Mark Morris, P.E. #126, 1317-M, Summerville, SC 29483

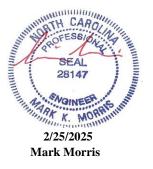
843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

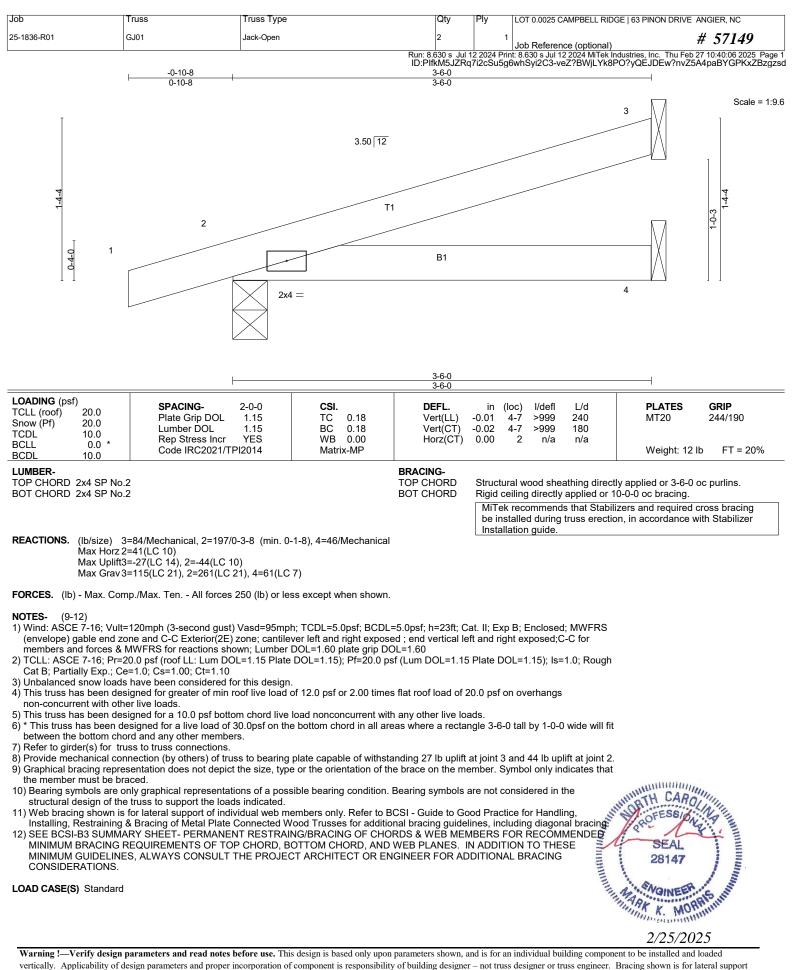
AST #: 57149 JOB: 25-1836-R01 JOB NAME: LOT 0.0025 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 65 Truss Design(s)

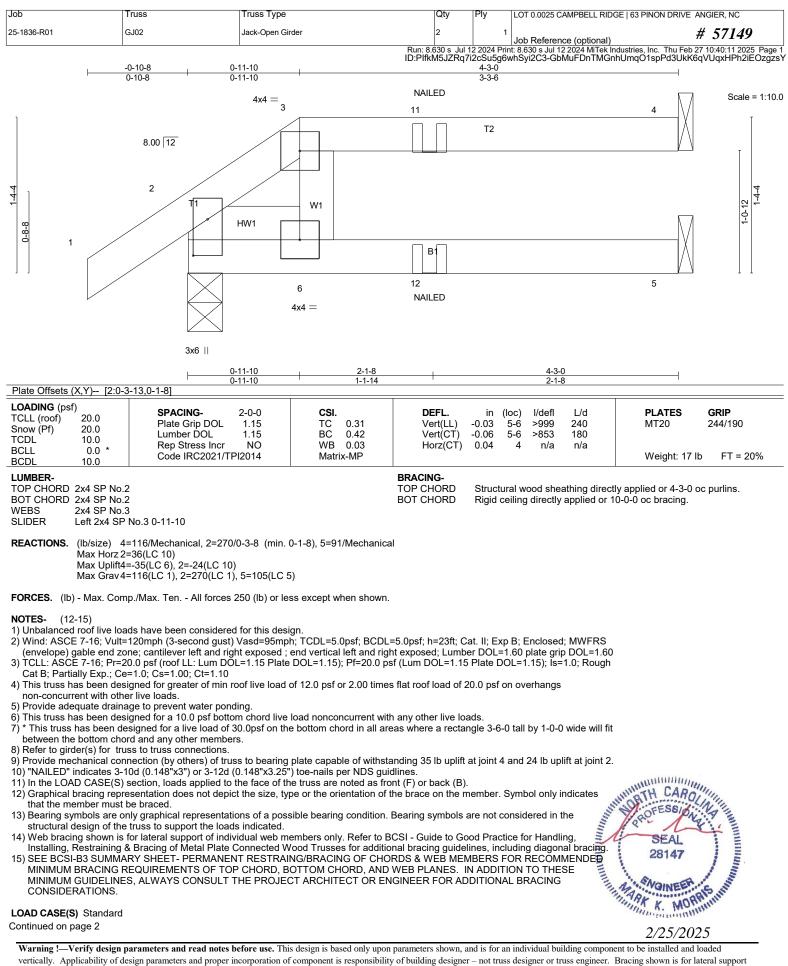
Trusses:

GJ01, GJ02, GJ03, GJ04, GJ05, GJ06, GJ07, GR01, GR02, GR02A, GV01, J01, J01A, J02, J03, J04, J05, J06, J07, PB01, PB02, PB03, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, R19, R20, R21, R22, R23, R24, R27, R27A, R28, R29, R30, R31, R32, R33, SP01, SP02, V01, V02, V03, V04, V05, V06, V07, V08, V09



Warning !- Verify design parameters and read notes before use.



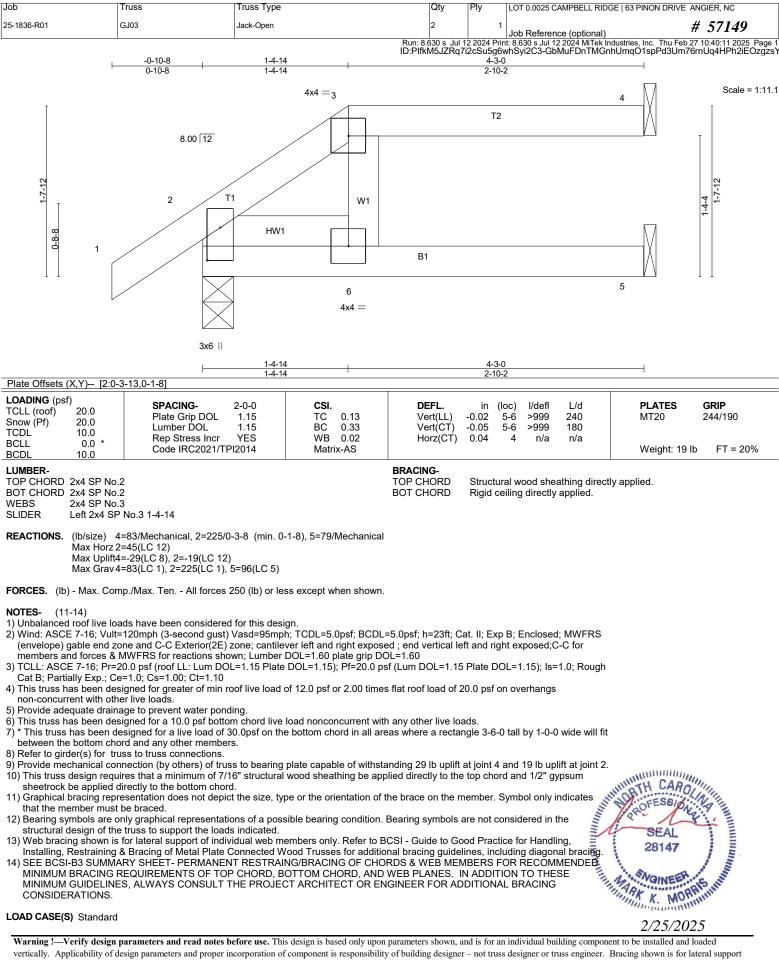


Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE ANGIER, NC
25-1836-R01	GJ02	Jack-Open Girder	2	1	Job Reference (optional) # 57149
					ıt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:40:11 2025 Page $\widehat{2}$ vhSyi2C3-GbMuFDnTMGnhUmqO1spPd3UkK6qVUqxHPh2iEOzgzsY

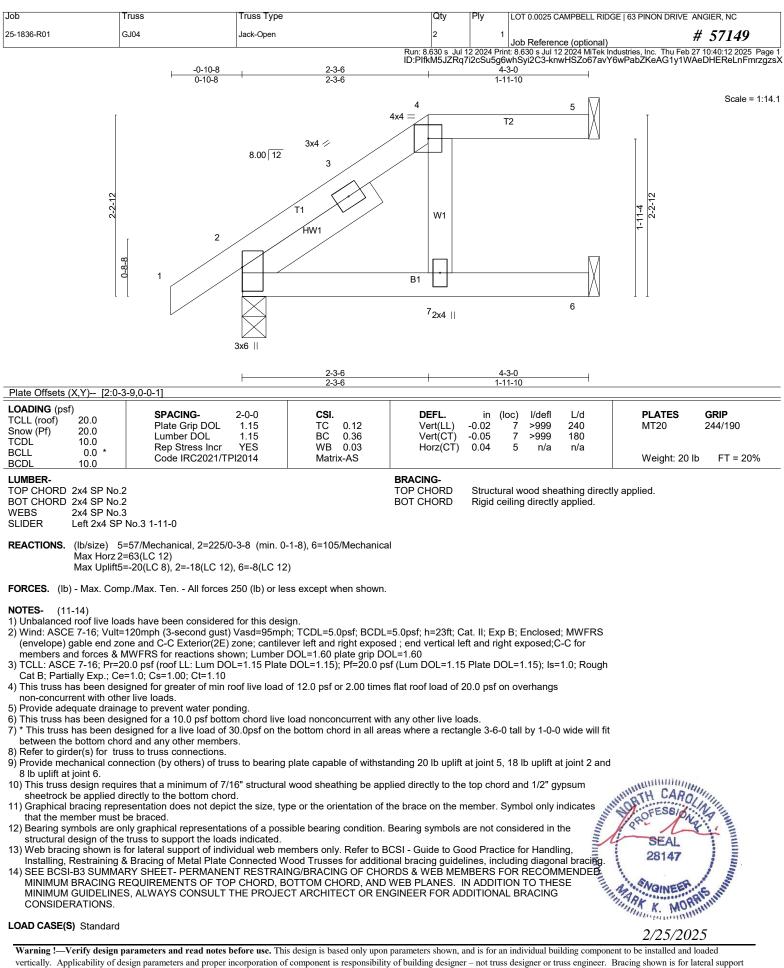
LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb) Vert: 11=-55(F) 12=-34(F)

> SEAL 28147 2/25/2025 The installed and loaded



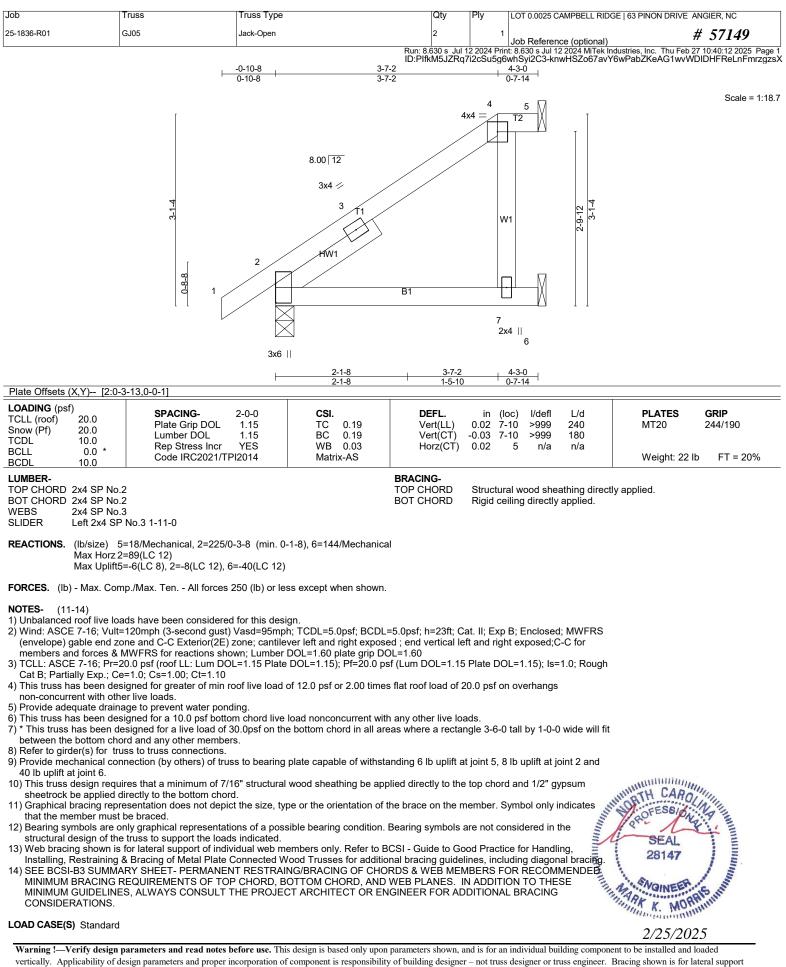
vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



LOAD CASE(S) Standard

Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

2/25/2025

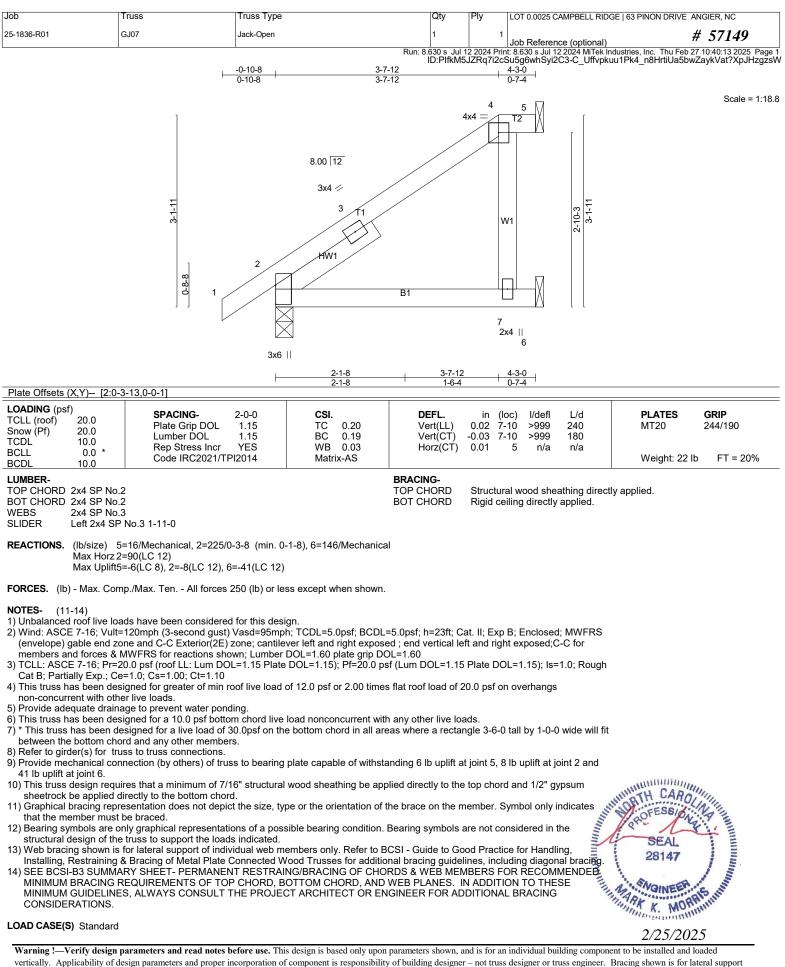


LOAD CASE(S) Standard

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2/25/2025

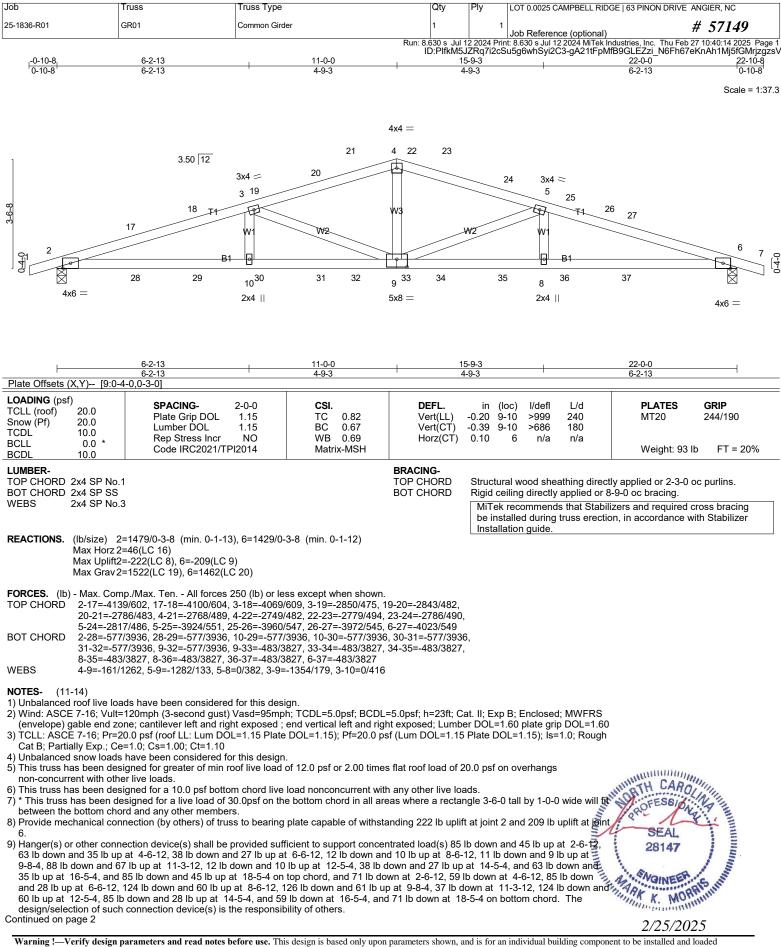
Job	Truss		Truss Type		Qty	Ply I	_OT 0.0025 C	AMPBELI RID	GE 63 PINON DRIVE ANGIER, NC
25-1836-R01	GJ06		Jack-Open		1	1	201 0.0020 0		# 57149
					Run: 8.630 s Jul	12 2024 Print:	Job Referer 8.630 s Jul 1	ice (optional) 2 2024 MiTek li	
			-0-10-8		ID:PIfkM5JZRq 4-1-1	7i2cSu5g6w	hSyi2C3-kn 4 _⊤ 3-0	wHSZo67avY	ndustries, Inc. Thu Feb 27 10:40:12 2025 Page /6wPabZKeAG1wIWDUDHgReLnFmrzgz
			0-10-8		4-1-1		4 ₇ 3-0 0-1-15		
							4 🗆		Scale = 1:20
		[
				8.00 12	/	/ /			
				3x4 🥢					
		3-5-3		3	T1				
					\langle				
				$\langle \chi \rangle$					
			2 /	HW1					
		0-8-8	. / /	1/			M	ŝ	
		[1		<u> </u>	B1		Μ	0-3-8	
			<pre>k</pre>	$\stackrel{\sim}{\rightarrow}$					
			⊴ 3x6				5		
			+	<u>2-1-8</u> 2-1-8		4-3-0 2-1-8			
Plate Offsets (X,Y)	[2:0-3-0,0-0-1]								1
LOADING (psf) TCLL (roof) 20	0	PACING-	2-0-0	CSI .	DEFL.	in (loc		L/d	PLATES GRIP
Snow (Pf) 20. TCDL 10.	.0 Lu	ate Grip DOL Imber DOL	1.15 1.15	TC 0.23 BC 0.18	Vert(LL) Vert(CT)	0.02 5- -0.03 5-	8 >999	240 180	MT20 244/190
BCLL 0	.0 * Re	ep Stress Incr ode IRC2021/TP	YES PI2014	WB 0.00 Matrix-AS	Horz(CT)	0.01	2 n/a	n/a	Weight: 19 lb FT = 20%
BCDL 10	.0				BRACING-				
TOP CHORD 2x4					TOP CHORD			athing direct	ly applied.
BOT CHORD 2x4 SLIDER Left	SP No.2 2x4 SP No.3 1-11	1-0			BOT CHORD	Rigid cell	ing directly	applied.	
REACTIONS. (Ib/s	size) 4=110/Mec	hanical 2=225/	0-3-8 (min 0-1-8	3), 5=52/Mechanica	al				
Max	k Horz 2=100(LC 1	12)		,, , , , , , , , , , , , , , , , , , , ,					
	< Uplift4=-60(LC 1 < Grav 4=115(LC 2								
FORCES. (Ib) - Ma	ax. Comp./Max. T	en All forces 2	250 (lb) or less e	cept when shown.					
				toopt when shown.					
NOTES- (9-12) 1) Wind: ASCE 7-1	6; Vult=120mph ((3-second gust)	Vasd=95mph; T(CDL=5.0psf; BCDL	=5.0psf; h=23ft; C	at. II; Exp E	; Enclosed	; MWFRS	
				eft and right expose .=1.60 plate grip D0		ft and right	exposed;C	C-C for	
2) TCLL: ASCE 7-1		oof LL: Lum DOI				Plate DOL	.=1.15); ls=	1.0; Rough	
3) This truss has be	een designed for	greater of min ro	oof live load of 12	2.0 psf or 2.00 times	s flat roof load of 2	20.0 psf on	overhangs		
4) This truss has be	with other live load een designed for a		m chord live load	nonconcurrent wit	h any other live loa	ads.			
5) * This truss has	been designed for tom chord and an			ttom chord in all are	eas where a rectar	ngle 3-6-0 t	all by 1-0-0) wide will fit	
6) Refer to girder(s) for truss to truss	s connections.							
 Provide mechan This truss design 									
	plied directly to the			the orientation of th	e brace on the m	ember Svn	hol only in	dicates that	
the member mus	st be braced.		(100)20, type of t			, .			ANNIHI HITTITA
10) Bearing symbo structural desig	is are only graphic in of the truss to s	support the load	s indicated.	bearing condition.	Bearing symbols a	are not con	sidered in	ine	IN ORTH CANOLANI
11) Web bracing st Installing Rest	nown is for lateral	support of indiv of Metal Plate (idual web membe	ers only. Refer to B Trusses for additic	CSI - Guide to Go anal bracing guide	od Practice	e for Handl ling diagor	ing,	2ROF NR. STIM
12) SEE BCSI-B3 S	SUMMARY SHEE	T- PERMANEN	IT RESTRAING/E	3RACING OF CHO	RDS & WEB MEN	ABERS FO	RRECOM	MENDE	SEAL
	JELINES, ALWA	YS CONSULT T	HE PROJECT A	RCHITECT OR EN	IGINEER FOR AD		BRACING	SE III	SEAL 28147
CONSIDERAT	ONS.							IIIII	SEAL 28147 2/25/2025
LOAD CASE(S) St	andard							in.	ARE GREERE
									Municipality Monthly
									2/25/2025
									_, ,



