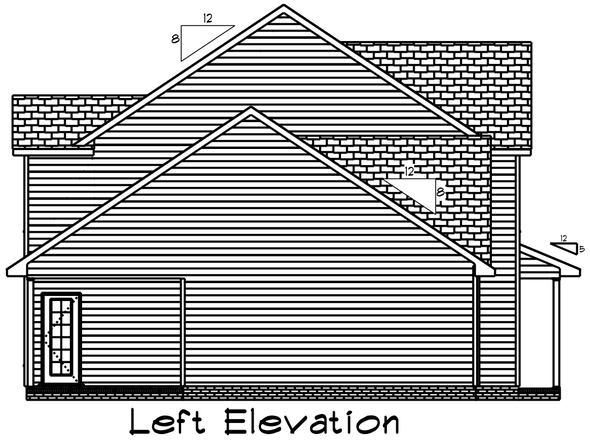
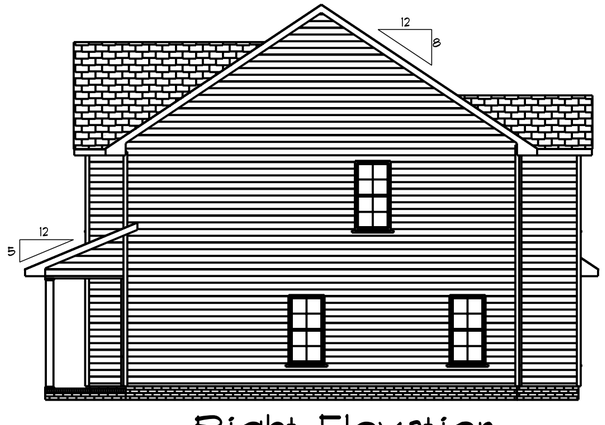
Front Elevation
Scale: 1/4"= 1'0"

NOTICE TO CONTRACTOR
All construction must comply with current NC Building Codes and is subject to field inspection and verification.
APPROVED
Limited building only review
Permit holder responsible for full compliance with the code
04/10/2025
Harnett COUNTY NORTH CAROLINA



Left Elevation
Scale: 1/8"= 1'0"

Rear Elevation
Scale: 1/8"= 1'0"



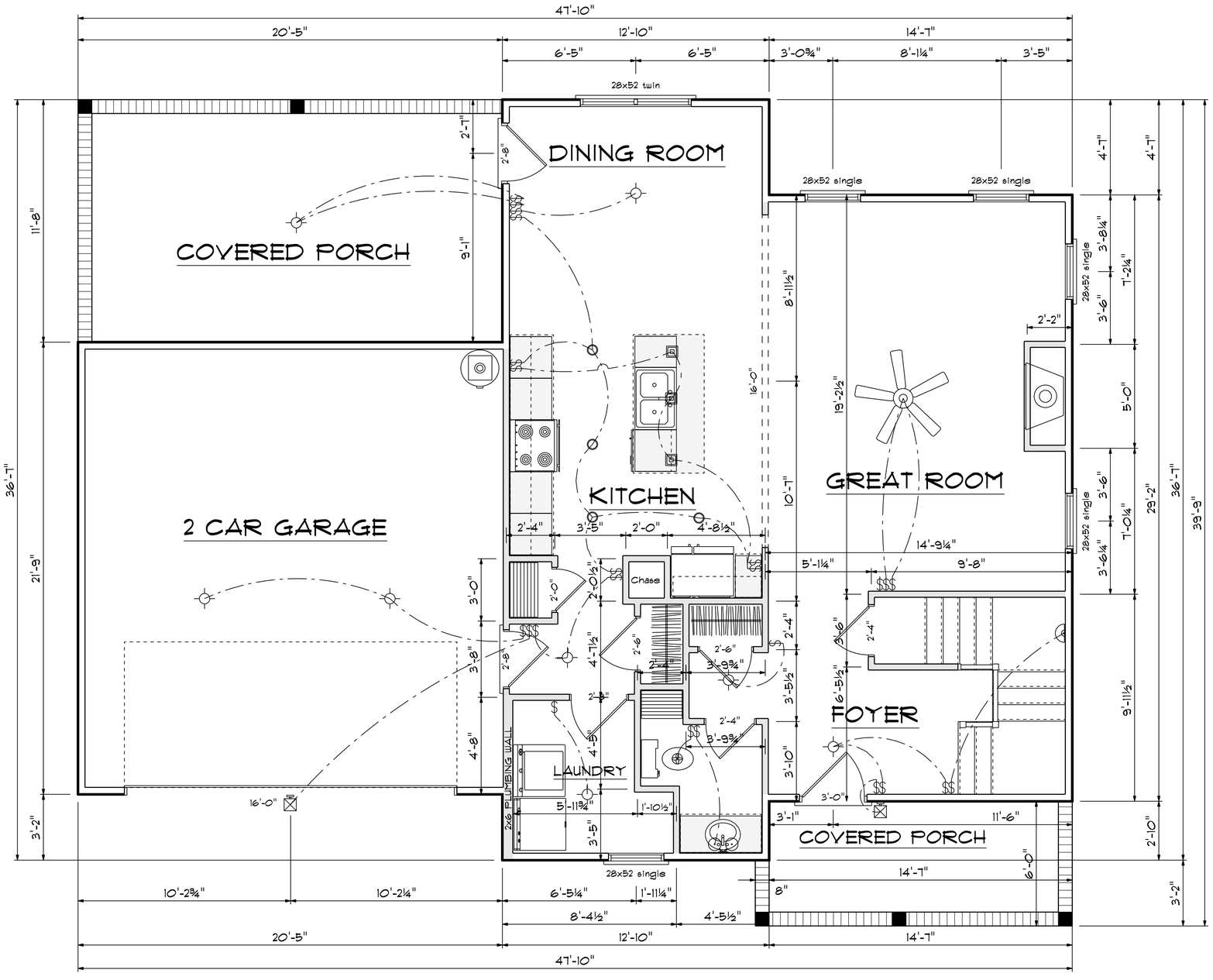
Right Elevation
Scale: 1/8"= 1'0"

