GENERAL NOTES

DESIGN NOTES

1. Floor: 40 lbs. Live load, 15 lbs. Dead load
2. Roof: 30 lbs. Live load, 20 lbs. Dead load
3. Soil bearing capacity-2000 PSF
4. Live loads, dead loads, wind loads, snow loads, lateral loads, seismic zoning and any specialty loading conditions will need to be confirmed before construction and adjustments to plans made accordingly. See your local building officials for verification of your specific load data, zoning restrictions and site conditions.

CONCRETE AND FOUNDATIONS

1. All slabs on grade shall be 4 inch 3000 PSI (28-day compressive strength concrete), unless noted otherwise.
2. All slabs on grade shall bear on four inch compacted granular fill with 6 by 6 10-10 welded wire mesh.
3. Interior slabs shall have 6 mil, polyethylene vapor barrier underneath.
4. Provide proper expansion joints and control joints as per local requirements.
5. Provide additional bearing points as required by floor "I" joist manufacturer, and loading transfers.
6. Foundation details may vary with local codes and conditions, verify with contractor or engineer.
7. Provide foundation access and vents as required by local codes and conditions.
8. Foundation wall and footing sizes reinforcing must conform with your local building requirements.
9. Foundation walls are not to be backfilled until house is completely framed and roof is in place.
10. Verify depth of footings with your local codes.
11. Provide termite protection as required by HUD minimum property standards.

BASEMENT

1. Basement stairs are calculated as 9 inch treads with 1 inch nosing (10 inch total) and 7.75 inch risers.
2. Water heater and air conditioner may be located in basement when using basement option.
3. Provide sump pumps as required.
4. Some soil conditions may require a 12 inch concrete retaining wall, verify with contractor or engineer.
5. Provide exterior windows and door as grade allows.
6. Provide venting as local codes and conditions dictate.

FRAMING

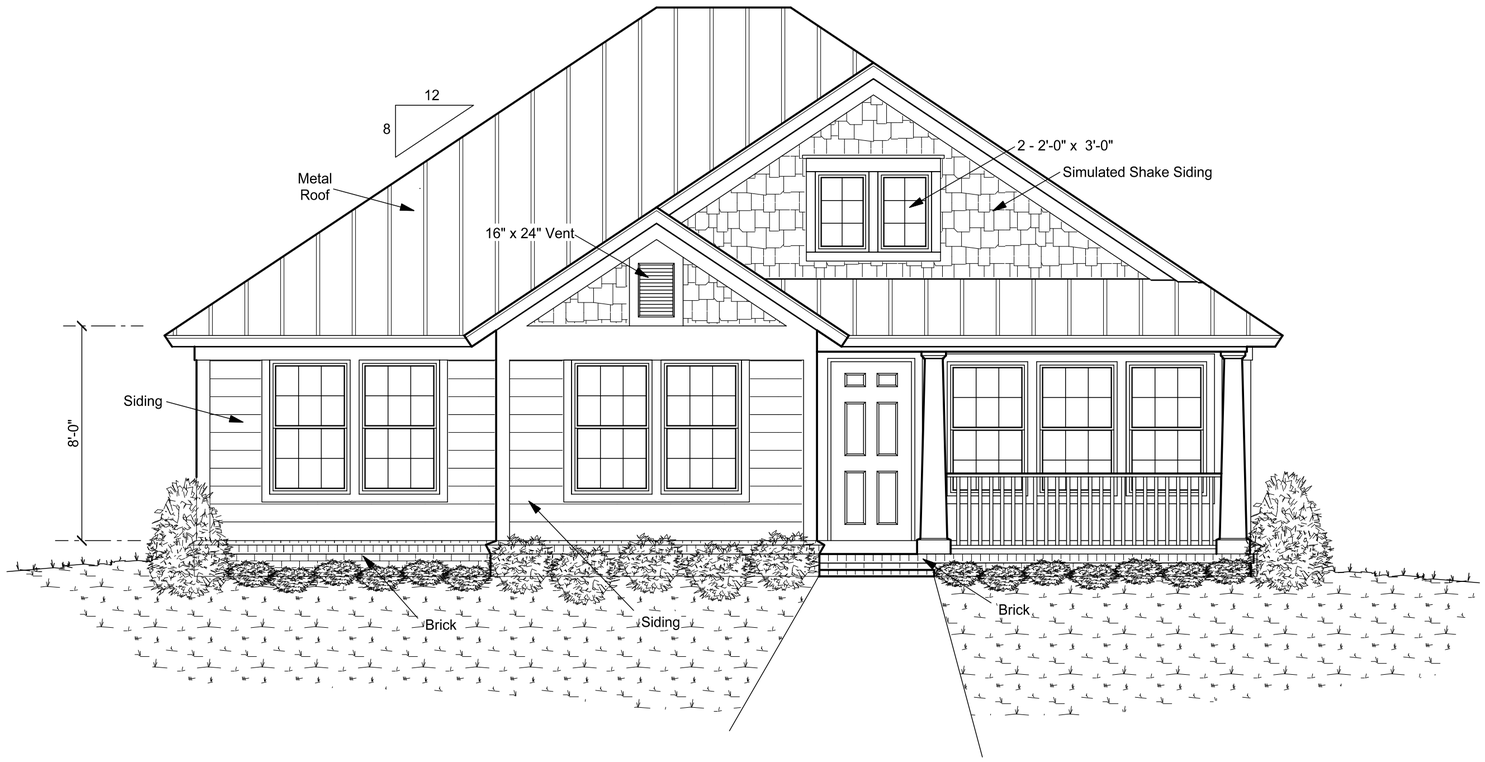
1. Contractor to confirm the size, spacing and species of all framing and structural members to meet your local code requirements.
2. Any structural or framing members not indicated on the plan are to be sized by the contractor.
3. Double floor joists under all partition walls, unless otherwise noted.
4. All angled walls are 45 degree angles unless noted otherwise.
5. Provide collar ties, cross-bridging and bracing as required.
6. Provide additional bearing points as required by loading transfers.
7. Framing lay-out and size may vary with local codes and conditions.
8. Roof framing plan is for general layout only, do no use for rafter count.

MISC. NOTES

1. Prefabricated fireplaces and flues are to be U.L. approved and installed per manu. specifications.
2. All materials, supplies and equipment to be installed per manu. specifications and local codes.
3. Provide type "x" firecode sheetock on garage walls and ceilings.
4. Confirm window openings for your local egress requirements and minimum light and venting.
5. The mechanical and electrical layouts are suggested only. Consult your mechanical and electrical contractors for exact specifications, locations and sizes.
6. Minor alterations to this plan can be made by the builder. Please contact our drafting department for information price quotes if major changes are required.

This plan was designed and drafted by W.L. Martin Home Designs to meet average conditions and codes in the state of Oklahoma at the time it was designed. Because codes and regulations can change and may vary from jurisdiction to jurisdiction, W.L. Martin Home Designs cannot warrant compliance with any special code or regulation. Consult your local building official to determine the suitability of these plans for your specific site and applicaion.
This plan can be adapted to your local building codes and requirements, but also, it is the responsibility of the purchaser and/or builder of thes plan to see that the structure is built in strict compliance with all governing municipal codes [city,county,state, and federal].
The purchaser and/or builder of this plan releases W.L. Martin Home Designs, its owner and employees from any claims or lawsuits that may arise during the construction of this structure or any time thereafter.