LOAD CASE(S) Standard

Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

2/25/2025



Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE ANGIER, NC
25-1836-R01	GR01	Common Girder	1	1	Job Reference (optional) # 57149
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:40:14 2025 Page u5g6whSyi2C3-gA21tFpMfB9GLEZzi N6Fh67eKnAh1Mj5fGMrjzgz:

NOTES- (11-14)

- 10) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).
- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard

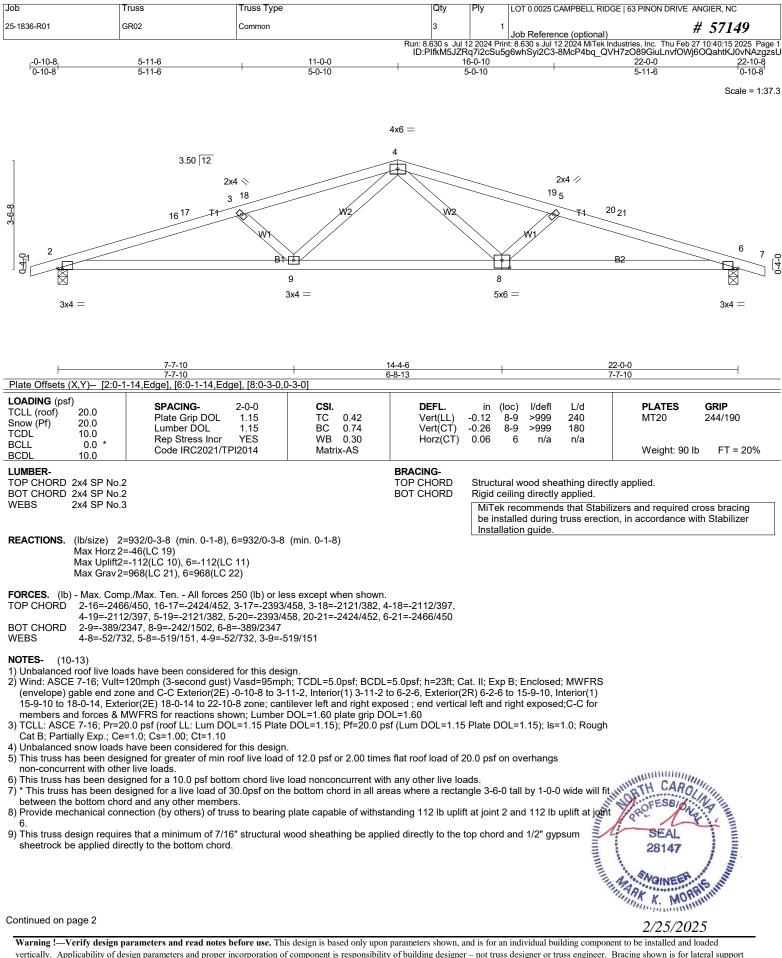
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-14=-20

Concentrated Loads (lb)

Vert: 17=-56(F) 18=-23(F) 22=-50(F) 25=-23(F) 27=-56(F) 28=-71(F) 29=-59(F) 30=-85(F) 31=-124(F) 32=-126(F) 33=-32(F) 34=-124(F) 35=-85(F) 36=-59(F) 37=-71(F) 29=-59(F) 30=-85(F) 31=-124(F) 32=-126(F) 32=-32(F) 34=-124(F) 35=-85(F) 36=-59(F) 37=-71(F) 30=-85(F) 30=-85(F) 30=-85(F) 30=-85(F) 32=-126(F) 32=-126(F) 32=-32(F) 32=-32(F) 34=-124(F) 35=-85(F) 36=-59(F) 37=-71(F) 30=-85(F) 3





Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 P	PINON DRIVE ANGIER, NC
25-1836-R01	GR02	Common	3	1	Job Reference (optional)	# 57149
						Inc. Thu Feb 27 10:40:15 2025 Page 2 9GiuLnvfOWj6OQahtKJ0vNAzgzsU

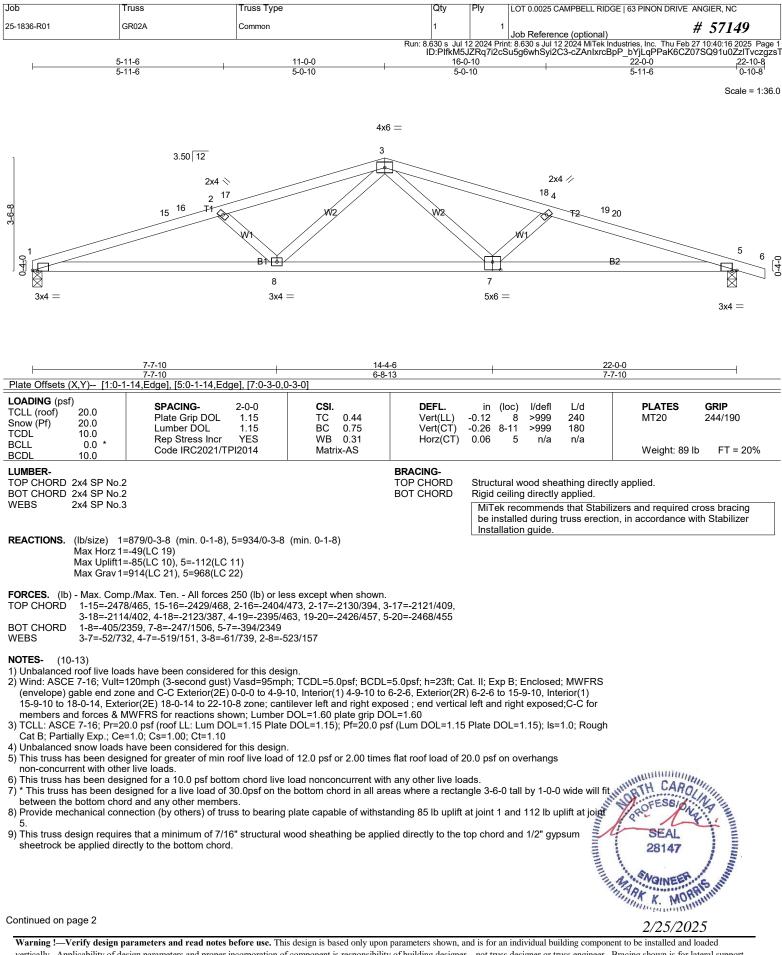
10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE	ANGIER, NC
25-1836-R01	GR02A	Common	1	1	Job Reference (optional)	# 57149
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb u5g6whSyi2C3-cZAnIxrcBpP_bYjLqPPaK6CZ	

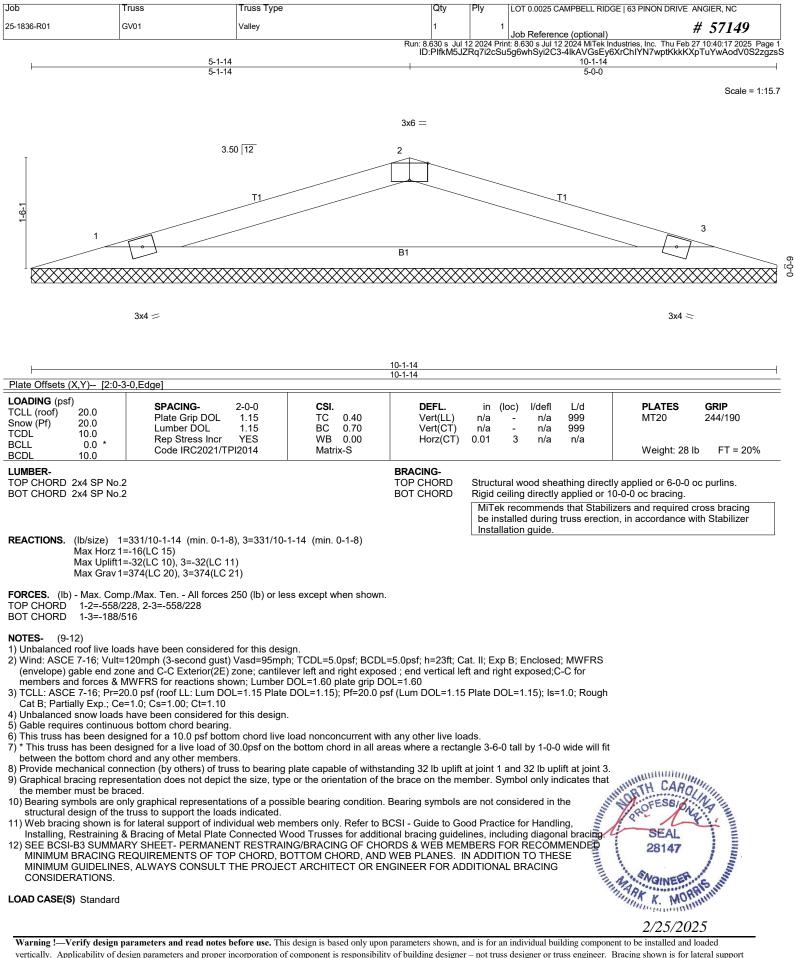
10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

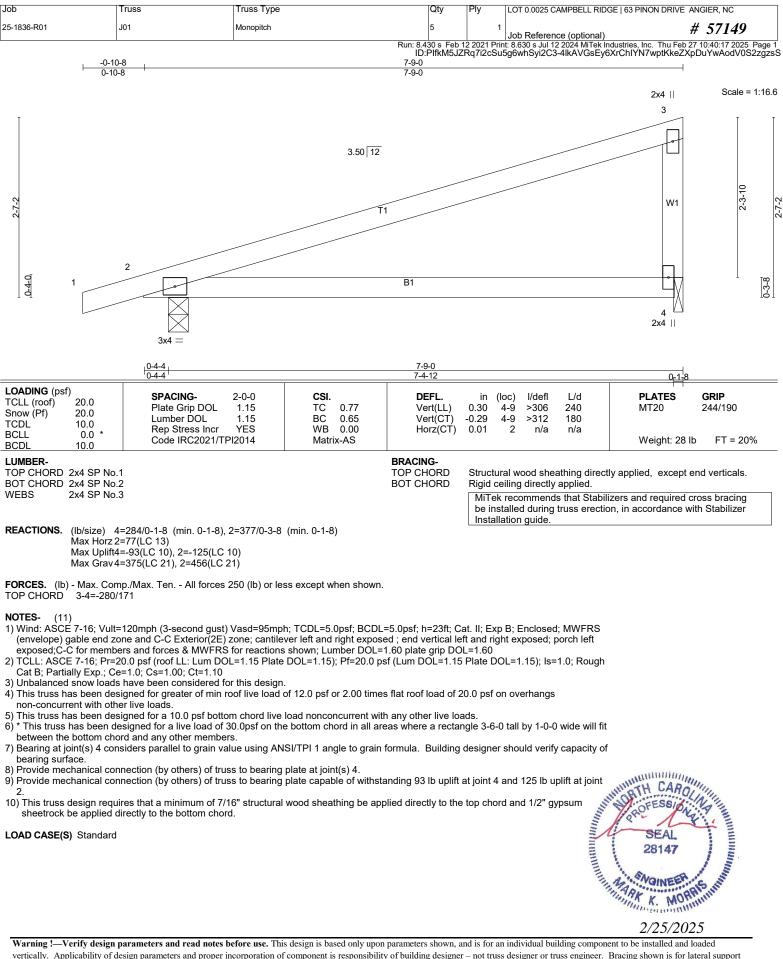
12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

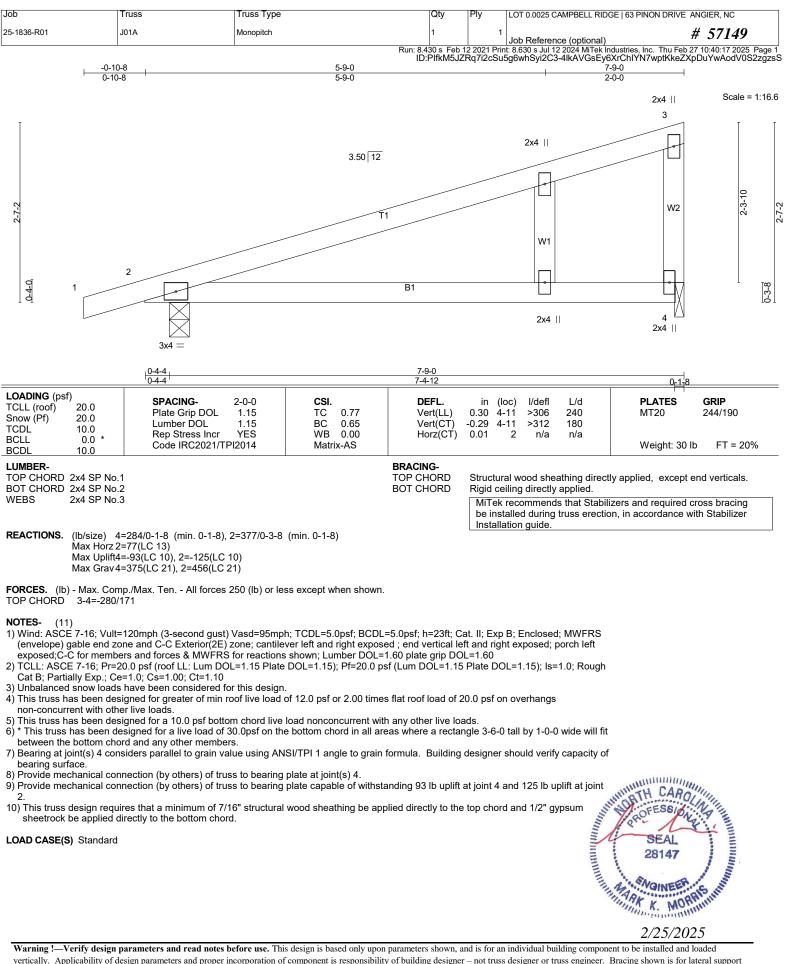
Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

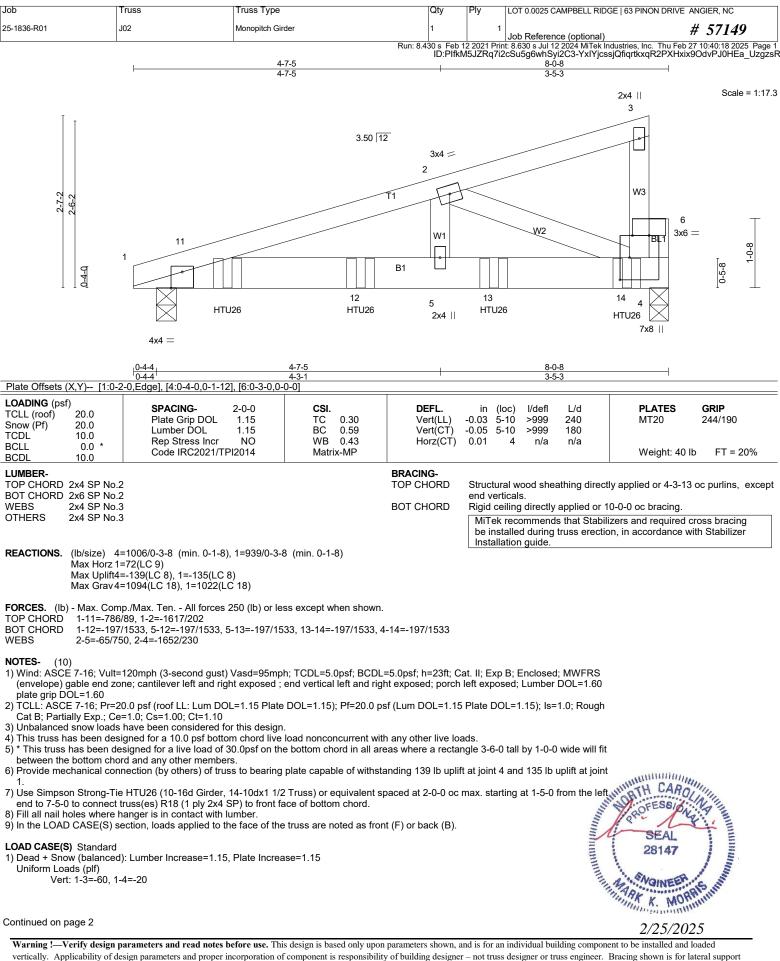
LOAD CASE(S) Standard











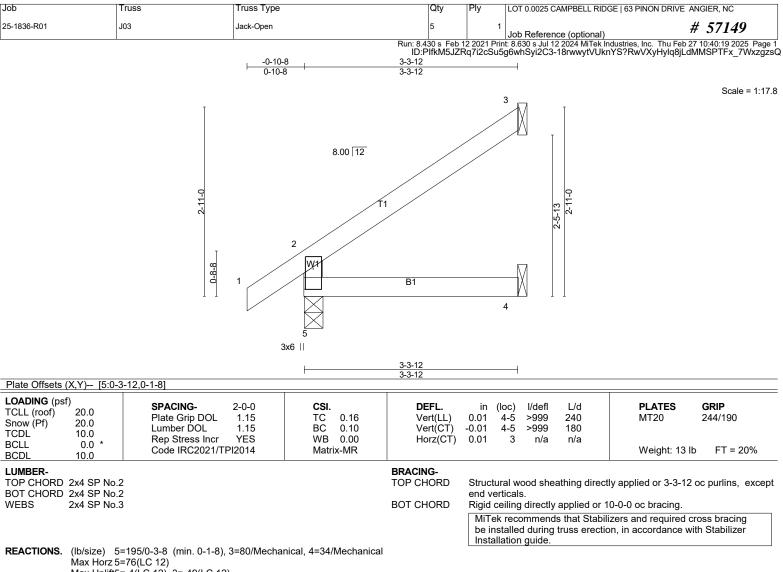
Warning 1—Verity design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

[Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE ANGIER, NC	
	25-1836-R01	J02	Monopitch Girder	1	1	Job Reference (optional) # 57149	
			Run: 8.4			t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:40:18 2025 cSu5g6whSyi2C3-YxIYjcssjQfiqrtkxqR2PXHxix9OdvPJ0HEa_	

LOAD CASE(S) Standard Concentrated Loads (Ib)

Vert: 10=-332(F) 12=-332(F) 13=-332(F) 14=-338(F)





Max Uplift5=-4(LC 12), 3=-49(LC 12) Max Grav 5=195(LC 1), 3=85(LC 20), 4=58(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

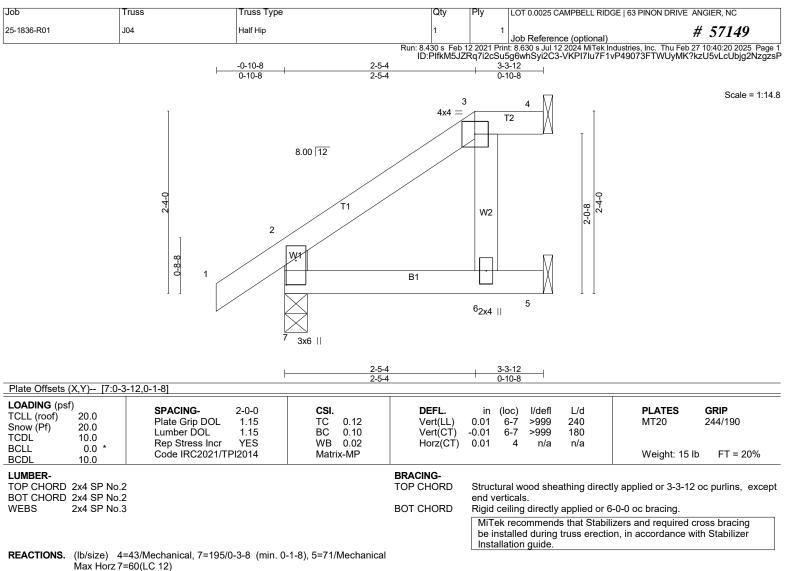
5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 5 and 49 lb uplift at joint 3.