Base Designs
2727 Chimney Pt.
Linden, N.C. 28356
910-263-0405

DATE: 1/15/2022
REVISED
DRAWING#

SCALE: 1/4"
DRAWN BY
APPROVED

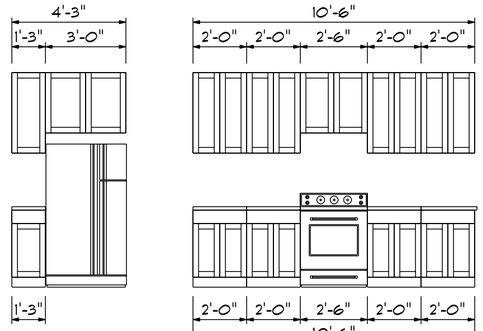
The Almirante



First Floor Plan

Scale: 1/4"= 1'-0"

Kitchen Cabinets



FIRST FLOOR OPENING SCHEDULE				
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT
36X80 COLONIAL A 1	3'-0"	L	NO	1
32X80 FRENCH A 1	2'-8"	L	NO	1
7' x 16' GARAGE DOOR	16'-0"	U	NO	1
2-0 Door Unit	2'-0"	R	NO	1
2-4 Door Unit	2'-4"	R	NO	1
2-4 Door Unit	2'-4"	L	NO	1
2-6 Door Unit	2'-6"	R	NO	2
2-8 Door Unit	2'-8"	L	NO	1
2-8 Door Unit	2'-8"	R	NO	1
28x52 single	2'-8" x 5'-2"	N	NA	5
28x52 twin	5'-4" x 5'-2"	NN	NA	1

Areas

First Floor	899
Second Floor	774
Total Heated	1673
Garage	447
Front Porch	89
Rear Porch	237

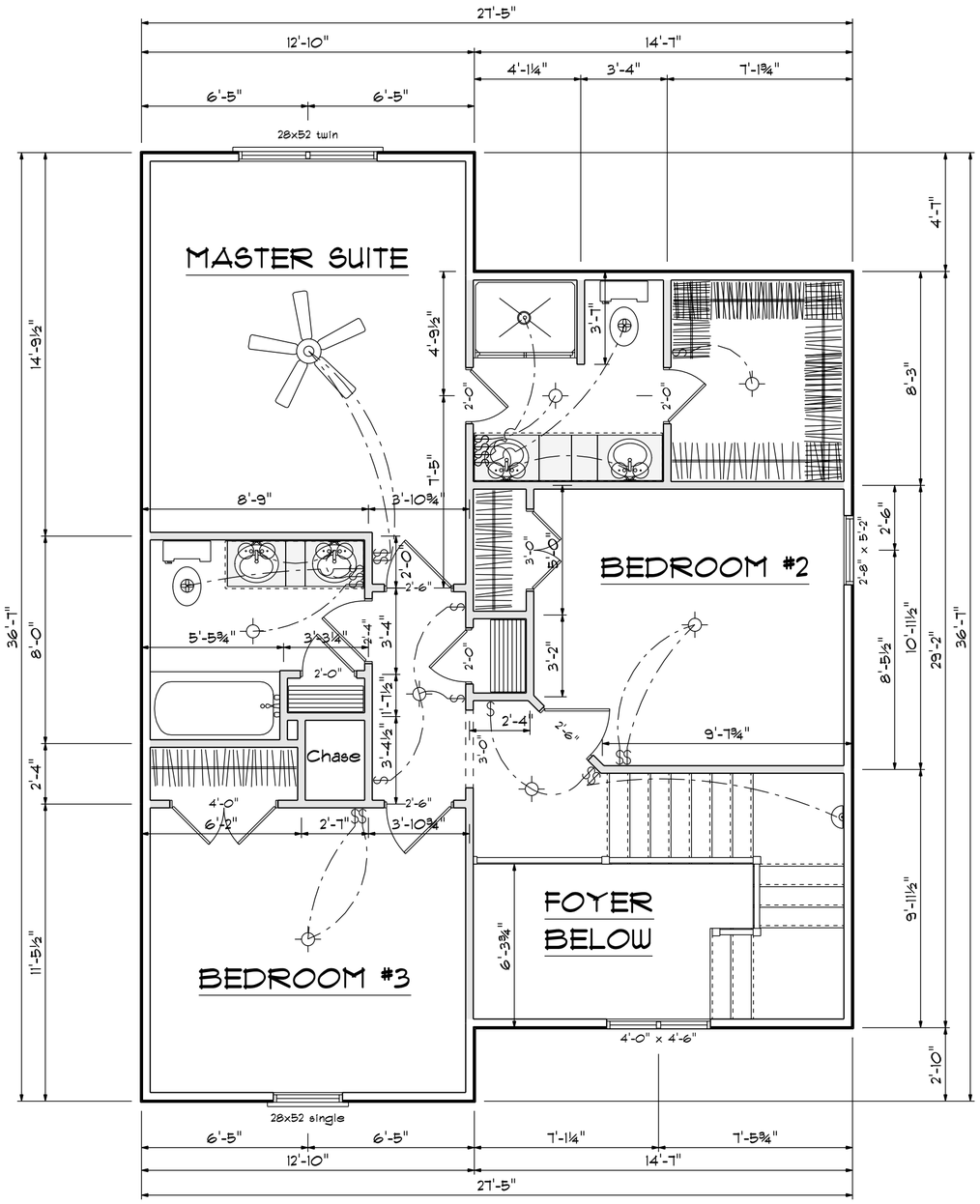
Bass Designs
2127 Chimney Pt.
Linden, N.C. 28356
910-263-0405

