## Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 57653 JOB: 25-2083-R01 JOB NAME: LOT 0.0005 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 26 Truss Design(s)

Trusses:

J01, R01, R02, R02A, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, SP01, SP02, VT01, VT02, VT03, VT04



## Warning !--- Verify design parameters and read notes before use.



- (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2.

LOAD CASE(S) Standard









Job	Truss	Truss Type	Qty	Ply	LOT 0.0005 CAMPBELL RID	GE   160 ALDEN WAY AM	IGIER, NC
25-2083-R01	R03	Roof Special	7	1		#	57653
			Run: 85.430 s Feb 1	  2 2021 Prin	Job Reference (optional) t: 8.630 s Jul 12 2024 MiTek li	ndustries, Inc. Tue Mar 11	12:29:52 2025 Page 1
	-0-10-8 6-8-12	13-1-15	ID:WI8rkg6BK5	SaRYCYG	f9_0xywFJ5-2CMJtu3fWE	EhQ2Ja8QauGzusDZF4	1HR6bwd?4Y?_zc0jz
	0-10-8 6-8-12	6-5-4	6-5-4	5-11-1	0 6-3-2		
			649 /				Scale = 1.72.6
			0x0 2				
Т		6.00 12	7				
				$\sim$			
		4x4 ≈ 20	21	$\mathcal{N}$	9.00 12		
		6 <sup>T2</sup>	// \/	$\langle \rangle \gg$	22 ac 4x6		
		3x8 =			23		
5-10	2x	4 11 5			24		
10-		4	XV/4	vke			
	4x4 / 10				W7 W8	2-4	
	4x4 = 3	VV2 VV3		//		9 3x4    9	
	2 1101		\// B2	\	\ //		
α					Ê₩/		
* C	5v5	18 25 26	17 15 <sup>16</sup> 27 <sup>31</sup>	28 32	2 11 <b>33</b> 29 30	10	
		4x4 =	4x8 = 2x4		4x4 =	4x4 —	
		4	4x4 = 2x4	2	$x_{4} =$	+A+ —	
			2x4 =				
	786	15 1 3	10.7.3	24 1 3	31 10 0		
	7-8-6	7-4-14	4-6-0	4-6-0	7-8-13		
Plate Offsets (X,Y) [2:0	-3-1,0-0-4], [7:0-6-0,0-3-0]						
LOADING (psf) TCLL (roof) 20.0	SPACING-	2-0-0 CSI.	DEFL.	in (lo	oc) l/defl L/d	PLATES	GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.25 TC 0.89	Vert(LL)	-0.43	14 >892 240 14 >589 180	MT20	244/190
TCDL 10.0	Rep Stress Incr	YES WB 0.71	Horz(CT)	0.07	10 n/a n/a		
BCDL 10.0	Code IRC2021/TP	I2014 Matrix-SH				Weight: 204 I	b FT = 20%
LUMBER-			BRACING-				
TOP CHORD 2x4 SP No BOT CHORD 2x4 SP SS	.2 *Excent*		TOP CHORD	Structura Rigid cei	al wood sheathing direct	tly applied, except ei	nd verticals.
B2: 2x4 SF	No.2		Berenera	6-0-0 oc	bracing: 13-16	to o o oo bracing. E	loopt.
WEBS 2x4 SP No SLIDER Left 2x4 SP	.3 2 No 3 3-8-8		WEBS	1 Row a	t midpt 8-10	1	· · · · · · · · · · · ·
				be inst	alled during truss erection	on, in accordance wit	h Stabilizer