NOTICE TO CONTRACTOR:
All construction must comply with current NC Building Codes and is subject to field inspection and verification.
APPROVED
Limited building only review
Permit holder responsible for full compliance with the code
04/03/2025
Harnett COUNTY NORTH CAROLINA



FRONT ELEVATION

REVISIONS	BY

24222
" Adrian "

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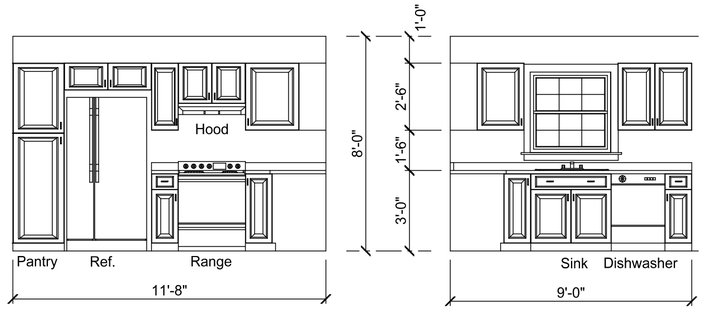
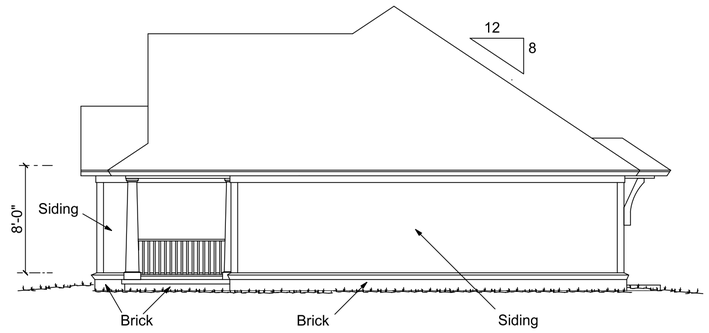
Date
Scale 1/4" = 1'-0"
Drawn
Job
Sheet 1
Of 7 Sheets

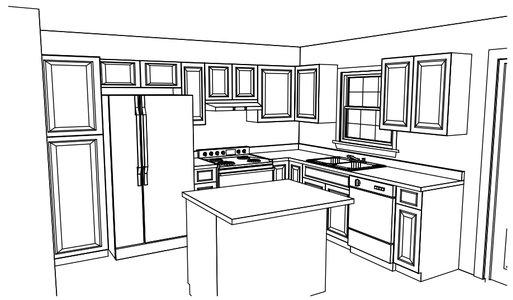


ELEVATION A 1/4" = 1'-0"

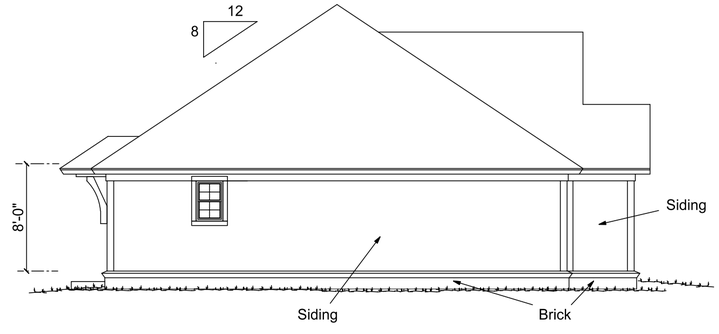
ELEVATION B 1/4" = 1'-0"



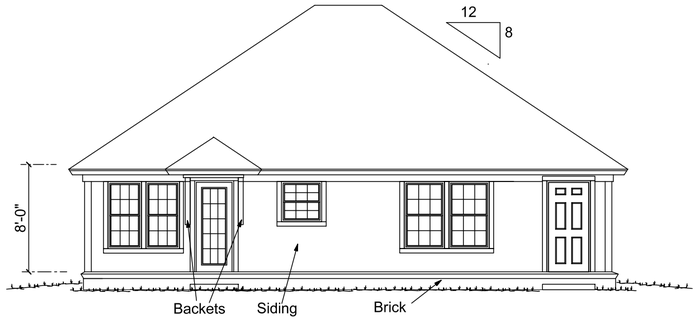
RIGHT SIDE ELEVATION 1/8" = 1'-0"



KITCHEN PERSPECTIVE



LEFT SIDE ELEVATION 1/8" = 1'-0"



REAR ELEVATION 1/8" = 1'-0"

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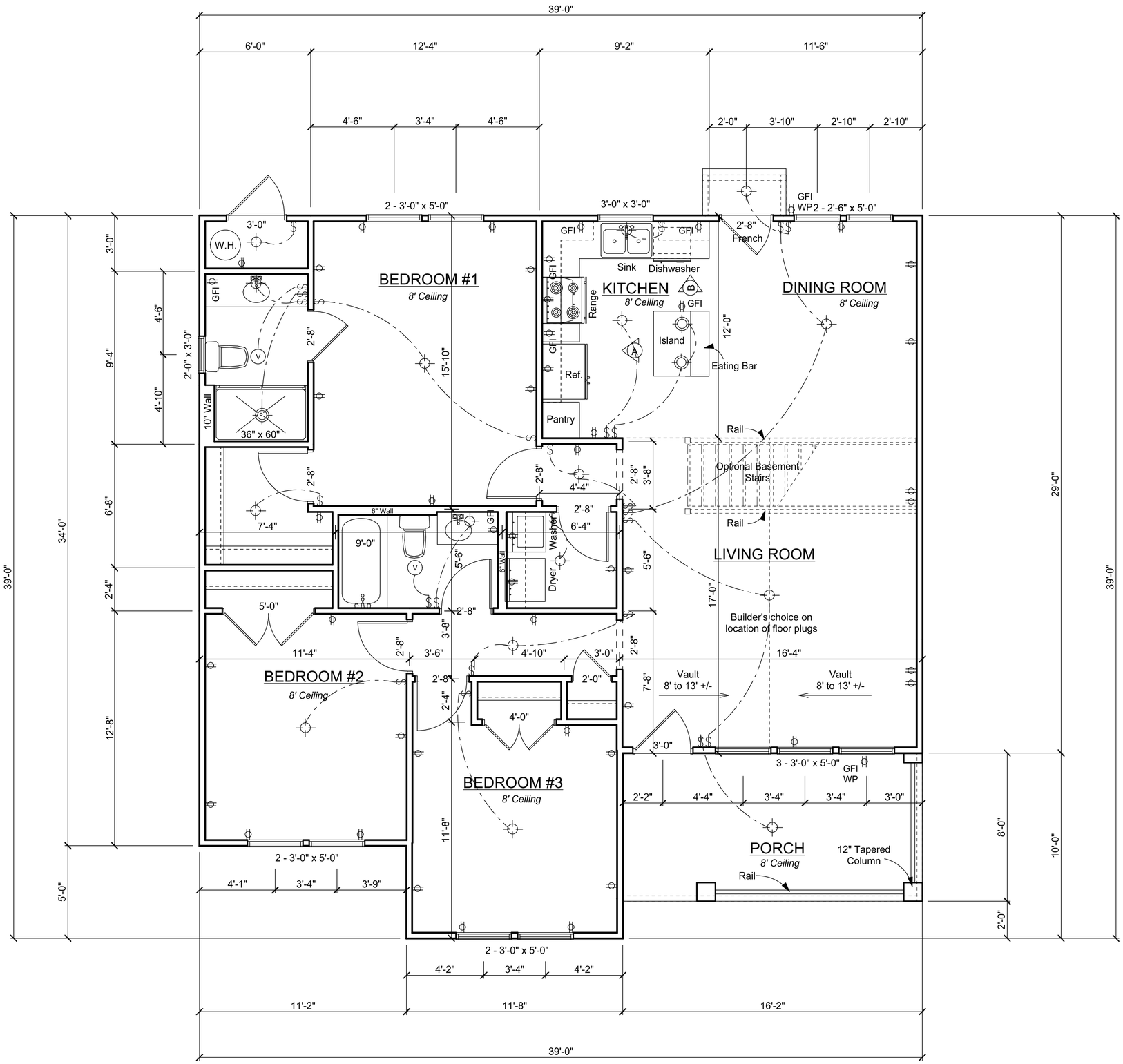
Of	7 Sheets



Floor Plan

Notes:
1. Builders choice on plumbing wall locations.
2. A/C may be relocated as required.
3. Provide safety glass where required.

