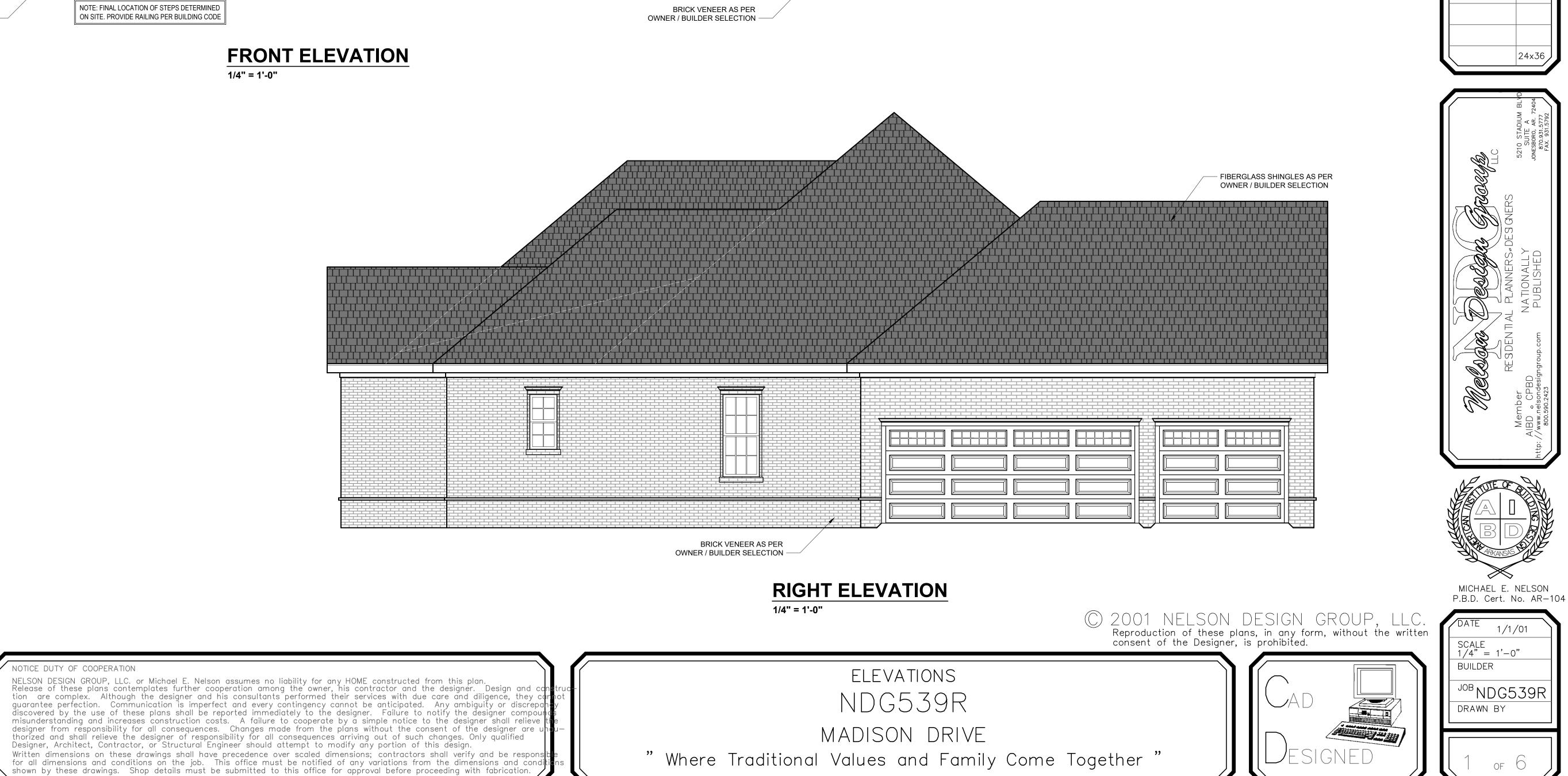


1/4" = 1'-0"

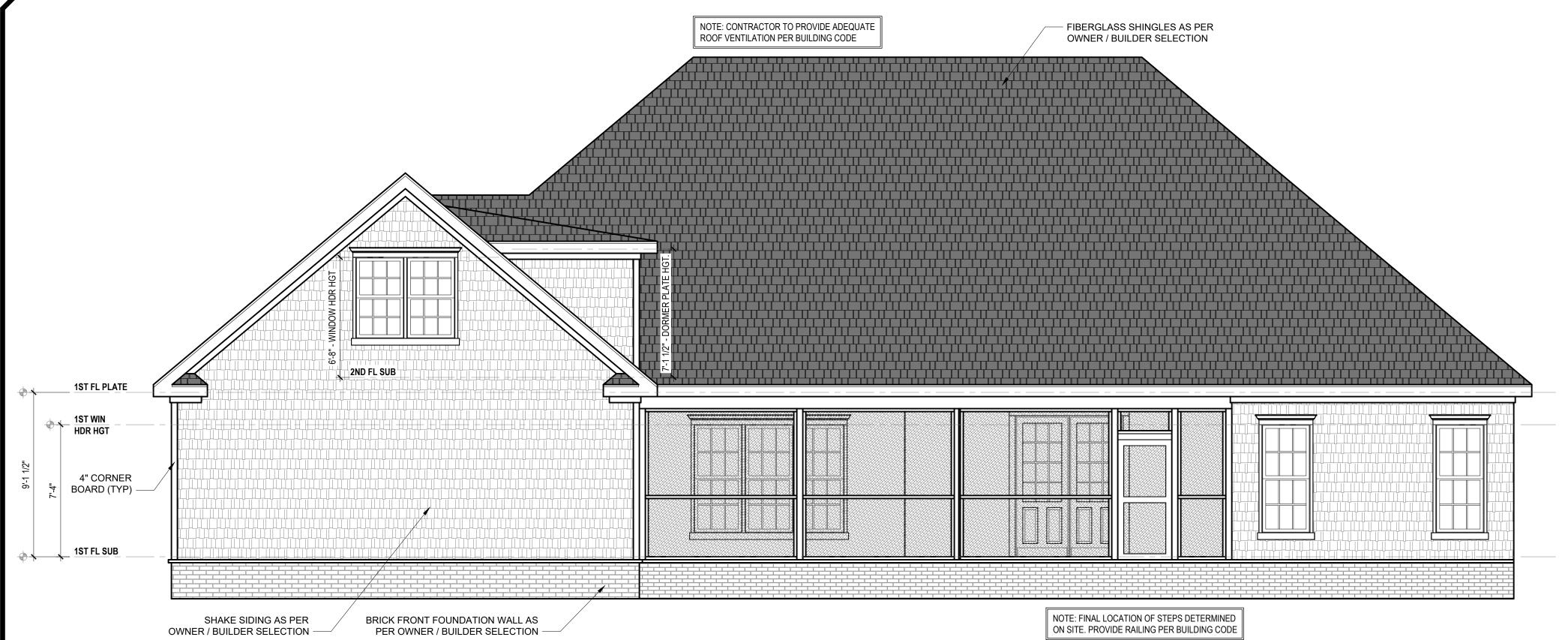
NOTICE DUTY OF COOPERATION

Written dimensions on these drawings shall have precedence over scaled dimensions; contractors shall verify and be respon

# **FRONT ELEVATION**



REVISIONS	BY
5-16-05	DH/RP AS
8-22-07	TCS
10-10-07	TCS
	24x36



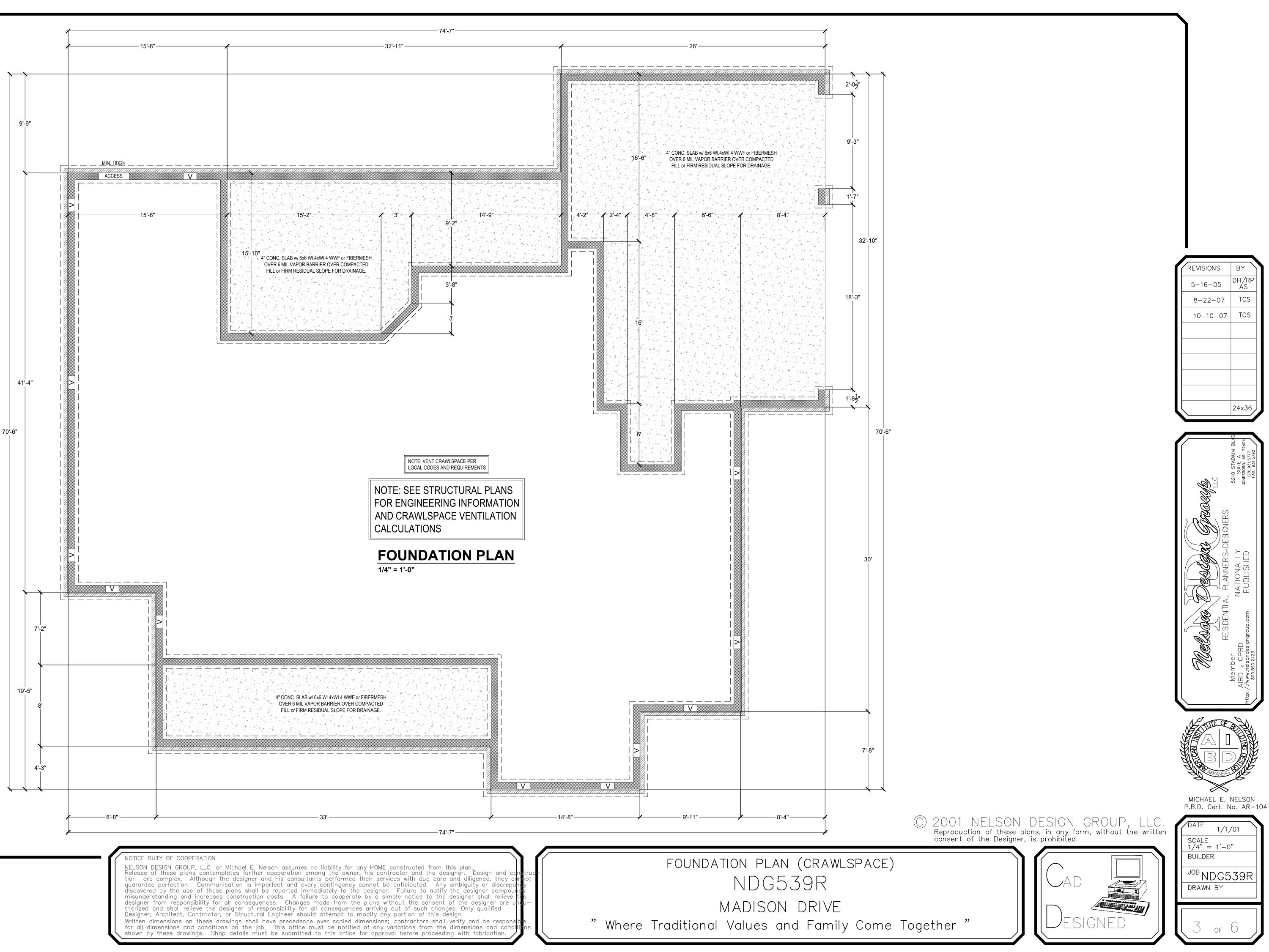
**REAR ELEVATION** 1/4" = 1'-0"