	0 4000/0 0 0 / 0 4 40	) 40 4000/0 0 0 (min 0 4 45)		Installa	tion guide.		
Max Horz	2=1383/0-3-8 (min. 0-1-12 2=211(LC 11)	), 10=1368/0-3-8 (min. 0-1-15)					
Max Uplift	2=-98(LC 14), 10=-15(LC 1	5)					
Max Grav.	2=1496(LC 5), 10=1627(LC	, 20)					
FORCES. (Ib) - Max. Con	np./Max. Ten All forces 2	250 (lb) or less except when show	vn.				
6-20=-20	0/149, 3-19=-2516/151, 4-61/199, 20-21=-2020/203,	7-21=-1954/225, 7-22=-1937/246	6=-2394/215, 6, 22-23=-1984/220,				
8-23=-20	37/213, 9-24=-313/107, 9-7	10=-299/117	45 47 04050				
15-27=0/	5/2210, 18-25=-88/1909, 2 1253, 12-27=0/1253, 12-28	5-26=-88/1909, 17-26=-88/1909, 3=0/1253, 28-33=0/1253, 11-33=	)/1253, 11-29=-35/15	52,			
29-30=-3	5/1552, 10-30=-35/1552	7- 000/000 40 47- 404/4440 7	40- 400/4050				
VEBS 4-18=-28 7-13=-10	1/164, 6-18=-118/514, 6-17 5/785. 11-13=-136/678. 8-1	/=-683/230, 16-17=-134/1148, 7-  1=-254/239, 8-10=-1907/13	16=-102/1256,				
	,,-	· · · · <b>,</b> · · · · ·					
1) Unbalanced roof live lo	ads have been considered	for this design.					
2) Wind: ASCE 7-16; Vult	=120mph (3-second gust)	Vasd=95mph; TCDL=5.0psf; BC	DL=5.0psf; h=23ft; Ca	at. II; Exp	B; Enclosed; MWFRS		
(envelope) gable end z	one and C-C Exterior(2E) - 6-10-10. Exterior(2E) 26-10	0-10-8 to 3-11-2, Interior(1) 3-11 -10 to 31-8-4 zone:C-C for memi	-2 to 14-9-10, Exterio pers and forces & MV	r(2R) 14-9 VFRS for	9-10 to 24-4-13, reactions shown:		
Lumber DOL=1.60 plat	e grip DOL=1.60					WHOMAN CAR	10.
3) ICLL: ASCE 7-16; Pr= Cat B: Partially Exp : C	20.0 psf (roof LL: Lum DOI e=1 0 <sup>.</sup> Cs=1 00 <sup>.</sup> Ct=1 10	_=1.25 Plate DOL=1.25); Pt=20.0	pst (Lum DOL=1.15	Plate DO	L=1.15); Is=1.0; Rough	IN GESSIA	NUL
4) Unbalanced snow loads have been considered for this design.							
5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs							
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.							
() * I his truss has been designed for a live load of 30.0pst on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with fit to between the bottom chord and any other members, with BCDL = 10 0pst							
8) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of with	standing 100 lb uplifi	at joint(s	) 2, 10.	A ANOINEER	S MA
I OAD CASE(S) Standard	1					MARK & MORP	annut .
LOND CACLOF Standard						Man a. Weight	2.00
						3/11/202	5