1286 Square Feet Heat & Cool

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Date	
Scale	1/4" = 1'-0"
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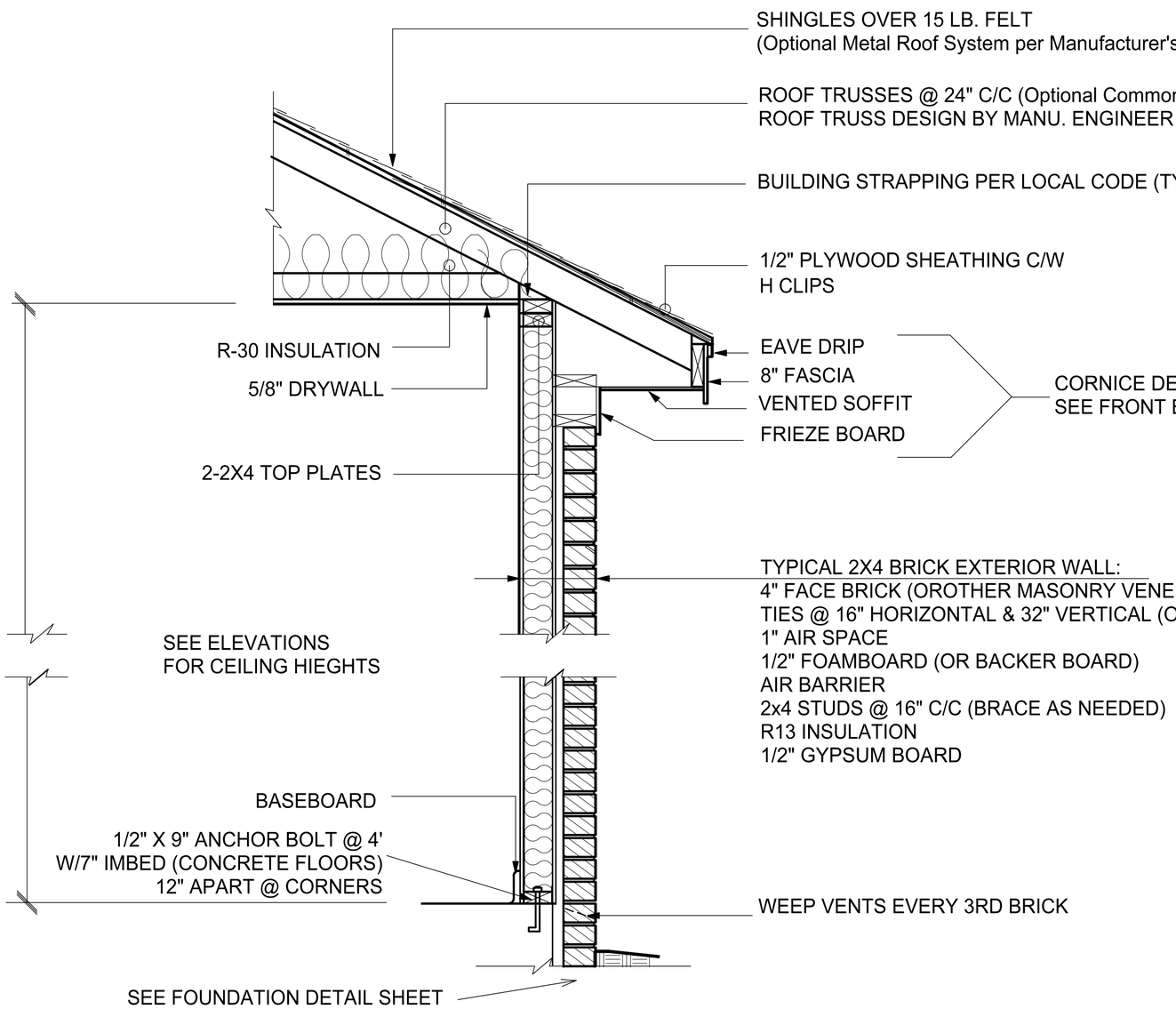
Of	7 Sheets

NOTE: ALL STRUCTURAL MEMBERS MUST
COMPLY W/ LOCAL BUILDING CODES.



TYPICAL BUILDING SECTION

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Date

Scale

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Job

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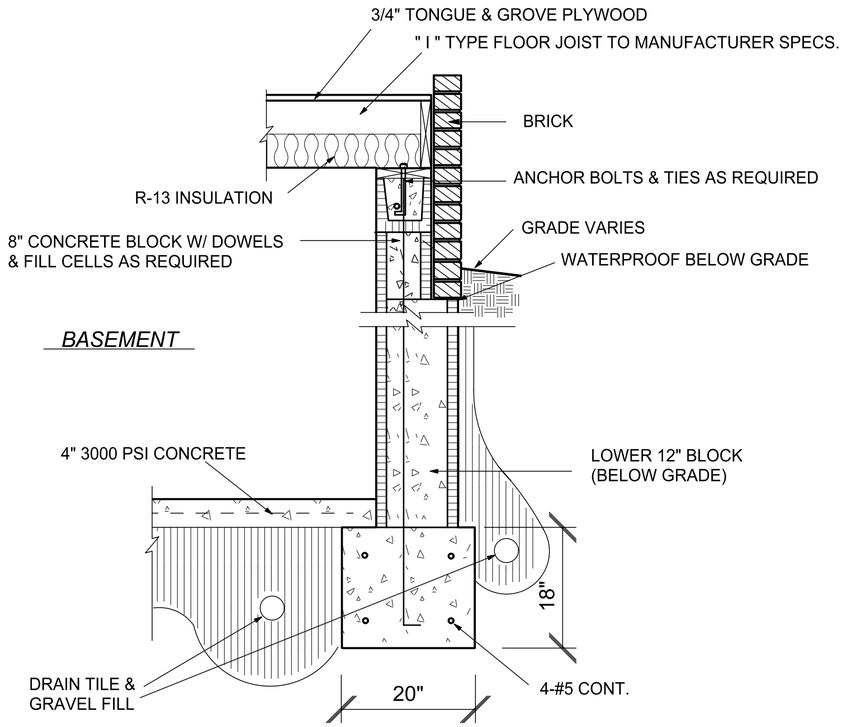
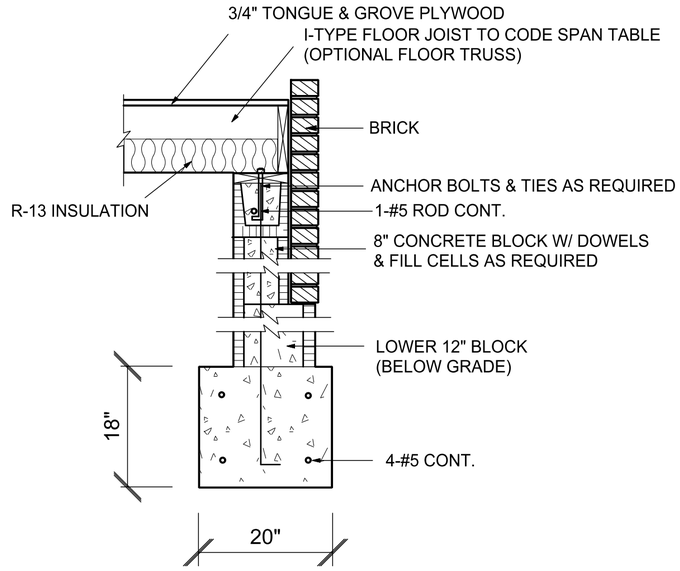
Of 7 Sheets

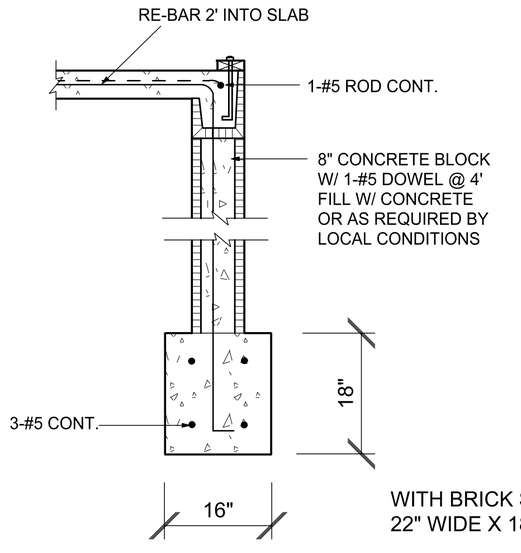
NOTE: SEE ADDITIONAL NOTES ON FRONT ELEVATION SHEET.