DATE: 1/15/2022
REVISED
DRAWING#

SCALE: 1/4"
DRAWN BY
APPROVED

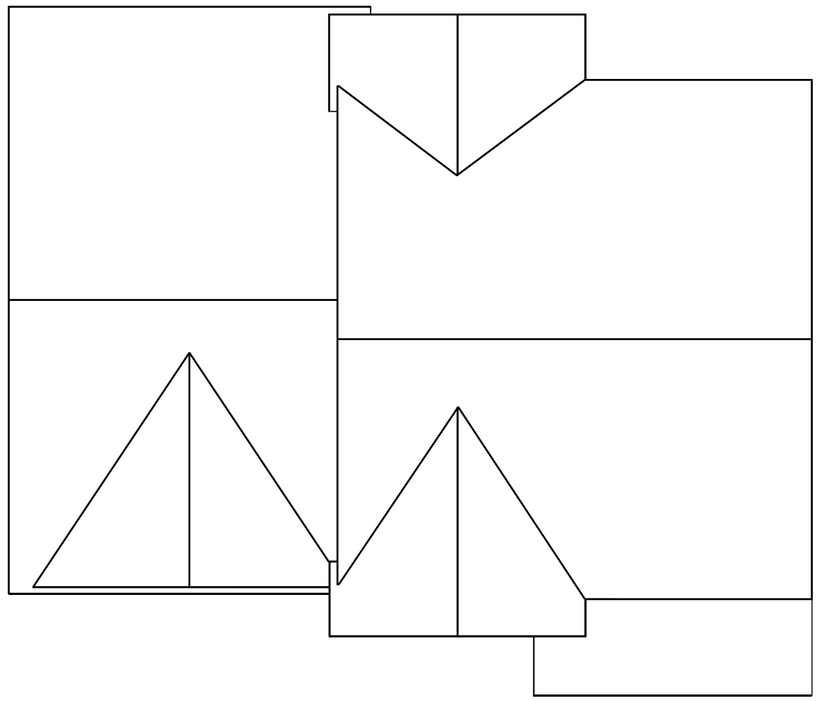
The Almirante

SECOND FLOOR OPENING SCHEDULE				
PRODUCT CODE	SIZE	HINGE	REVERSED	COUNT
2-0 Door Unit	2'-0"	R	NO	3
2-0 Door Unit	2'-0"	L	NO	1
2-4 Door Unit	2'-4"	L	NO	1
2-6 Door Unit	2'-6"	R	NO	1
2-6 Door Unit	2'-6"	L	NO	2
3-0 Doublehung Door Unit	3'-0"	LR	NO	1
4-0 Doublehung Door Unit	4'-0"	LR	NO	1
20x46 twin	4'-0" x 4'-6"	NN	NA	1
28x52 single	2'-8" x 5'-2"	N	NA	2
28x52 twin	5'-4" x 5'-2"	NN	NA	1



