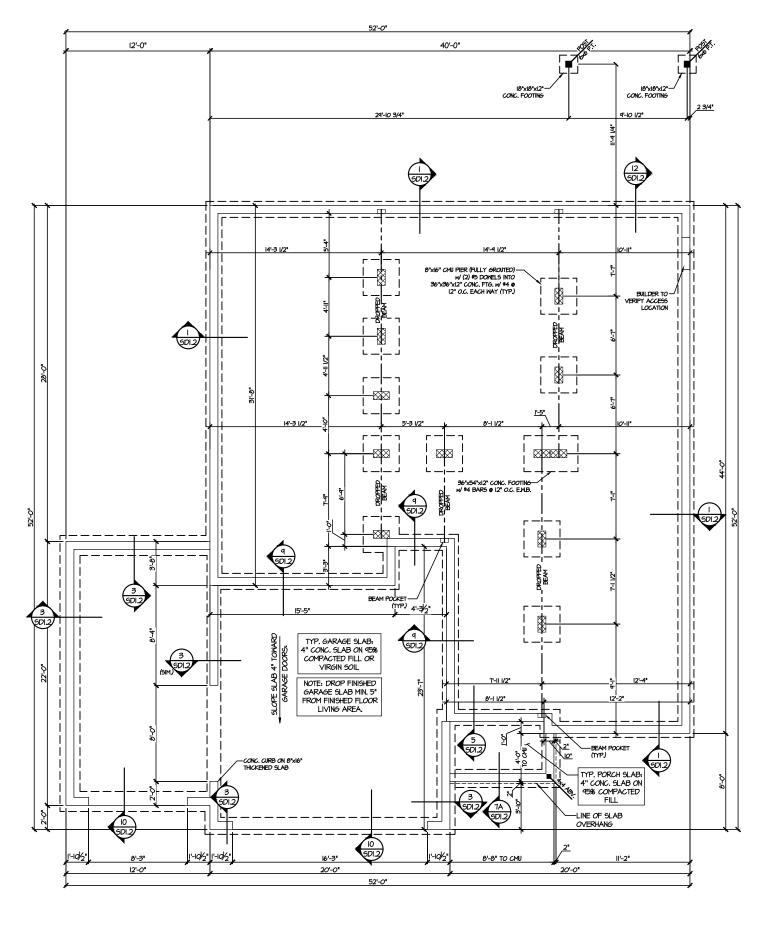
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> LOT 107 Raleigh BL

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M&K project number: 126-2306

drawn by: issue date: 03-21-25

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LEGEND

• = = INDICATES SHEAR WALL & EXTENT

\* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE,

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES

& SCHEDULES

• IIIIII INTERIOR BEARING WALL

● □===□ BEARING WALL ABOVE • ---- BEAM / HEADER

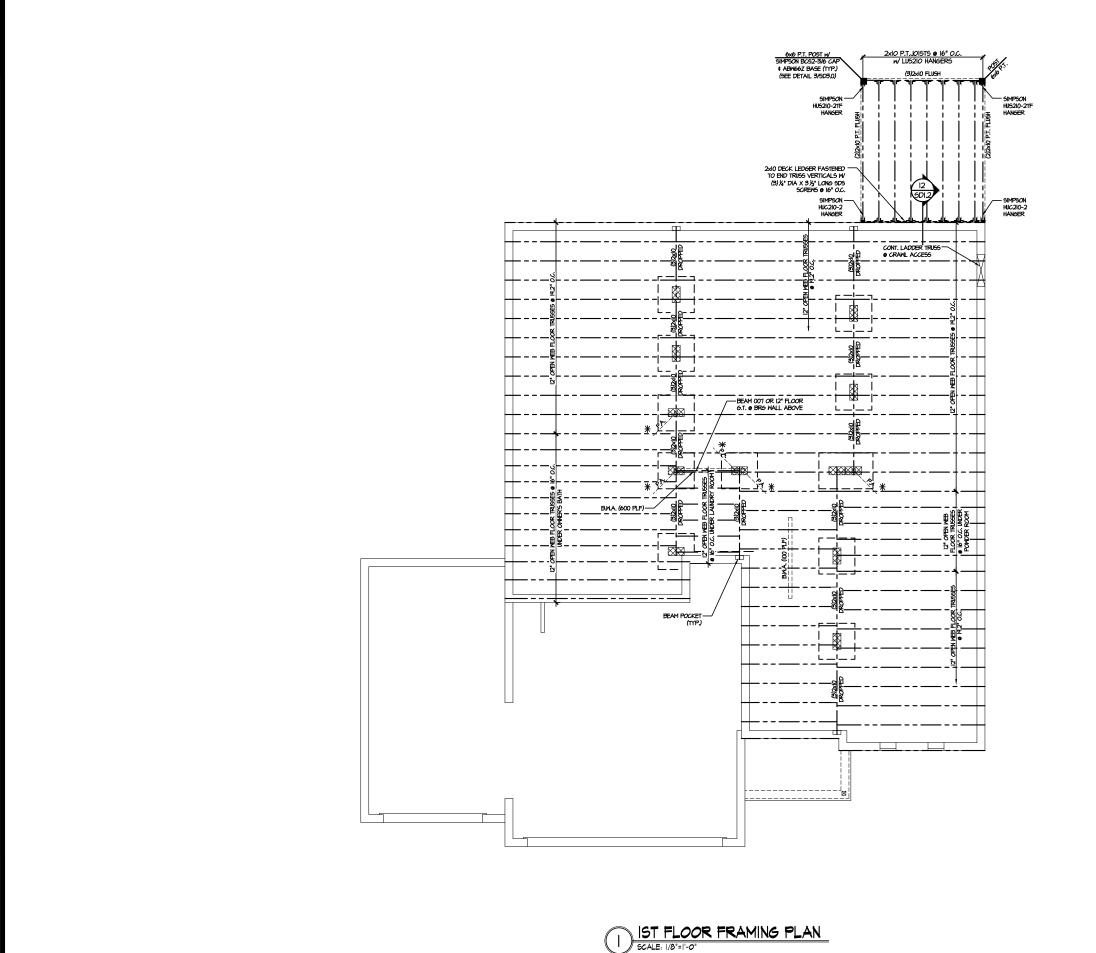
• EXTENT OF OVERFRAMING

INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

JL METAL HANGER

BLAKE POND COMMUNITY Lot 107 - Middleton 2 Raleigh, nc

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M&K project number: 126-2306