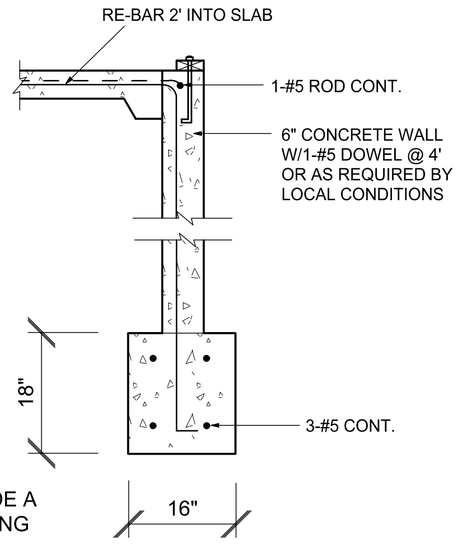
OPTIONAL BASEMENT SECTION



DETAIL - A

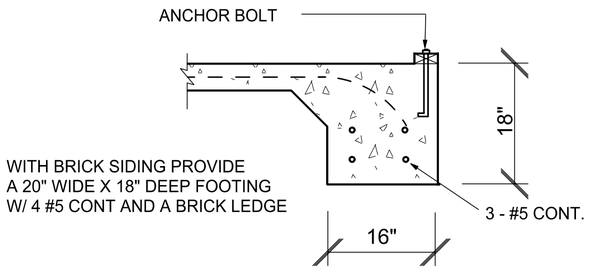


DETAIL - B



WITH BRICK SIDING PROVIDE A
22" WIDE X 18" DEEP FOOTING

DETAIL - C



DETAIL - D

1 STORY FOUNDATION DETAILS (OPTIONS)

NOTES:
* WALLS ABOVE 4'-0" FROM FINISH GRADE MAY REQUIRE ADDITIONAL STRUCTURAL SIZING, SEE ENGINEER (FOOTING SIZE & RETAINING WALL REQ.)
* FOOTING DETAILS MAY VARY IN REQUIREMENTS, IN YOUR AREA, VERIFY WITH CONTRACTOR OR ENGINEER.
* CONCRETE FLOOR TO BE 8" MINIMUM ABOVE FINISH GRADE.
* PROVIDE COMPACTED SAND FILL AS REQUIRED UNDER FOOTINGS.

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Date	
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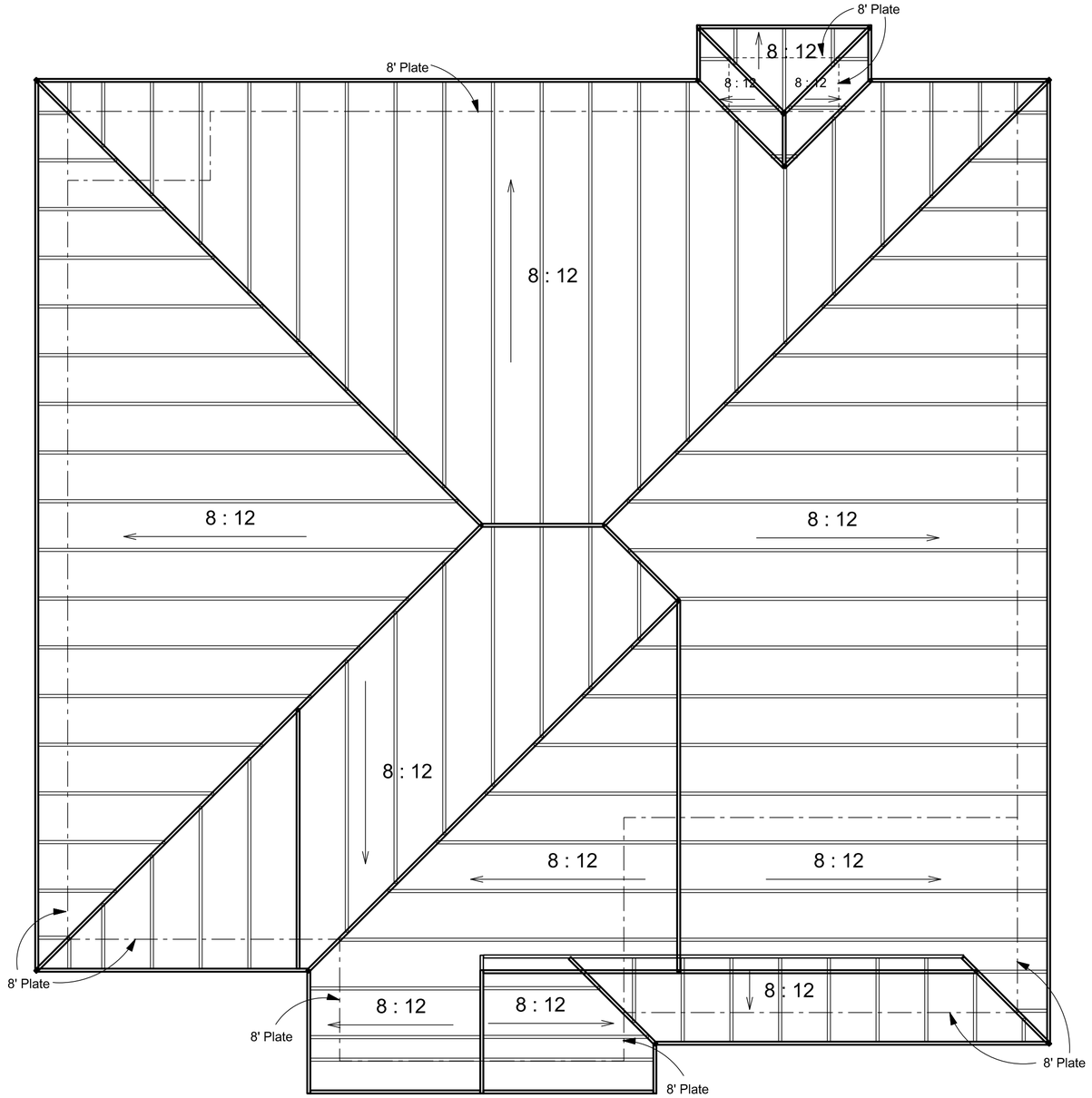
Of	7 Sheets



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Scale	1/4" = 1'-0"
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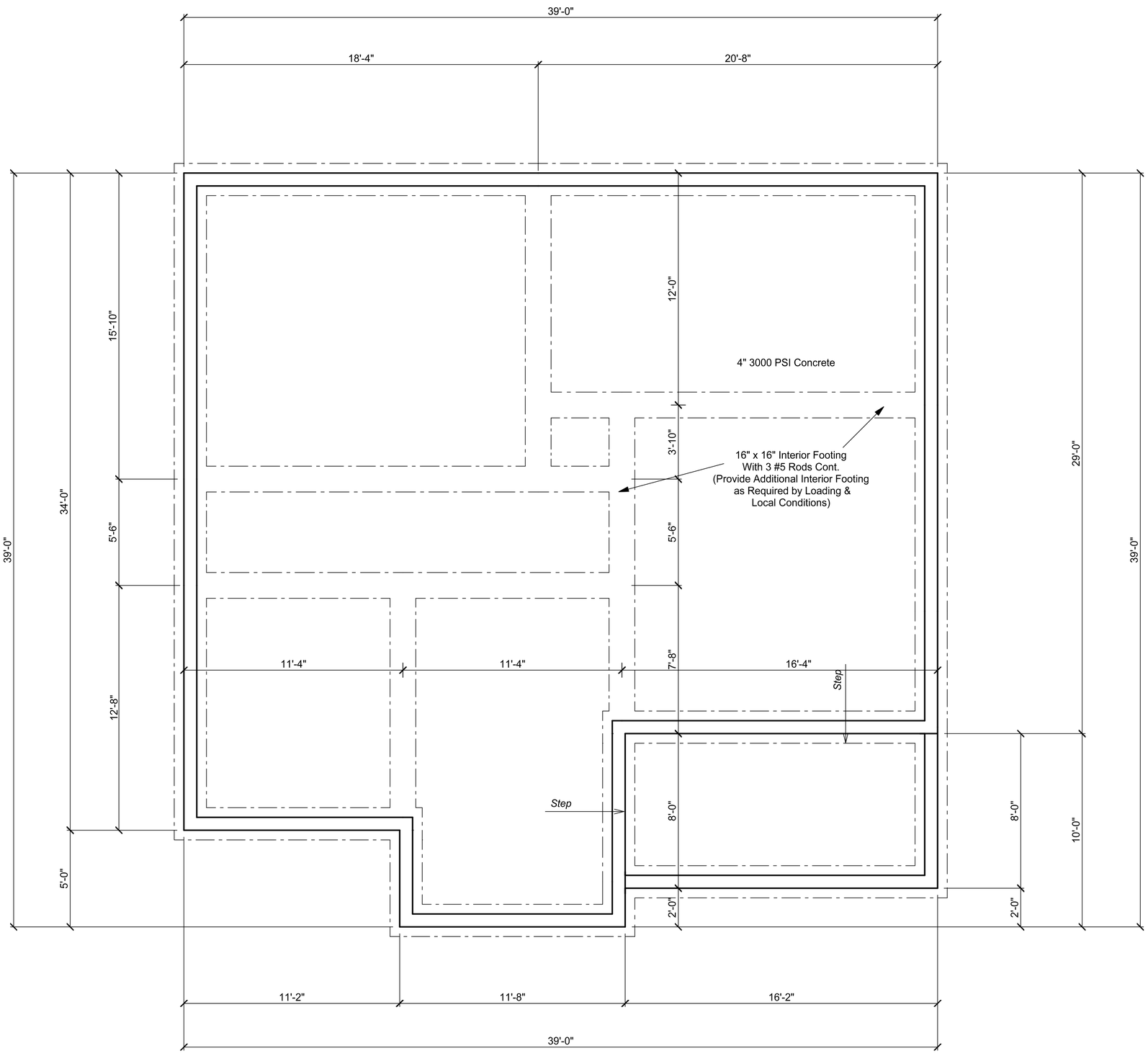
Of	7 Sheets