LOAD CASE(S) Standard





Max Uplift4=-5(LC 9), 7=-14(LC 12), 5=-24(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.
 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

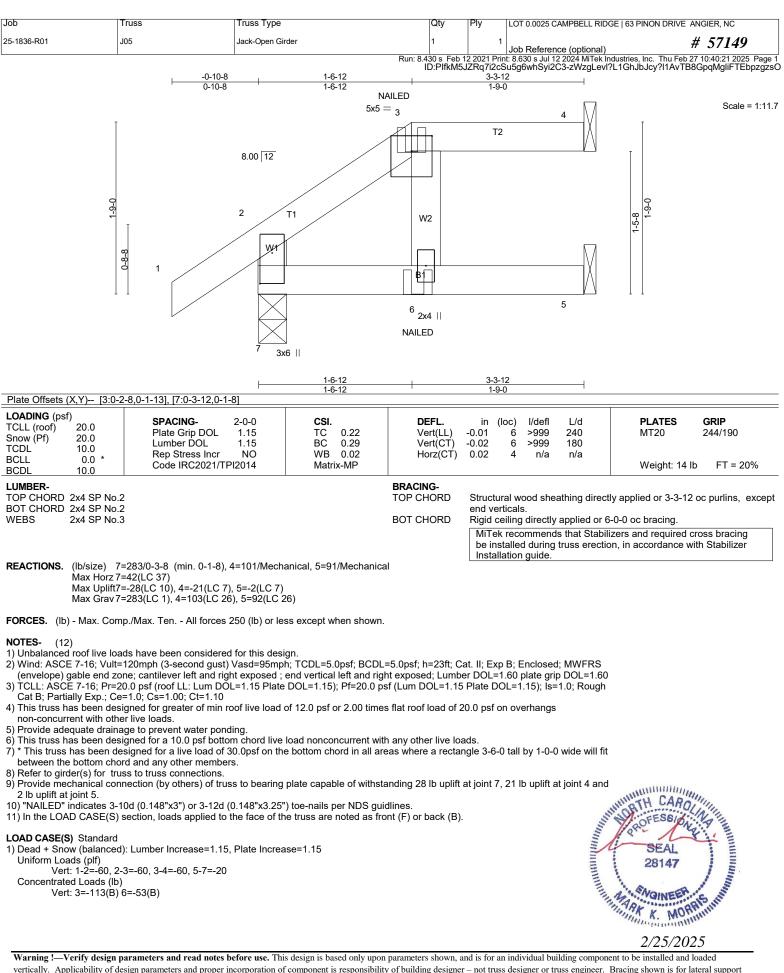
* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 4, 14 lb uplift at joint 7 and 24 lb uplift at joint 5.

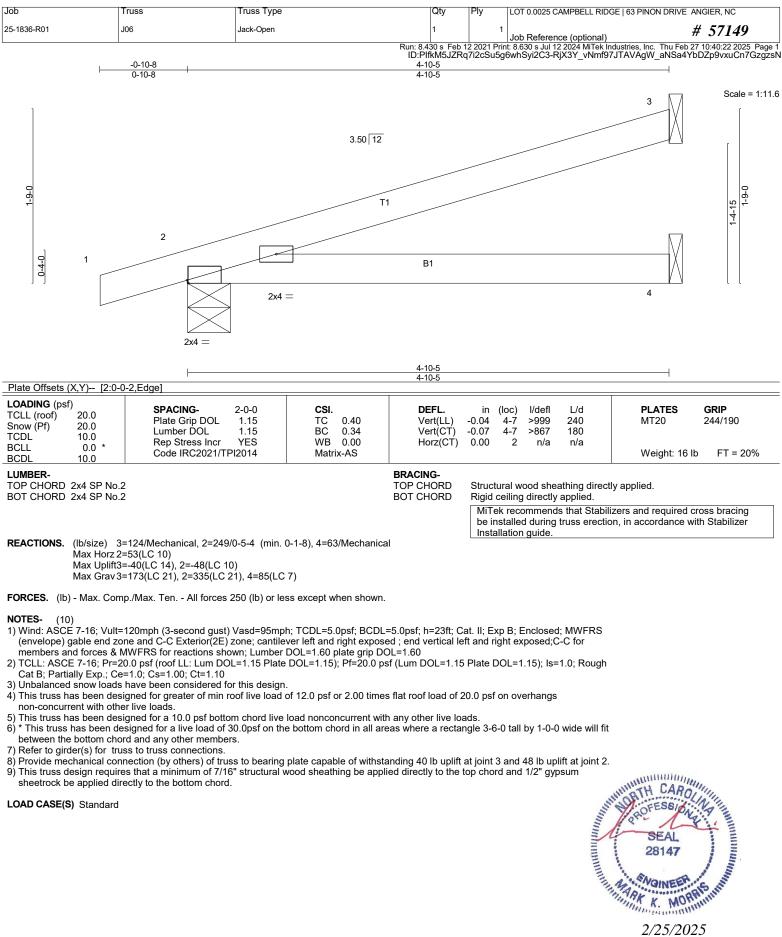
LOAD CASE(S) Standard



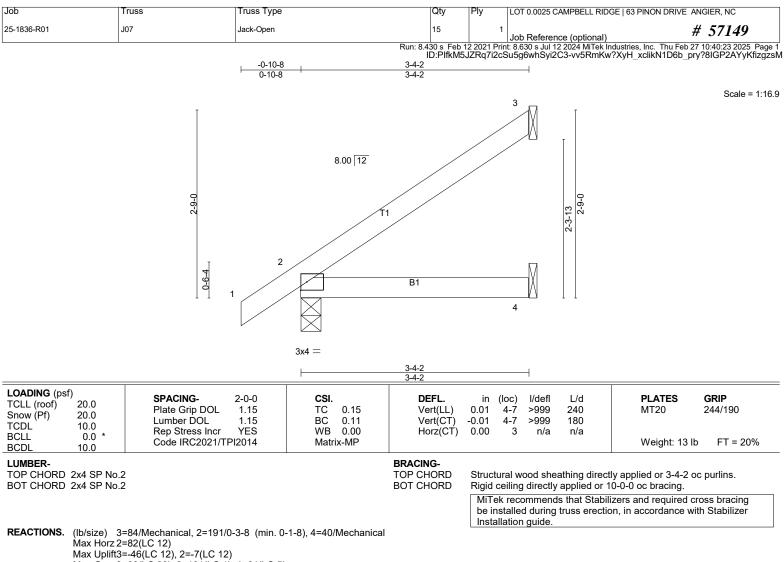


Concentrated Loads (lb) Vert: 3=-113(B) 6=-53(B)

^{2/25/2025}







Max Grav 3=88(LC 20), 2=191(LC 1), 4=61(LC 5)

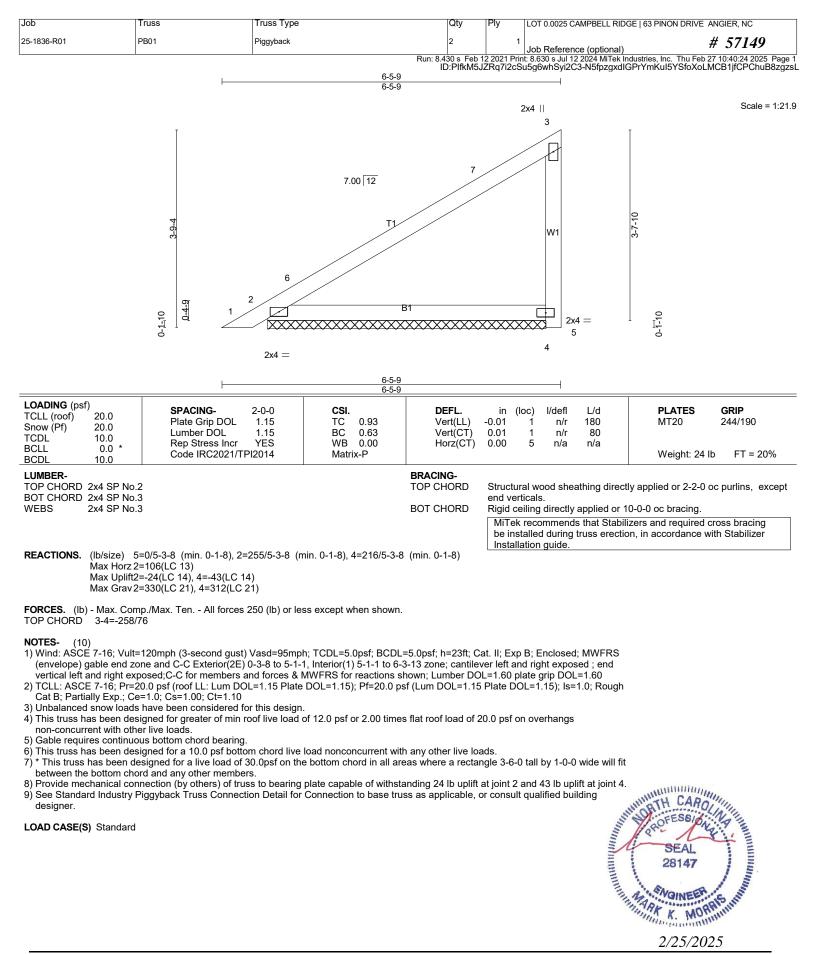
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

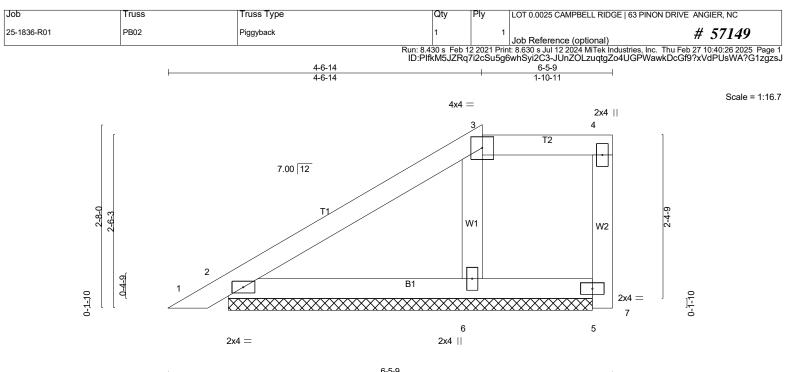
NOTES- (8

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 3 and 7 lb uplift at joint 2.

LOAD CASE(S) Standard







			5-9		4
		6-	5-9		1
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL TCDL 0.0 BCLL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.43 BC 0.17 WB 0.05	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d 0.00 1 n/r 180 0.01 1 n/r 80 0.00 5 n/a n/a	PLATES GRIP MT20 244/190
BCDL 10.0	Code IRC2021/TPI2014	Matrix-P			Weight: 25 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2			BRACING- TOP CHORD		tly applied or 6-5-9 oc purlins, except
BOT CHORD2x4 SP No.3WEBS2x4 SP No.3			BOT CHORD	end verticals. Rigid ceiling directly applied or 6	6-0-0 oc bracing.
					lizers and required cross bracing on, in accordance with Stabilizer

REACTIONS. (lb/size) 7=0/5-3-8 (min. 0-1-8), 2=171/5-3-8 (min. 0-1-8), 5=59/5-3-8 (min. 0-1-8), 6=240/5-3-8 (min. 0-1-8) Max Horz 2=71(LC 13) Max Uplift2=-22(LC 14), 5=-15(LC 10), 6=-18(LC 14)

Max Grav 2=280(LC 36), 5=98(LC 35), 6=298(LC 36)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(12)

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

- 6) Provide adequate drainage to prevent water ponding.
- 7) Gable requires continuous bottom chord bearing.

8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

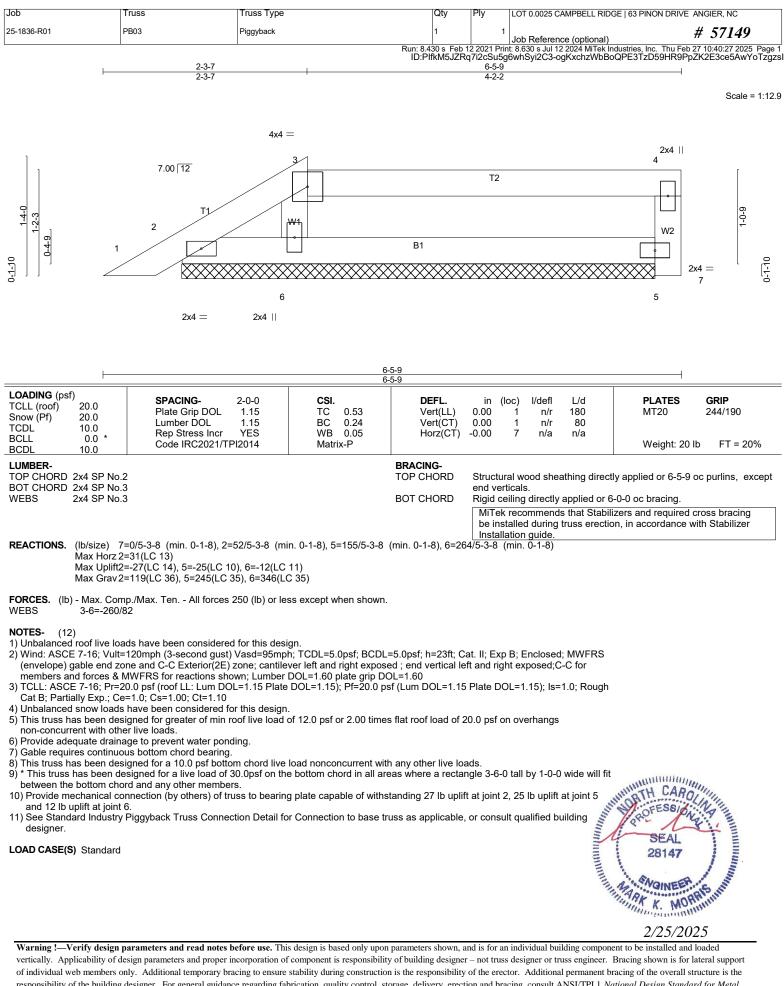
* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 9) between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 2, 15 lb uplift at joint 5 and 18 lb uplift at joint 6.

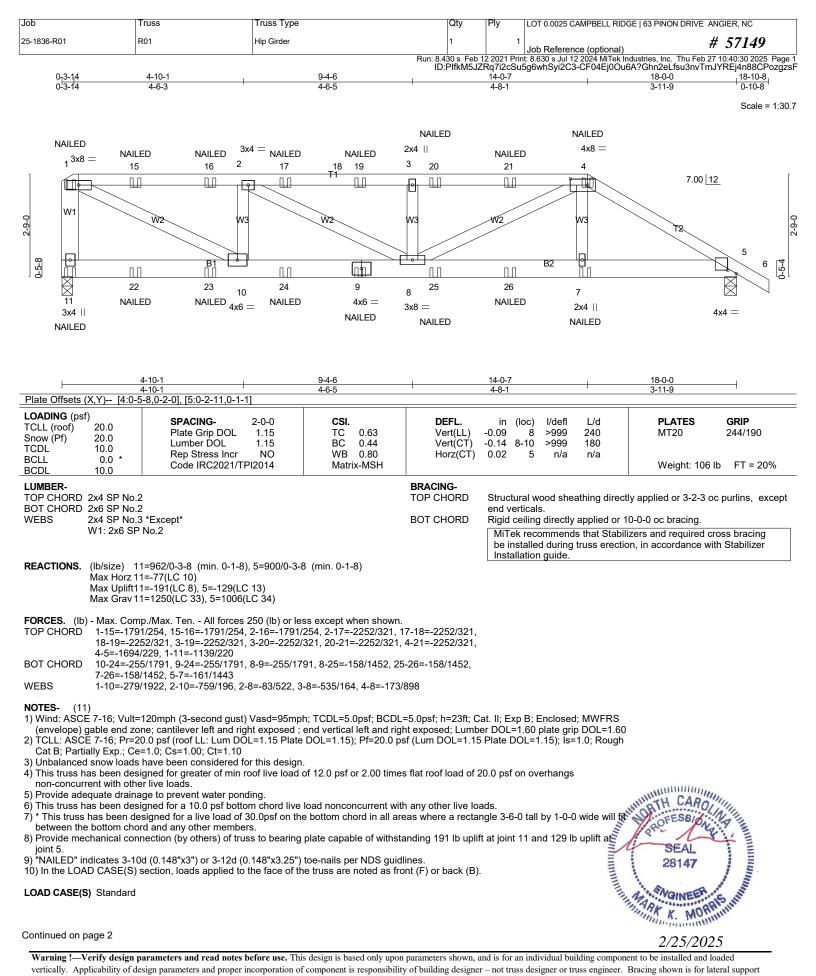
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIV	/E ANGIER, NC
25-1836-R01	R01	Hip Girder	1	1	Job Reference (optional)	# 57149
			Run: 8.430 s Feb 12	2 2021 Prir	t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu F	eb 27 10:40:31 2025 Page 2

n: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:40:31 2025 Page 2 ID:PlfkM5JZRq7i2cSu5g6whSyi2C3-gRaSR300fQlrurMEC3A5RHJ4DAenAhzE0oumxEzgzsE

LOAD CASE(S) Standard

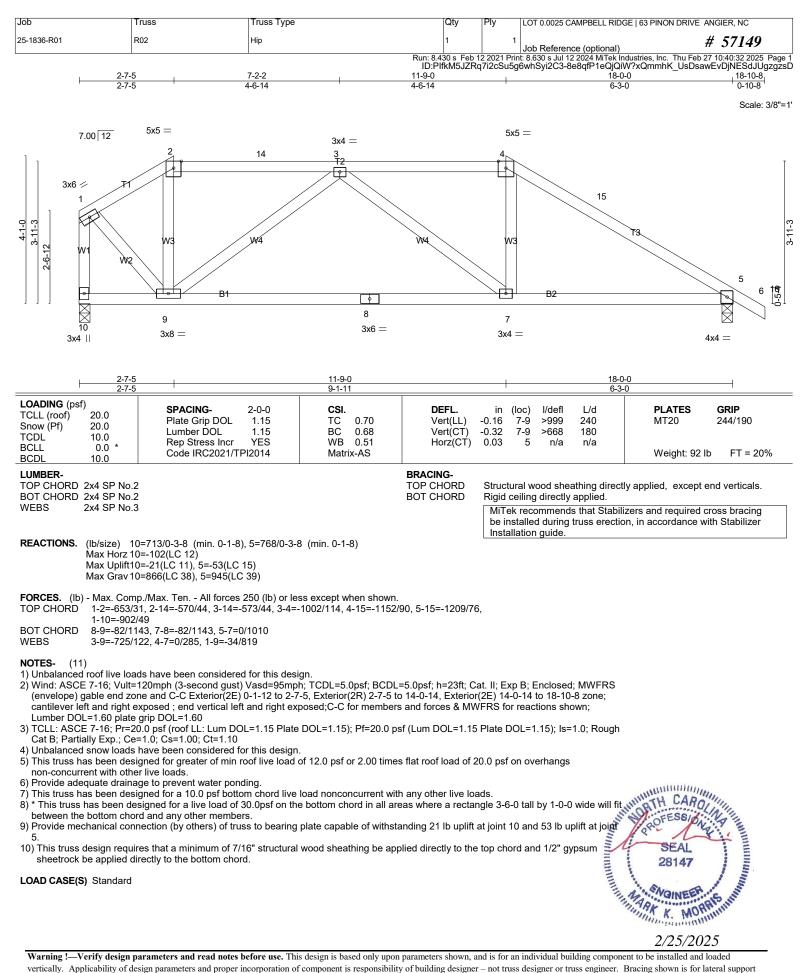
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