drawn by: issue date: 03-21-25

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SEE SO.O FOR BEAM SCHEDULE

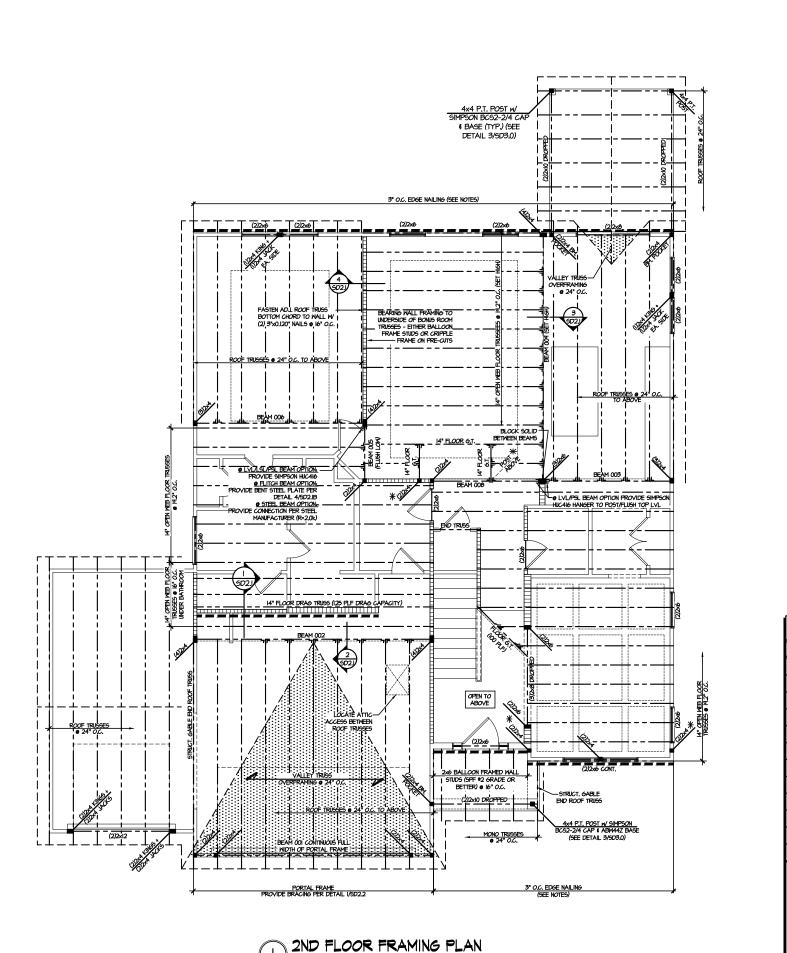
#### LEGEND

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE
- BEAM / HEADER
- ■ INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- JL METAL HANGER
- \* INDICATES POST ABOVE. PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES \$ SCHEDULES

OOR

BLAKE POND COMMUNITY Lot 107 - Middleton 2 Raleigh, nc



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SUBMISSION DESIGNATION OF 18802 Y

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SD2.I REFERS TO SD2.IA FOR LVL/PSL/LSL BEAMS OR SD2.IB FOR FLITCH BEAMS OR SD2.IC FOR STEEL BEAMS

#### LEGEND

SEE SO.O FOR BEAM SCHEDULE

- INTERIOR BEARING WALL
- □===□ BEARING WALL ABOVE • --- BEAM / HEADER
- = = INDICATES SHEAR WALL & EXTENT
- EXTENT OF OVERFRAMING
- \* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.
- INDICATES HOLD-DOWN OR STRAP.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

#### ENGINEERED BEAM MATERIAL SCHEDULE LVL OPTION STEEL OPTION PSL OPTION LSL OPTION FLITCH OPTION (2)2xl2 + (I) 片"xlik" STEEL FLITCH PLATE - H 001 (2)15%"\-117%" - H (2)13%"\11%" - H N/A 36"xII%" - H 00IA (2)2xi2 + (i) 为"xii以" STEEL FLITCH PLATE - H (2)13/4"×117/4" - H (2)13/4"×113/4" - H N/A 3K"xII%" - H (3)2x12 + (2) ½"x1½" STEEL FLITCH PLATES - FB (2)134"x18" - FT 5¼"xi8" - FT N/A WI2xI9 - F (2)2xl2 + (I)¼\*xl¼\* STEEL FLITCH PLATE - FB 003 (2)13/4"x14" - F 3½"x|4" - F (2)13/4"x14" - F (2)2xl2 + (l)从"xli¼" STEEL FLITCH PLATE - FB 004 (2)13/4"x14" - F 3½"x14" - F (2)13/4"x14" - F WI2xI4 - F (2)2xl2 + (I)从"xli以" STEEL FLITCH PLATE - FB 005 (2)|3/4"x|4" - F (2)|3/4"x|4" - F WI2xI4 - F 36"x14" - F (2)2xl2 + (l)从"xll以" STEEL FLITCH PLATE - FB 006 (2)13/4"x14" - F (2)13/4"×14" - F 3%"x14" - F WI2xI4 - F (2)2x12 + (1) ¼"x11¼" STEEL FLITCH PLATE - F (2)134"x1136" - F 3½"xII%" - F (2)1¾"x11%" - 1 WIOxI2 - F (2)2xl2 + (I) ¼"xll¼" STEEL FLITCH PLATE - FB 008 (2)13/4"×14" - F (2)13/4"x14" - F 3½"x|4" - F '4)2xl2 + (3) ½"xl以" STEE! FLITCH PLATES - FB 009 (3)134"x18" - FT 5¼"xl8" - FT WI2x26 - F (4)2xl2 + (3) %"xlK" STEEL FLITCH PLATES - FB 010 (3)|%"x20" - FT 5½"x20" - FT N/A WI2x35 - F (2)2xl2 + (I)从"xli以" STEEL FLITCH PLATE - FB OII (2)1¾"x11¾" - FB 3½"xll%" - FB (2)13/4"×14" - FB WIOxI2 - FB

- BEAM NOTATION:

   "F" INDICATES FLUSH BEAM
   "FT" INDICATES FLUSH TOP BEAM
   "FB" INDICATES FLUSH BOTTOM BEAM
   "D" INDICATES FLUSH BOTTOM BEAM
   "I" INDICATES DROPPED BEAM
   "I" INDICATES DROPPED OPENING HEADER

PLATES IN SUCCESSION w/ (2) 3"x0,120" NAILS @ 8" O.C.