Slab Foundation Plan

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Date

Scale 1/4" = 1'-0"

Drawn

Job

Sheet 7A

Of 7 Sheets

BUILDING SECTION

For more details see; Typical wall section, foundation plan, foundation details and roof plan

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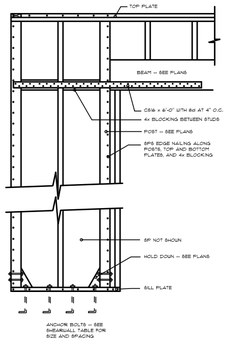
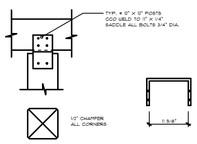
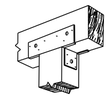


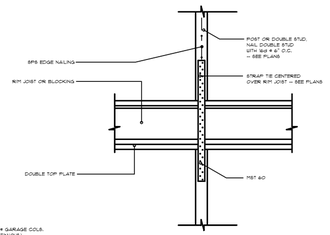
STRAP TIE A



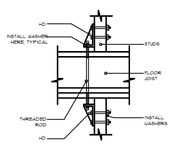
BEAM CONNECTION SADDLE BRACKET



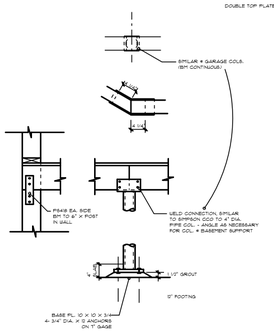
ISOMETRIC T-CONNECTION



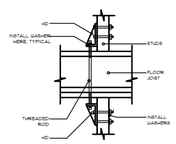
STRAP TIE B



HD2 HOLDOWN



BEAM CONNECTIONS WITH CONNECTOR PLATES



HD2

HD2 HOLDOWN

HOLD DOWN KEY

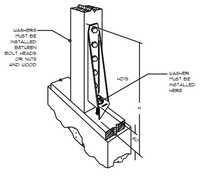
1. SIMPSON LSTHD8 OR LSTHD8RJ (FOR RIM JOIST APPLICATION) WITH 24-16d SINKERS INTO A DBL STUD, EDGE NAIL SHEAR TO BOTH STUDS. AT FOUNDATION USE 8" CONCRETE EMBEDMENT WITH #4 REBAR ABOVE THE EMBEDMENT PORTION.

2. TO CLEAR SPAN FLOOR USE SIMPSON MSTC40 WITH 36-16d SINKERS. USE 18-16d NAILS TO DBL OR 4x STUD ABOVE FLOOR, BELOW FLOOR USE 18-16d SINKERS TO DBL OR 4x STUD OR HEADER. SEE DETAIL FROM SIMPSON CATALOG. 16" CLEAR SPAN.

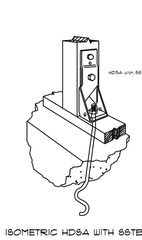
3. SIMPSON PHD5 WITH 14-SDS 1/4"x3" WOOD SCREWS INTO DBL OR 4x STUD, EDGE NAIL SHEAR TO BOTH STUDS. AT FOUNDATION USE SIMPSON SSTB20 ANCHOR BOLT WITH 16" CONCRETE EMBEDMENT WITH SINGLE POUR FOUNDATION. IF REQUIRED, USE CONNECTOR NUT AND CONTINUOUS ALLTHREAD TO CONNECT ANCHOR BOLT TO HOLD DOWN.

4. TO CLEAR SPAN FLOOR USE SIMPSON MSTC78 WITH 80-16d SINKERS. USE 40-16d NAILS TO DBL OR 4x STUD ABOVE FLOOR, BELOW FLOOR USE 40-16d SINKERS TO DBL OR 4x STUD. SEE DETAIL FROM SIMPSON CATALOG. 18" CLEAR SPAN.

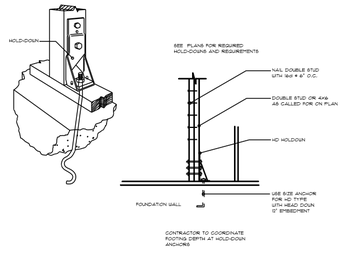
5. SIMPSON STHD10 OR STHD10RJ (FOR RIM JOIST APPLICATION) WITH 28-16d SINKERS INTO A DBL STUD, EDGE NAIL SHEAR TO BOTH STUDS. AT FOUNDATION USE 10" CONCRETE EMBEDMENT WITH #4 REBAR ABOVE THE EMBEDMENT PORTION.



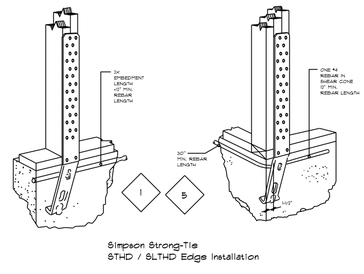
ISOMETRIC HD15 HOLDOWN

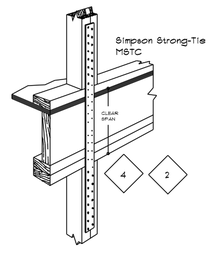


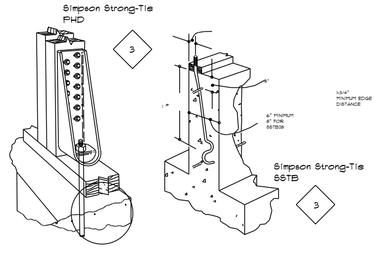
ISOMETRIC HDSA WITH SSTB HOLDOWN



HOLDOWNS

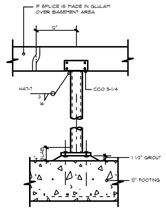




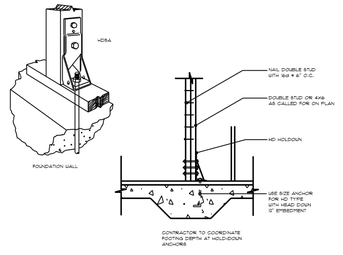


Details shown will not all apply to your build.
Details will vary based on local codes, conditions,
foundation and construction methods and materials used.