Second Floor Plan

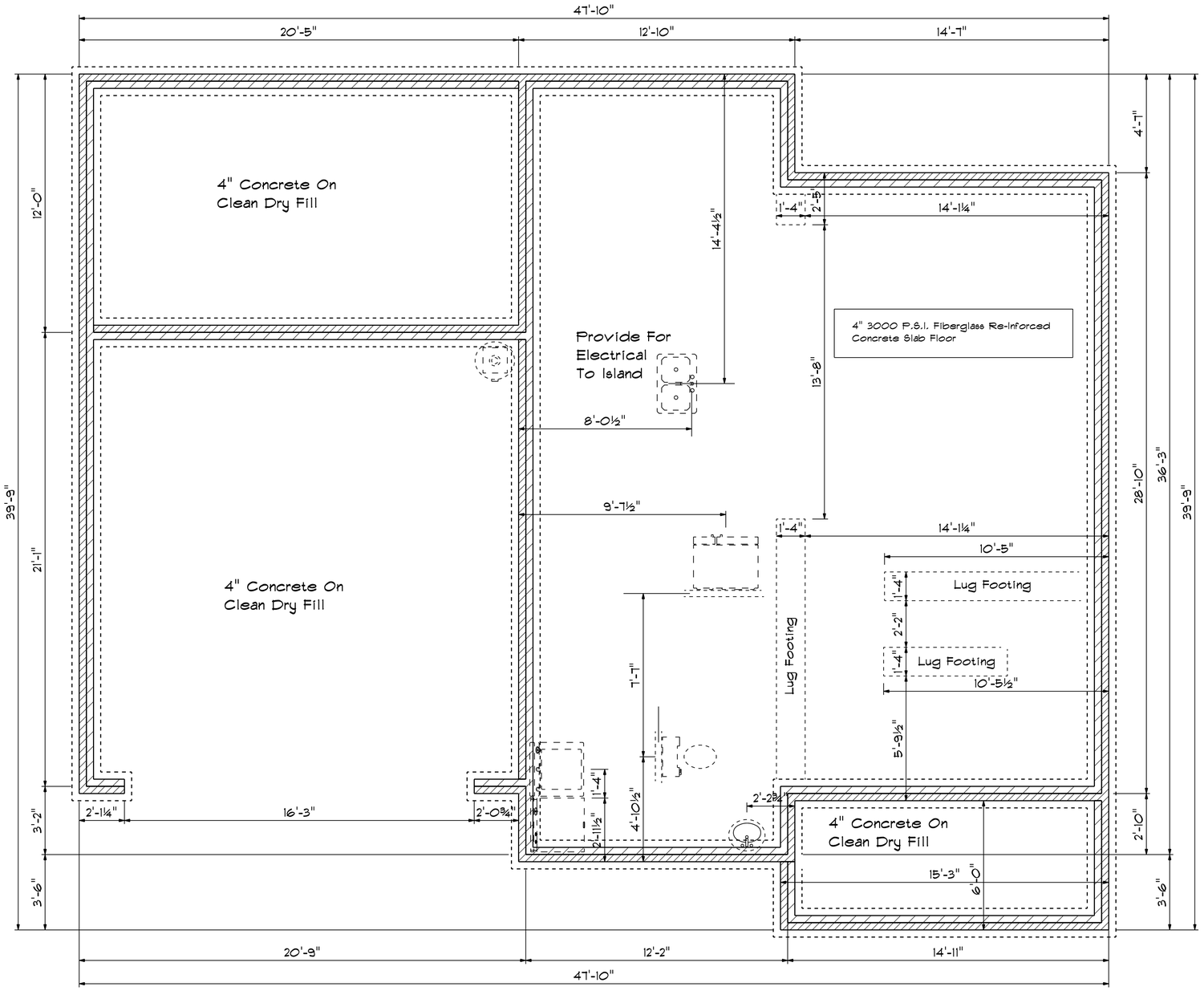
Scale: 1/4"= 1'-0"



Roof Layout

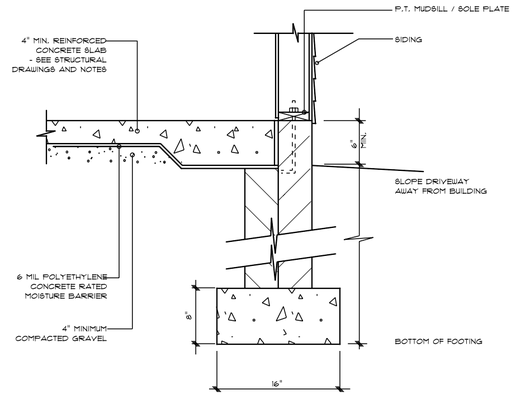
Base Designs 2121 Chimney Pt. Linden, N.C. 28356 910-263-0405	DATE: 1/15/2022		
	REVISED		
	DRAWING#		
SCALE: 1/4"			
DRAWN BY			
APPROVED			

The Almirante

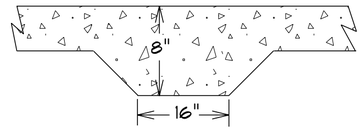


Foundation Plan

Scale: 1/4"= 1'-0"



STEM WALL FOOTING DETAIL



LUG FOOTING DETAIL

Bass Designs
2127 Chimney Pt.
Linden, N.C. 28356
910-263-0405

DATE: 1/15/2022
REVISED
DRAWING#

SCALE: 1/4"
DRAWN BY
APPROVED

The Almirante

General Notes:

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.

** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

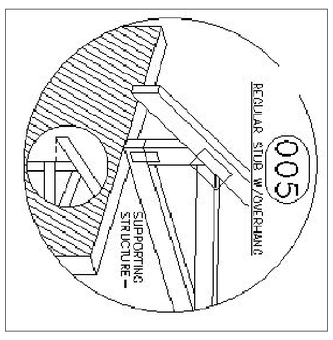
** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