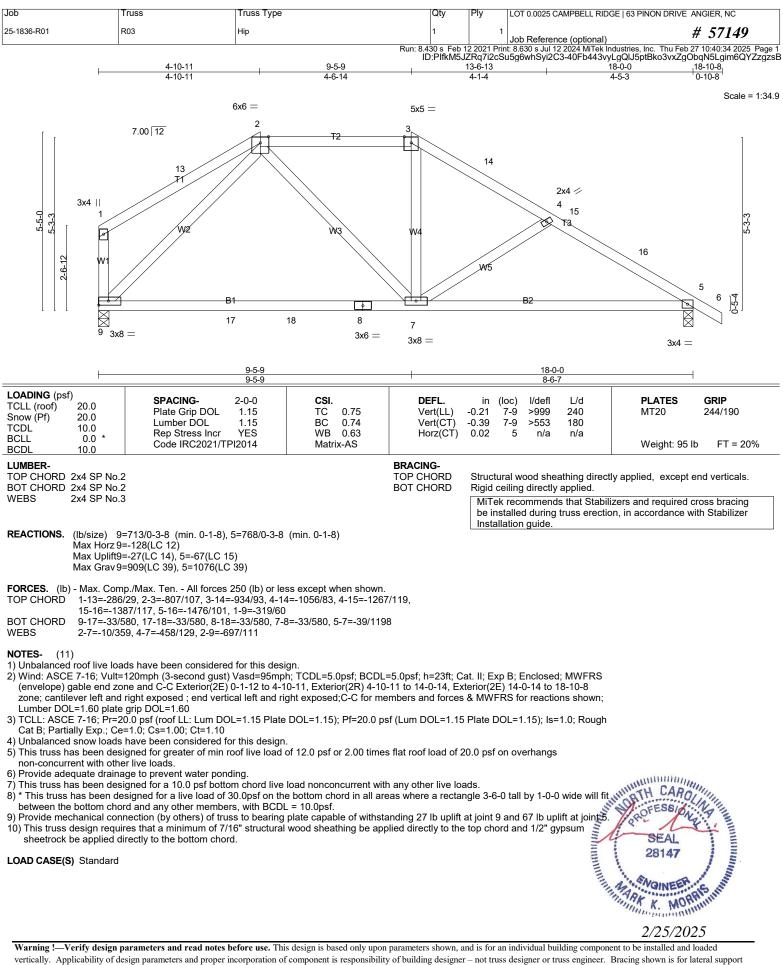
Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 11-12=-20

Concentrated Loads (lb)

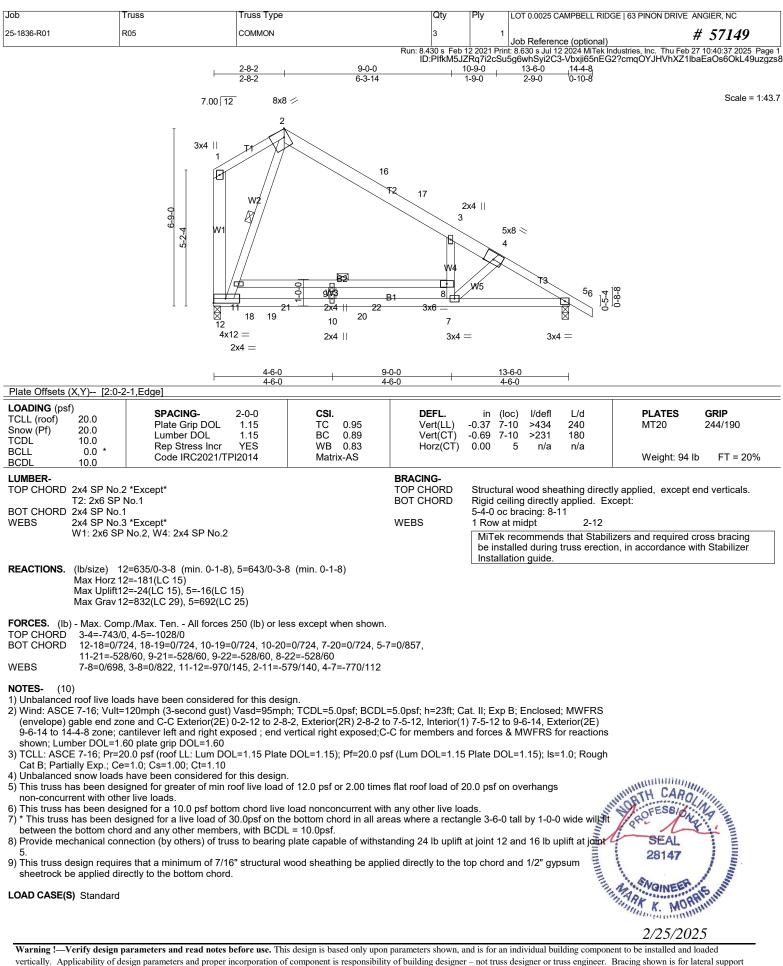
Vert: 1=-50(B) 4=-24(B) 11=-28(B) 9=-20(B) 7=-20(B) 15=-24(B) 16=-24(B) 17=-24(B) 19=-24(B) 20=-24(B) 21=-24(B) 22=-20(B) 23=-20(B) 24=-20(B) 25=-20(B) 26=-20(B) 26=-



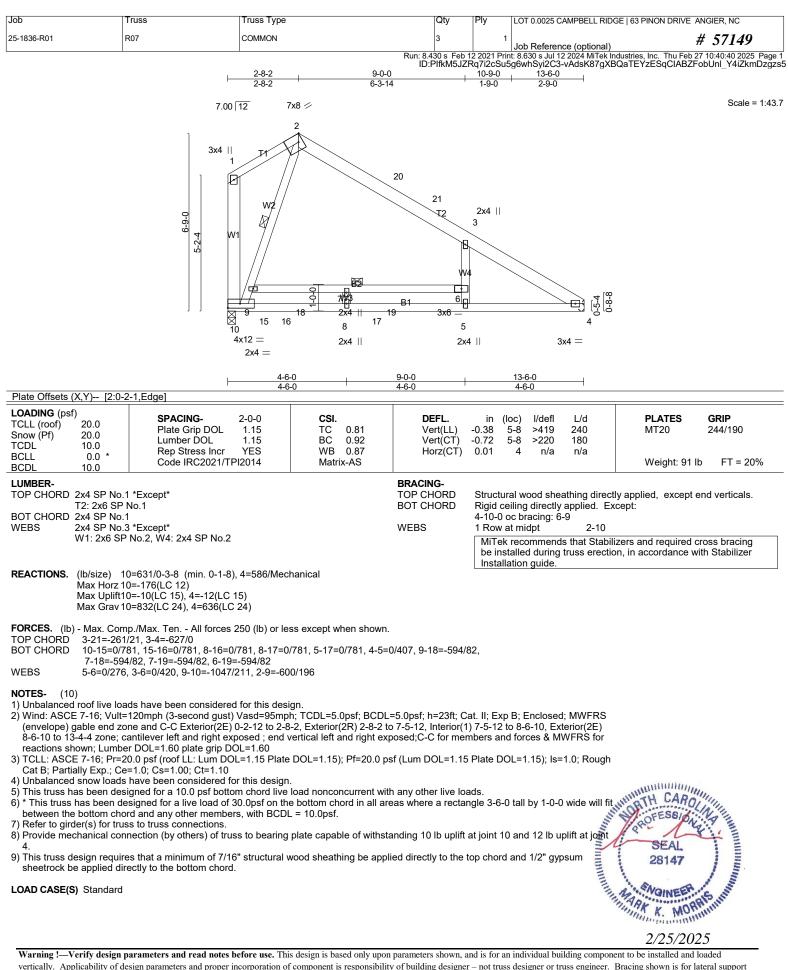


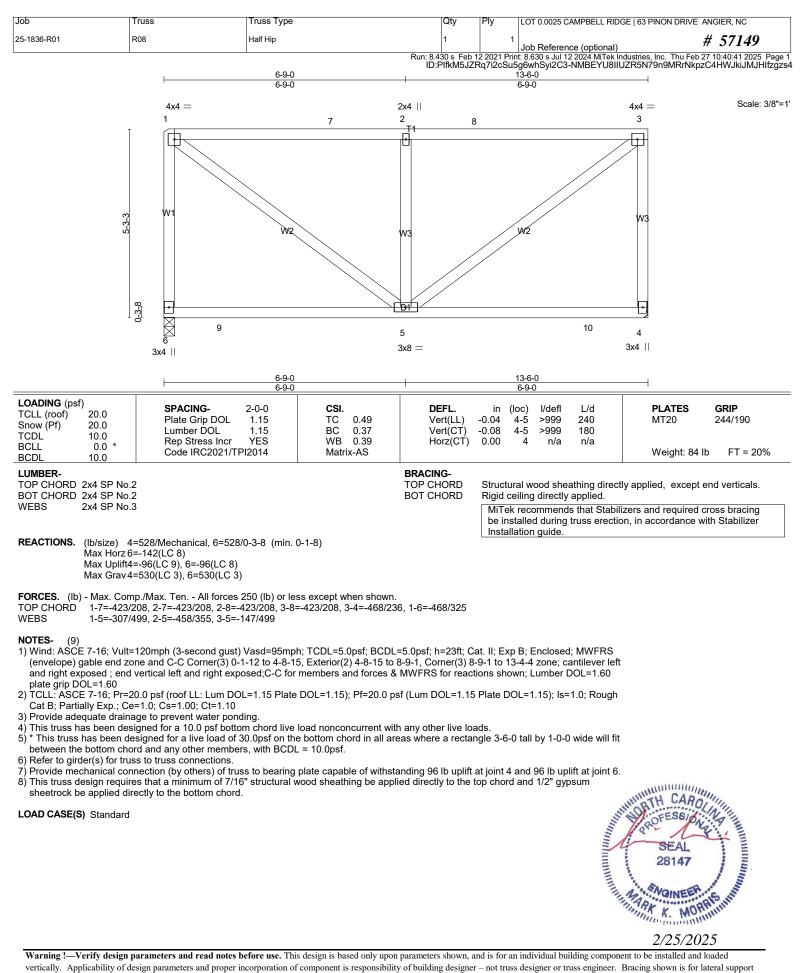


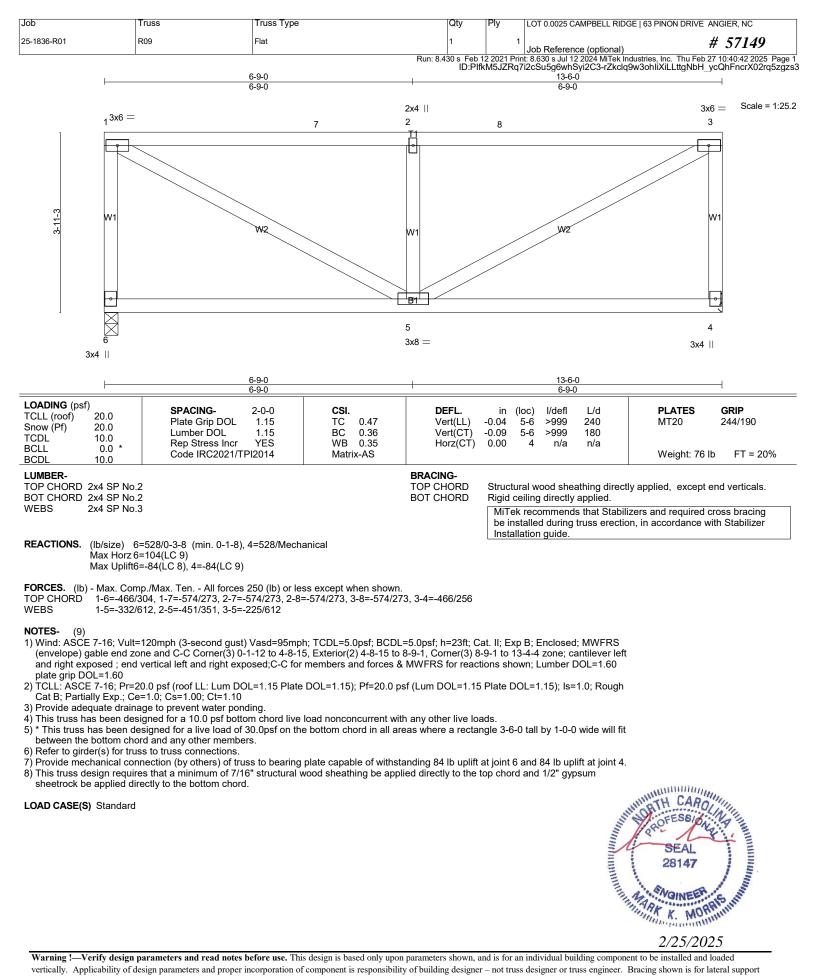
Job 25-1836-R01	Truss R04		Truss Type Common		Qty 3 Run: 8.430 s Feb ID:PlfkM5JZRc	1 Jot	Reference (optional	DGE 63 PINON DRIVE AN) Industries, Inc. Thu Feb 27 /w8_cFB?cmG8K1vsBH	[£] 57149
		F	7-2-2 7-2-2		12-5-2 5-2-15		18-0-0 5-6-14	18-10-8 0-10-8	
				5x6 = 2					Scale = 1:42.5
		7.00 1	2 13 14			14 ^{2x4} ⁄⁄	,		
0-6-9	4x4	12	W2			14 3 14 12			
	2-6-12	1 2 W1		w3	W4		15		
			B1			B2		45 F	
		8 3x8 =	16	7 3x6 =	6 3x4 =			⊠	
		<u> </u>	9-0-0			18-0			
Plate Offsets (X,Y) [4:0	-0-15,0	-1-8]	9-0-0			9-0	-0	, 	
LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0		SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TI	2-0-0 1.15 1.15 YES PI2014	CSI. TC 0.77 BC 0.74 WB 0.29 Matrix-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.24 6-8 -0.35 6-8 0.02 4	l/defl L/d >879 240 >615 180 n/a n/a	PLATES MT20 Weight: 93 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No BOT CHORD 2x4 SP No WEBS 2x4 SP No	.2	ept*			BRACING- TOP CHORD BOT CHORD WEBS		directly applied.	ctly applied, except er	nd verticals.
W1: 2x4 S	P No.2						d during truss erect	ilizers and required cr ion, in accordance wit	
	8=-155 4=-75(L		4)	n. 0-1-8)			<u>.</u>		
1-8=-355 BOT CHORD 8-16=0/5	8/74, 2- 5/113 64, 7-1		=-873/76, 3-15=- 4, 4-6=-21/944	xcept when shown. 1009/111, 4-15=-11	33/97,				
NOTES- (10) 1) Unbalanced roof live lo 2) Wind: ASCE 7-16; Vull (envelope) gable end z Exterior(2E) 14-0-14 to MWFRS for reactions s 3) TCLL: ASCE 7-16; Pre- Cat B; Partially Exp.; C 4) Unbalanced snow load	t=120m cone and 18-10- shown; 20.0 ps ce=1.0; ls have	ph (3-second gust) d C-C Exterior(2E) 8 zone; cantilever Lumber DOL=1.60 f (roof LL: Lum DC Cs=1.00; Ct=1.10 been considered for	Vasd=95mph; T 0-1-12 to 4-11-6, left and right expo plate grip DOL= DL=1.15 Plate DO or this design.	Exterior(2R) 4-11-6 osed ; end vertical le 1.60 L=1.15); Pf=20.0 ps	to 11-11-12, Inte ft and right expos f (Lum DOL=1.15	rior(1) 11-11- ed;C-C for m Plate DOL=1	12 to 14-0-14, embers and forces .15); Is=1.0; Rough	1	
 5) This truss has been de non-concurrent with ott 6) This truss has been de 7) * This truss has been de 7) * This truss has been de 8) Provide mechanical co 9) This truss design requires the sheetrock be applied de LOAD CASE(S) Standard 	her live esigned lord and onnectio ires that lirectly to	loads. for a 10.0 psf botto d for a live load of any other membe n (by others) of tru t a minimum of 7/1	om chord live load 30.0psf on the bo rs, with BCDL = 1 ss to bearing plat 6" structural wood	I nonconcurrent with ttom chord in all are 0.0psf. e capable of withsta	any other live loa as where a rectar nding 75 lb uplift :	ads. ngle 3-6-0 tall at joint 4 and	Ū	SEAL 28147 2/25/202	A Statement
LOND ONDE(D) Standard	4						(III)	ARK	Summer
								2/25/202	5
Warning !—Verify design	-				-		vidual building compo	nent to be installed and lo Bracing shown is for lat	adea