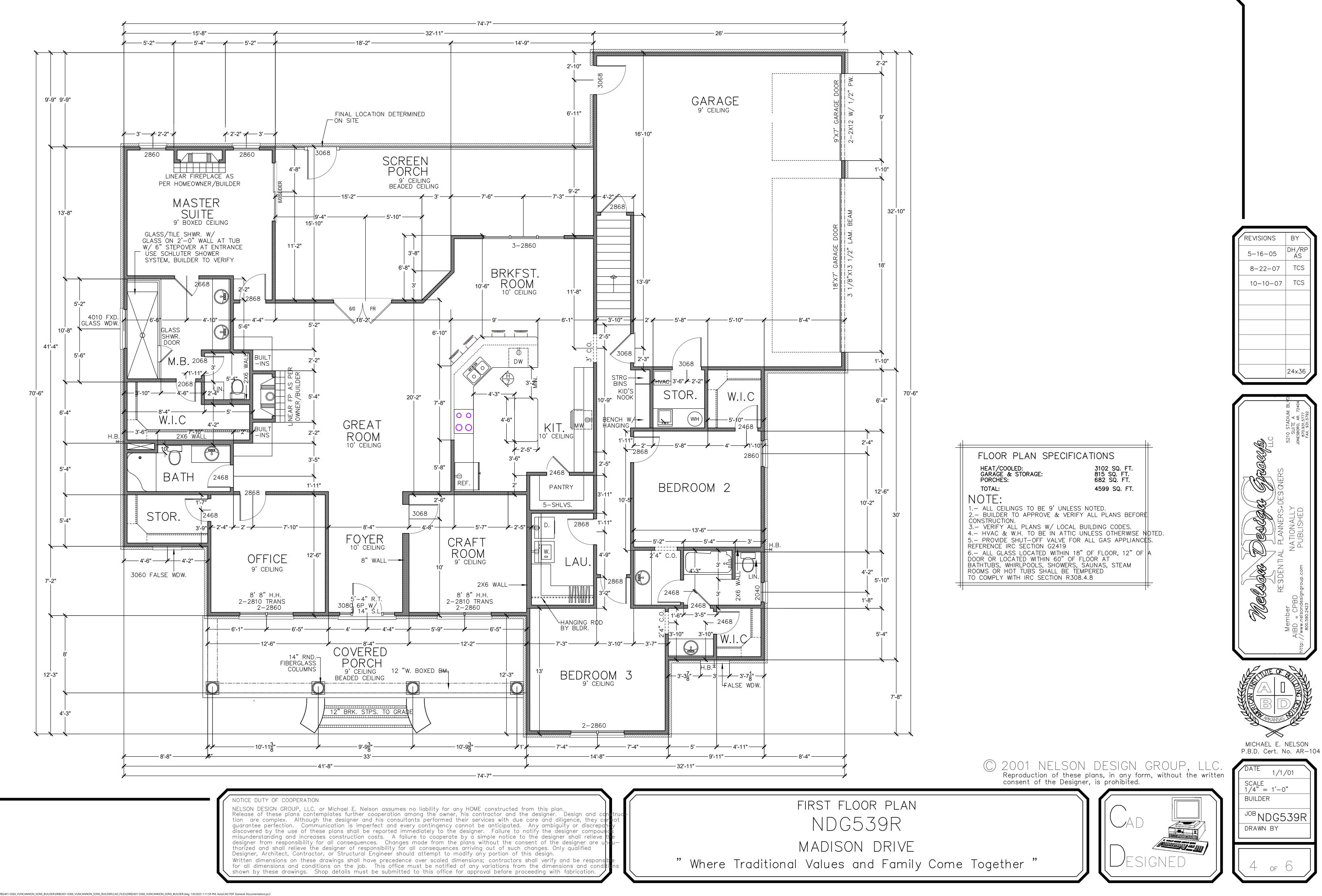
NOTICE DUTY OF COOPERATION

guarantee perfection. Communication is imperfect and every contingency cannot be anticipated. Any ambiguity or discrepar discovered by the use of these plans shall be reported immediately to the designer. Failure to notify the designer compour misunderstanding and increases construction costs. A failure to cooperate by a simple notice to the designer shall relieve designer from responsibility for all consequences. Changes made from the plans without the consent of the designer are ur thorized and shall relieve the designer of responsibility for all consequences arriving out of such changes. Only qualified Designer, Architect, Contractor, or Structural Engineer should attempt to modify any portion of this design. Written dimensions on these drawings shall have precedence over scaled dimensions; contractors shall verify and be respor