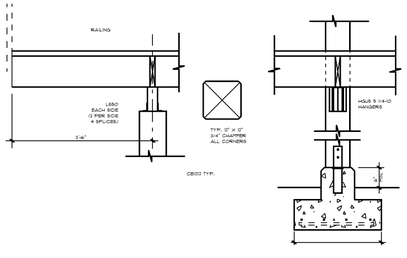
STRAP DETAILS



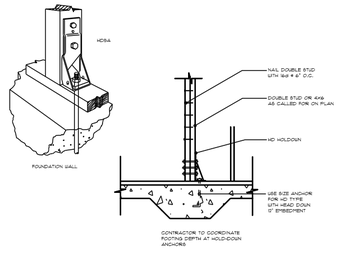
BEAM CONNECTION STEEL POST AND FOOTING



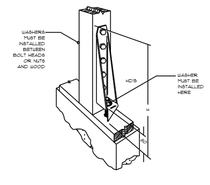
HDSA HOLDOWN



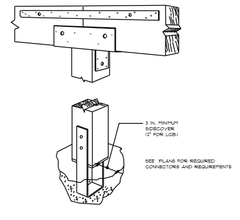
BEAM AND POST



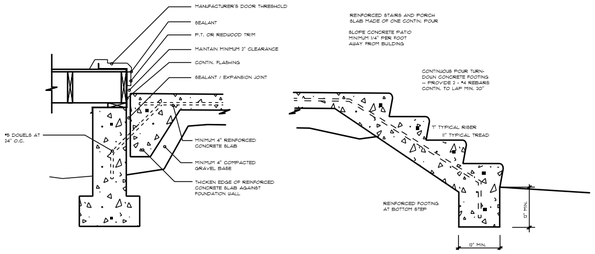
HD5A HOLDOWN



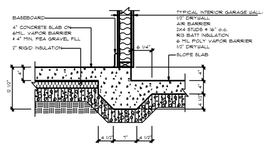
ISOMETRIC HD15 HOLDOWN



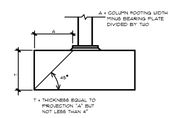
POST BASE AND CAP CONNECTORS



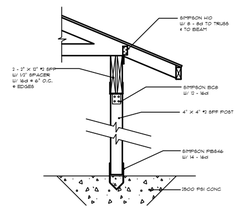
PATIO/STEPS DETAIL



2x4/GARAGE WALL THICKENED SLAB



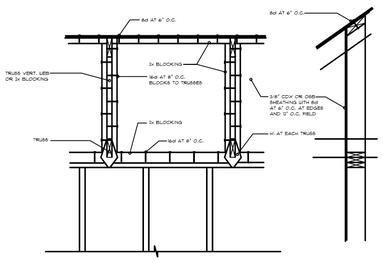
COLUMN FOOTING



POST AND BEAM DETAIL B

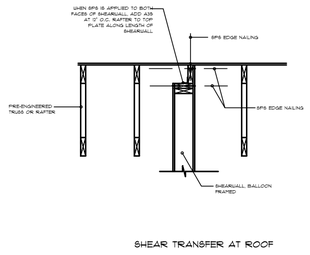
Details shown will not all apply to your build.
Details will vary based on local codes, conditions,
foundation and construction methods and materials used.

FOUNDATION DETAILS

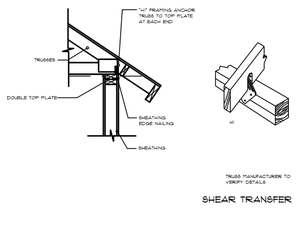


TOP OF SHEAR WALL TO ROOF DIAPHRAGM

SHEAR TRANSFER DETAIL "A"

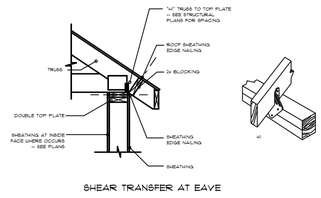


SHEAR TRANSFER AT ROOF

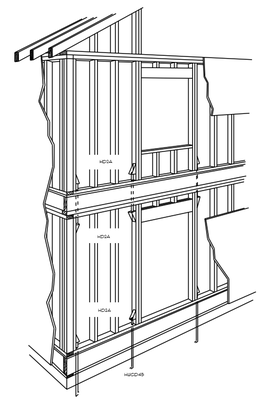


TRUSS MANUFACTURER TO VERIFY DETAILS

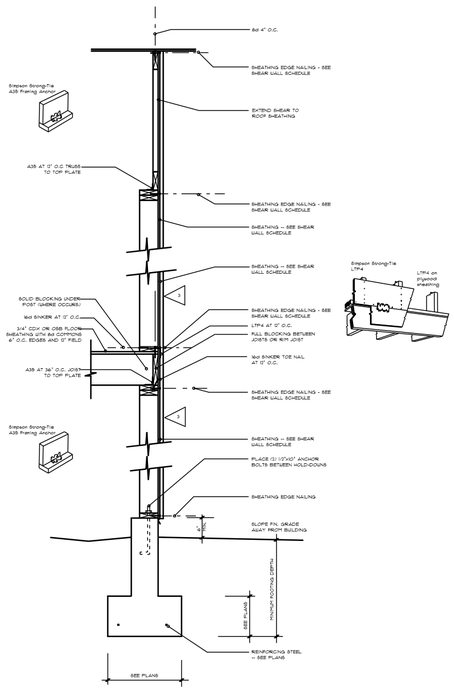
SHEAR TRANSFER



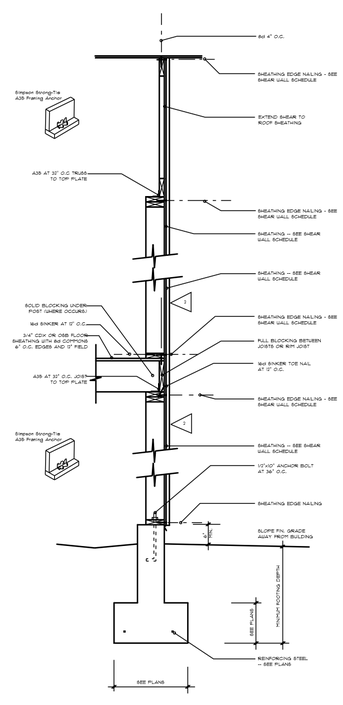
SHEAR TRANSFER AT EAVE



ISOMETRIC SHEAR-WALL LOCATION



SHEAR TRANSFER DETAIL "D"



SHEAR TRANSFER DETAIL "E"

Details shown will not all apply to your build.
Details will vary based on local codes, conditions,
foundation and construction methods and materials used.

SHEAR WALL KEY

NOTE FOR SHEAR WALLS 8d IS FOR GALVANIZED BOX NAIL OR COMMON NAIL

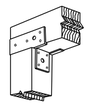
1 — AT ALL EXTERIOR WALLS AND WHERE NOTED

3/8" CDX OR OSB OR 5/8" T-1-11 WITH 8d AT 6" O.C. AT PANEL EDGES AND 12" O.C. FIELD. STUDS AT 16" O.C. MAX.

2 — 3/8" CDX OR OSB OR 5/8" T-1-11 WITH 8d AT 4" O.C. AT PANEL EDGES AND 12" O.C. FIELD. STUDS AT 16" O.C. MAX.

3 — 1/2" CDX OR OSB WITH 10d AT 3" O.C. AT PANEL EDGES AND 12" O.C. FIELD. USE 4x6's AT ALL VERTICAL EDGES. STUDS AT 16" O.C. MAX.