- REFER TO DETAIL D/SD2.0 FOR TYPICAL FLITCH BEAM CONNECTIONS
  REFER TO DETAIL E/SD2.0 FOR TYPICAL STEEL BEAM CONNECTIONS
  FOR FLUSH TOP BEAMS PROVIDE 2X STACKED PLATES DENEATH BEAM AS REQ'D. FASTEN
- PLATES IN SUCCESSION W (2) 3"X0.120" NAILS @ 8" O.C. FOR FLUSH BOTTOM BEAMS PROVIDE 2x STACKED PLATES ATOP BEAM AS REQ'D. FASTEN

POND COMMUNIT
- MIDDLETON 2
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BLAKE I LOT 107 -RALEIGH,  $\overline{\mathsf{O}}$ 

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M&K project number: 126-2306

drawn by: issue date: 03-21-25

REVISIONS:

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LEGEND

• == INDICATES SHEAR WALL & EXTENT EXTENT OF OVERFRAMING

\* INDICATES POST ABOVE, PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

REFER TO SO.O FOR TYPICAL STRUCTURAL NOTES & SCHEDULES

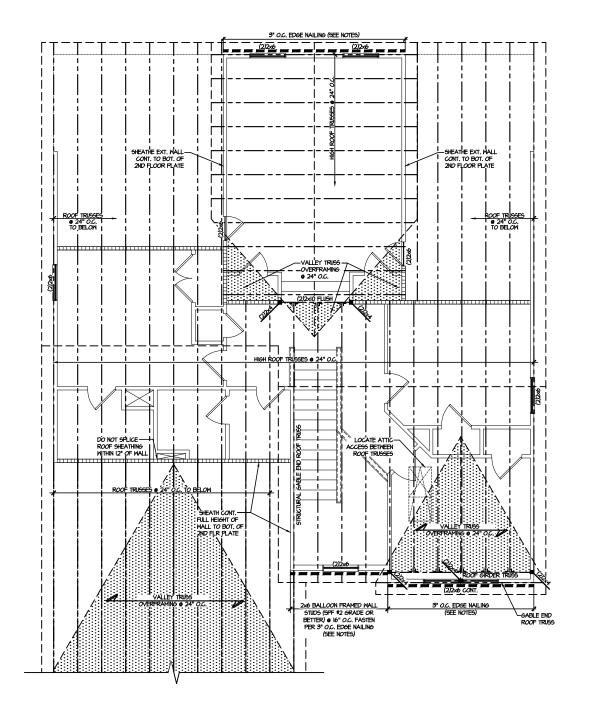
INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.

 INTERIOR BEARING WALL ● □===□ BEARING WALL ABOVE • --- BEAM / HEADER

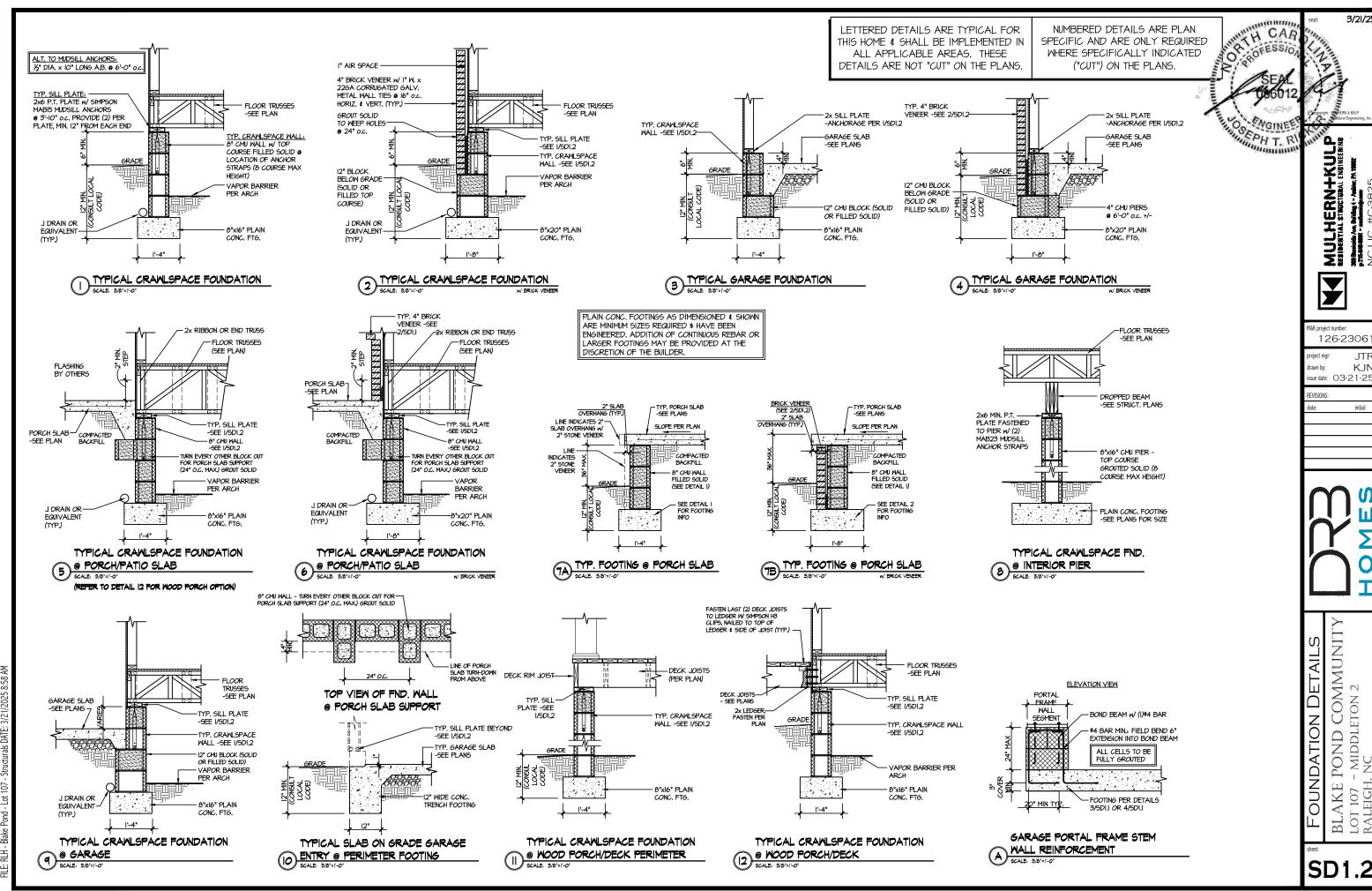
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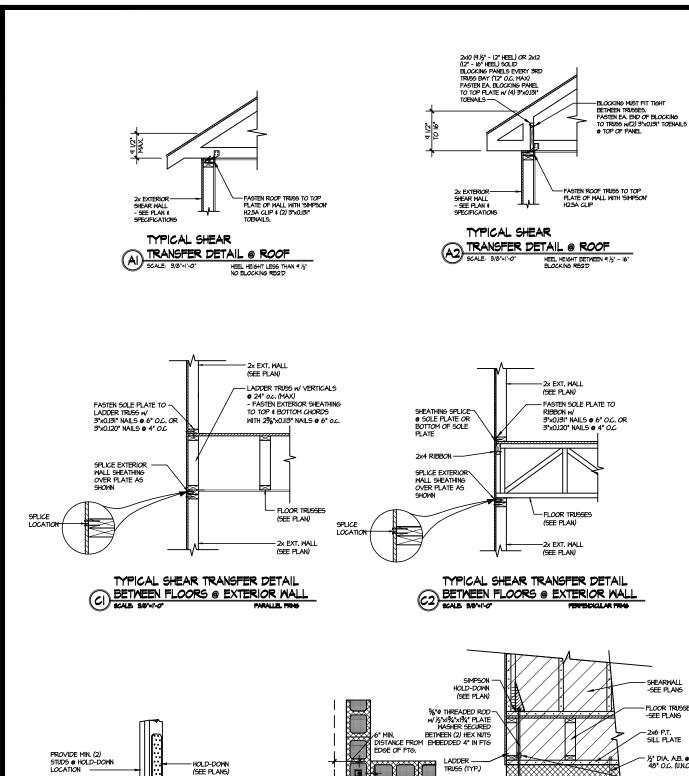