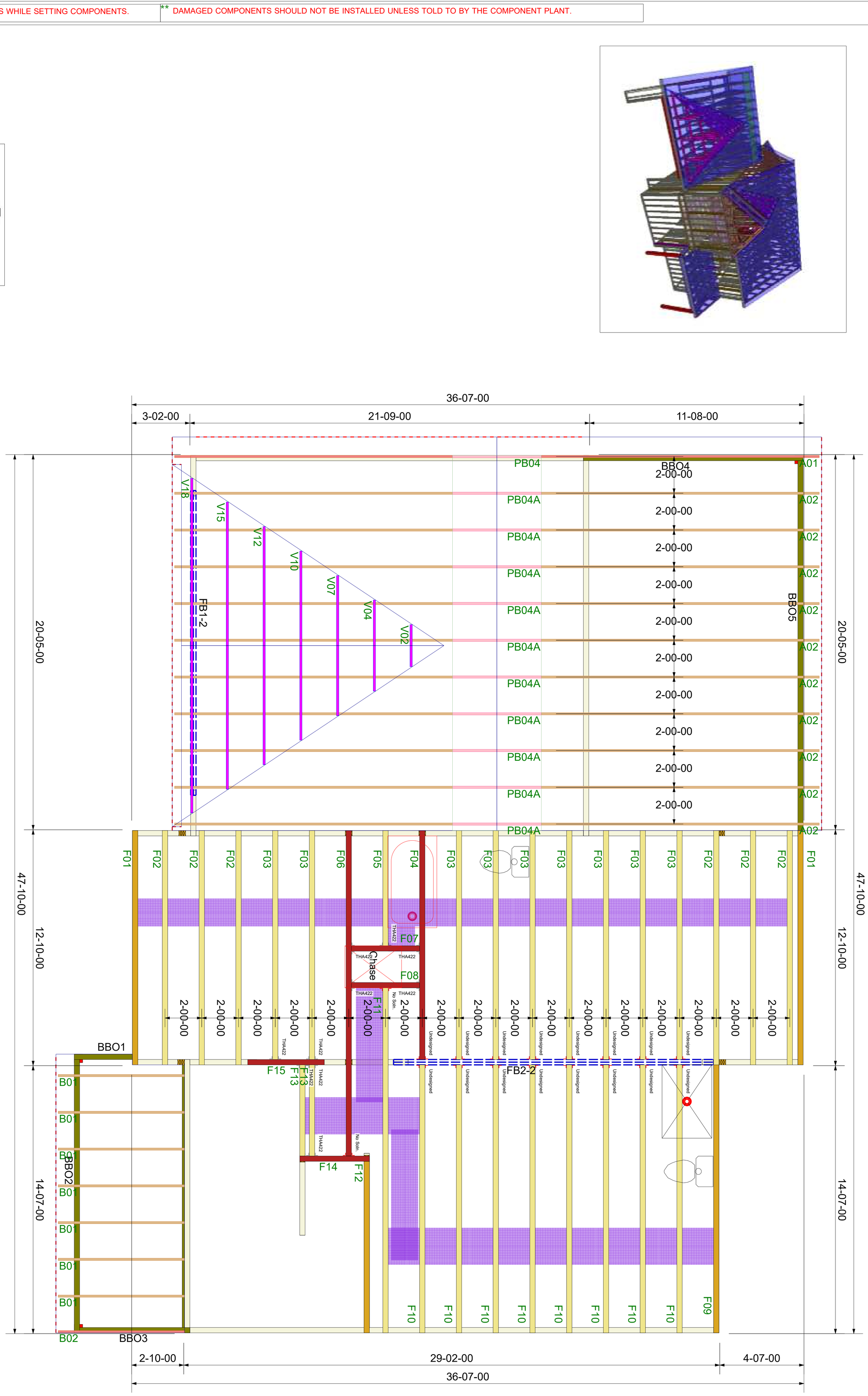
** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

Truss Drawing Left End Indicator





** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.

Scale: NTS
Date: 5/16/2023
Designer: NC
Project Number: 2302018
Sheet Number: 1/1