Job	Truss	Truss Type	Qty	Ply	LOT 0.0005 CAMPBELL RIDGE   160 ALDEN WAY	ANGIER, NC
25-2083-R01	R07	Common Girder	1	2	Job Reference (optional)	# 57653
		Run: 85.4	30 s Feb 1 ID:Wl8rkg	2 2021 Prin BK5SaR	t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Ma YCYGf9_0xywFJ5bU3Ia4v2rx8HckXX?xk2J	r 11 12:29:54 2025 Page 2 xZu2p9v?_D5JZf3tzc0jx

## LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 6=-1242(B) 9=-1247(B) 10=-1242(B) 11=-1242(B) 12=-1242(B) 13=-1242(B) 14=-1242(B) 15=-1348(B) 16=-1348(B)





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer of truss designer of truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Max Grav 6=112(LC 21), 2=206(LC 21), 7=276(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (11)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7.

10) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard





Max Uplift4=-32(LC 14), 2=-51(LC 10) Max Grav 4=243(LC 21), 2=351(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES- (10)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.







vertically. Applicability of design parameters and roter that notes of or easing it is used only upon parameters and not true on the state of the intervention of component is responsibility of building designer – not trues designer of trues general support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0005 CAMPBELL	RIDGE   160 ALDEN WAY ANGIER, NC
25-2083-R01	R13	Monopitch	10		1 Job Reference (optio	nal) # <b>57653</b>
			Run: 85.430 s Feb ID:WI8rkg6Bk	12 2021 Pr (5SaRYC)	int: 8.630 s Jul 12 2024 Mi YGf9 0xvwFJ5-wzcpiG	Tek Industries, Inc. Tue Mar 11 12:29:56 2025 Page 1 6AZSCrWwtwfQzC7k0vosWoN2tWYd2m8lzc0iv
		-0-10-8 6-8-5	13-6	6-0		
		0-0-0	0-9-	•11		
				3x	4	Scale: 3/16"=1
		T	9.00 12		4	
			_			
			T	2/		
		5x6	"			
		5	3		14/2	
		0-7-			XV3	
		٦ //				
		Т	W	2 D		
			w			
		HW1	<b>B</b> 1			
			6 8		5	
		0.00 11	2x4		3x4 =	
		6-8-5	13-6	5-0 .11		
Plate Offsets (X,Y) [2:0	)-3-8,Edge], [3:0-3-0,0-3-4]		1			
LOADING (psf)	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in (	(loc) l/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.25 TC 0.61	Vert(LL)	-0.05	5-6 >999 240	MT20 244/190
TCDL 10.0	Rep Stress Incr	YES WB 0.24	Horz(CT)	0.01	5 n/a n/a	
BCDL 0.0	Code IRC2021/TF	Pl2014 Matrix-SH				Weight: 82 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x4 SP No BOT CHORD 2x4 SP No	o.2 o 2		TOP CHORD	Structu end ve	ıral wood sheathing d rticals	irectly applied or 6-0-0 oc purlins, except
WEBS 2x4 SP No	p.3		BOT CHORD	Rigid c	eiling directly applied	or 10-0-0 oc bracing.
WEDGE Left: 2x4 SP No.3			WEBS	1 Row	at midpt 4-	5, 3-5
				be ins	stalled during truss er	ection, in accordance with Stabilizer
REACTIONS (lb/size)	5=526/Mechanical 2=592	/0-3-8 (min 0-1-8)		Instal	lation guide.	
Max Horz	2=324(LC 12)	10-0-0 (mm. 0-1-0)				
Max Uplif Max Grav	t5=-189(LC 12) /5=629(LC 20), 2=626(LC 2	20)				
TOP CHORD 2-3=-67	omp./Max. Ten All forces 3/0	250 (lb) or less except when shown	1.			
BOT CHORD 2-7=-150	0/524, 6-7=-150/524, 6-8=-	151/521, 5-8=-151/521				
VVEBS 3-6=0/3	70, 3-5=-656/190					
<b>NOTES-</b> (8)	It=100mph (2 accord quat)	Vand-Ofmanh: TCDI = E Onaf: BCDI	-E Onof: h=22ft; C	at III Ev	n Di Englagodi MM//E	
(envelope) gable end	zone and C-C Exterior(2E)	-0-10-8 to 3-11-2, Interior(1) 3-11-2	to 8-6-10, Exterior	at. II; Ex (2E) 8-6-	-10 to 13-4-4 zone;C-	C
for members and force	es & MWFRS for reactions	shown; Lumber DOL=1.60 plate gri	p DOL=1.60	Diata D	OI = 1 15); la=1 0; Day	uch
Cat B; Partially Exp.; (	Ce=1.0; Cs=1.00; Ct=1.10	L-1.25 Plate DOL-1.25), PI-20.0 p	DSI (LUIII DOL-1.15	Plate D	OL = 1.15, $IS = 1.0$ , $ROI$	ugn
3) This truss has been do	esigned for greater of min r	oof live load of 12.0 psf or 2.00 time	es flat roof load of 2	20.0 psf o	on overhangs	
4) This truss has been de	esigned for a 10.0 psf botto	m chord live load nonconcurrent wi	ith any other live loa	ads.		
5) * This truss has been	designed for a live load of a	30.0psf on the bottom chord in all a	reas where a rectai	ngle 3-6-	0 tall by 1-0-0 wide w	ill fit
6) Refer to girder(s) for the	russ to truss connections.	13, With DODE - 10.0p31.				WINGTH CARO
7) Provide mechanical co	onnection (by others) of true	ss to bearing plate capable of withs	tanding 100 lb uplif	t at joint(	s) except (jt=lb) 5=18	9. SOFESSION VA
LOAD CASE(S) Standar	d					and the second s
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						Municipal Monthly

3/11/2025





Job	Truss	Truss Type	Qty	Ply	LOT 0.0005 CAMPBELL RIDGE   160 ALDEN W	AY ANGIER, NC
25-2083-R01	R15	Half Hip Girder	1	1	Job Reference (optional)	# 57653
		Run: 85. ID:WI8	130 s Feb 1 Brkg6BK5S	2 2021 Prin aRYCYG1	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue f9_0xywFJ5-PAACwc7oKmKi84S6C7URgx.	Mar 11 12:29:57 2025 Page 2 Z3OGud6U2fnHnJgCzc0ju

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 3=-8(B) 6=-1(B) 7=-8(B) 8=-8(B) 9=-1(B) 10=-1(B)