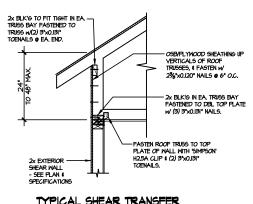


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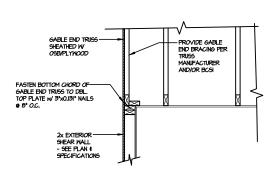
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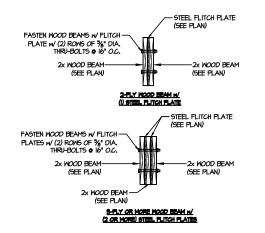














SHEARWALL -SEE PLANS FLOOR TRUSSES

2x6 P.T. SILL PLATE

COUPLER MAY BE

USED TO FASTEN ADD'L LENGTH OF THREADED ROD TOGETHER

CMU FND. WALL

FND. PLANS)

. 6" MIN. TYPICAL CMU FOUNDATION HOLD-DOWN

(CORNER SHOWN - APPLICABLE TO ALL CONDITIONS)

\_\_\_ (REFER TO ARCH.

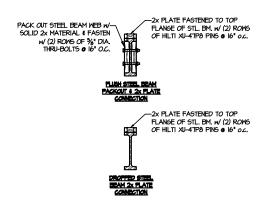
F2 INSTALLATION SCALE: N.T.S.

6" MIN.
DISTANCE FROM
EDGE OF FTG.

HOLD-DOWN LOCATION

ANCHOR (REFER TO HOLD-DOWN SCHEDULE ON PLAN) INTO
CONCRETE OR 3,000 PSI GROUTED
CMU CELLS.

TYPICAL HOLD DOWN INSTALLATION SCALE, N.T.S.



TYPICAL STEEL BEAM CONNECTION DETAIL SCALE 8/4\*-1-0\*

LETTERED DETAILS ARE TYPICAL FOR THIS HOME & SHALL BE IMPLEMENTED IN ALL APPLICABLE AREAS. THESE DETAILS ARE NOT "CUT" ON THE PLANS.

NUMBERED DETAILS ARE PLAN SPECIFIC AND ARE ONLY REQUIRED WHERE SPECIFICALLY INDICATED ("CUT") ON THE PLANS.

ETAILS

COMMUNIT BLAKE POND CON Lot 107 - Middleton Raleigh, NC

3/21/2

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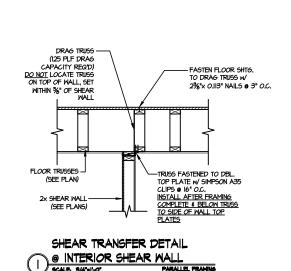
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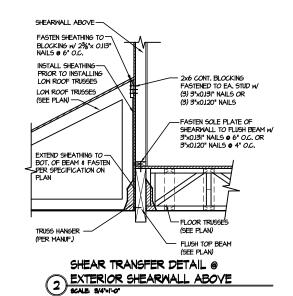
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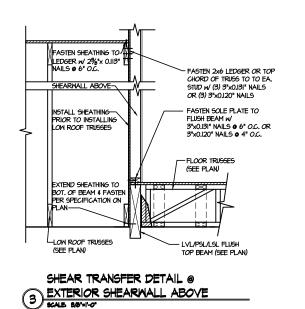
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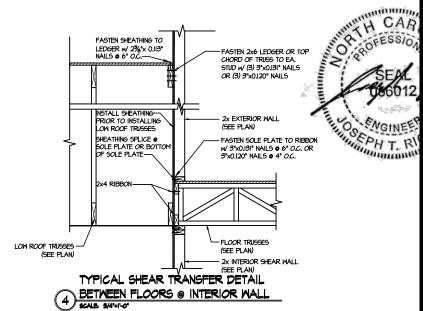
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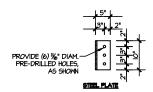
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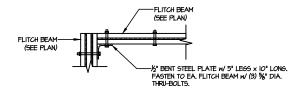
BLAKE POND COMMUNIT' Lot 107 - Middleton 2 Raleigh, nc FRAMING DETAILS