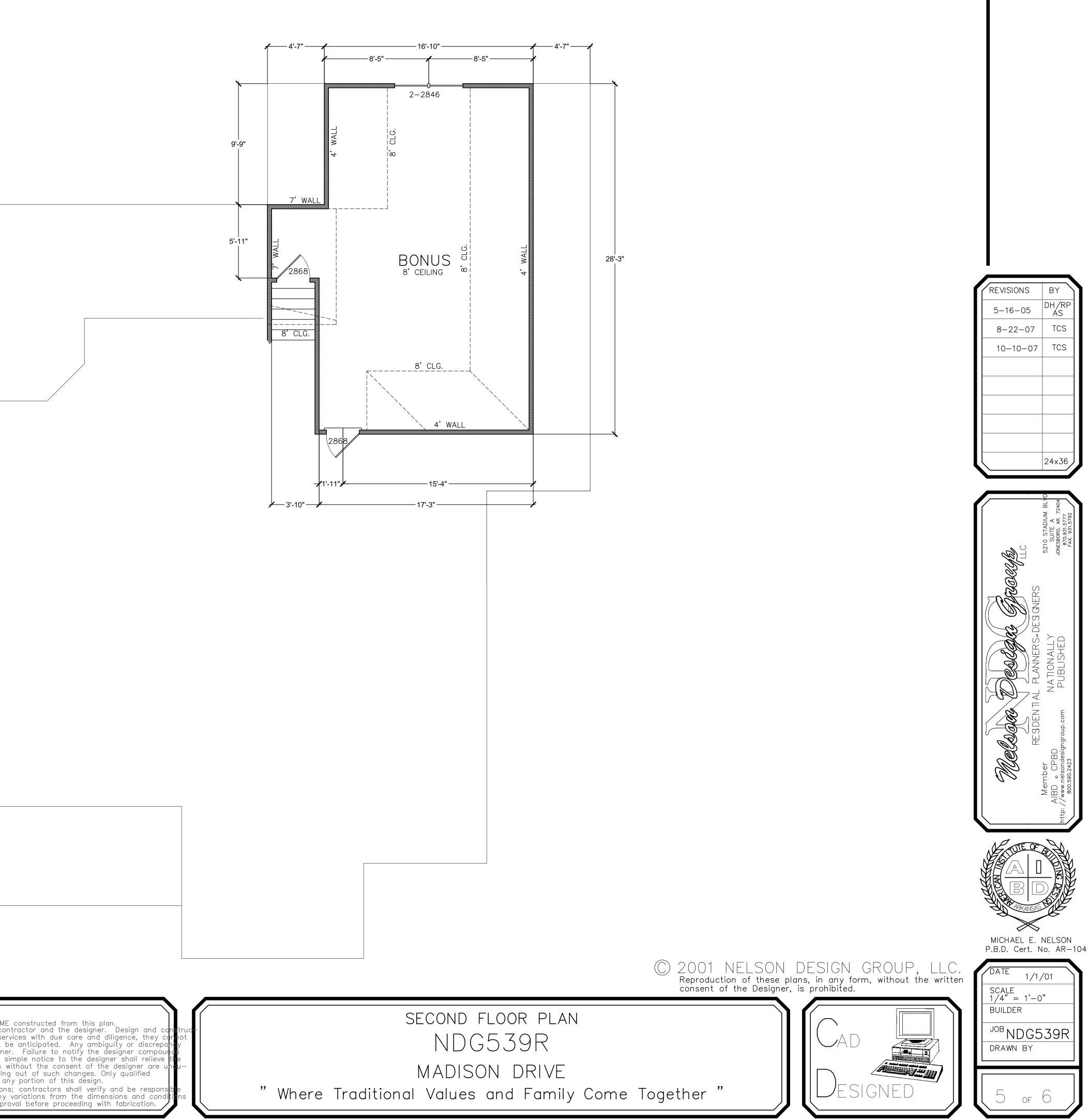


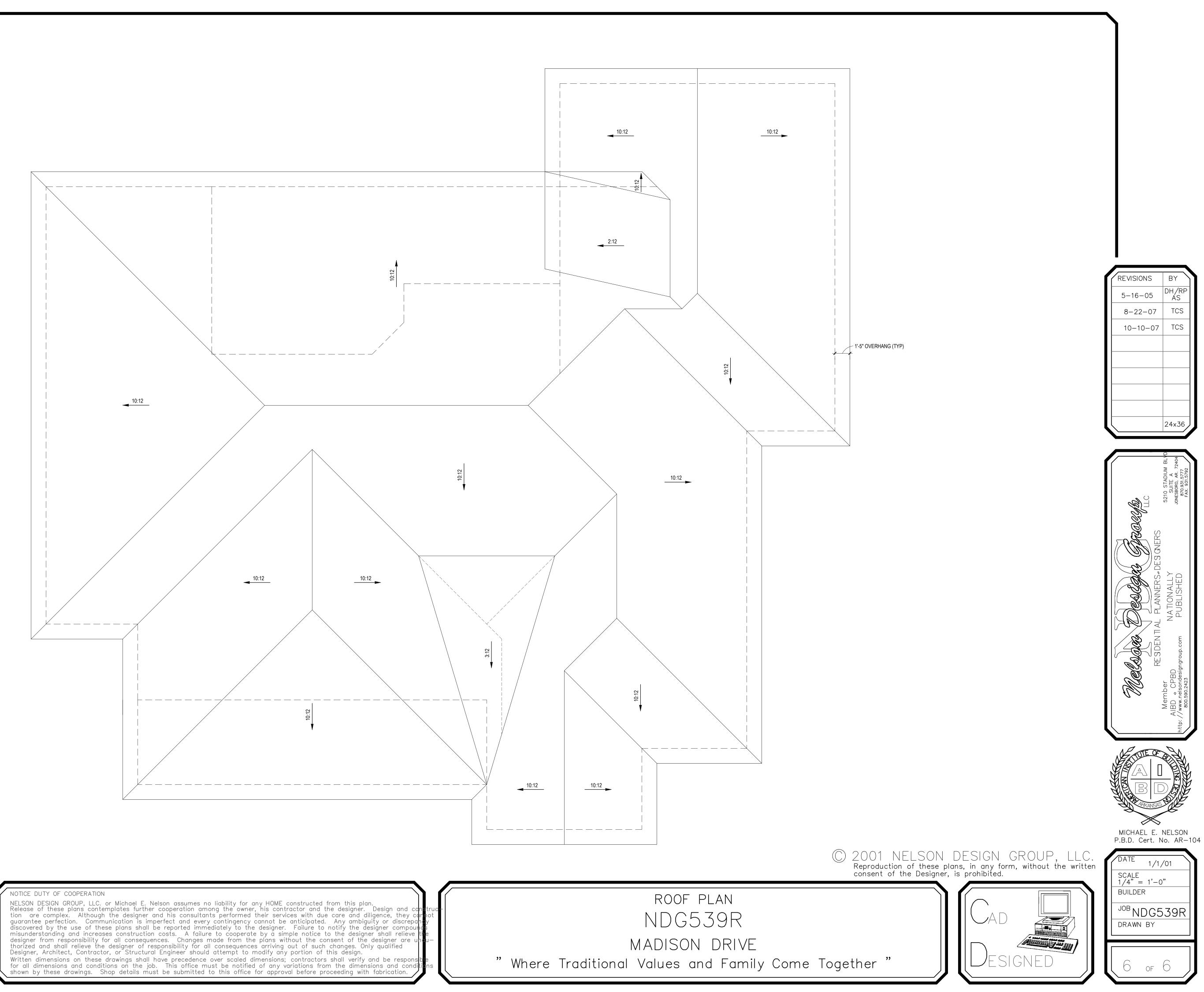


NOTICE DUTY OF COOPERATION

M:Raleigh Office\DRB\DRB 2024\DRB2401-0360 VUNCANNON SONS BUILDER\DRB2401-0360 VUNCANNON SONS BUILDER\CAD FILES\DRB2401-0360 VUNCANNON SONS BUILDER.dvg, 1/9/2025 1:11:59 PM, AutoCAD PDF (General

NELSON DESIGN GROUP, LLC. or Michael E. Nelson assumes no liability for any HOME constructed from this plan. Release of these plans contemplates further cooperation among the owner, his contractor and the designer. Design and con tion are complex. Although the designer and his consultants performed their services with due care and diligence, they car guarantee perfection. Communication is imperfect and every contingency cannot be anticipated. Any ambiguity or discrepan discovered by the use of these plans shall be reported immediately to the designer. Failure to notify the designer compound misunderstanding and increases construction costs. A failure to cooperate by a simple notice to the designer shall relieve to designer from responsibility for all consequences. Changes made from the plans without the consent of the designer are un thorized and shall relieve the designer of responsibility for all consequences arriving out of such changes. Only qualified Designer, Architect, Contractor, or Structural Engineer should attempt to modify any portion of this design. Written dimensions on these drawings shall have precedence over scaled dimensions; contractors shall verify and be responsibility Written dimensions on these drawings shall have precedence over scaled dimensions; contractors shall verify and be response for all dimensions and conditions on the job. This office must be notified of any variations from the dimensions and cond shown by these drawings. Shop details must be submitted to this office for approval before proceeding with fabrication.





NOTICE DUTY OF COOPERATION

M:Raleigh Office\DRB\DRB\_2024\DRB2401-0360\_VUNCANNON\_SONS\_BUILDER\DRB2401-0360\_VUNCANNON\_SONS\_BUILDER\DRB2401-0360\_VUNCANNON\_SONS\_BUILDER.dwg, 1/9/2025 1:12:00 PM, AutoCAD PDF (General D

# DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	( )	( )	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

# STRUCTURAL NOTES:

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- ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER) ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
- (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2).
- ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT. ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN.
- PRESUMPTIVE BEARING CAPACITY = 2000 PSF 10) 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION. 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY

15.966 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 19 VENTS REQ'D (BASED ON 8" X 16" VENTS)

-OR

2395 SQ. FT. OF CRAWL SPACE / 1500 = 1.59 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION

CRAWL SPACE VENTILATION CALCULATION

VENT LOCATIONS MAY VARY FROM THOSE SHOWN ON PLAN, HOWEVER VENTS SHALL BE PLACED TO

GROUND AREA WHERE THE REQUIRED OPENINGS ARE PLACED SO AS TO PROVIDE CROSS VENTILATION OF THE CRAWL SPACE. THE INSTALLATION OF OPERABLE LOUVERS SHALL NOT BE PROHIBITED.

ONE FOUNDATION VENT SHALL BE WITHIN 3 FEET OF EACH CORNER OF THE BUILDING. TO PREVENT

RAINWATER ENTRY WHEN THE CRAWL SPACE IS BUILT ON A SLOPED SITE, THE UPHILL FOUNDATION

WHEN THE BOTTOM OF THE FOUNDATION VENT OPENING IS LESS THAN 4 INCHES ABOVE THE FINISHED

WALLS MAY BE CONSTRUCTED WITHOUT WALL VENT OPENINGS. VENT DAMS SHALL BE PROVIDED

WALL VENTED CRAWL SPACES REQUIRE FULL COVERAGE GROUND VAPOR RETARDERS.

THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 1/1500 OF THE CRAWL SPACE

PROVIDE ADEQUATE VENTILATION AT ALL POINTS AND TO PREVENT DEAD AIR POCKETS.