New Castle Contractors

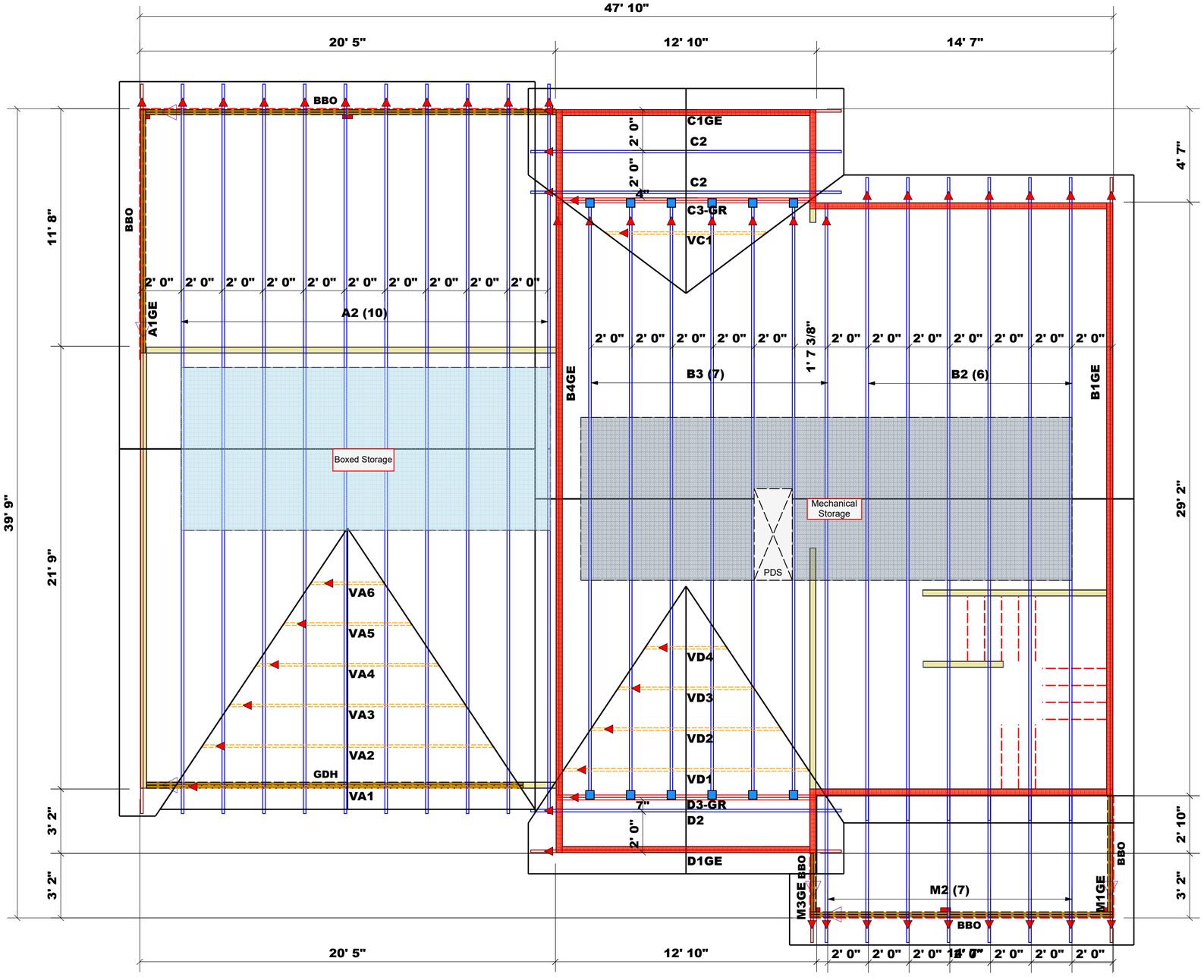
Almirante Plans

COMPONENT PLACEMENT PLAN

Kempsville Building Materials
A Division of the Carter Lumber Company

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Truss" available from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name



Truss Placement Plan
SCALE: NTS

Plumbing Drop Notes
1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24"oc.

Dimension Notes
1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of frame wall unless noted otherwise
3. All exterior wall to truss dimensions are to face of frame wall unless noted otherwise

Roof Area	= 2303.27 sq.ft.
Ridge Line	= 74.02 ft.
Hip Line	= 0 ft.
Horiz. OH	= 131.49 ft.
Raked OH	= 240.11 ft.
Decking	= 79 sheets

All Walls Shown Are Considered Load Bearing

▲ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do Not Erect Trusses Backwards

WALL SCHEDULE
1st Floor Brg. Wall
2nd Floor Brg. Wall

Products				
PlotID	Length	Product	Plies	Net Qty
GDH	19' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM1	18' 0"	1-3/4"x 18" LVL Kerto-S	2	2

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	12	Varies	16d/3-1/2"	16d/3-1/2"

COMTECH
ROOF & FLOOR TRUSSES & BEAMS
Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature_____

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

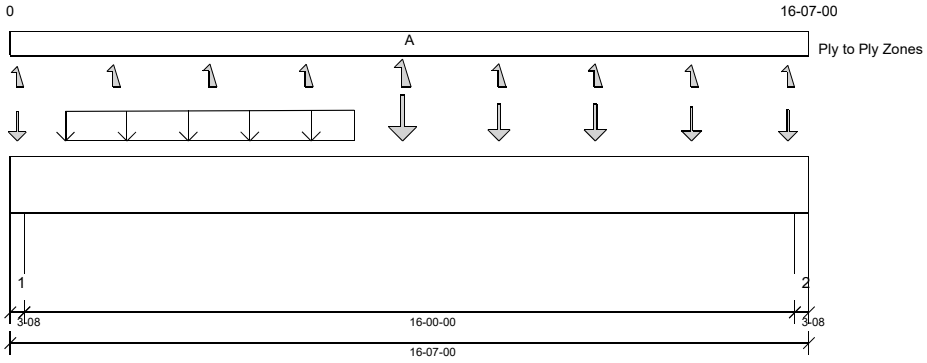
NO. REACTION (UP TO)	Q'TY STUDS FOR 2) PLY HEADER	NO. REACTION (UP TO)	Q'TY STUDS FOR 3) PLY HEADER	NO. REACTION (UP TO)	Q'TY STUDS FOR 4) PLY HEADER

BUILDER	Cash/Jaime Soto
JOB NAME	215 Montana Lane
PLAN	The Almirante
SEAL DATE	1/15/22
QUOTE #	Quote #
JOB #	J0522-2738

COUNTY	Cumberland
ADDRESS	215 Montana Lane, Spring Lake NC
MODEL	Roof
DATE REV.	5/23/22
DRAWN BY	Johnnie Baggett
SALESMAN	Marshall Naylor

MiTek	Customer: Street 1: City: Customer Ph...	Job Name: **B** Level: **1st FLOOR** Label: **FB1-2 - i64** Type: **Beam**	**2 Ply Member** **2.0 RigidLam DF LVL 1-3/4 x 14**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.7.3.303.Update9.26 | Report Version: 2021.03.26 06/13/2024 14:48



DESIGN INFORMATION

Building Code:	IRC 2018
Design Methodology:	ASD
Risk Category:	II (General Construction) Residential
Service Condition:	Dry
LL Deflection Limit:	L/360, 0.75" (absolute)
TL Deflection Limit:	L/240, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 1'- 10 1/2" Bottom: 16'- 7"

Bearing Stress of Support Material:

- 1323 psi Wall @ 0'- 2 1/2"
- 1323 psi Wall @ 16'- 4 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	8'- 1 3/4"	D + Lr	1.15	26863 lb ft	33318 lb ft	Passed - 81%
Max Neg. Moment:	8'- 1 3/4"	0.6D + 0.6W	1.60	6180 lb ft	27051 lb ft	Passed - 23%
Max Shear:	1'- 5 1/2"	D + Lr	1.15	5569 lb	10894 lb	Passed - 51%
Live Load (LL) Pos. Defl.:	8'- 3 9/16"	Lr		0.435"	L/360	Passed - L/441
Total Load (TL) Pos. Defl.:	8'- 3 9/16"	D + Lr		0.766"	L/240	Passed - L/250