**SD2.1A** 

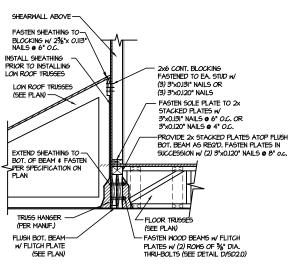
FILE: RLH - Blake

SHEAR TRANSFER DETAIL @ INTERIOR SHEAR WALL
SCALE 5/4'=1'-0' PARALL

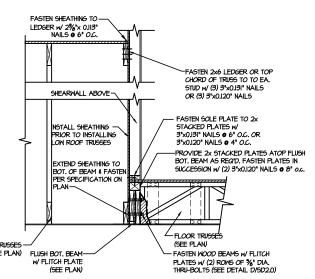




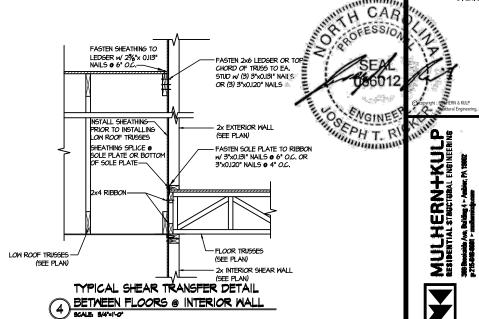
TYPICAL FLITCH BEAM TO FLITCH BEAM CONNECTION DETAIL



SHEAR TRANSFER DETAIL @ 2 EXTERIOR SHEARWALL ABOVE



3 EXTERIOR SHEARWALL ABOVE SCALE SAS'SING"



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drawn by: issue date: 03-21-25

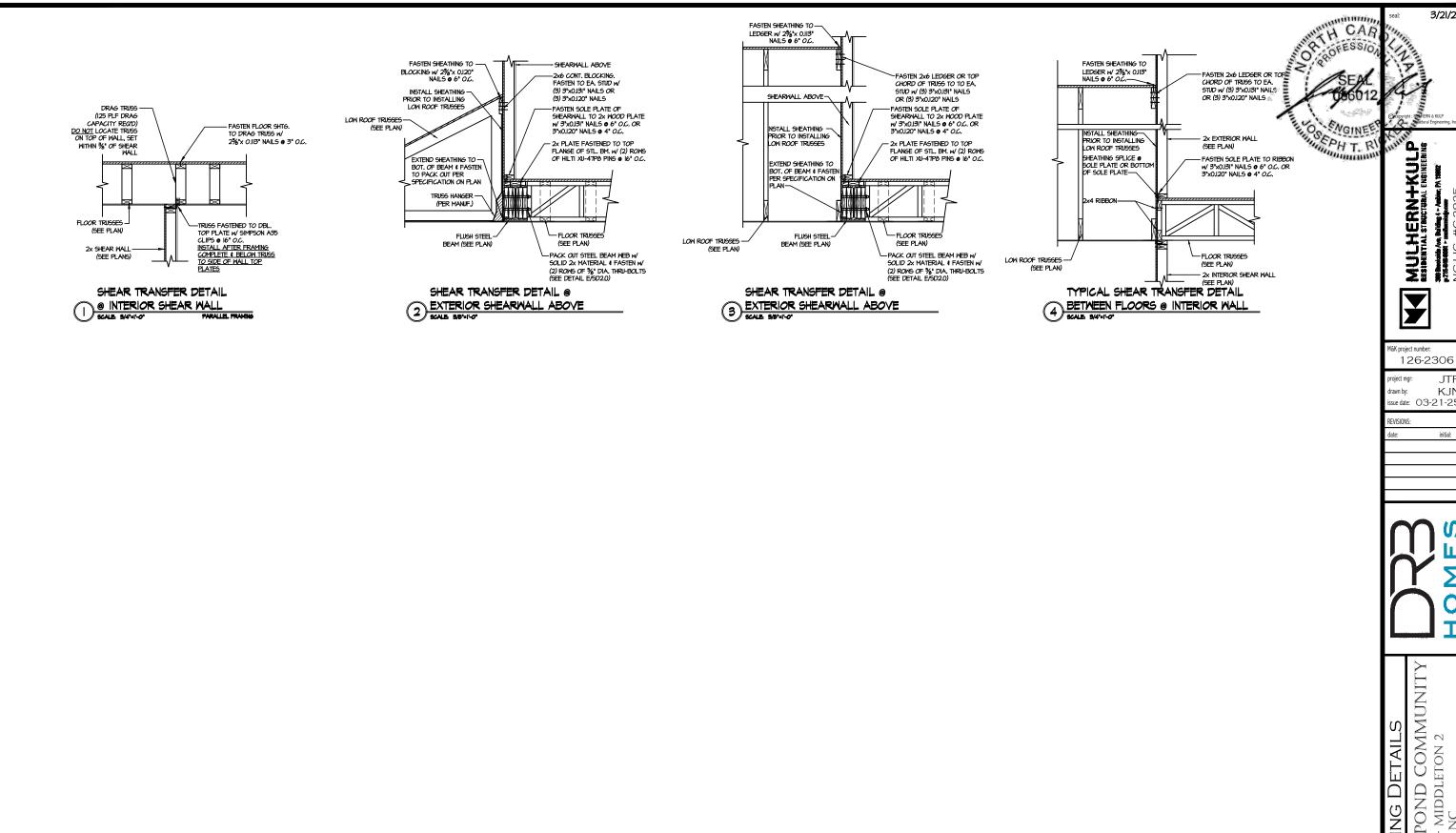
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BLAKE POND COMMUNIT Lot 107 - Middleton 2 Raleigh, nc DETAILS