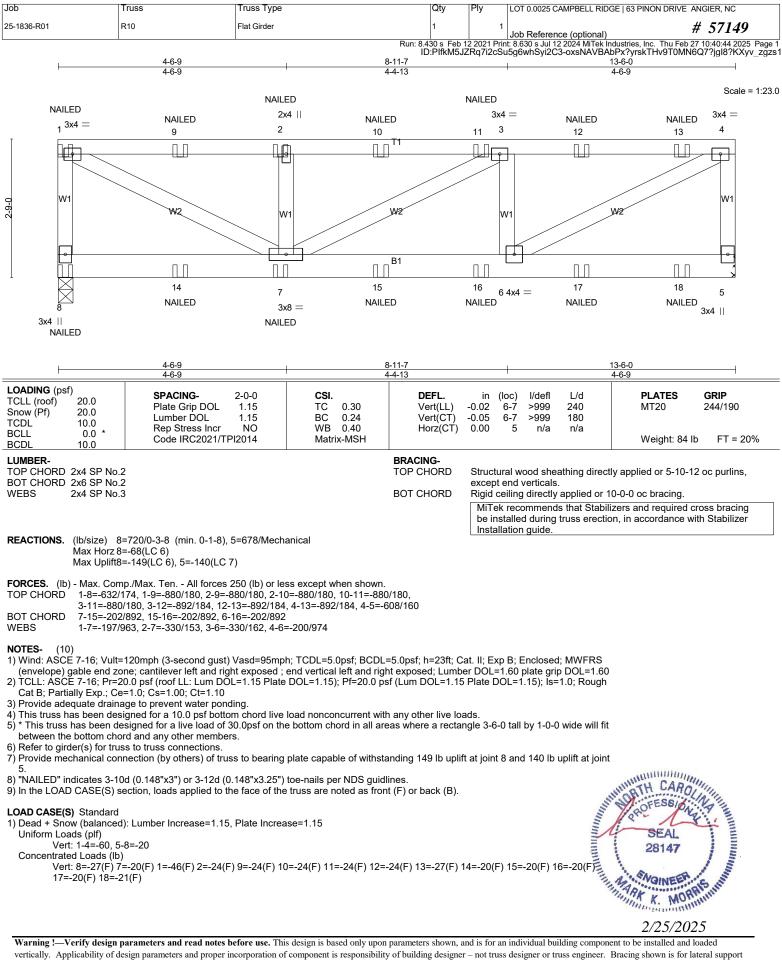


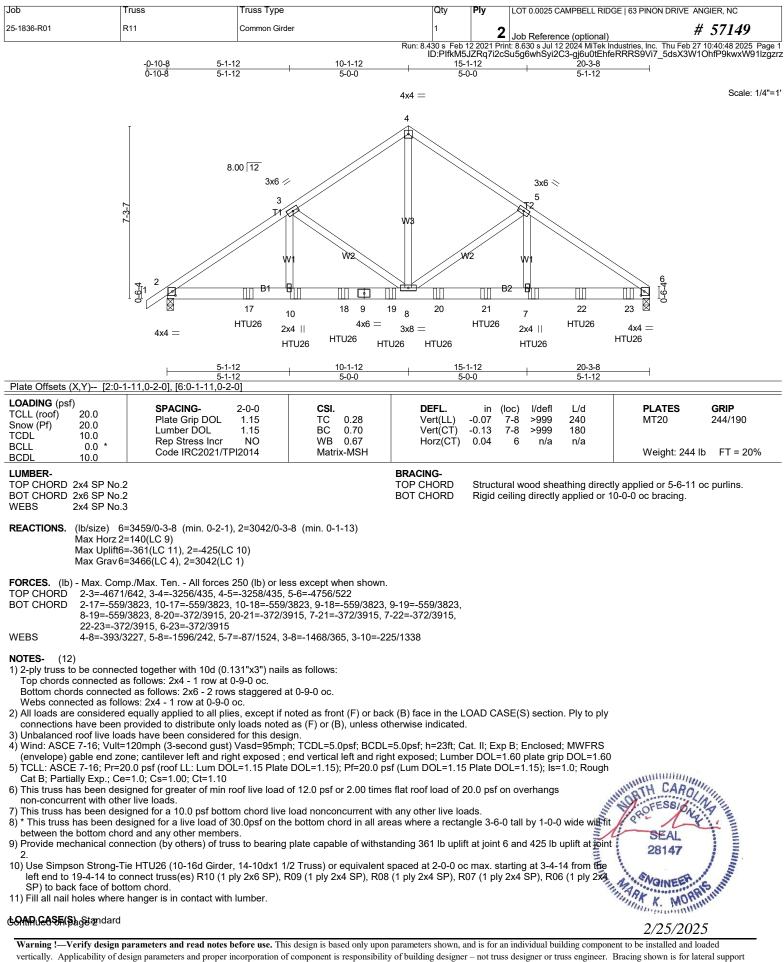
Job	Tru	SS	Truss Type		Qty	Ply	LOT 0.0025 CAMPBELL RID	IGE 63 PINON DRIVE ANGIER, NC
25-1836-R01	R06	i	COMMON		3	1	Ich Poforance (antional)	# 57149
					Run: 8.430 s Feb	12 2021 Prii 17 2021 Prii 17 8 a 7 i 2 c 9	Job Reference (optional) ht: 8.630 s Jul 12 2024 MiTek I Su5g6wbSvi2C3-R 31 I7o7	ndustries, Inc. Thu Feb 27 10:40:39 2025 Page 1mtljr4zmgkJzmyfOIPFs2HNPr2qBDnzgzs
			2-4-10	8-8-8 6-3-14		10-5-8	13-2-8 2-9-0	
		7.00		0-3-1-	•	1-5-0	2-3-0	Scale = 1:43.
		7.00	12 8x8 🖉					Stale - 1.45.
		3x4 	2 1 1 W2 W2		20 21 T2	2x4 3		
		6 5.4-5	$ \begin{array}{c} $	2x4 1 8 2x4 2x4 2x4	B B1 6 9 3x6 - 5 2x4	=	4 3x4 =	
			4-2-8 4-2-8		8-8-8 4-6-0		13-2-8 4-6-0	
	(X,Y) [2:0-2-1	,Edge]						
LOADING (ps TCLL (roof)	20.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC 0.82	DEFL. Vert(LL)	in (l -0.40	oc) l/defl L/d 6-7 >396 240	PLATES GRIP MT20 244/190
Snow (Pf) TCDL	20.0 10.0	Lumber DOL Rep Stress Incr	1.15 YES	BC 0.94 WB 0.96	Vert(CT) Horz(CT)		6-7 >208 180 4 n/a n/a	
BCLL BCDL	0.0 * 10.0	Code IRC2021/T		Matrix-AS		0.01	i ii/a ii/a	Weight: 86 lb FT = 20%
	Max Horz 10≕	.1 Except* Except* P No.3 622/Mechanical, 4=57 -177(LC 12)			BRACING- TOP CHORD BOT CHORD WEBS	Rigid ce 6-0-0 oc 1 Row a MiTek be inst	eiling directly applied. Ex bracing: 6-9 at midpt 2-10 recommends that Stabi	tly applied, except end verticals. xcept: lizers and required cross bracing on, in accordance with Stabilizer
FORCES. (IL TOP CHORD BOT CHORD WEBS	Max Grav 10=) - Max. Comp./ 3-4=-593/0 10-15=0/723 7-18=-563/7	-12(LC 15), 4=-12(LC 826(LC 24), 4=622(LC Max. Ten All forces , 15-16=0/723, 8-16=0 3, 7-19=-563/73, 6-19 -6=0/428, 9-10=-1058	: 24) 250 (lb) or less e> /723, 8-17=0/723, =-563/73	5-17=0/723, 4-5=		73,		
NOTES- (1 1) Unbalance 2) Wind: ASC (envelope) 8-6-10 to 1 reactions s 3) TCLL: ASC Cat B; Part 4) Unbalance 5) This truss between th 7) Refer to gir 8) Provide me 4. 9) This truss of	0) d roof live loadss E 7-16; Vult=12 gable end zone 3-4-4 zone; can hown; Lumber L E 7-16; Pr=20.0 ially Exp.; Ce=1 d snow loads ha has been design b has been design b has been design b has been design chance and the source base bottom chord der(s) for truss bechanical conne design requires be applied direct	have been considere 0mph (3-second gust and C-C Exterior(2E) tilever left and right ex OCL=1.60 plate grip D 0 psf (roof LL: Lum DC .0; Cs=1.00; Ct=1.10	d for this design. Vasd=95mph; TC 0-5-4 to 2-8-2, Ex posed ; end vertic OL=1.60 VL=1.15 Plate DOI or this design. Or this design. Or this design. Or this design. So the bot rs, with BCDL = 1 ss to bearing plate 6" structural wood	CDL=5.0psf; BCDL terior(2R) 2-8-2 to al left and right ex _=1.15); Pf=20.0 p nonconcurrent wit tom chord in all ar 0.0psf. e capable of withst	7-5-12, Interior(1) posed;C-C for mer sf (Lum DOL=1.15 h any other live loa eas where a rectar anding 12 lb uplift	7-5-12 to nbers and Plate DC ads. ngle 3-6-0 at joint 10	d forces & MWFRS for DL=1.15); Is=1.0; Rough) tall by 1-0-0 wide will fil	











Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE ANGIER, NC	
25-1836-R01	R11	Common Girder	1	2	Job Reference (optional) # 57149	
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:40:48 2025 Pa Su5g6whSyi2C3-gj6u0tEhfeRRRS9Vi7_5dsX3W1OhfP9kwxW91Iz	

LOAD CASE(S) Standard

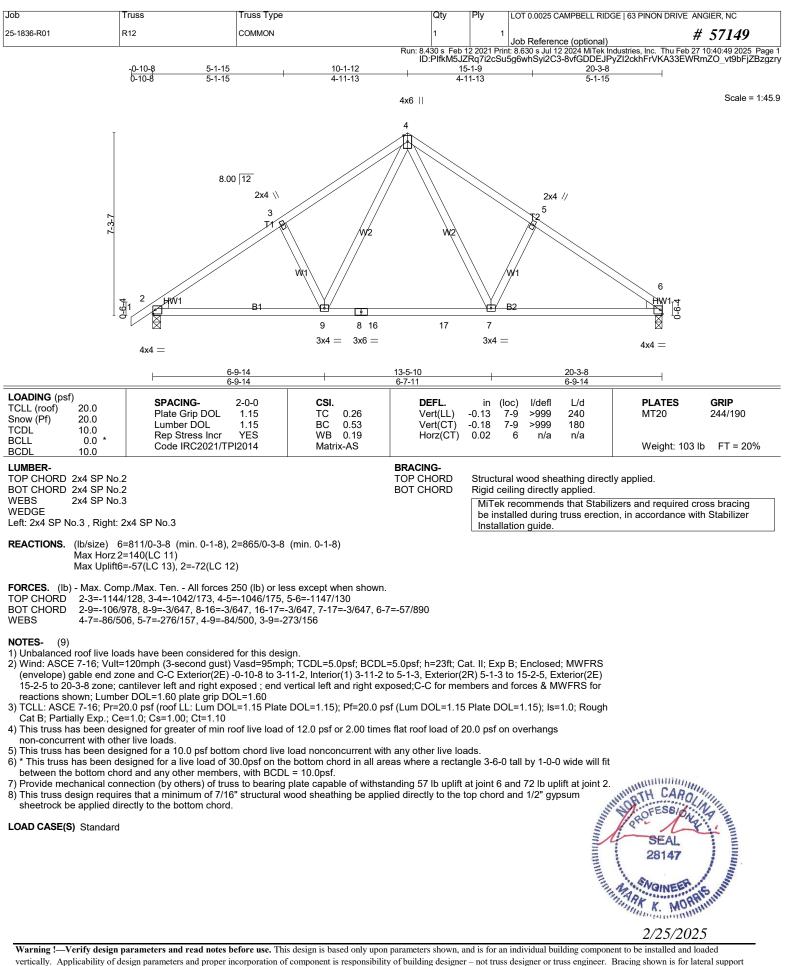
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

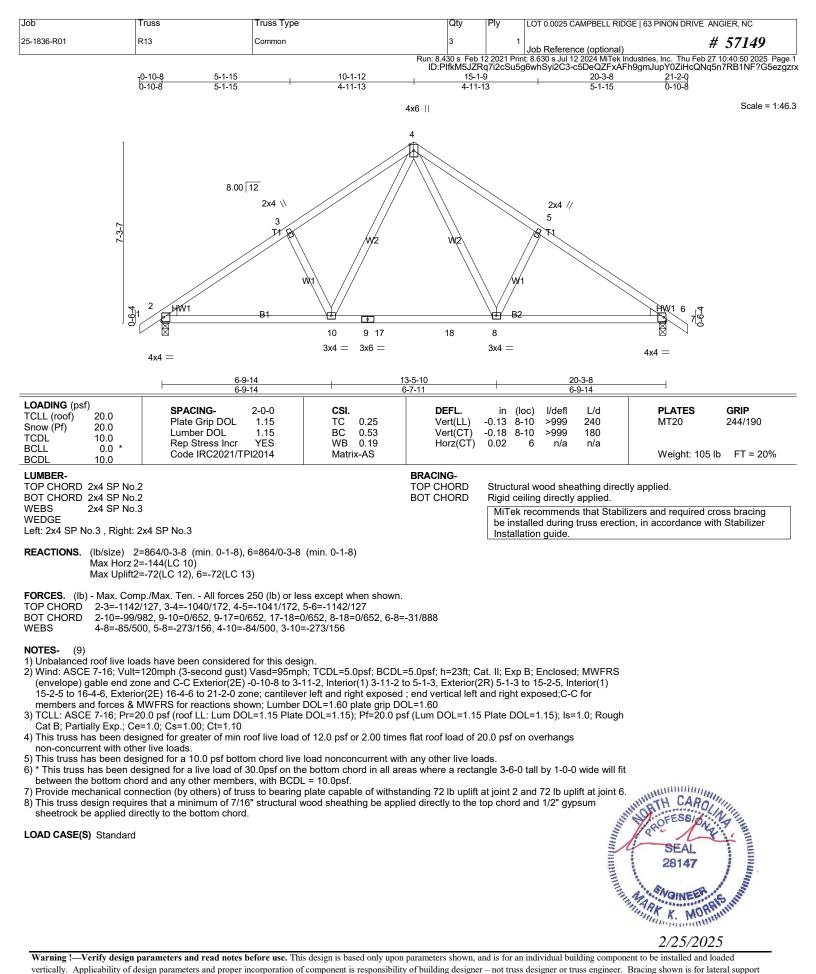
Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 11-14=-20

Concentrated Loads (lb)

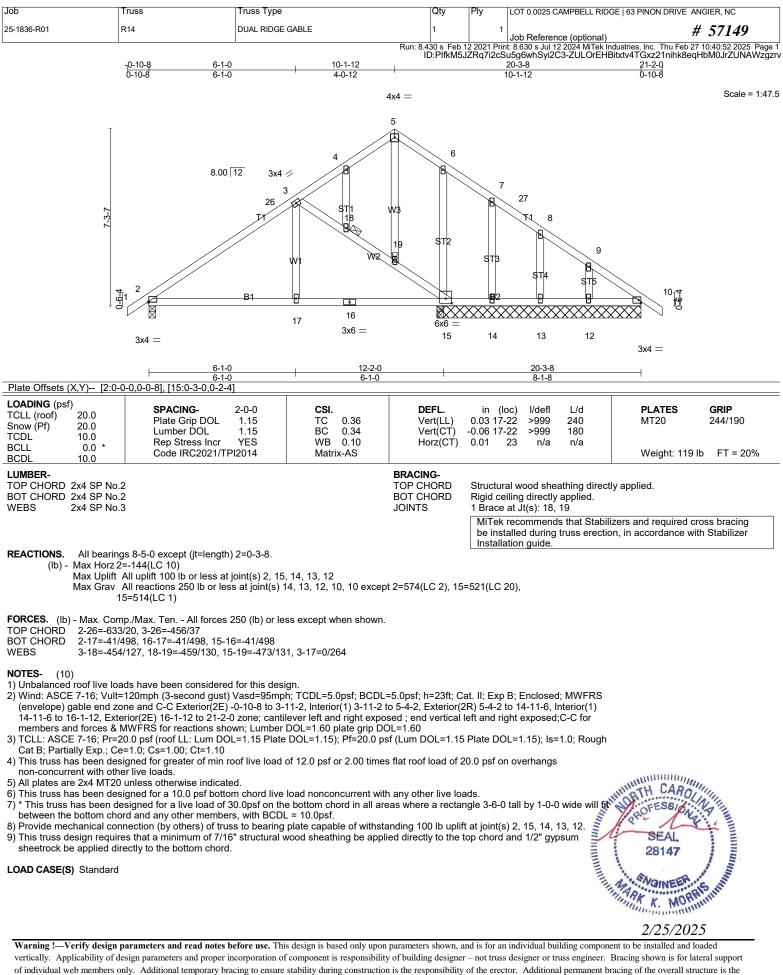
Vert: 7=-517(B) 10=-508(B) 17=-658(B) 18=-508(B) 19=-533(B) 20=-533(B) 21=-533(B) 22=-517(B) 23=-518(B)



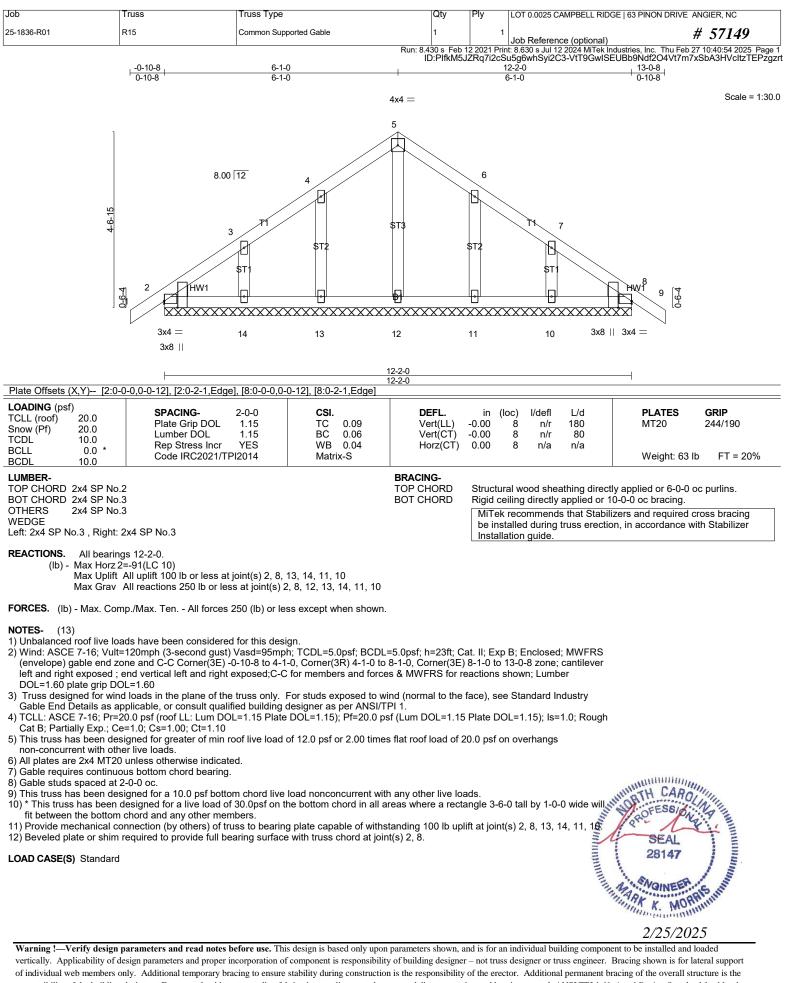




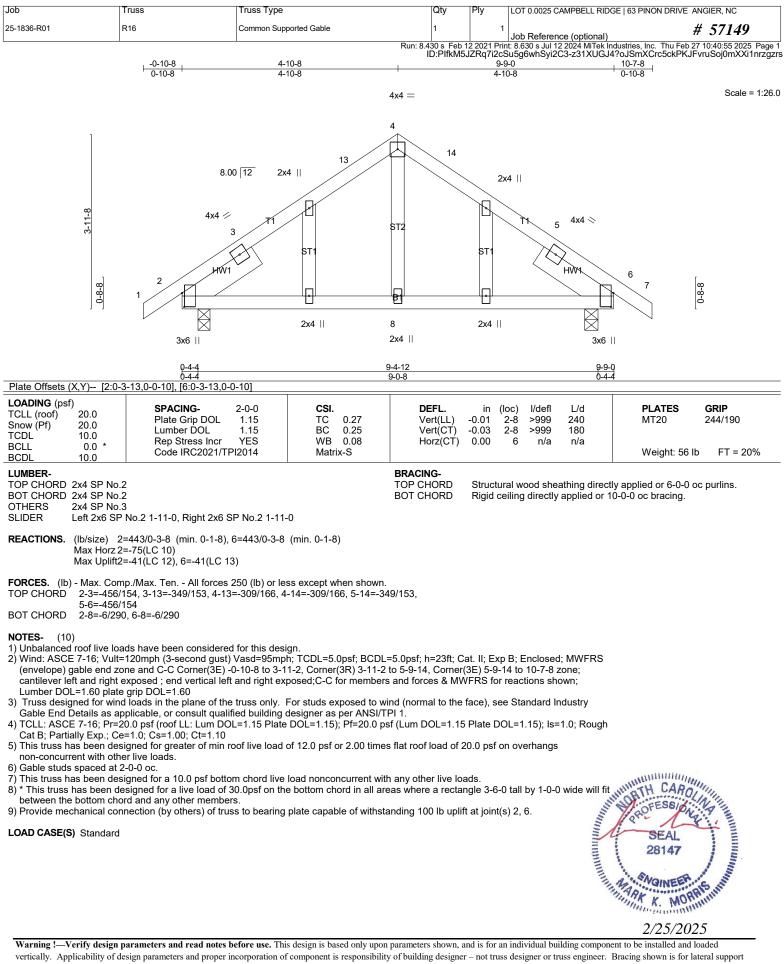
2/25/2025



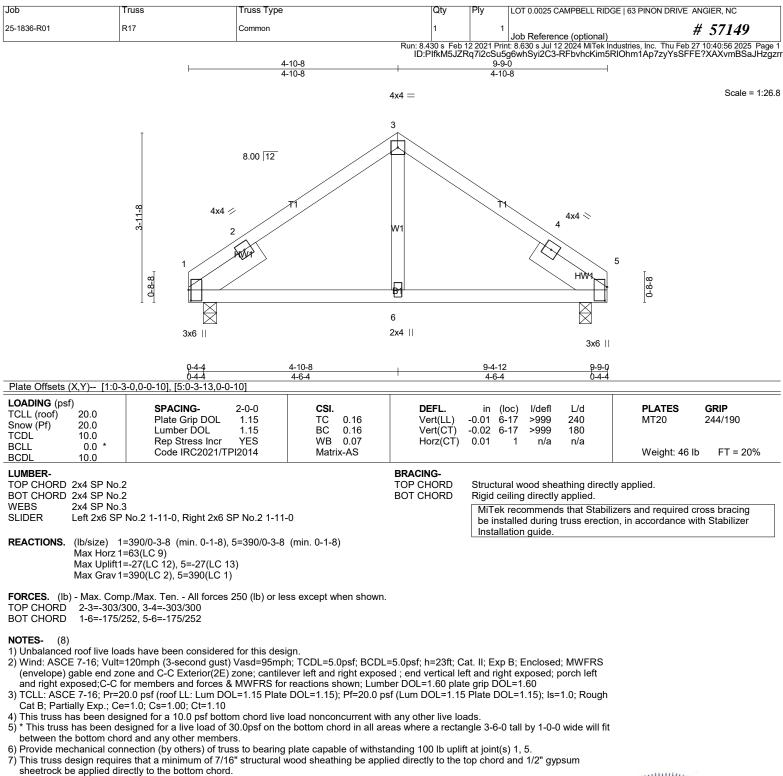
of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



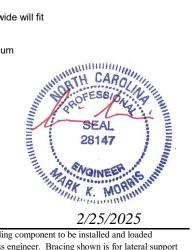
of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

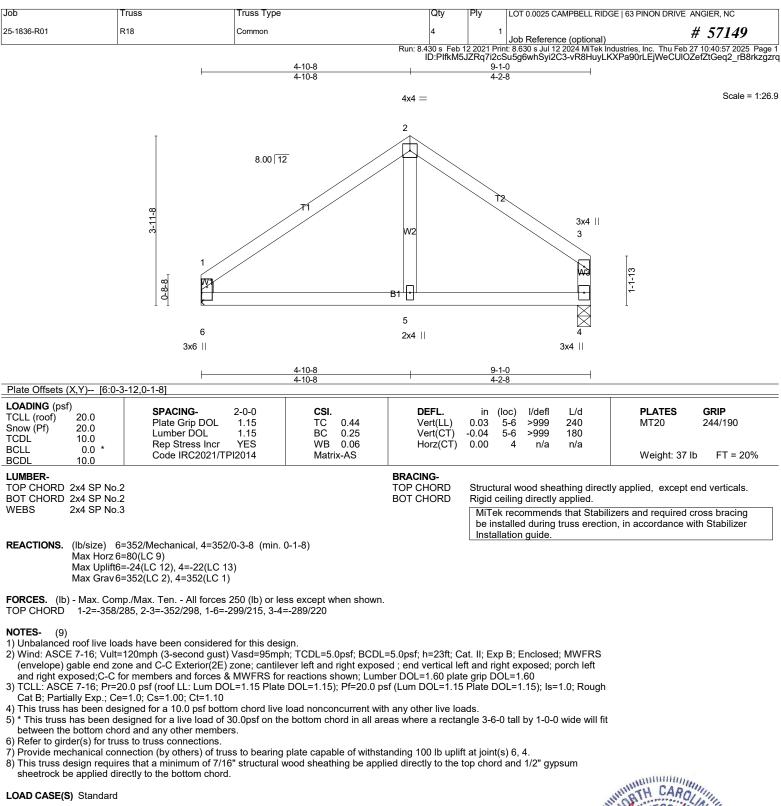


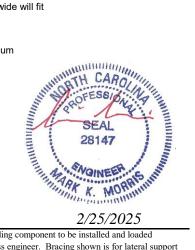
vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