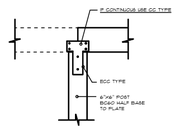
SHEAR WALL DETAILS



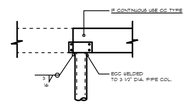
PROVIDE SIMPSON TYPE END POST CAP AT ALL END BEAM CONNECTIONS

SEE STRUCTURAL PLANS AND NOTES FOR REQUIRED CONNECTORS AND ANCHOR REQUIREMENTS

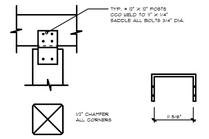
ISOMETRIC L-CONNECTION



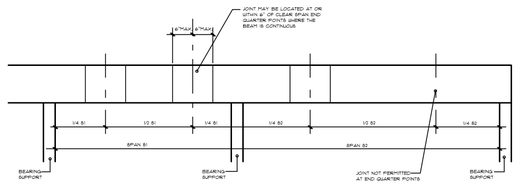
BEAM CONNECTION WOOD POST



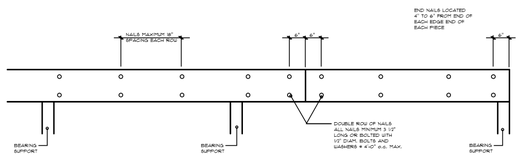
BEAM CONNECTION STEEL POST



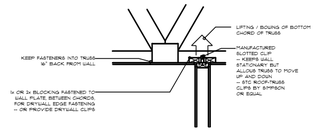
BEAM CONNECTION SADDLE BRACKET



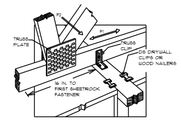
ELEVATION

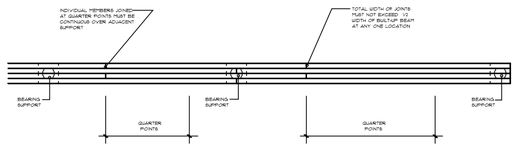


NAILING OF BEAMS

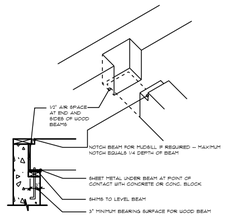


NON-BEARING INT. WALL ATTACHMENT TO TRUSSES

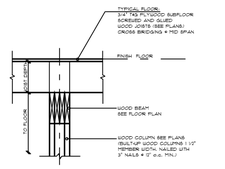




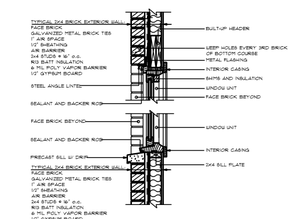
PLAN



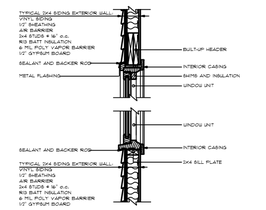
BEAM POCKET



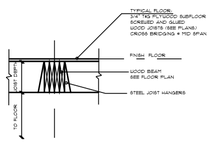
DROPPED WOOD BEAM WOOD COLUMN DETAIL



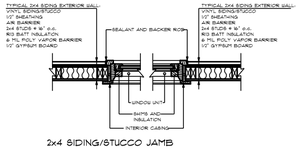
2x4/BRICK HEADER & SILL



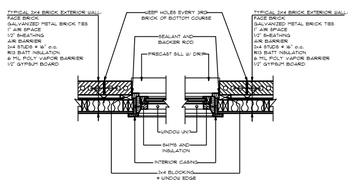
2x4/SIDING HEADER & SILL



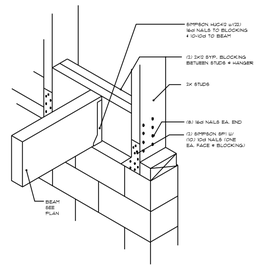
FLUSH WOOD BEAM DETAIL



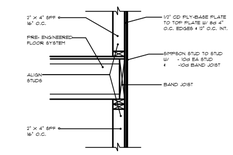
2x4 SIDING/STUCCO JAMB



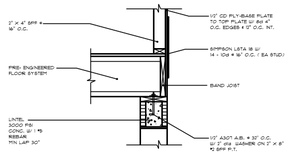
2x4 BRICK JAMB



ISOMETRIC SIMPSON HUC412 CONNECTION B



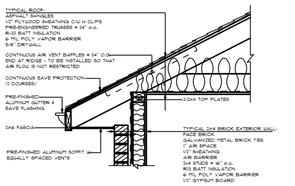
ENGINEERED FLOOR CONNECTION DETAIL A



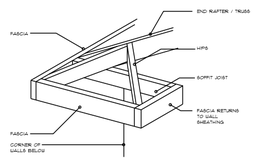
ENGINEERED FLOOR CONNECTION DETAIL B

Details shown will not all apply to your build.
Details will vary based on local codes, conditions,
foundation and construction methods and materials used.

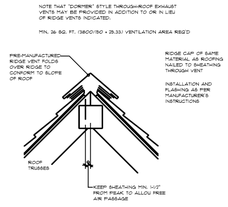
WALL AND BEAM DETAILS



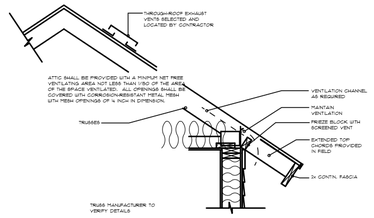
2x4/BRICK EAVE



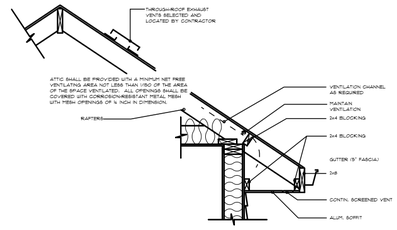
HIP RETURN DETAIL



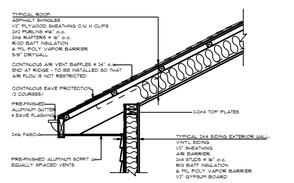
RIDGE VENT



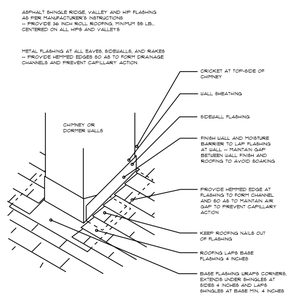
ROOF VENTILATION EAVE DETAIL TRUSS



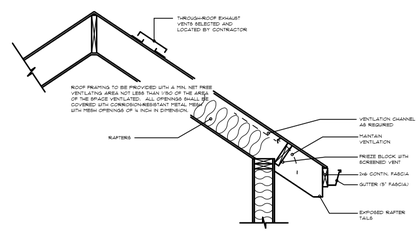
ROOF VENTILATION SOFFITED EAVE SIDING



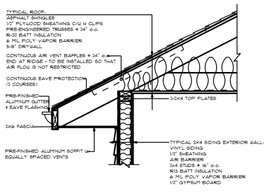
2x4/SIDING CATHEDRAL EAVE W/ PURLINS



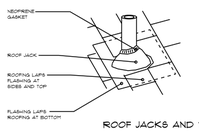
FLASHING DETAILS / NOTES



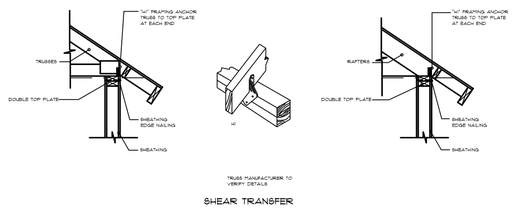
ROOF VENTILATION EAVE DETAIL STICK FRAME