.59 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 2 VENTS REQ'D (BASED ON 8" X 16" VENTS)2

- ANCHORED TO THE FOUNDATION.
- 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

- 15'-8"  $\mathbf{X}$ 9'-9" (2) 2X10 BAND, FIELD LOCATE ACCESS, NO POINT LOADS ABOVE BAND -3'-0" MAX. 22'-2" <u>\_\_\_\_\_MIN. 18X24</u> \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ \_ ACCESS -15'-8"┡<mark>- Ҿ</mark>┢ 2 X 10 @ 16" OC 10'-6" 4'-6" 7/// <u>2) 2 X 10,</u> FUUSH 5'-10" 9'-10" 8" X 16" MASONRY 41'-4" PILASTER ON A 22" X 30" X 10" CONC. FTG., TYP. UNO -2 X 10 @ 16" OC 2 X 10 @ 16" OC 70'-6" 58" X 30" X 10" CONC. FTG. -2 X 10 @ 16" OC 3'-8" 8'-10" ī 16" X 16" FLUSH MASONRY PIER ON A 30" X 30" X 10" CONC. FTG., TYP. UNO -2395 SQ. FT. OF CRAWL SPACE / 150 = 15.966 SQ. FT. OF REQ'D VENTILATION WITHOUT CROSS VENTILATION 2 X 10

7'-2"

4'-3"

- 8'-8" -

-

19'-5"

@ 16" OC

11'-8"

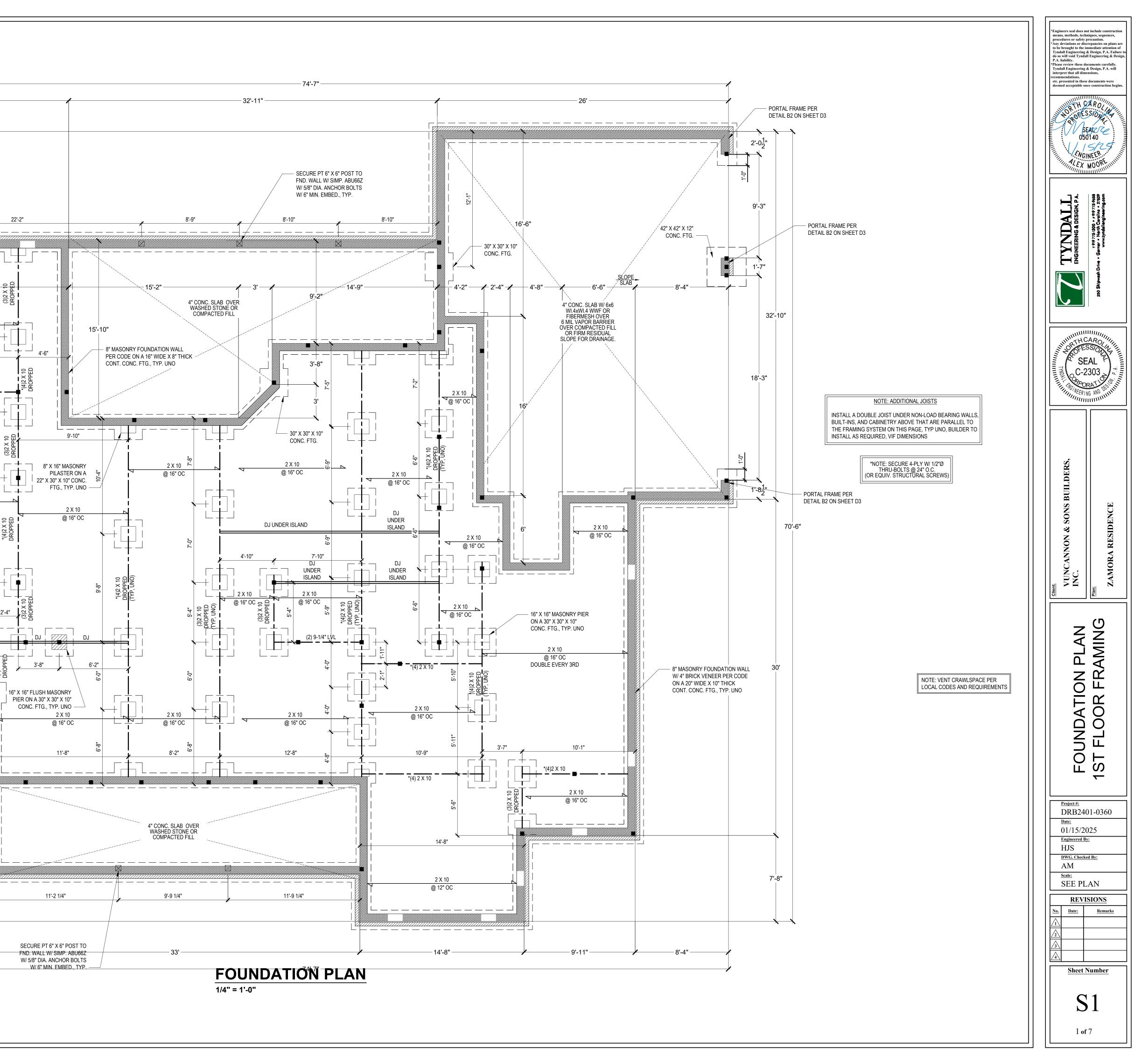
11'-2 1/4"

W/ 6" MIN. EMBED., TYP.



EXTERIOR GRADE.

NO SCALE



# DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	
	40	10	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B) BASED ON SEISMIC ZONES A, B & C			
SEISMIC				

BWL 1

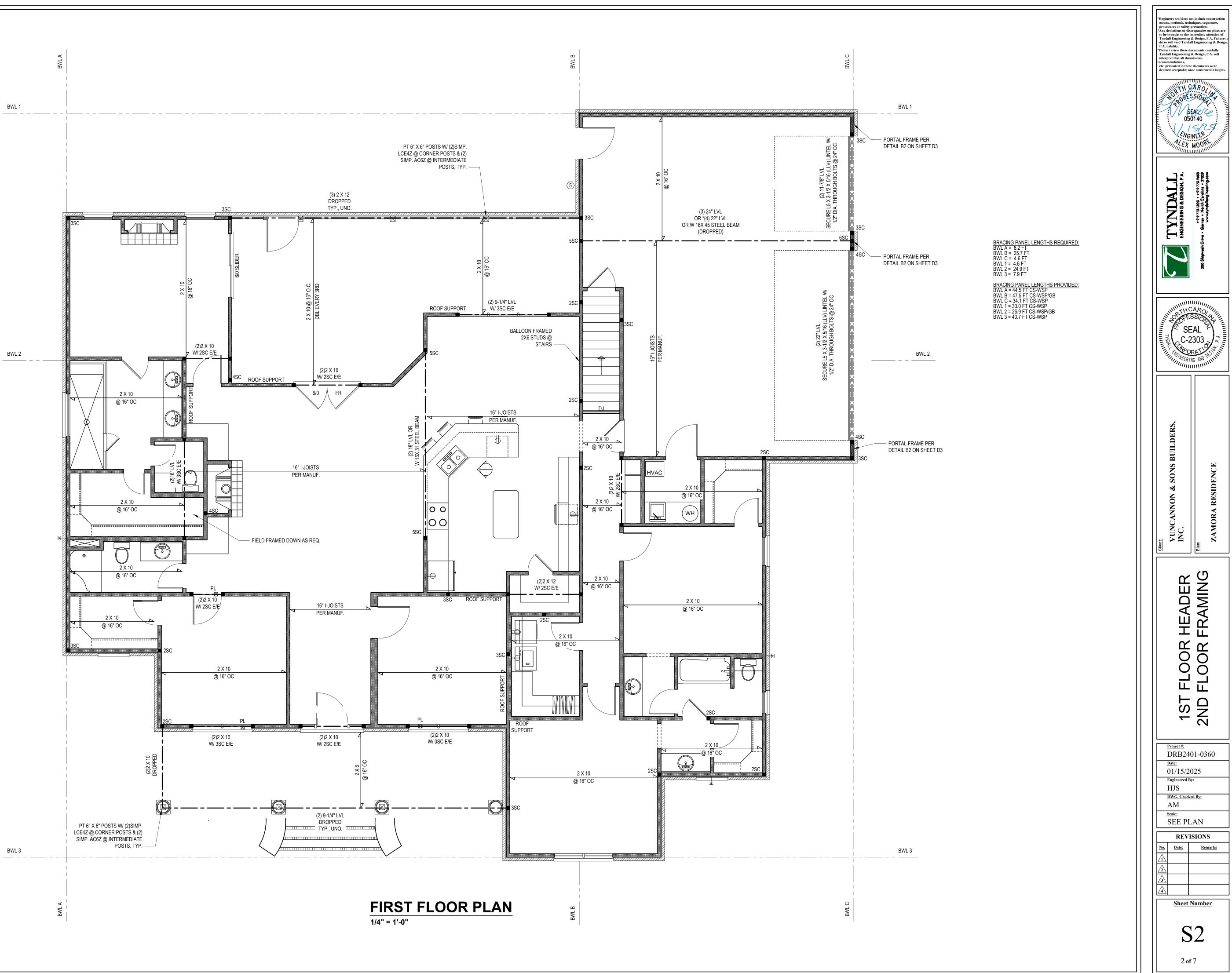
BWL 2

# STRUCTURAL NOTES:

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- ALL LUMBER SHALL BE SYP #2 (UNO) ALL LVL LUMBER TO BE 1.75" WIDE (ACTUAL) EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (OR GREATER) (I.E. iLEVEL MICROLAM)
- ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI) (OR GREATER) ALL PSL LUMBER IS TO BE 1.8E (Fb = 2,400 PSI) (OR GREATER) ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
- (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND TOGETHER w/ (2) 10d NAILS @ 8" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6'-8", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1'-6". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFER
- TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO) REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION
- OF ALL WALLS OVER 10'-0" IN HEIGHT. ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
- Fy = 50 KSI MIN. (UNO)
- ALL EXTERIOR LUMBER TO BE #2 SYP PT
- ALL CONCRETE, fc = 3000 PSI MIN. PRESUMPTIVE BEARING CAPACITY = 2000 PSF 10)
- 11) 1/2"Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY.
- 12) PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO) 13) PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- 14) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC. 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
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- ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.