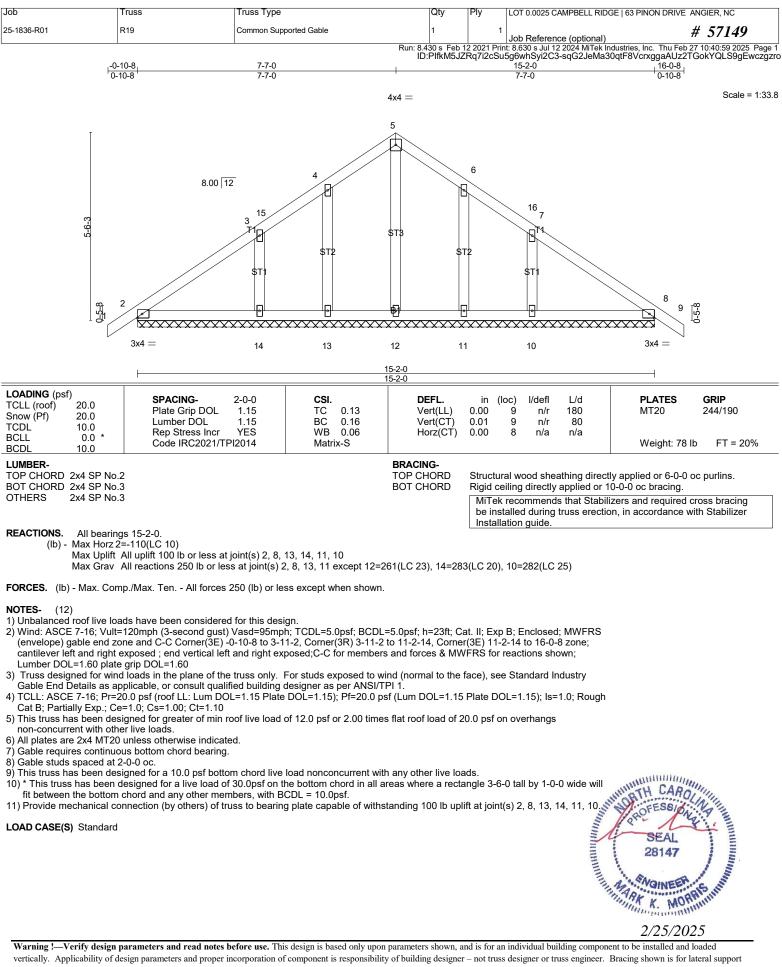


LOAD CASE(S) Standard

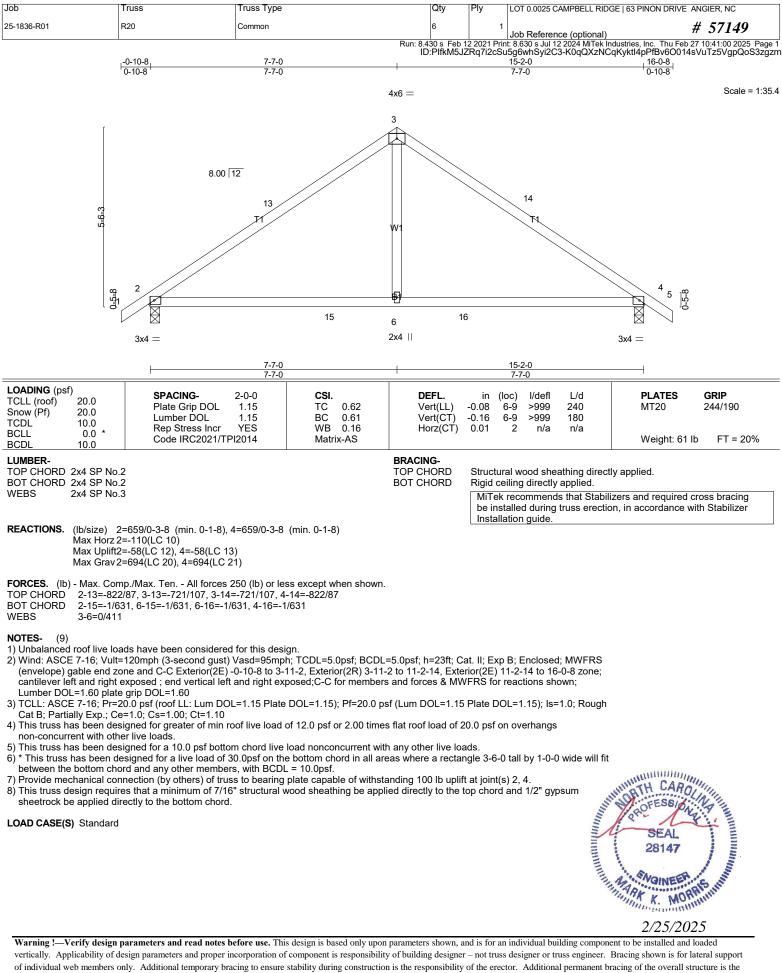




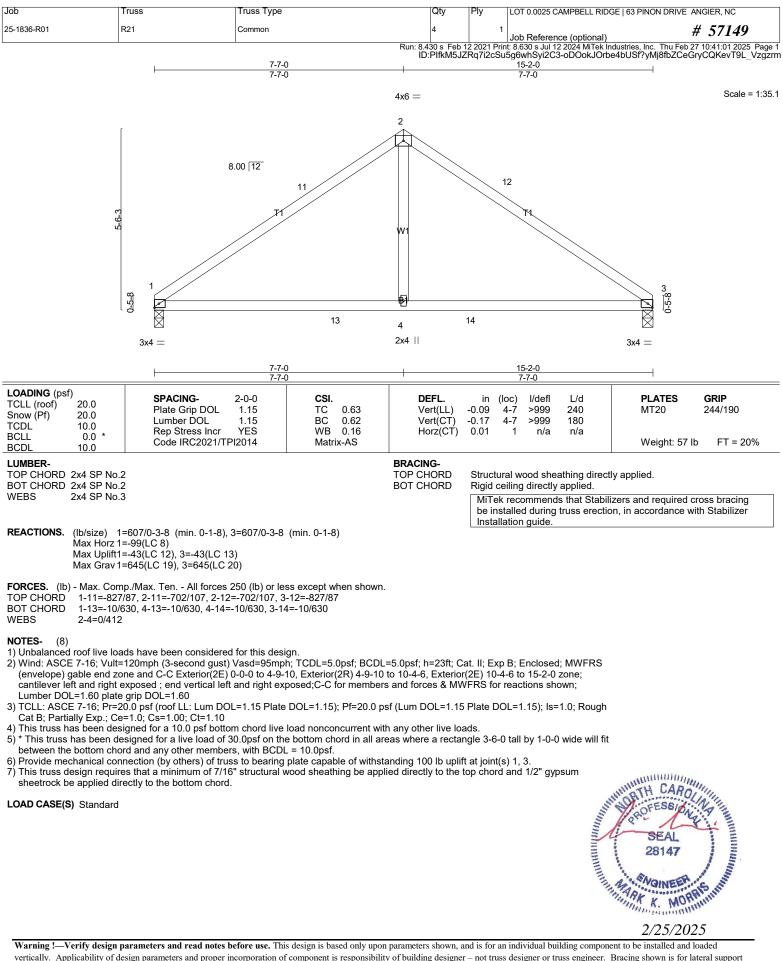


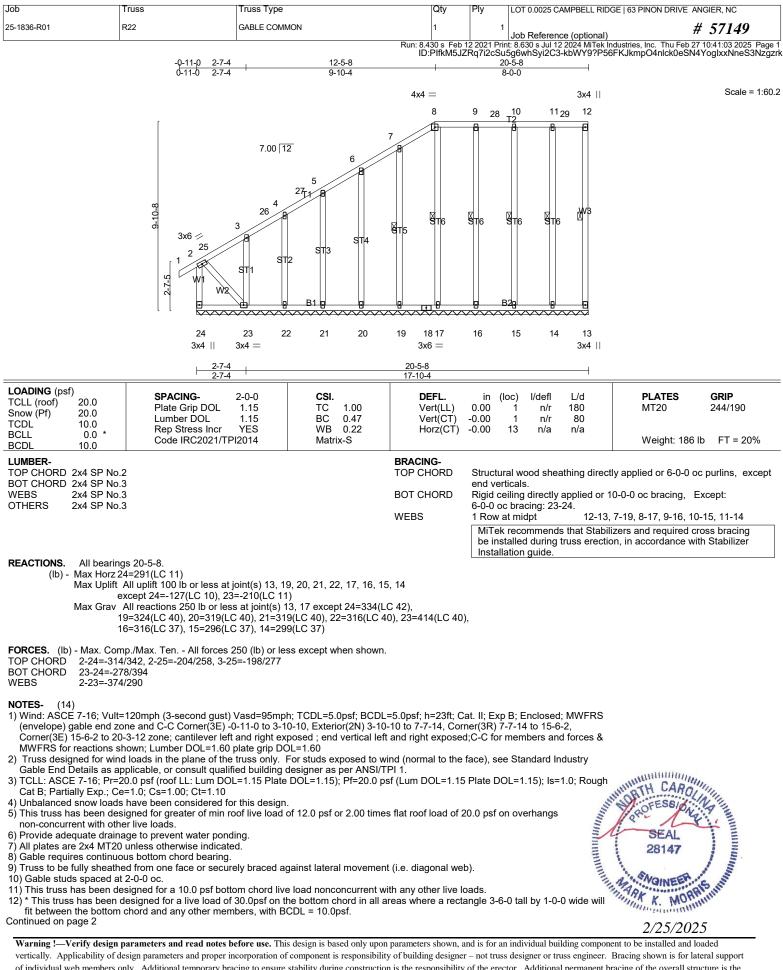


vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





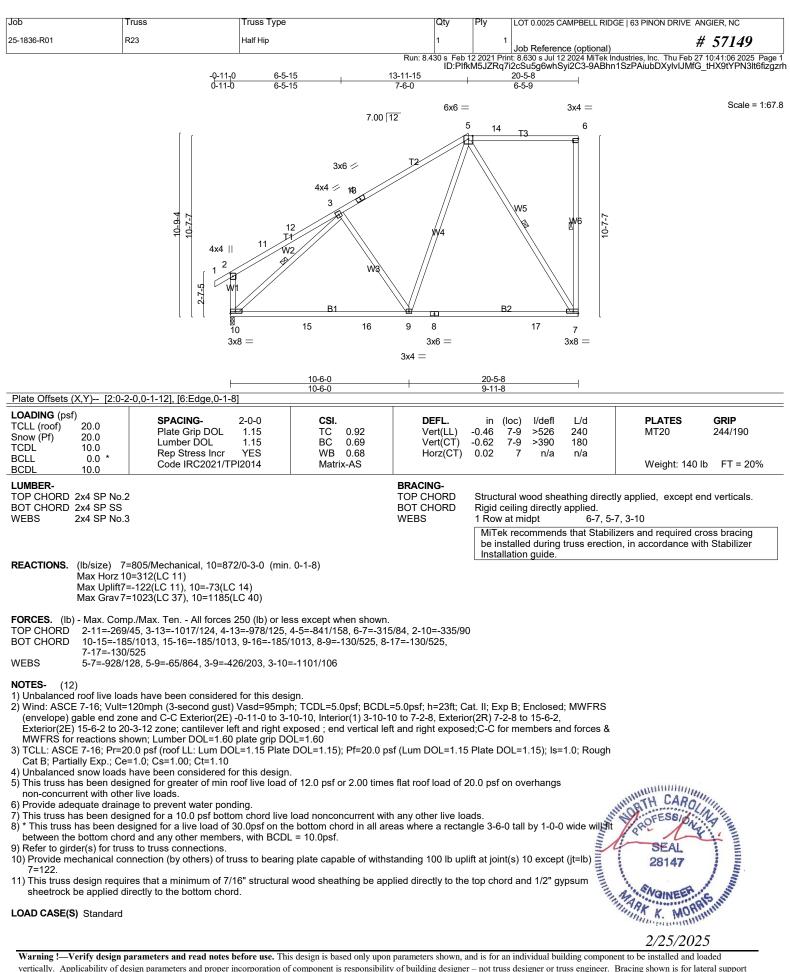
vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

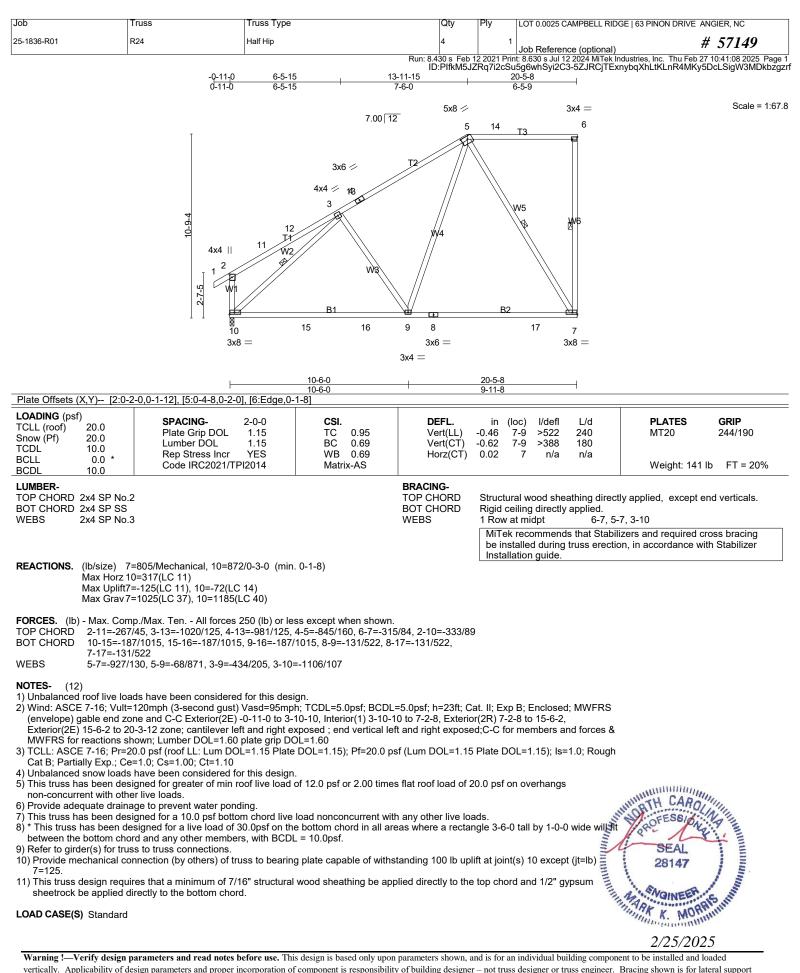
Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE ANGIER, NC		
25-1836-R01	R22	GABLE COMMON	1	1	Job Reference (optional) # 57149		
Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:41:04 2025 Pa ID:PlfkM5JZRg7i2cSu5g6whSvi2C3-Co3xNLQitZSALwOaeUGrHEBd7Uu1PIA4bRO?bg							

NOTES- (14) 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 19, 20, 21, 22, 17, 16, 15, 14 except (jt=lb) 24=127, 23=210.