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	3-08	D + Lr	1.15	6687 lb		9188 lb	16207 lb	Passed - 73%
1	3-08	0.6D + 0.6W	1.60		-1513 lb	-	-	
2	3-08	D + Lr	1.15	6587 lb		9188 lb	16207 lb	Passed - 72%
2	3-08	0.6D + 0.6W	1.60		-1489 lb	-	-	

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	16'- 7"	Self Weight	Top	13 lb/ft	-	-	-	-
Tapered	1'- 1 3/4"	7'- 1 3/4"	Smoothed Load	Top	310 lb/ft	-	252 lb/ft	379 To 432 lb/ft	116 To 130 lb/ft
Point	0'- 1 3/4"	0'- 1 3/4"	A02(c09)	Top	510 lb	-	388 lb	589/-29 lb	178/-820 lb
Point	2'- 1 3/4"	2'- 1 3/4"	A02(c01)	Top	-	-	-	-20 lb	-986 lb
Point	4'- 1 3/4"	4'- 1 3/4"	A02(c05)	Top	-	-	-	-17 lb	-1055 lb
Point	6'- 1 3/4"	6'- 1 3/4"	A02(c04)	Top	-	-	-	-19 lb	-1032 lb
Point	8'- 1 3/4"	8'- 1 3/4"	A02(c03)	Top	827 lb	-	725 lb	1226/-14 lb	375/-1505 lb
Point	10'- 1 3/4"	10'- 1 3/4"	A02(c07)	Top	657 lb	-	544 lb	880/-14 lb	268/-1080 lb
Point	12'- 1 3/4"	12'- 1 3/4"	A02(c02)	Top	644 lb	-	531 lb	871/-15 lb	260/-1070 lb
Point	14'- 1 3/4"	14'- 1 3/4"	A02(c10)	Top	598 lb	-	481 lb	762/-14 lb	232/-983 lb
Point	16'- 1 3/4"	16'- 1 3/4"	A02(c06)	Top	540 lb	-	419 lb	659/-47 lb	197/-902 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	0'- 3 1/2"	E22(i63)	2955 lb	-	2319 lb	3725/-90 lb	254 lb/ -5475 lb
2	16'- 3 1/2"	16'- 7"	E11(i7)	2899 lb	-	2284 lb	3696/-99 lb	254 lb/ -5475 lb

DESIGN NOTES

- CAUTION: The maximum net analysis reaction exceeds the user-defined maximum uplift value at one or more supports.
- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 0.99

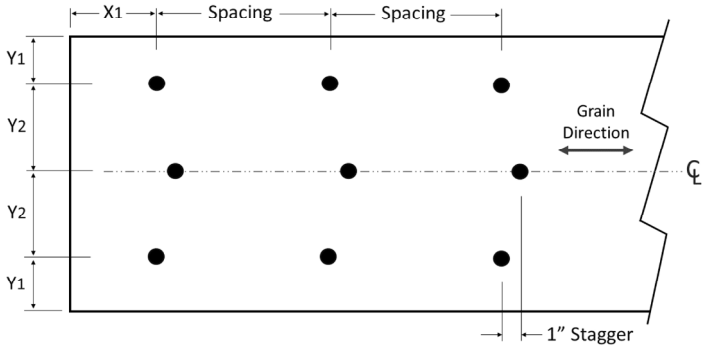
PLY TO PLY CONNECTION

- Zone A: Factored load = 0 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 51. Row = 3, Spacing = 12"
  12d (0.148"x3.25") nails properties: D = 0.148" , L = 3.25". Fastener capacity = 117 lbs. X1 = 2.25" , Y1 = 0.75", Y2 = 1.5"
  Install fasteners from one face.
  X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.

MiTek	Customer: Street 1: City: Customer Ph...	Job Name: **B** Level: **1st FLOOR** Label: **FB1-2 - i64** Type: **Beam**	**2 Ply Member** **2.0 RigidLam DF LVL 1-3/4 x 14**	Status: **Design Passed**

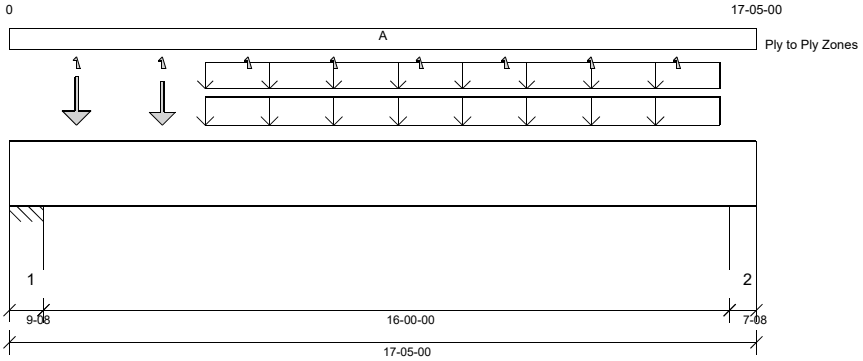
PLY TO PLY CONNECTION

FASTENER INSTALLATION – 3 ROWS (FROM ONE FACE)



MiTek	Customer: Street 1: City: Customer Ph...	Job Name: **B** Level: **1st FLOOR** Label: **FB2-2 - i74** Type: **Beam**	**2 Ply Member** **2.0 RigidLam DF LVL 1-3/4 x 18**	Status: **Design Passed**

Illustration Not to Scale. Pitch: 0/12 | Designed by Single Member Design Engine in MiTek® Structure Version 8.7.3.303.Update9.26 | Report Version: 2021.03.26 06/13/2024 14:48