### STRUCTURAL SHEATHING NOTES

- 1) DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR
- 2) WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF
- THE 2018 NCRC.
- 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- (1) REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- $\langle 2 \rangle$  1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8'-0" / (ISOLATED PANELS) OR 4'-0" (CONTINUOUS SHEATHING). SECURE w/ 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON AILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- 6) ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP) SHEATHING WITH A MINIMUM THICKNESS OF 3/8". SHEATHING SHALL BE SECURED WITH MINIMUM 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS. 7) MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
- BE AS FOLLOWS: - 24" ADJACENT TO OPENINGS NOT MORE THAN
  - 67% OF WALL HEIGHT
  - 30" ADJACENT TO OPENINGS GREATER THAN 67% AND LESS THAN 85% OF WALL HEIGHT.
  - 48" FOR OPENINGS GREATER THAN 85% OF
- WALL HEIGHT
- $\langle 4 \rangle$  SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- (5) MINIMUM 800# HOLD-DOWN DEVICE



BWL 3

# DESIGN LOADS

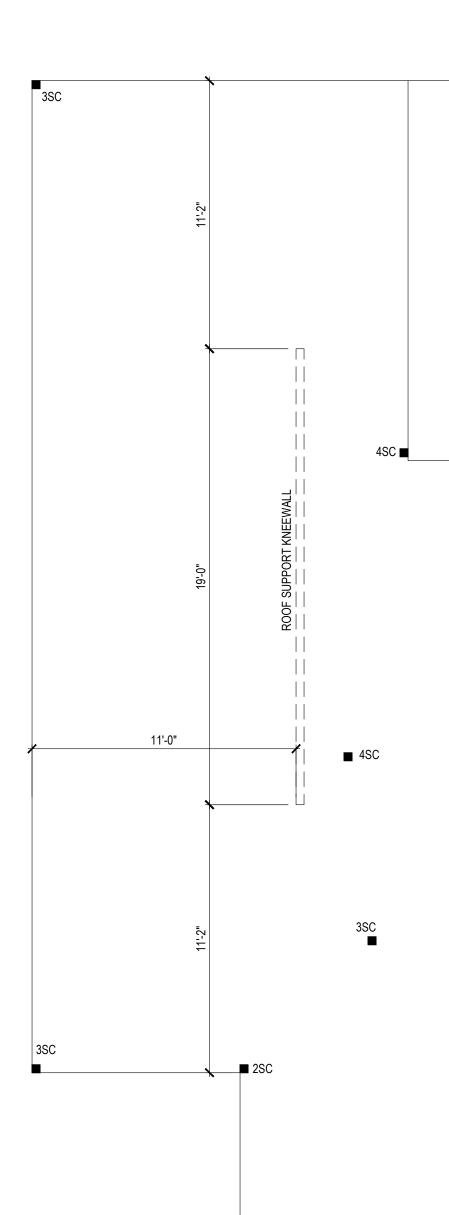
	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	( )	( )	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

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- PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
   PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
   MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS
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   UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION.
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- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
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- 30" ADJACENT TO OPENINGS GREATER THAN
- 67% AND LESS THAN 85% OF WALL HEIGHT. - 48" FOR OPENINGS GREATER THAN 85% OF
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- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, EITHER A MIN. 48" BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800# SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- $\langle 5 \rangle$  MINIMUM 800# HOLD-DOWN DEVICE



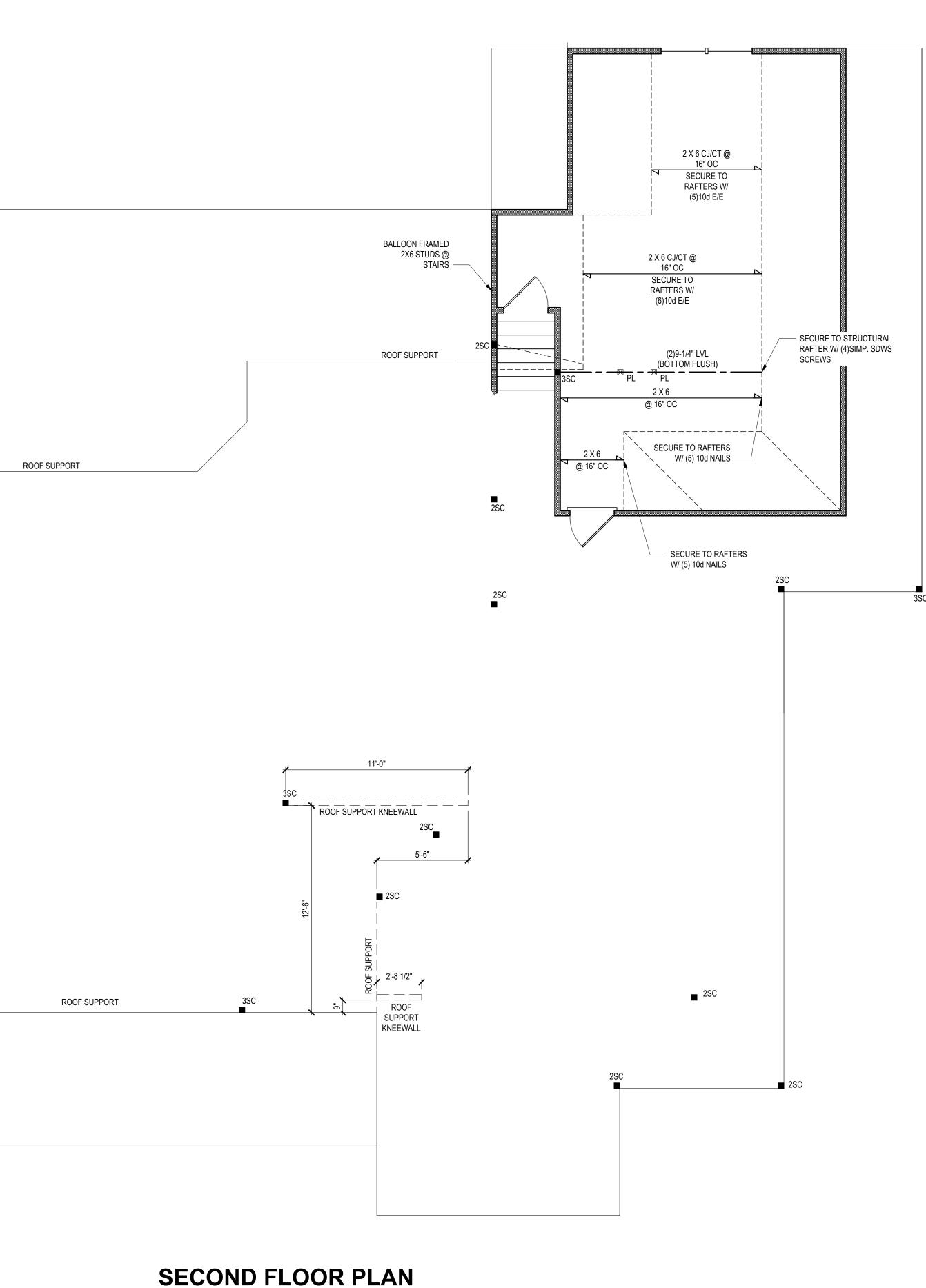
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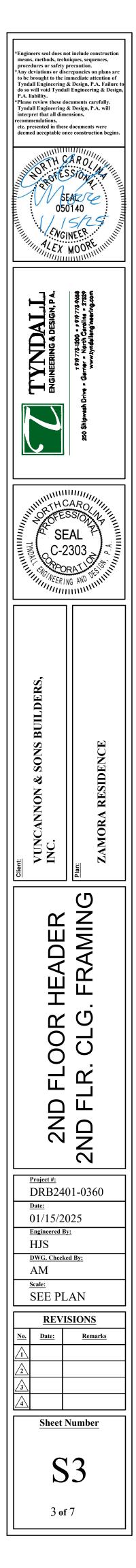
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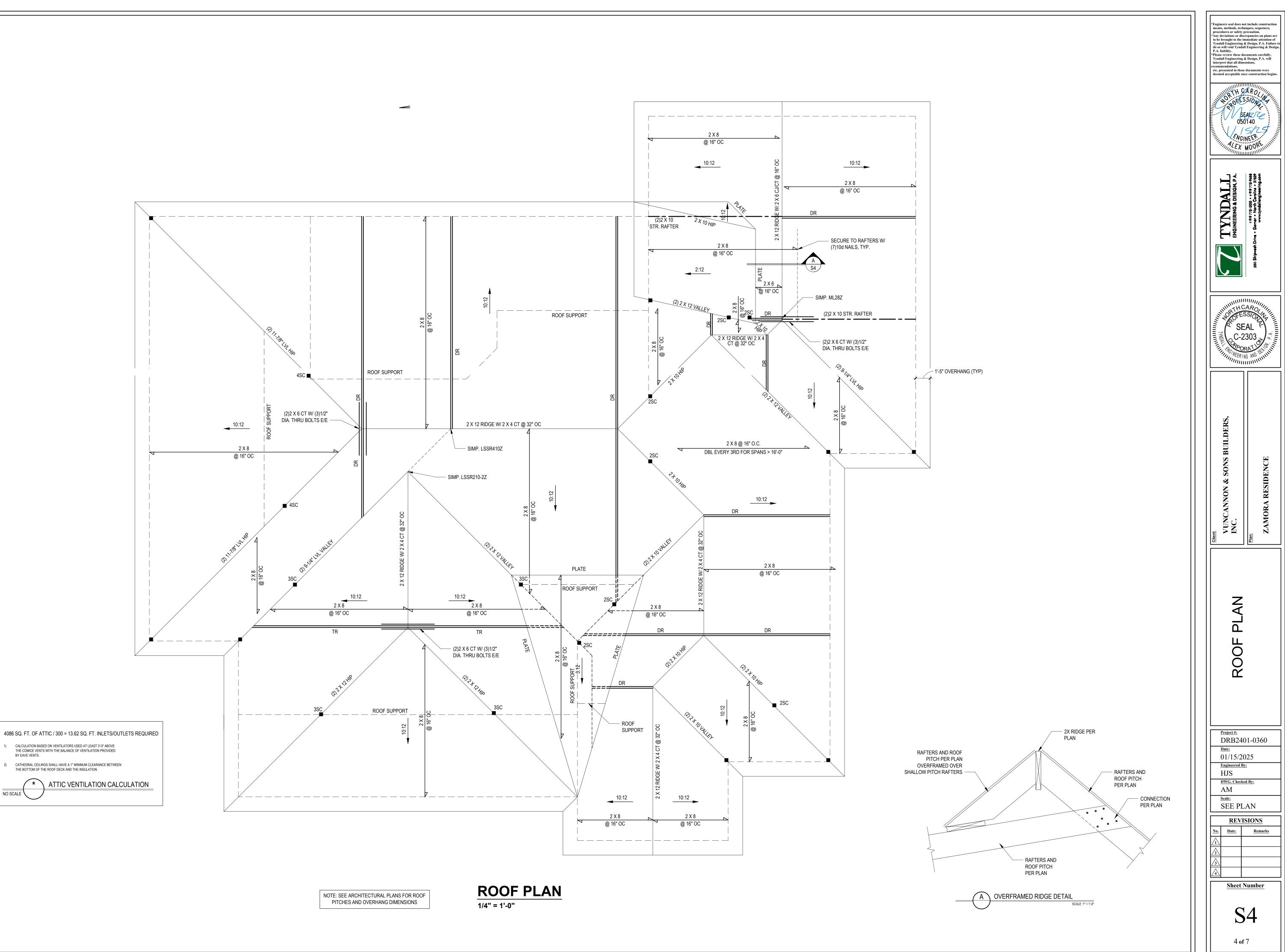
5/2025