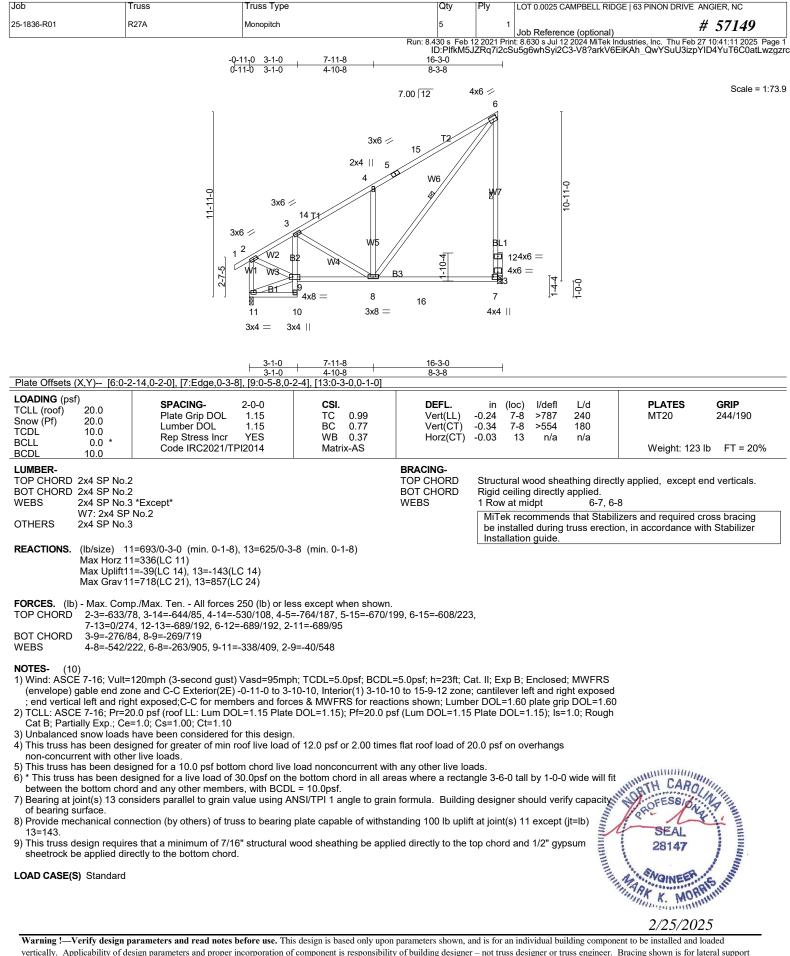
LOAD CASE(S) Standard



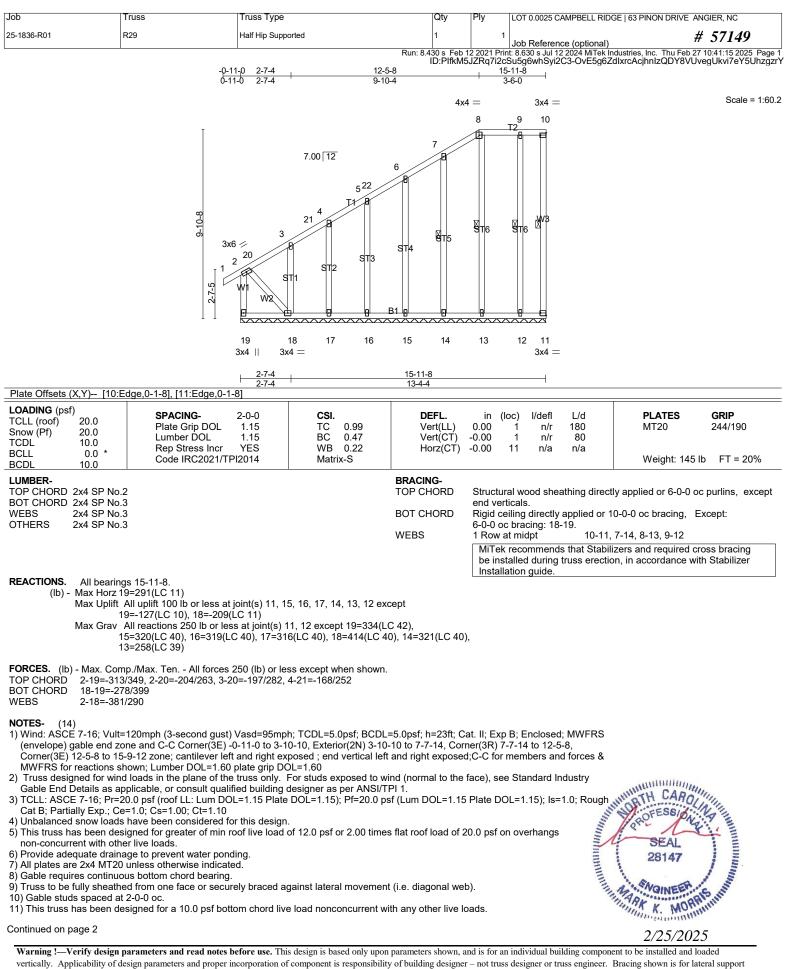




Job		Truss	Truss Type		Qty		Ply	10	T 0 0025 (DGE 63 PINON	DRIVE AN	GIER NC
25-1836-R01		R27	Monopitch		2		,	1					57149
					Run: 8.430 s	Feb 12	2 2021 P	rint: 8.6	630 s Jul 1	nce (optional)	Industries, Inc.	Thu Feb 27 '	10:41:10 2025 Page 1 ONz MrKpUzgzr
			-0-11-0 0-11-0	7-11-8		15-1 8-0	1-8	logow		-TyrcdOv0	TPCJ3IIK_INF	wwkgpuiz	φοινε_ινιτκροέχει
			0-11-0	7-11-0			-0	4x6 -	1.				Scale = 1:70.6
					7.00	12		4x0	5				
			Ī					Λ	Ŧ				
					3x6 🛩	n /	72						
				2	2x4 4		$\langle $						
					3	W4	ıX						
		C 7 7				\$	$/ \setminus$	~	W5				
				9 Tt			·	10:3.8					
			5x5		WB	/							
									X				
			2-7-5 1	W2									
					7 1	1			6				
			3x4		4x8 =				3x4 =				
				7-11-8		15-1	1-8						
Plate Offsets	(X.Y) [2:0-)	2-0,0-1-12], [5:0-2-14,0-2-	-0]. [6:Edae.0-1	7-11-8		8-0			_				
LOADING (psi	f)	SPACING-	2-0-0	CSI.	DEFL.		in	(loc)	l/defl	L/d	PLA	TES	GRIP
TCLL (roof) Snow (Pf)	20.0 20.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC 0.86 BC 0.85	Vert(LL Vert(C		-0.29 -0.38	6-7 6-7	>641 >493	240 180	MT2		244/190
TCDL BCLL	10.0 0.0 *	Rep Stress Incr	YES	WB 0.55	Horz(C		-0.01	6	n/a	n/a	10/-:-		FT - 200/
BCDL	10.0	Code IRC2021/TF	12014	Matrix-AS							vveič	ht: 115 lb	FT = 20%
LUMBER- TOP CHORD					BRACING- TOP CHORE						ctly applied,	except en	d verticals.
BOT CHORD WEBS	2x4 SP No.	3 *Except*			BOT CHORE WEBS	J	1 Row			/ applied. 5-6, 5	5-7		
	W5: 2x4 SF	? No.2									ilizers and re		
REACTIONS.	(lb/size) 6	=625/Mechanical, 8=693	/0-3-0 (min. 0-	1-8)			Insta	llatior	n guide.	·			
	Max Horz 8	=348(LC 13) =-143(LC 14), 8=-39(LC	,	- ,									
	Max Grav 6	=865(LC 24), 8=718(LC 2	21)										
FORCES. (Ib TOP CHORD		np./Max. Ten All forces 84, 3-9=-566/104, 3-4=-69				199							
BOT CHORD	2-8=-644/	137	55/190, 4-10	599/210, 5-10520/	234, 3-0090/1	100,							
WEBS		492 256, 5-7=-246/862, 2-7=-4	43/469										
NOTES- (10		100mmh (0. s. s. s. s. d. s. s. t)											
(envelope)	gable end zo	=120mph (3-second gust) one and C-C Exterior(2E)	-0-11-0 to 3-10	-10, Interior(1) 3-10-	-10 to 11-0-2, E	xteric	or(2E) 1	11-0-2	to 15-9	-12 zone;			
Lumber DO	L=1.60 plate	exposed ; end vertical left e grip DOL=1.60	U .										
		20.0 psf (roof LL: Lum DO =1.0; Cs=1.00; Ct=1.10	L=1.15 Plate D	OL=1.15); Pf=20.0	psf (Lum DOL=´	1.15 I	Plate D)OL=1	.15); ls:	=1.0; Rough	1		
		have been considered fo signed for greater of min r		12.0 psf or 2.00 time	es flat roof load	of 20) () nsf	on ov	erhangs				
											MULLIN	Hilliller.	
6) * This truss	has been de	esigned for a live load of a	30.0psf on the l	pottom chord in all a	reas where a re	ectan	gle 3-6	-0 tall	by 1-0-	0 wide will fi	it unenne TH	CAROL	11/11
7) Refer to gird	der(s) for tru	ss to truss connections.	s, with BCDL =					() 0	. /	Inne.	PROFE	SEIPNA	Pum
 Provide me This truss d 	chanical con lesign requir	er live loads. signed for a 10.0 psf botto asigned for a live load of 3 ord and any other member ss to truss connections. inection (by others) of trus es that a minimum of 7/16 rectly to the bottom chord	ss to bearing pl 5" structural wo	ate capable of withs od sheathing be app	standing 100 lb i blied directly to t	uplift the to	at joint op chor	(s) 8 d and	except (1/2" gy	jt=lb) 6=143 psum	1 st	AL	
sheetrock b	e applied dir	rectly to the bottom chord								UUM	28	147	
LOAD CASE(S	 Standard 									psum	ANDI	NEER	and the second s
											MARK K	MORA	inner.
											"HILLING	munnun	
		parameters and read notes b									2/2	5/2025)



Job	Tru	SS	Truss Type		Qty	Ply	I OT 0 0025 0		GE 63 PINON DRIVE ANGIER, NC
25-1836-R01	R28		Half Hip		1	1			# 57149
					Run: 8.430 s Feb 1	2 2021 Pri	nt: 8.630 s Jul 1	nce (optional) 2 2024 MiTek I	ndustries. Inc. Thu Feb 27 10:41:13 2025 Page
			-0-11-0 0-11-0	6-5-15	13-11-15	Rq7i2cSu	15-11-8	3-RW6KFQXI	/lmKbuwlaJftwy873AE6x50lyPgK3_Pozgz
			0-11-0	6-5-15	7-6-0		' 1-11-9 '		
					7.00 12	6x6 =	= 3x4 =		Scale = 1:65
		Τт			7.00 12	;	5 6	т	
						/1			
				3x6 🗢	13 T2				
				2x4 <u>a</u> r					
				3	W4//	/	w5		
		<u>10-9-4</u> 10-7-7		11	\$		N AV6	10-7-7	
		9 9		TA			159	10	
			4x4 /						
				W3					
		2-7-5		W2					
] 🚛		B1		<u> </u>		
			9	8	14		¹⁵ 7		
			3x4	3x10 =	=		4x6 =		
			L	6-5-15	15-11-8				
Plate Offsets (X	(,Y) [2:0-1-0	,0-1-12], [6:Edge,0-1-8	3]	6-5-15	9-5-9		1		
LOADING (psf)		SPACING-	2-0-0	CSI.	DEFL.	in (I	oc) l/defl	L/d	PLATES GRIP
	20.0 20.0	Plate Grip DOL Lumber DOL	1.15 1.15	TC 0.91 BC 0.60	Vert(LL)	-0.45 `	7-8 >417 7-8 >302	240 180	MT20 244/190
TCDL BCLL	10.0 0.0 *	Rep Stress Incr	YES	WB 0.57	Horz(CT)		7 n/a	n/a	
	10.0	Code IRC2021/TF	912014	Matrix-AS					Weight: 123 lb FT = 20%
LUMBER- TOP CHORD 2:	x4 SP No 2				BRACING- TOP CHORD	Structur	al wood she	athing direc	tly applied, except end verticals.
BOT CHORD 2					BOT CHORD WEBS		eiling directly	/ applied.	
VVED3 2.	X4 3F NU.3				WEBS			6-7, 5 Is that Stabi	lizers and required cross bracing
							talled during ation quide.	truss erecti	on, in accordance with Stabilizer
	lb/size) 7=6 Max Horz 9=3	25/Mechanical, 9=693/	/0-3-0 (min. 0-	1-8)			g		
N	Max Uplift7=-1	108(LC 14), 9=-50(LC ⁻							
N	Max Grav 7=9	73(LC 40), 9=947(LC 3	36)						
		′Max. Ten All forces : I, 10-11=-814/73, 3-11			8/194.				
		06, 5-13=-815/227, 2-9		,	,				
		9, 5-7=-883/196, 2-8=-2	24/746, 5-8=-20)9/999					
NOTES- (12)									
		have been considered Omph (3-second gust)			=5 0psf: h=23ft: Ca	at II:Exp	B. Enclose	d MWFRS	
	able end zone	and C-C Exterior(2E)	-0-11-0 to 3-10	-10, Interior(1) 3-10-1	0 to 7-2-8, Exterior	r(2R) 7-2	-8 to 13-11-	15,	
		own; Lumber DOL=1.6	60 plate grip D0	DL=1.60	U .				
Exterior(2E) 1 & MWFRS for			L=1.15 Plate D	OL=1.15); Pt=20.0 ps	st (Lum DOL=1.15	Plate DC)L=1.15); Is:	=1.0; Rough	
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE	7-16; Pr=20.0	.0; Cs=1.00; Ct=1.10							
Exterior(2É) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads ha	.0; Cs=1.00; Ct=1.10 ave been considered fo		12.0 psf or 2.00 times	s flat roof load of 2	0 0 psf o	n overhands		
Exterior(2É) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads ha	.0; Cs=1.00; Ct=1.10 ave been considered fo		12.0 psf or 2.00 times	s flat roof load of 20	0.0 psf o	n overhangs	5	ANTH CARO
Exterior(2É) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads ha	.0; Cs=1.00; Ct=1.10 ave been considered fo		12.0 psf or 2.00 times ad nonconcurrent with	s flat roof load of 20 n any other live loa	0.0 psf o ds.	n overhangs		TH CARO
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads ha	.0; Cs=1.00; Ct=1.10 ave been considered fo		12.0 psf or 2.00 times ad nonconcurrent with pottom chord in all are : 10.0psf.	s flat roof load of 20 n any other live loa eas where a rectan	0.0 psf o ds. gle 3-6-0	n overhangs) tall by 1-0-	s D wide will fit	REFESSION STREET
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads ha	.0; Cs=1.00; Ct=1.10 ave been considered fo		12.0 psf or 2.00 times ad nonconcurrent with bottom chord in all are to 10.0psf.	s flat roof load of 20 n any other live loa eas where a rectan	0.0 psf o ds. gle 3-6-0	n overhangs) tall by 1-0-1) wide will fit	SEAL
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s 5) This truss has non-concurrei 6) Provide adeq 7) This truss has between the b 9) Refer to girde 10) Provide mec 7=108.	7-16; Pr=20.(ly Exp.; Ce=1 snow loads ha s been design nt with other l uate drainage s been design as been design as been design oottom chord er(s) for truss chanical conn	.0; Cs=1.00; Ct=1.10 ave been considered for leed for greater of min r ive loads. a to prevent water pond and for a 10.0 psf botto gned for a 1ive load of 3 and any other member to truss connections. ection (by others) of tru	oof live load of ling. m chord live lo 30.0psf on the l rs, with BCDL = uss to bearing) wide will the will	SEAL 28147
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s 5) This truss has non-concurrer 6) Provide adeq 7) This truss has between the b 9) Refer to girde 10) Provide mec 7=108. 11) This truss de	7-16; Pr=20.(ly Exp.; Ce=1 snow loads ha s been design nt with other l uate drainage s been design as been design as been design cottom chord er(s) for truss chanical conn esign requires	.0; Cs=1.00; Ct=1.10 ave been considered fo	oof live load of ling. Im chord live lo 30.0psf on the l rs, with BCDL = uss to bearing 16" structural w) wide willing (jt=lb) ypsum	SEAL 28147
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s 5) This truss has non-concurrer 6) Provide adequ 7) This truss has 8) * This truss has between the b 9) Refer to girde 10) Provide mec 7=108. 11) This truss de sheetrock be	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads has s been desigr nt with other I uate drainage s been desigr as been desigr as been desigr bottom chord er(s) for truss chanical conn esign requires e applied dire	.0; Cs=1.00; Ct=1.10 ave been considered for leed for greater of min r ive loads. e to prevent water pono leed for a 10.0 psf botto gned for a live load of 3 and any other member to truss connections. ection (by others) of tru- s that a minimum of 7/1	oof live load of ling. Im chord live lo 30.0psf on the l rs, with BCDL = uss to bearing 16" structural w) wide will a solution of the	SEAL 28147
Exterior(2E) 1 & MWFRS for 3) TCLL: ASCE Cat B; Partiall 4) Unbalanced s 5) This truss has non-concurree 6) Provide adeq 7) This truss has between the b 9) Refer to girde 10) Provide mee 7=108. 11) This truss de	7-16; Pr=20.0 ly Exp.; Ce=1 snow loads has s been desigr nt with other I uate drainage s been desigr as been desigr as been desigr bottom chord er(s) for truss chanical conn esign requires e applied dire	.0; Cs=1.00; Ct=1.10 ave been considered for leed for greater of min r ive loads. e to prevent water pono leed for a 10.0 psf botto gned for a live load of 3 and any other member to truss connections. ection (by others) of tru- s that a minimum of 7/1	oof live load of ling. Im chord live lo 30.0psf on the l rs, with BCDL = uss to bearing 16" structural w					0 wide will the will	SEAL 28147 2/25/2025



Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE ANGIER, NC		
25-1836-R01	R29	Half Hip Supported	1	1	Job Reference (optional) # 57149		
Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 27 10:41:16 2025 Pag ID:PIfkM5JZRg7i2cSu5q6whSyi2C3-s5oTuSZF3FzTomItL0UfmmhgEJ vDB8rMIle07zg							

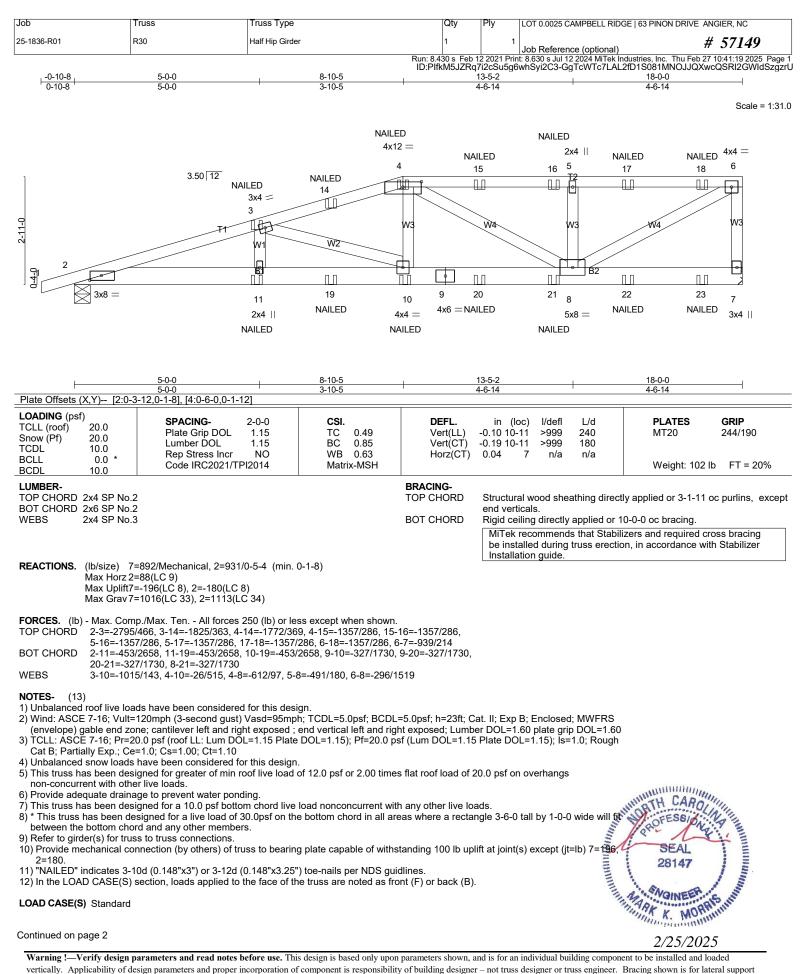
NOTES- (14)

12) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 15, 16, 17, 14, 13, 12 except (jt=lb) 19=127, 18=209.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0025 CAMPBELL RIDGE 63 PINON DRIVE	ANGIER, NC
25-1836-R01	R30	Half Hip Girder	1	1	Job Reference (optional)	# 57149
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Thu Feb 2 whSyi2C3-GgTcWTc7LAL2fD1S081MNOJJQX	

LOAD CASE(S) Standard

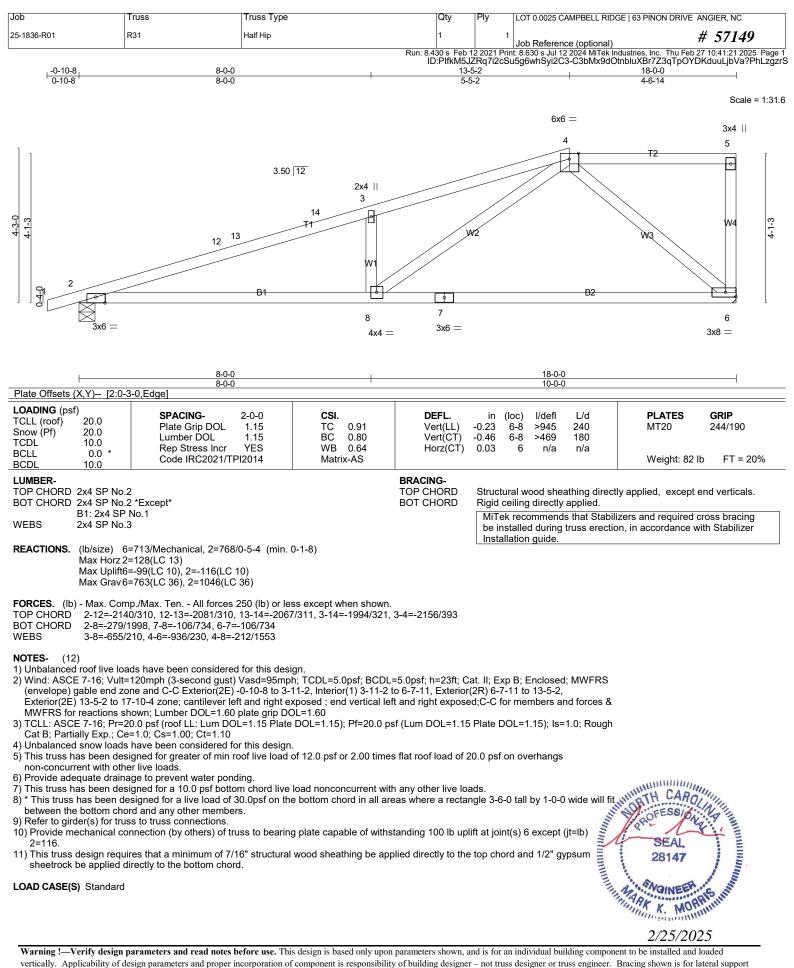
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

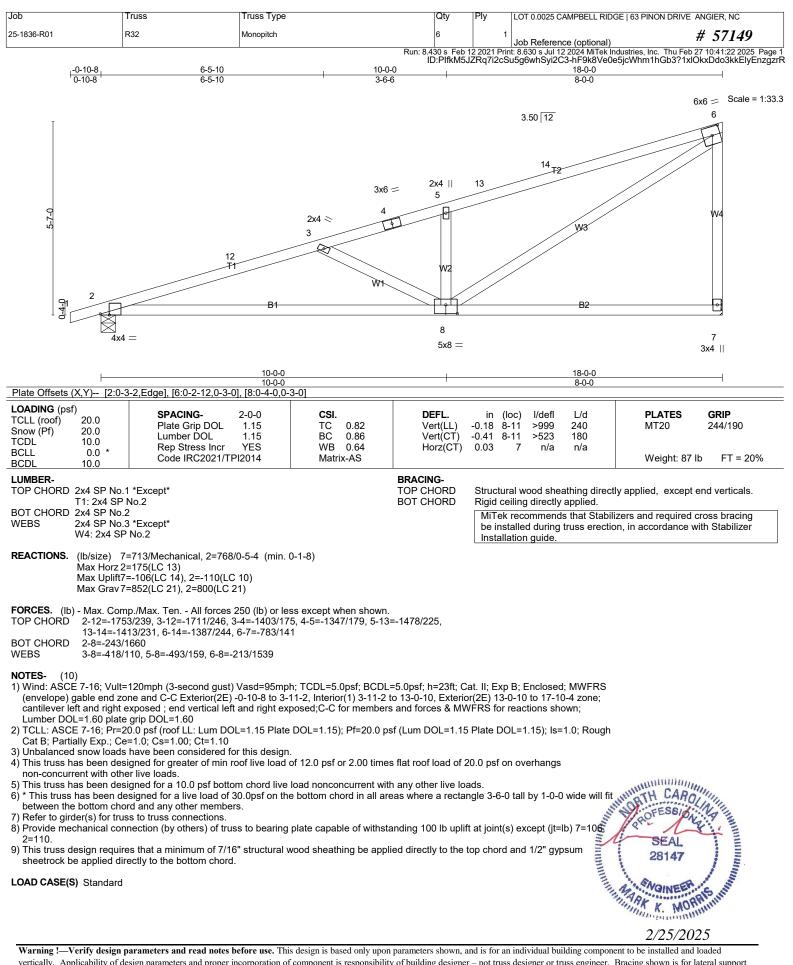
Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 2-7=-20

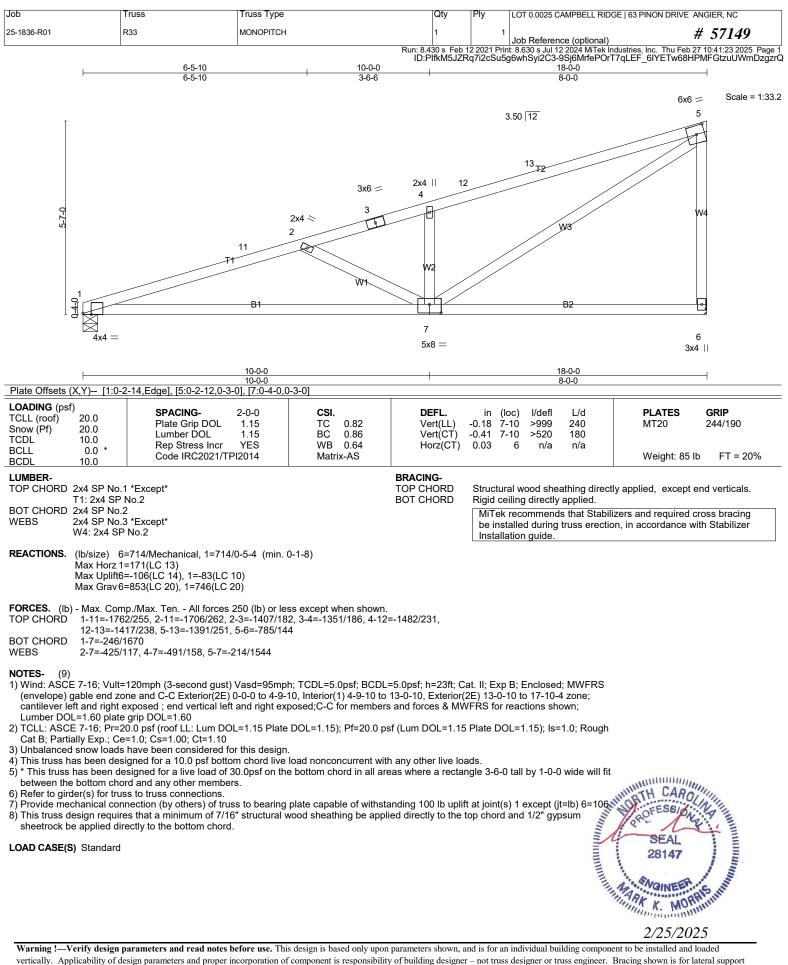
Concentrated Loads (lb)