SD2.1B

LOW ROOF TRUSSES (SEE PLAN) FLUGH BOT. BEAM— W FLITCH PLATE (SEE PLAN) SHEAR TRANSFER DETAIL @



RLH - Blake

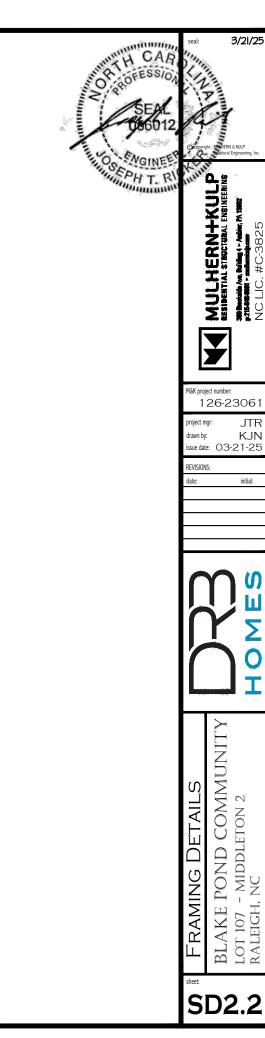
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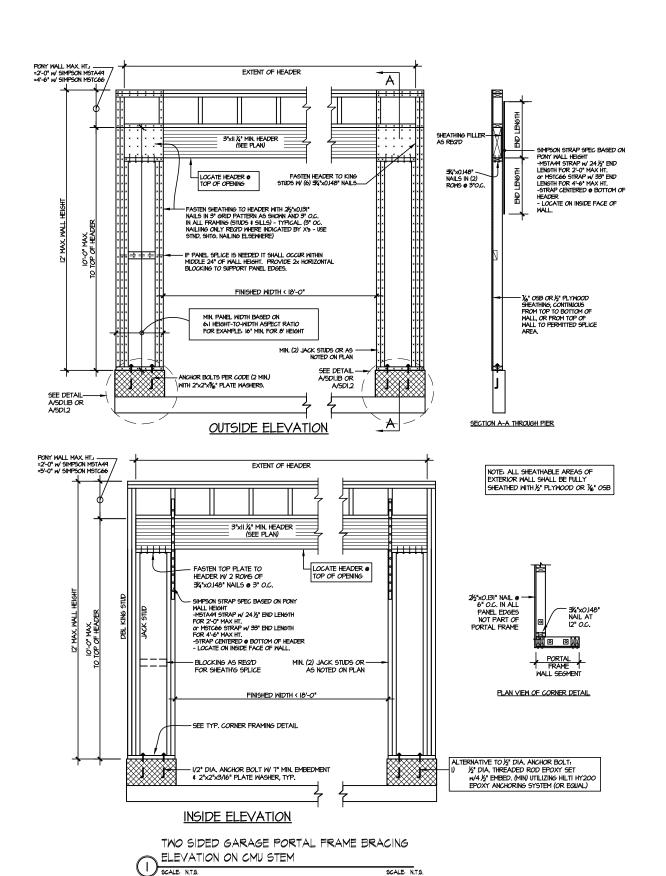
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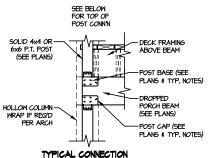
SD2.1C



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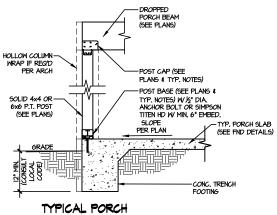






## TYPICAL CONNECTION DETAIL & 2nd FLOOR DECK

POST CONNECTION DETAIL
SCALE: NONE



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drawn by:

REVISIONS:

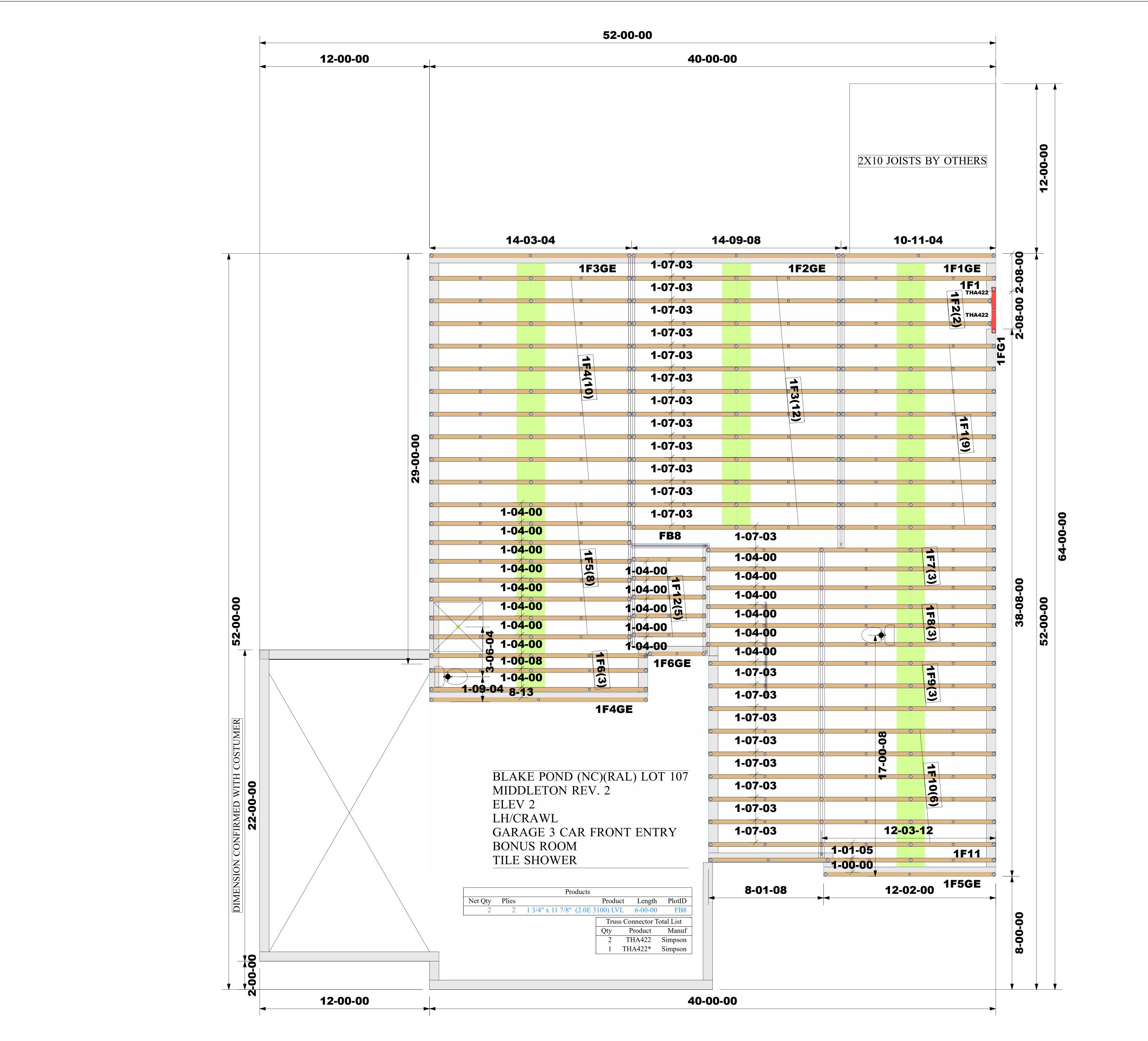
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issue date: 03-21-25

initial:

BLAKE POND COMMUNITY Lot 107 - Middleton 2 Raleigh, nc

**SD3.0** 



OWF TRUSS LAYOUT SCALE: NTS



Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

Stomer: DRB RaleighName: Blake Pond Lot 00.0107 OWF#: Lot 00.0107

Middleton

Name:

Model



NOTE:

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IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.

THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THI CONSTRUCTIION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.

## WARNING:

CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVEN TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.

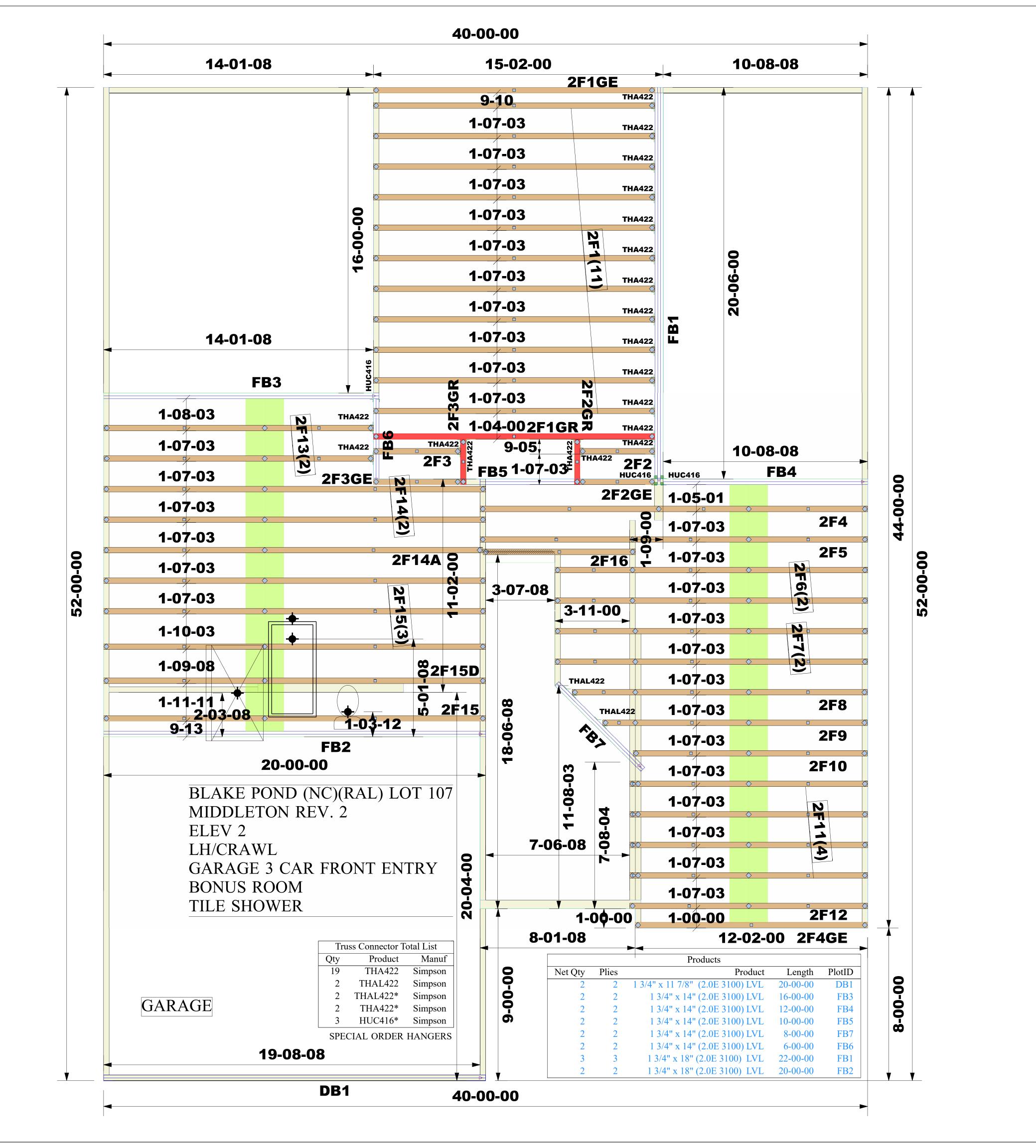
Job #:

2503-3655

signer:

Roger Espinoza
Sales Rep:

Robbie Zarobinski



OWF TRUSS LAYOUT SCALE: NTS



Structural, LLC 201 Poplar Avenue Thurmont, MD 21788 Phone: 301-271-7591

Sustomer: DRB Raleigh

Job Name:
Blake Pond Lot 00.0107 OWF

ot #: Lot 00.0107

Middleton

Name:

Model



NOTE:

ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOI TRUSSES TO SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATION FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THE CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.

IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR

RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTIION DESIGN, DRAWINGS, DOCUMENTS INCLUDIN THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.

## WARNING:

BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS
DESIGNER, PLATE MANUFACTURER, OR THE TRUSS
MANUFACTURER. PERSONS ERECTING TRUSSES ARE
CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE
ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENTOPPLING AND DOMINOING DURING ERECTION; AND
PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC
APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTAR
AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER

TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND PLUMB POSITION WHERE NO SHEATHING IS APPLIED DIRECTLY TO TO AND/OR BOTTOM CHORDS, THEY SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED DESIGN. TRUSSES SHALL BE HANDLED WITH REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.

Job #:

2503-3655

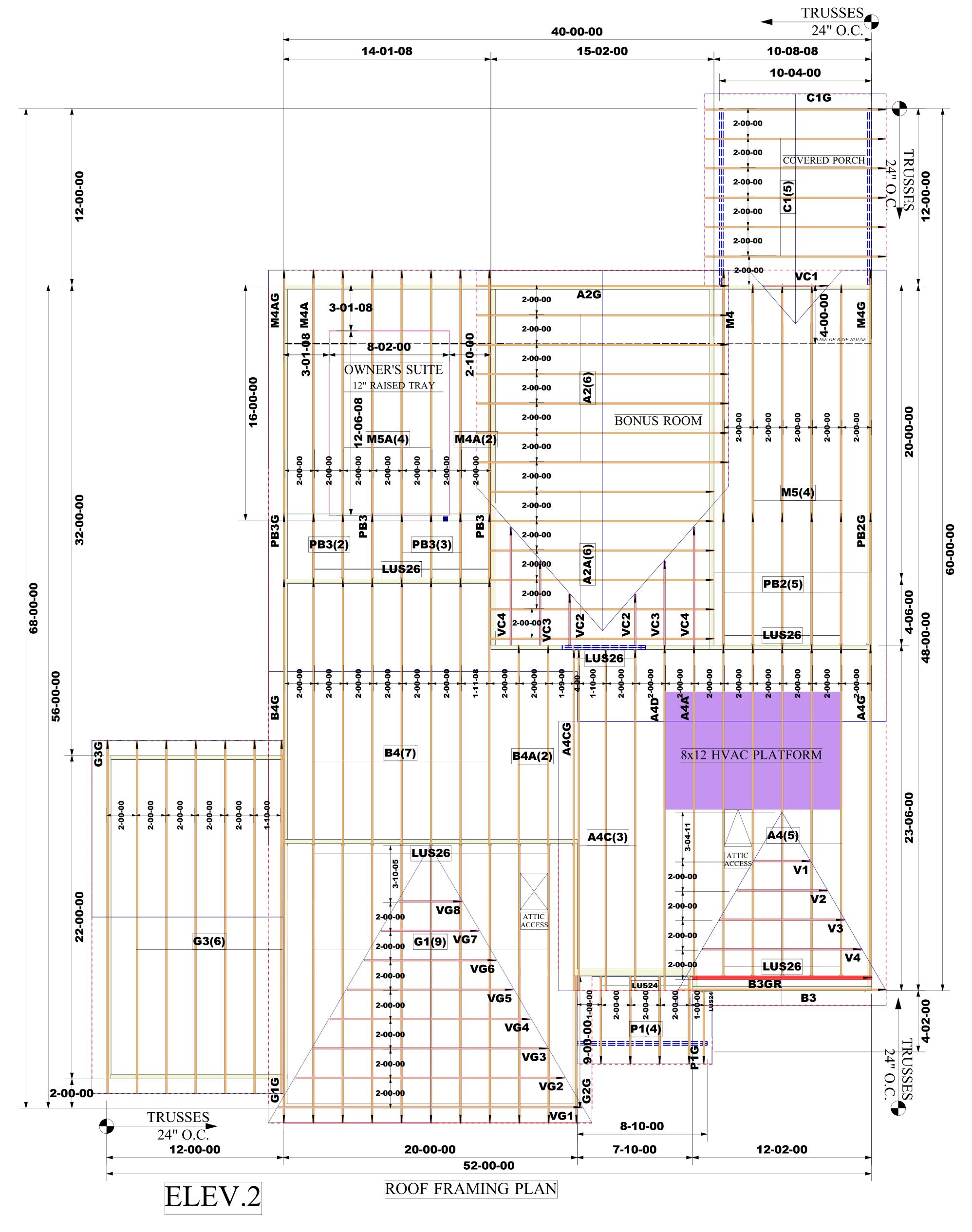
signer:

Roger Espinoza
Sales Rep:

Robbie Zarobinski

## ROOF TRUSS LAYOUT

SCALE: NTS



\*EXTERIOR DIMENSIONS ARE TO STUD. \*TRUSS 2' O.C U.N.O \*INSTALL SIMPSON H2.5A HURRICANE ANCHORS AT EACH BEARING POINT

Truss Connector Total List

Qty Product Manuf

7 LUS24 Simpson

32 LUS26 Simpson

140 One H2.5A Simpson

BLAKE POND SF(NC)(RAL) LOT 00.0107 PHASE MODEL-2183-1-MIDDLETON
GARAGE LEFT SIDE
GARAGE 3 CAR FRONT ENTRY
OPT COVERED PORCH
OPT TRAY CEILING OWNER'S BEDROOM

		1			
Job #:	WARNING:	NOTE:	Customer: DRB Raleigh		
2503-3656	CONVENTIONAL FRAMING, ERECTION AND/OR PERMANENT BRACING IS NOT THE RESPONSIBILITY OF THE TRUSS DESIGNER, PLATE MANUFACTURER, OR THE	IT IS THE RESPONSIBILITY OF THE BUILDING DESIGNER OR ARCHITECT TO PROVIDE AN APPROPRIATE CONNECTION FOR TRUSSES TO	<u> </u>		
	TRUSS MANUFACTURER. PERSONS ERECTING TRUSSES ARE CAUTIONED TO SEEK PROFESSIONAL ADVICE REGARDING THE ERECTION BRACING WHICH IS ALWAYS REQUIRED TO PREVENT TOPPLING AND DOMINOING DURING ERECTION; AND PERMANENT BRACING WHICH MAY BE REQUIRED IN SPECIFIC	SUPPORTING STRUCTURE PER REACTIONS SHOWN ON TRUSS ENGINEERING. SPECIAL CONSIDERATIONS FOR MECHANICAL EQUIPMENT AND/OR PLUMBING (AND THEIR CONNECTIONS) IN TRUSS SPACE MUST BE DIAGRAMMED BY BUILDER ON APPROVED TRUSS LAYOUT PRIOR TO FABRICATION.	Job Name: Blake Pond Lot 00.0107 Roof	Third-Party Quality Assurance Licensee TPI Plant W974	
Designer:	APPLICATIONS. SEE "BRACING WOOD TRUSSES COMMENTARY AND RECOMMENDATIONS" (BCSI 1) FOR FURTHER INFORMAITON.  TRUSSES SHALL BE INSTALLED IN A STRAIGHT AND	THIS COMPANY IS A TRUSS MANUFACTURER WHOSE RESPONSIBILITIES ARE LIMITED TO THOSE DESCRIBED IN WTCA 1-1995 "DESIGN RESPONSIBILITIES". ACCORDINGLY, IT DISCLAIMS	<u>Lot #:</u> Lot 00.0107	Structural, LLC 201 Poplar Avenue	
Sayan Roy Sales Rep:	PLUMB POSITION WHERE NO SHEATHING IS APPLIED  DIRECTLY TO TOP AND/OR BOTTOM CHORDS, THEY  SHALL BE BRACED AS SPCIFIED ON THE ENGINEERED  DESIGN. TRUSSES SHALL BE HANDLED WITH	ANY RESPONSIBILITIES AND/OR LIABILITY FOR THE CONSTRUCTION DESIGN, DRAWINGS, DOCUMENTS INCLUDING THE INSTALLATION, AND BRACING OF TRUSSES MANUFACTURED BY THIS COMPANY.		Thurmont, MD 21788 Phone: 301-271-7591	
Robbie Zarobinski	REASONABLE CARE DURING ERECTION TO PREVENT DAMAGE OR PERSONAL INJURY.	TROOPS MANUFACTORED BY THIS COMPANY.	Model Name: Middleton	1 110116. 301-271-7331	