3SC



1/4" = 1'-0"





BY EAVE VENTS.

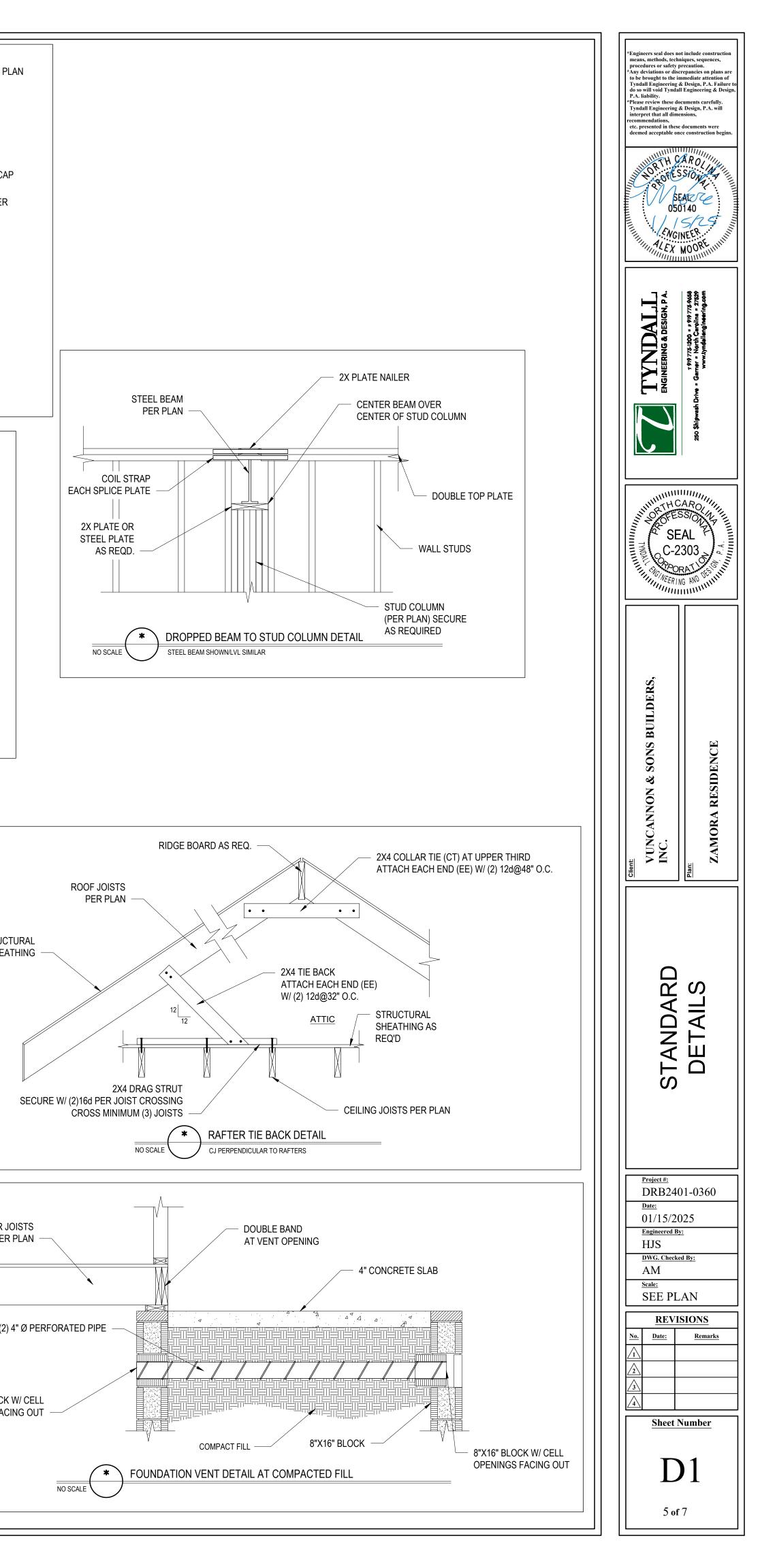
NO SCALE

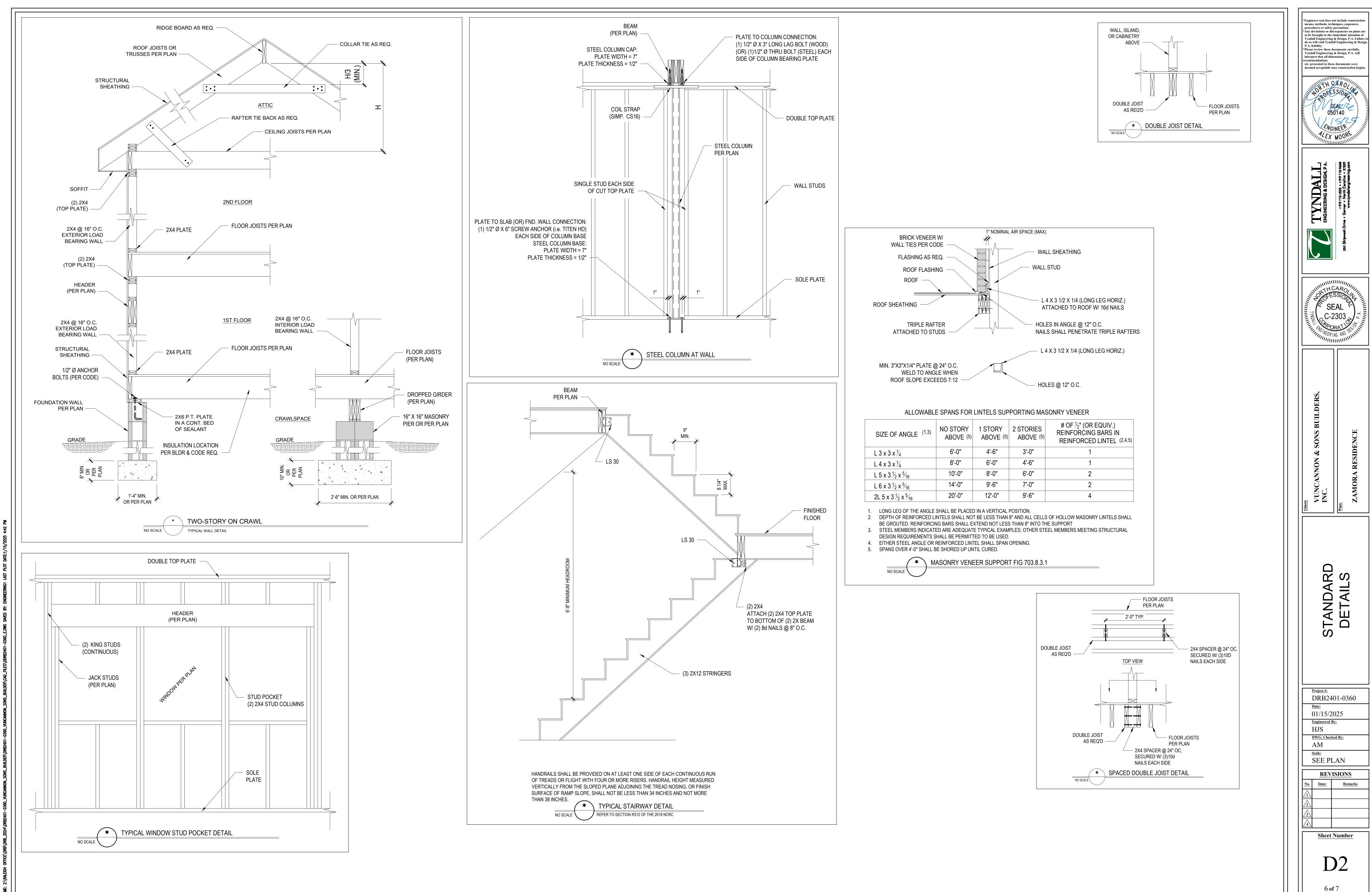
	CONSTRUCTION SHA				RTH CAROLINA ST	ATE 2018 RESIDEN	ITIAL BUILDING			ALT CANT	= =	ALTERN CANTILE		
	SIGN LOADS:									CJ CMU COL	= = =	CEILING CONCRE COLUM	ETE MASONRY UNIT	
				LIVE LC (PSF		SF)	DEFLEC	TION		CONC CONT	= =	CONCRE CONTIN		
		ALL	FLOORS	40	1	10	LL L/360	TL L/240		CT DBL	= =	COLLAR DOUBLE		
		ATTIC (w	/ walk up stairs)	30	1	10	L/360	L/240		DIA DJ	=	DIAMETI	ER	
			Ill down access)	20		10 5	L/240	L/180 L/180		DR DSP	=	DOUBLE	RAFTER STUD POCKET	
		EXTERN	IAL BALCONY	40		10	L/360	L/240		EA	=	EACH		
			ROOF DF TRUSS	20		10 20	L/240 L/240	L/180 L/180		EE FJ	=	EACH EN	JOIST	
		11W	ND LOAD		BASE	O ON 120 MPH (EX	POSURE B)			FND FTG	=	FOUNDA	G	
		S	EISMIC		5	SEISMIC ZONES A,	B & C			GALV HORIZ HT	= = =	galvan Horizo Height	NTAL	
MIN	NIMUM ALLOWABLE SC	DIL BEARING PRE	ESSURE = 2000 PSF							JSC KS	= =	JACK ST KING ST		
	NCRETE SHALL HAVE LESS NOTED OTHERW		AY COMPRESSIVE S	TRENGTH OF 3000	PSI AND A MAXIMU	JM SLUMP OF FIVE	INCHES							
BR/	XIMUM DEPTH OF UNE ACING. REFER TO SEC	TION R404 OF 20	018 NC BUILDING CO	DE FOR BACKFILL										
	ICKNESS, SOIL TYPE,													
ALL	_ Framing Lumber S _ Framing Lumber E _ LVL Lumber to be 1	<b>KPOSED TO THE</b>	ELEMENTS SHALL B	E TREATED MATE						1) MA	XIMUM HEIGH	IT OF DEC	K SUPPORT POSTS	AS FOLL
ALL	LISL LUMBER TO BE 3	.5" WIDE NOMIN	AL EACH SINGLE ME	MBER AND Fb = 23	25 PSI, E = 1.6M PS	SI (Ù.N.O.)				, Г				
	PSL LUMBER TO BE				·	( )	חוד				POST SIZI	Ξ	MAX. POST HEIC	3HT**
	LOAD BEARING EXTE										4 x 4		8'-0"	
	STRUCTURAL STEEL										6 x 6 ***		20'-0" OVER 20'-	
	_ STEEL ANGLES, PLA _ STEEL PIPE SHALL B			IM A36.										
-	EEL BEAMS SHALL BE OVIDE SOLID BEARING									* TH	MAXIMUN	/I TRIBUTA	NO. 2 TREATED SOU RY AREA IS BASED (	ON 128 T
LAC	G SCREWS (1/2"Ø x 4" I LE PLATES, AND THE S	ONG). LATERAL	SUPPORT IS CONSIL	DERED ADEQUATE	PROVIDED THE JO	DISTS ARE TOE NA				** FR(			CATED AT DIFFEREN O BOTTOM OF GIRDI	
					C C					*** DE			TS OVER 20'-0" SHAL ESSIONAL ENGINEE	
THI EXT	OVIDE ANCHOR BOLT E END OF EACH PLATE TEND 7" INTO CONCRE	SECTION. ANCI	HOR BOLTS SHALL B Y. THE BOLTS SHALL	E SPACED AT 3'-0" . BE LOCATED IN T	O.C. FOR BASEME	NTS. ANCHOR BOI	_T SHALL			,		E BRACED	TO PROVIDE LATER	
	ERE SHALL BE A MININ												IS LESS THAN 4'-0" /	AND THE
	LL AND ROOF CLADDI		G OK WATERFROOT	ING FER SECTION	403 AND 400 OF IN	S BOILDING CODE.					ABOVE. L	ATERAL B	STRUCTURE IN ACC RACING IS NOT REC	QUIRED.
WA	LL CLADDING SHALL E	BE DESIGNED FC		· ·	BS/SQFT) OR GREA	ATER POSITIVE AN	D NEGATIVE PR	ESSURE.		B. 4 x			S MAY BE PROVIDED THE KNEE BRACES	
39.0	0 LBS/SQFT FOR ROOI 0 LBS/SQFT FOR ROOI	PITCHES 0/12 T	O 1.5/12	ollowo.									SS THAN 1/3 OF THE AND THE BRACES S	
18.0	) LBS/SQFT FOR ROOF IEAN ROOF HEIGHT 30	PITCHES 6/12 T											THE HORIZONTAL. M GIRDER WITH ONE S	
										C. FO			) OF THE BRACE. (S WITHOUT KNEE E	RACES C
			,			<i>κ</i> .					BRACING	, LATERAL	STABILITY MAY BE	PROVIDE
	FER TO SECTION R602									Γ				
	OVIDE CONTINUOUS S	-									POST SI	ZE	MAX. TRIBUTARY AREA	
-	LIFT LOADS GREATER										4 x 4		48 SQ. FT.	
	FER TO TABLE N1102.				COMPONENT CRITE	RIA.					4 x 4 6 x 6		46 SQ. FT. 120 SQ. FT.	
	L COLUMNS DESIGNE		Υ.	,										
	OVIDE A MINIMUM OF					, , , , , , , , , , , , , , , , , , ,				D. 2 x		-	CROSS BRACING M R DIRECTIONS FOR	