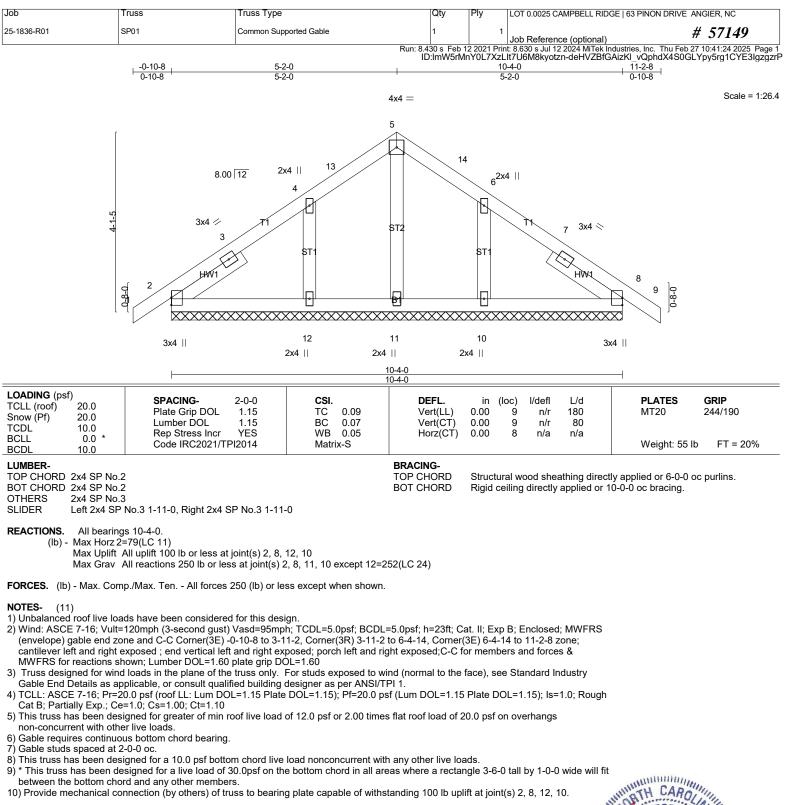
Vert: 3=-43(F) 11=-72(F) 10=-14(F) 4=-20(F) 14=-3(F) 15=-20(F) 16=-20(F) 17=-20(F) 18=-22(F) 19=-51(F) 20=-14(F) 21=-14(F) 22=-14(F) 23=-15(F)



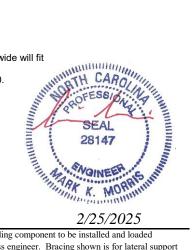


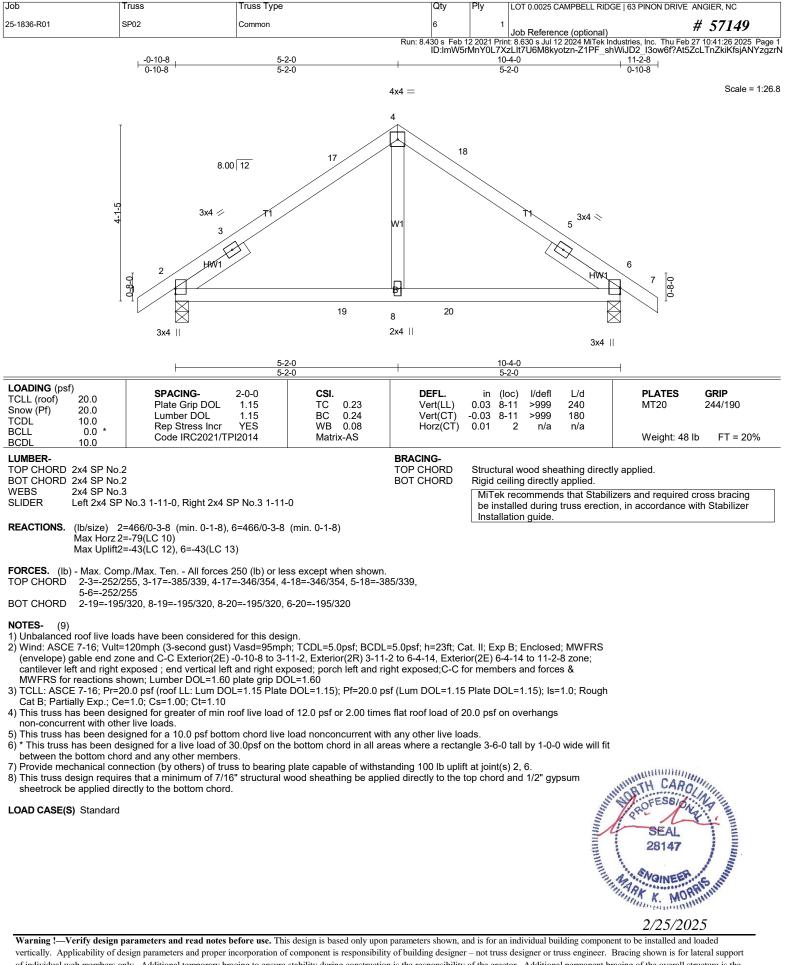




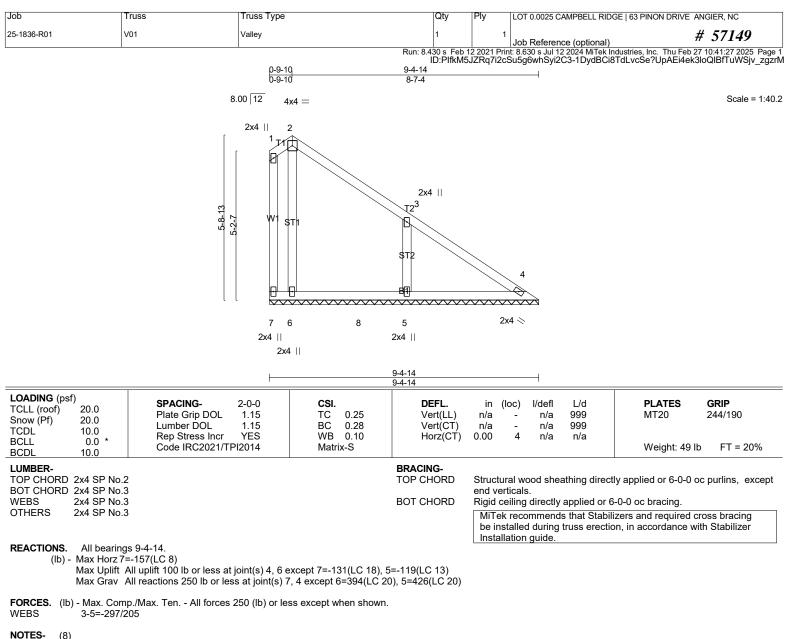








of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 0-9-10, Exterior(2R) 0-9-10 to 4-1-8, Exterior(2E) 4-1-8 to 8-11-1 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

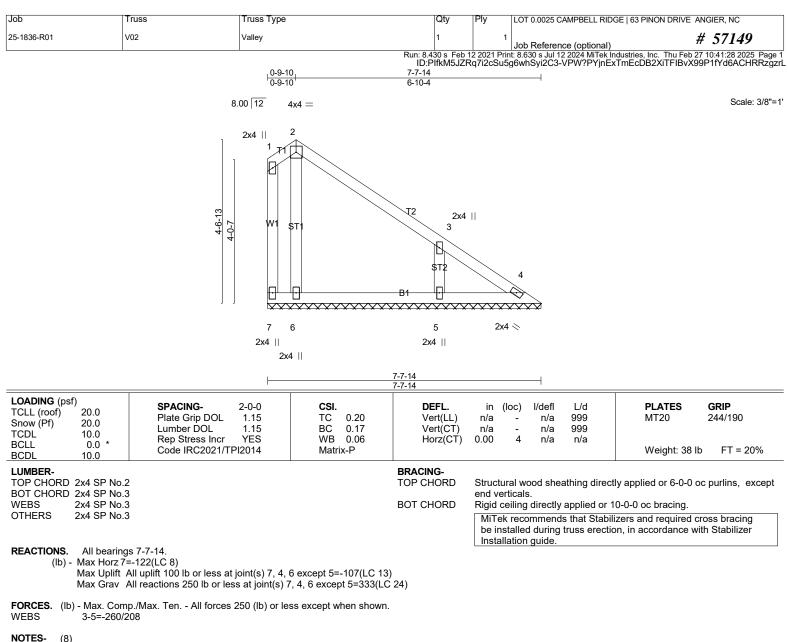
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 6 except (jt=lb) 7=131, 5=119.

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LOAD CASE(S) Standard
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1) Unbalanced roof live loads have been considered for this design.

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 0-9-10, Exterior(2R) 0-9-10 to 2-4-8, Exterior(2E) 2-4-8 to 7-2-1 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

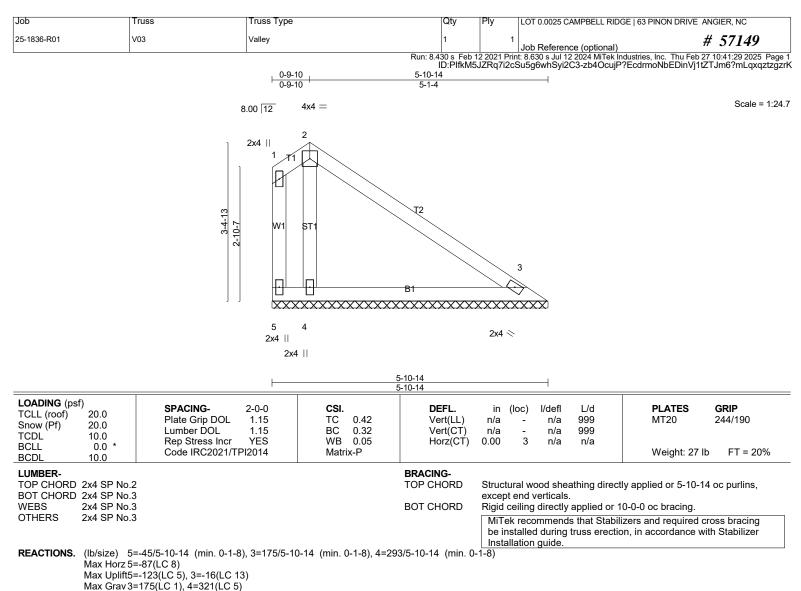
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 4, 6 except (jt=lb) 5=107.

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LOAD CASE(S) Standard
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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

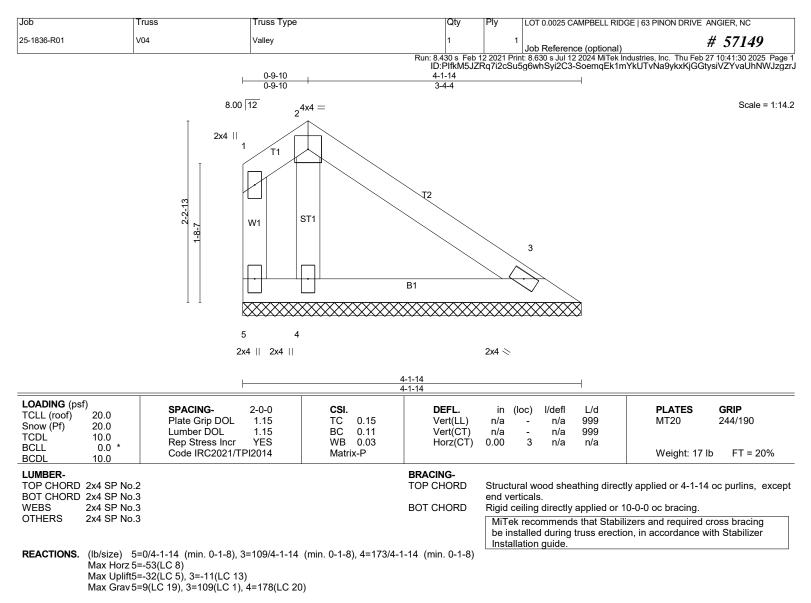
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 5=123.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

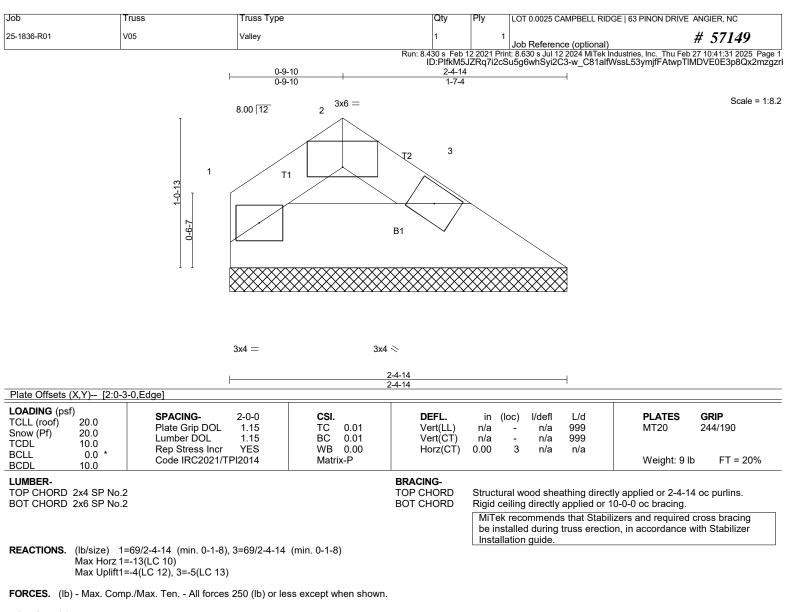
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.

LOAD CASE(S) Standard





- NOTES-(8)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

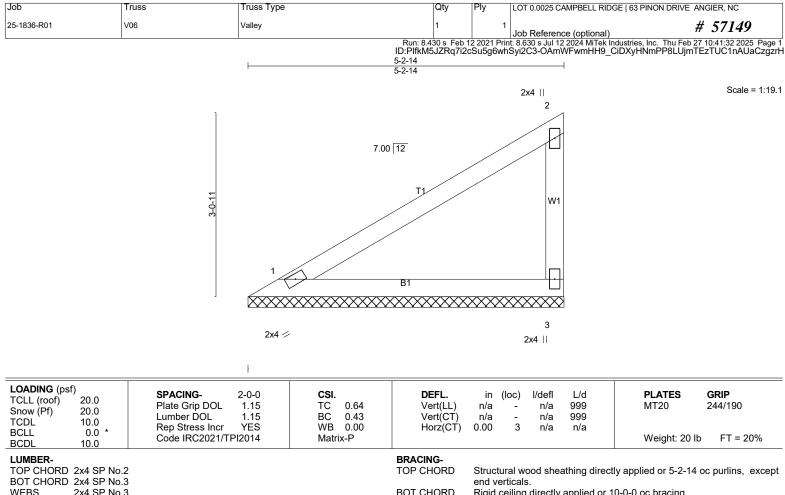
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=182/5-2-14 (min. 0-1-8), 3=182/5-2-14 (min. 0-1-8) Max Horz 1=79(LC 11) Max Uplift1=-12(LC 14), 3=-36(LC 14) Max Grav 1=261(LC 20), 3=261(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

WFBS

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

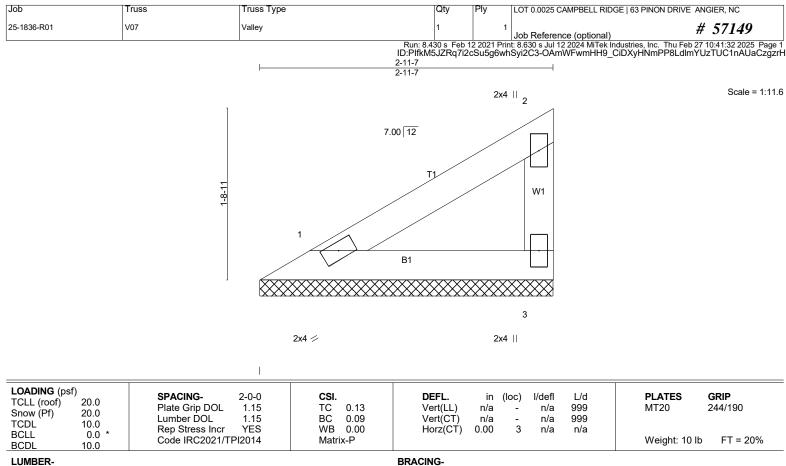
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





TOP CHORD

BOT CHORD

LUM	BE	R-
	~ .	

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No 3

Structural wood sheathing directly applied or 2-11-7 oc purlins, except end verticals Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=91/2-11-7 (min. 0-1-8), 3=91/2-11-7 (min. 0-1-8) Max Horz 1=39(LC 11) Max Uplift1=-6(LC 14), 3=-18(LC 14) Max Grav 1=118(LC 20), 3=118(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

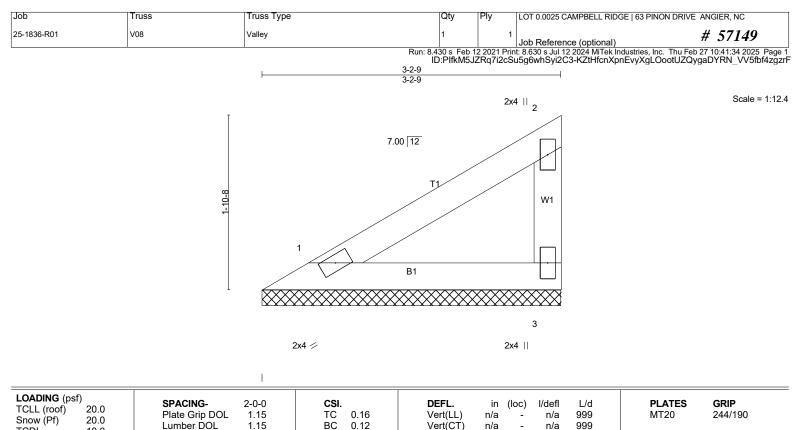
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





TCDL BCLL BCDL	10.0 0.0 * 10.0	Rep Stress Incr YES Code IRC2021/TPI2014	WB 0.00 Matrix-P	Horz(CT)	0.00 3	n/a	n/a	Weight: 11 lb	FT = 20%
	RD 2x4 SP No.2 RD 2x4 SP No.3 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	end vertica	ls.	0	tly applied or 3-2-9 oc j 10-0-0 oc bracing.	purlins, except
					MiTek red	commend ed during	s that Stabi	lizers and required cros on, in accordance with	

REACTIONS. (lb/size) 1=101/3-2-9 (min. 0-1-8), 3=101/3-2-9 (min. 0-1-8) Max Horz 1=44(LC 11) Max Uplift1=-7(LC 14), 3=-20(LC 14) Max Grav 1=134(LC 20), 3=134(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

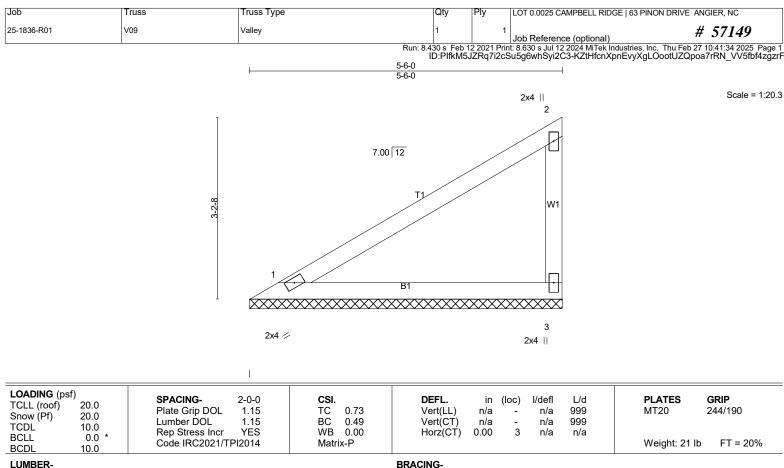
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





LUMBER-	BRACING-
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 5-6-0 oc purlins, exce
BOT CHORD 2x4 SP No.3	end verticals.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 1=193/5-6-0 (min. 0-1-8), 3=193/5-6-0 (min. 0-1-8) Max Horz 1=84(LC 11) Max Uplift1=-13(LC 14), 3=-38(LC 14) Max Grav 1=278(LC 20), 3=278(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