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0005 CAMPBELL RIDG	E   160 ALDEN WAY ANGIER, NC
25-2083-R01	SP02	COMMON	5	1		# 57653
			Run: 85.630 s 3 Jul	 12 2024 Pr	Job Reference (optional) int: 8.630 s Jul 12 2024 MiTek Ind	dustries. Inc. Tue Mar 11 12:29:58 2025 Page 1
0.40.0	-	0.0	ID:3tOeK4	4qXnLTmN	NBax9UYSrlyf11m-tMja8y8Q	54SZmE1Imr?gC96JXgBxrzco0xXtCezc0
0-10-8	6- 6-	0-0 0-0			6-0-0	
						Seels - 1:21 (
						Scale = 1:21.6
			4x6 =			
,			3			
	4.00 12					
		12	$\square$		14	
		13		$\sim$		
ထု	_					
7-7	1		W1			
2						4
4-8	/		B			
		15			16	
		15	6	I	10	
3x4 =			2x4			
						3x4 =
L	6-	0-0			12-0-0	
Plate Offsets (X Y) [2:0	-6- 1-1-13 Edge] [4:0-1-13 Edge]	0-0 >]			6-0-0	· · · ·
$\frac{1}{1} \frac{1}{1000} \frac$						
TCLL (roof) 20.0	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in (	loc) I/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Lumber DOL	1.15 IC 0.54 1.15 BC 0.55	Vert(LL)	0.08	6-9 >999 240 6-9 >999 180	MT20 244/190
TCDL 10.0	Rep Stress Incr	YES WB 0.10	Horz(CT)	0.01	4 n/a n/a	
BCDL 10.0	Code IRC2021/TP	2014 Matrix-AS				Weight: 42 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x4 SP No	0.2		TOP CHORD	Structu	ral wood sheathing directly	y applied.
BOT CHORD 2x4 SP No	0.2		BOT CHORD	Rigid c	eiling directly applied.	· · · · · · · · · · · · · · · · · · ·
WEDS 2X4 OF NO	1.0			Milek be ins	c recommends that Stabilized during truss erection	zers and required cross bracing
				Install	lation guide.	
REACTIONS. (lb/size)	2=532/0-3-8 (min. 0-1-8), 4	=532/0-3-8 (min. 0-1-8)				
Max Horz Max Uplift	2=-30(LC 15) 2=-162(I C 10) 4=-162(I C)	11)				
Max Grav	2=614(LC 21), 4=614(LC 2)	2)				
	mn May Tan All faraga ?	EQ (Ib) or loss except when shown				
TOP CHORD 2-13=-89	110.//wax. Ten All forces 2 06/947. 3-13=-825/957. 3-14	=-825/957. 4-14=-896/947				
BOT CHORD 2-15=-84	7/800, 6-15=-847/800, 6-16	=-847/800, 4-16=-847/800				
WEBS 3-6=-349	0/263					
<b>NOTES-</b> (10)						
1) Unbalanced roof live lo	ads have been considered	for this design.				
2) Wind: ASCE 7-16; Vul	t=120mph (3-second gust) \ zone and C-C Exterior(2E) -	/asd=95mph; TCDL=5.0psf; BCDI 0-10-8 to 3-11-2_Exterior(2R) 3-11	_=5.0psf; h=23ft; C _2 to 8-0-14 Exter	at. II; Exp ior(2E) 8	D B; Enclosed; MWFRS	
cantilever left and right	exposed ; end vertical left	and right exposed; porch left and r	ight exposed;C-C	for memb	ers and forces &	
MWFRS for reactions	shown; Lumber DOL=1.60	plate grip DOL=1.60				
3) TCLL: ASCE 7-16; Pr= Cat B: Partially Exp : C	:20.0 psf (roof LL: Lum DOL :e=1 0: Cs=1 00: Ct=1 10	.=1.15 Plate DOL=1.15); Pt=20.0 p	ost (Lum DOL=1.18	o Plate Do	OL=1.15); Is=1.0; Rough	
4) Unbalanced snow load	Is have been considered for	this design.				
5) This truss has been de	esigned for greater of min ro	of live load of 12.0 psf or 2.00 time	es flat roof load of 2	20.0 psf c	on overhangs	
6) This truss has been de	ner live loads. esigned for a 10.0 psf bottor	n chord live load nonconcurrent wi	th any other live lo	ads		
7) * This truss has been of	designed for a live load of 3	0.0psf on the bottom chord in all a	reas where a recta	ngle 3-6-	0 tall by 1-0-0 wide will fit	ANNELINI CASTINI
between the bottom ch	ord and any other members	).		<b>6</b>	·) · · · · · · · · · · · · · · · · · ·	IN ATH CAHOLINI
<li>4=162.</li>	ninection (by others) of trus	s to bearing plate capable of withs	tanding 100 ib upli	n ar joint(	s) except (jt=1b) 2=162,	POFESSION SIL
9) This truss design requ	ires that a minimum of 7/16	structural wood sheathing be app	lied directly to the	top chorc	l and 1/2" gypsum 🛭 💈 🦼	1 Alti
sheetrock be applied d	lirectly to the bottom chord.					SEAL
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Warning !	parameters and read notes be	fore use. This design is based only upo	n parameters shown,	and is for a	n individual building compone	ent to be installed and loaded



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LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

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LOAD CASE(S) Standard