	XIMUM MASONRY PEI										TÓ THE S	STRUCTUR	E AT THE EXTERIOF	R COLUMN
	S THE CONTRACTORS							N BEGINS.		E. FO	DIPPED C	GALVANIZE	E ATTACHED TO TH ED BOLT AT EACH EN S IN COASTAL REGI	ND OF EAG
										L. FU				JINO, OEE
			GLAZED		WOOD	MASS		BASEMENT <sup>c,</sup>	© SLAB <sup>d</sup>	CRAWL SPA	CE <sup>c</sup>			
IATE NES	FENESTRATION U-FACTOR <sup>b, j</sup>	SKYLIGHT <sup>b</sup> U-FACTOR	FENESTRATION SHGC <sup>b,<u>k</u></sup>	CEILING <sup>m</sup> R-VALUE	FRAMED WALL R-VALUE	WALL R-VALUE	FLOOR R-VALUE	WALL R-VALUE	R-VALUE AND DEPTH	WALL R-VALUE				
3	0.35	0.55	0.30	<u>38 or 30</u>	<u>15</u> or	<u>5/13 or</u>	19	<u>5/13</u> f	0	5/13				
4	0.35	0.55	0.30	<u>cont</u> 38 or 30	13 + <u>2.5</u>	<u>5/10 cont</u>	19	10/15	10	10/15				
5		0.55	<u>0.30</u> NR	cont j <u>38 or 30</u>	13 + <u>2.5</u> " " <u>19, or 13 + 5</u> h	<u>5/10 cont</u> 13/17 <u>or</u>	30 <sup>g</sup>	10/15						
	0.35	0.55	NR	cont	<u>or 15 + 3</u>	<u>13/12.5 cont</u>	30	10/15	10	<u>10/19</u>				
	{ }	N1102.1 CLIN	MATE ZONES 3-	5										
) SCALE	OF THE	INSULATION, THE INSTA	RS AND SHGC ARE MAXIMUMS LLED R-VALUE OF THE INSULA	TION SHALL NOT BE LESS 1	THAN THE R-VALUE SPECIFI		OR DESIGN THICKNESS							
	b. THE FENES	RATION U-FACTOR COL	UMN EXCLUDED SKYLIGHTS. T	HE SOLAR HEAT GAIN COE	FFICIENT									

- OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE WHICHEVER IS LESS. FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. R-5 SHALL BE ADDED TO THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS.
- e. <u>DELETED</u>
- f. BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID LOCATIONS AS DEFINED BY FIGURE N1101.7 AND TABLE N1101.7.
- g. OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY. R-19 MINIMUM.
- h. THE FIRST VALUE IS CAVITY INSULATION, THE SECOND VALUE IS CONTINUOUS INSULATION, SO "13+5" MEANS R-13 CAVITY INSULATION PLUS R-5 INSULATED SHEATHING. "15+3" MEANS R-15 CAVITY INSULATION. PLUS R-3 INSULATED SHEATHING. IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR,
- INSULATING SHEATHING IS NOT REQUIRED WHERE THE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25 PERCENT OF THE EXTERIOR, SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2. "13 + 2.5" MEANS R-13 CAVITY
- INSULATION PLUS R-2.5 SHEATHING. i. FOR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WALL.
- j. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A U-FACTOR NO GREATER THAN 0.55 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- k. IN ADDITION TO THE EXEMPTION IN SECTION N1102.3.3, A MAXIMUM OF TWO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHGC NO GREATER THAN 0.70 SHALL BE PERMITTED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- L R-30 SHALL BE DEEMED TO SATISFY THE CEILING INSULATION REQUIREMENT WHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-30 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. OTHERWISE R-38 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN 1 INCH OF THE ATTIC ROOF DECK.
- m, TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS LIMITED BY THE PITCH OF THE ROOF; THERE THE INSULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE. n R-19 FIBERGLASS BATTS COMPRESSED AND INSTALLED IN A NOMINAL 2 × 6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2X4 WALL IS NOT DEEMED TO COMPLY.
- 9. BASEMENT WALL MEETING THE MINIMUM MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT.