DESIGN INFORMATION

Building Code:	IRC 2018
Design Methodology:	ASD
Risk Category:	II (General Construction) Residential
Service Condition:	Dry
LL Deflection Limit:	L/360, 0.75" (absolute)
TL Deflection Limit:	L/240, 1.00" (absolute)

Lateral Restraint Requirements:

Both ends of the member and the outer supports must be laterally restrained. Top and bottom edges of the member must be fully restrained or have the following maximum unbraced length:

Top: 0' Bottom: 1'- 8 1/2"

Bearing Stress of Support Material:

- 1323 psi Wall @ 0'- 8 1/2"
- 725 psi Wall @ 16'- 10 1/2"

ANALYSIS RESULTS

Design Criteria	Location	Load Combination	LDF	Design	Limit	Result
Max Pos. Moment:	9'- 6 3/4"	D + L	1.00	25538 lb ft	46413 lb ft	Passed - 55%
Max Shear:	15'- 3 1/2"	D + L	1.00	6029 lb	12180 lb	Passed - 50%
Live Load (LL) Pos. Defl.:	8'- 9 7/16"	L		0.252"	L/360	Passed - L/761
Total Load (TL) Pos. Defl.:	8'- 9 7/16"	D + L		0.354"	L/240	Passed - L/543

SUPPORT AND REACTION INFORMATION

ID	Input Bearing Length	Controlling Load Combination	LDF	Downward Reaction	Uplift Reaction	Resistance of Member	Resistance of Support	Result
1	9-08	D + L	1.00	6607 lb		24937 lb	43990 lb	Passed - 26%
2	7-08	D + L	1.00	6094 lb		19687 lb	19031 lb	Passed - 32%

LOADING

Type	Start Loc	End Loc	Source	Face	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
Self Weight	0'	17'- 5"	Self Weight	Top	17 lb/ft	-	-	-	-
Uniform	4'- 6 3/4"	16'- 6 3/4"	Smoothed Load	Front	111 lb/ft	299 lb/ft	-	-	-
Uniform	4'- 6 3/4"	16'- 6 3/4"	Smoothed Load	Back	95 lb/ft	255 lb/ft	0 lb/ft	0 lb/ft	0 lb/ft
Point	1'- 6 3/4"	1'- 6 3/4"	-	Front	485 lb	1267/-2 lb	1 lb	1 lb	0/-2 lb
Point	3'- 6 3/4"	3'- 6 3/4"	-	Front	410 lb	1105 lb	0 lb	1 lb	0/-2 lb
Point	5'- 6 3/4"	5'- 6 3/4"	-	Front	-	-	-	-	-2 lb
Point	7'- 6 3/4"	7'- 6 3/4"	-	Front	-	-	-	-	-2 lb
Point	9'- 6 3/4"	9'- 6 3/4"	-	Front	-	-	-	-	-2 lb
Point	11'- 6 3/4"	11'- 6 3/4"	-	Front	-	-	-	-	-2 lb
Point	13'- 6 3/4"	13'- 6 3/4"	-	Front	-	-	-	-	-2 lb
Point	15'- 6 3/4"	15'- 6 3/4"	-	Front	-	-	-	-	-2 lb

UNFACTORED REACTIONS

ID	Start Loc	End Loc	Source	Dead (D)	Live (L)	Snow (S)	Roof Live (Lr)	Wind (W)
1	0'	0'- 9 1/2"	E24(i72)	1907 lb	4700/-2 lb	2 lb	3 lb	2 lb/ -8 lb
2	16'- 9 1/2"	17'- 5"	E25(i73)	1754 lb	4340 lb	2 lb	3 lb	2 lb/ -8 lb

DESIGN NOTES

- The dead loads used in the design of this member were applied to the structure as projected dead loads.
- Analysis and Design has been performed using precision loading from actual modeled conditions. Some loads may have been modified to simplify reporting.
- Tributary Loads have been generated based on actual spacing between members in the model which may differ from the default system spacing. The actual loads applied to the member are shown in the Specified Loads table.
- Transfer reactions may differ from design results as allowed per building codes and standard load distribution practices.
- This report is based on modeled conditions input by the user. Source information for the loads and supports are provided for reference only. Verify that all loads and support conditions are correct.
- Review all loads and reactions to ensure that the member/bearing/connector/structure can resist adequately. Unless already specified on this report, anchorage for uplift reactions to be specified by others. Installation of member and accessories (if required) as per manufacturer's instruction.
- Beam Stability Factor used in the calculation for Allowable Max Pos Moment (CL) = 1.00

PLY TO PLY CONNECTION

- Zone A: Factored load = 468 plf. Use 12d (0.148"x3.25") nails. LDF = 1.00. Qty = 54. Row = 3, Spacing = 12"
  12d (0.148"x3.25") nails properties: D = 0.148" , L = 3.25". Fastener capacity = 117 lbs. X1 = 2.25" , Y1 = 0.75", Y2 = 1.5"
  Install fasteners from one face.
  X1 = Minimum end distance, X2 = Minimum edge distance, Y2 = Minimum row spacing.

MiTek	Customer: Street 1: City: Customer Ph...	Job Name: **B** Level: **1st FLOOR** Label: **FB2-2 - i74** Type: **Beam**	**2 Ply Member** **2.0 RigidLam DF LVL 1-3/4 x 18**	Status: **Design Passed**

PLY TO PLY CONNECTION

FASTENER INSTALLATION – 3 ROWS (FROM ONE FACE)