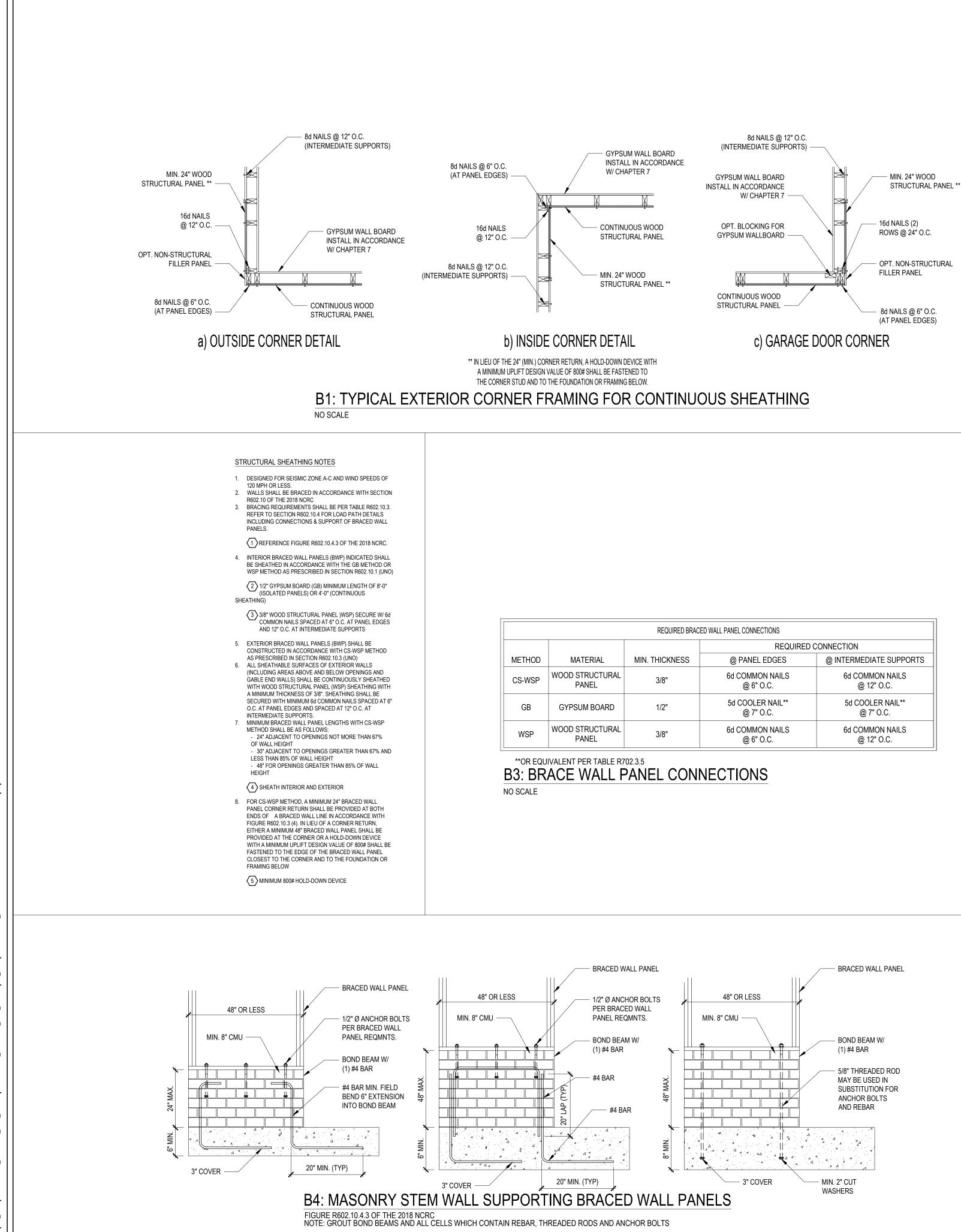
M

EFINITIONS FOR CC	OMMON ABBREVIATIONS	STRUCTURAL SHEATHING FLOOR JOISTS PER PLA	٩N
IATE EVER	MANUF = MANUFACTURER MAX = MAXIMUM		
G JOIST ETE MASONRY UNIT N	MIN = MINIMUM NOM = NOMINAL O.C. = ON CENTER		
ETE IUOUS R TIE	PL = POINT LOAD PT = PRESSURE TREATED REINF = REINFORCED	(3) 2X10 GIRDER OR PER PLAN OVERLAP JOISTS	
E ER E JOIST	REQ'D = REQUIRED RJ = ROOF JOIST RS = ROOF SUPPORT	8" SOLID MASONRY CAP	
E RAFTER E STUD POCKET	SC = STUD COLUMN SCH = SCHEDULE SPEC = SPECIFIED	CRAWL SPACE     16"X16" MASONRY PIER       OR PER PLAN     0R PER PLAN	
ND JOIST	TH = THICK TJ = TRIPLE JOIST		
ATION IG NIZED	TRTD = TREATED TSP = TRIPLE STUD POCKET TYP = TYPICAL		
NTAL FUD	UNO = UNLESS NOTED OTHERV W = WIDE FLANGE BEAM WWF = WELDED WIRE FABRIC	SE	
UD	XJ = EXTRA JOIST	2'-6" MIN OR PER PLAN	
		NO SCALE	
CK SUPPORT POSTS AS FO	DLLOWS:		
MAX. POST HEIGHT**		STRUCTURAL SHEATHING FLOOR JOISTS PER PLAN	
8'-0"			
20'-0" OVER 20'-0"		OR PER PLAN	
NO. 2 TREATED SOUTHER		2X6 (MIN) TREATED SILL	
RY AREA IS BASED ON 128 CATED AT DIFFERENT LEV O BOTTOM OF GIRDER	ÆLS.	220 (MIN) TREATED SILL     16"X16" MASONRY PIER       CRAWL SPACE     OR PER PLAN	
TS OVER 20'-0" SHALL BE I FESSIONAL ENGINEER OR	DESIGNED AND REGISTERED ARCHITECT.		
) TO PROVIDE LATERAL ST	ABILITY BY ONE OF		
IS LESS THAN 4'-0" AND T STRUCTURE IN ACCORDA RACING IS NOT REQUIREI	ANCE WITH SECTION (4)		
S MAY BE PROVIDED ON EA . THE KNEE BRACES SHAL	ACH COLUMN IN L ATTACH TO EACH POST	2'-6" MIN OR PER PLAN	
	BE ANGLED BETWEEN BRACES SHALL BE BOLTED		
GIRDER WITH ONE 5/8"Ø H D OF THE BRACE. KS WITHOUT KNEE BRACE		NO SCALE FLUSH GIRDER DETAIL	
L STABILITY MAY BE PROV ANCE WITH THE FOLLOWI			]
MAX. TRIBUTARY AREA	MAX. POST EMBEDMENT CONCRET HEIGHT DEPTH DIAMETER	SIMP. CS16 COIL STRAP EACH SIDE OF BEAM BEAM SPLICE	
48 SQ. FT.	4'-0"         2'-6"         1'-0"		
120 SQ. FT.	6'-0" 3'-6" 1'-8"	BEAM PER PLAN BEAM PER PLAN	
	STANDING DECKS OR PARALLEL		
BE ATTACHED TO THE POS ED BOLT AT EACH END OF	EACH BRACING MEMBER.	POST CAP POST PER PLAN	
ES IN COASTAL REGIONS, S	SEE CHAPTER 46.	PER PLAN STRUCTI	
		SHEAT	пп
		* BEAM SPLICE AT COLUMN	
		BOTTOM FLUSH BEAM PER PLAN INSTALL 2 X 4 KICKERS TO JOISTS W/ (2)10d	
		2 X 4 DRAG STRUT, SECURE W/ (2)16d PER JOIST CROSSING CROSS	
		MINIMUM (2) JOISTS	S
		CEILING / FLOOR JOISTS PER PLAN	
		LATERAL BEAM BRACING     FLOOR JO     PER F	
		NO SCALE WHEN BOTTOM FLUSH BEAMS ARE ≥ TWICE THE DEPTH OF THE ADJACENT JOISTS	
	Γ		-
		FLOOR JOISTS PER PLAN	
		OVERLAPPED (2) 4	Q
		8"X16" BLOCK V	
		STEEL BEAM 2X FLAT PLATE NAILER ATTACH TO STEEL BEAM W/ 1/4" Ø BOLTS @	٩G
		PER PLAN 24" O.C. ALT. SIDES OF WEB, DRILL HOLE 5/16" Ø MAX.	
		NO SCALE STEEL BEAM SHOWN/LVL SIMILAR	





4:42 SAVED E.DWG



ED WALL PANEL CONNECTIONS	
REQUIRED C	CONNECTION
@ PANEL EDGES	@ INTERMEDIATE SUPPORTS
6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.
5d COOLER NAIL** @ 7" O.C.	5d COOLER NAIL** @ 7" O.C.
6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.