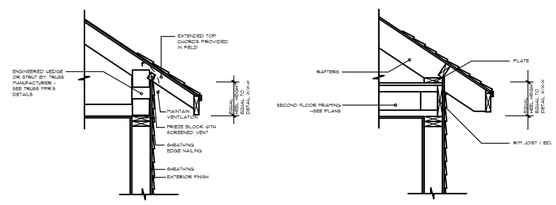
2x4/SIDING EAVE



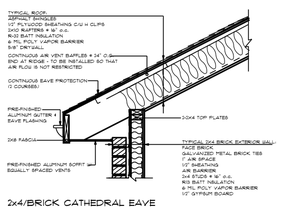
ROOF JACKS AND VENTS



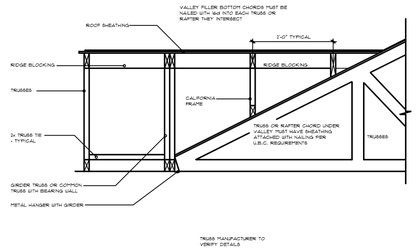
SHEAR TRANSFER



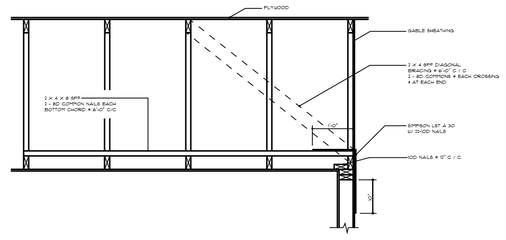
TRUSS ROOF WITH ACCESSIBLE STICK FRAME ROOF



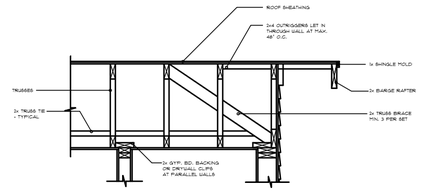
2x4/BRICK CATHEDRAL EAVE



TRUSS ROOF WITH GIRDER TRUSS



END WALL BRACING FOR CEILING DIAPHRAGM

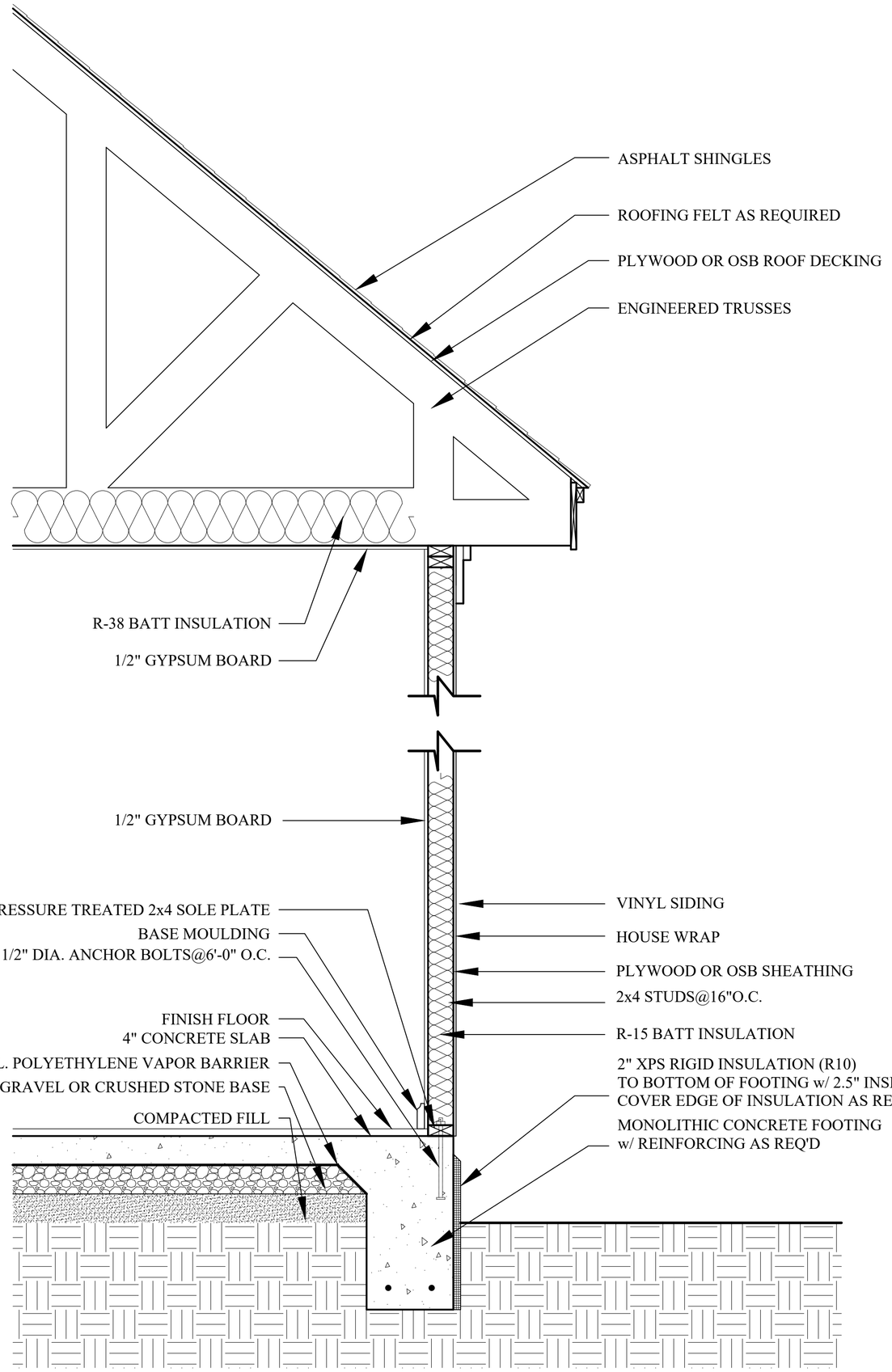


TRUSS FRAME GABLE END ROOF

Details shown will not all apply to your build.
Details will vary based on local codes, conditions,
foundation and construction methods and materials used.

ROOF DETAILS

NOTE:
THESE DETAILS REFERENCE
ONE ANOTHER



TYPICAL WALL SECTION

1"=1'-0"

WHITE HOUSE BUILDING ASSOCIATES LLC
WWW.WHITEHOUSEBUILDINGASSOCIATES.COM
WHITEHOUSEBLDGASSOC@GMAIL.COM
P.O. BOX 1682 LELAND, NORTH CAROLINA
910-978-5195

DRAWN BY: LRW
DATE: 30-Mar-25
REVISED:
SCALE: AS NOTED

DETAILS

BUILDER SHALL ENSURE COMPLIANCE WITH APPLICABLE BUILDING CODES AND ZONING ORDINANCES. ANY ERRORS OR OMISSIONS SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGNER BEFORE CONSTRUCTION BEGINS.

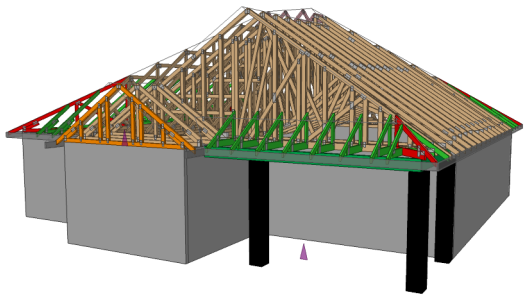
PRIME ESTATE RENOVATIONS

5076 OLD US HWY 421
LILLINGTON, NORTH CAROLINA 27546

G1

Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: **Walk In Customer**

Model: **24222 Adrian**

THE PLACEMENT PLAN NOTES:
1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____ **Date:** _____

General Notes:

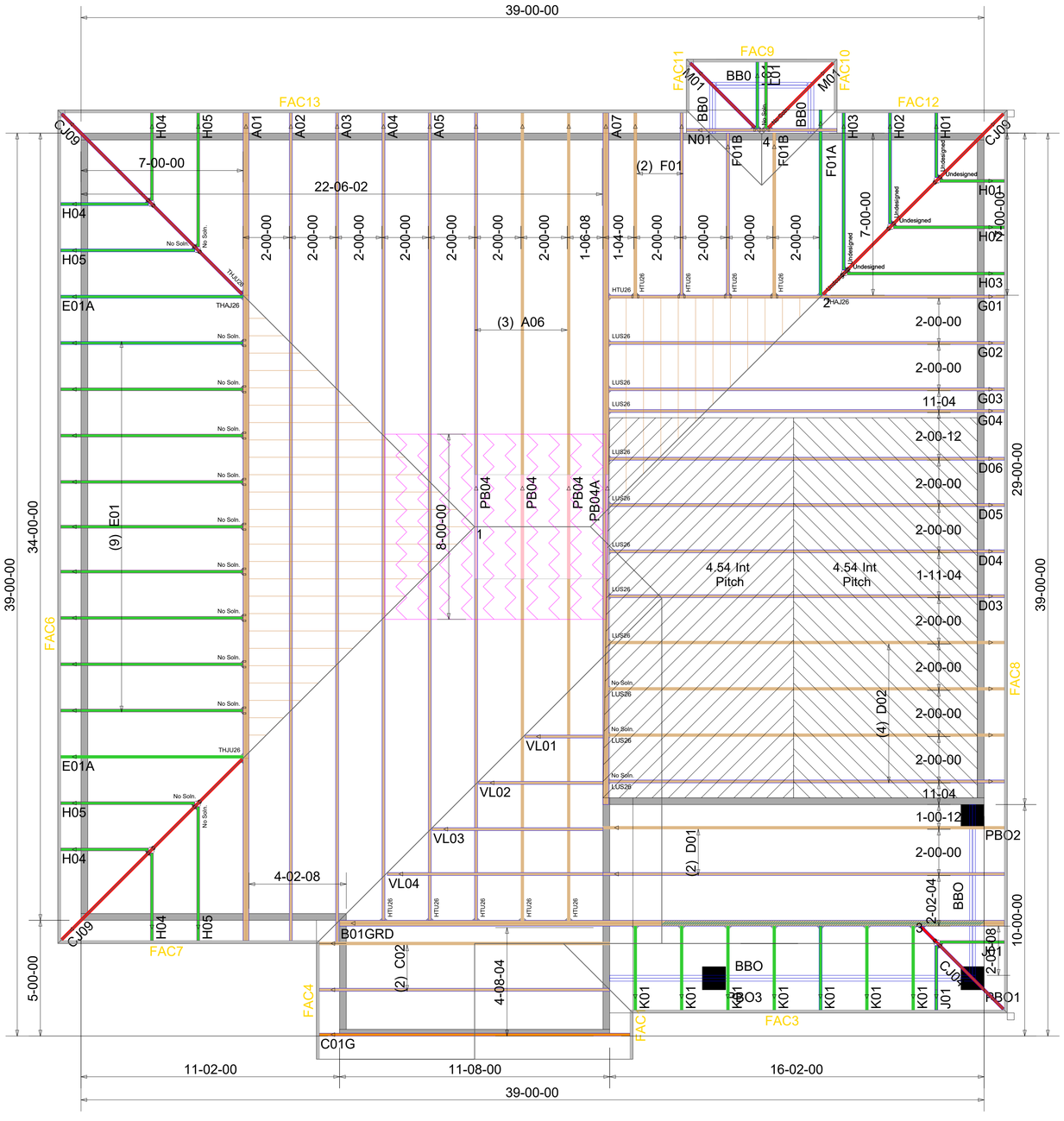
** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.

** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT.

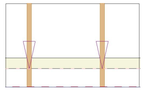
** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.



Truss Connector Total List		
Manuf	Product	Qty
Simpson	HTU26	10
Simpson	LGT2	5
Simpson	LUS26	11
Simpson	One H2.5A	79
Simpson	THJU26	3

Hatch Legend

- 4.54 Int Pitch
- HVAC Platform



Truss Drawing Left End Indicator

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

** All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the bldg designer and or contractor.

** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.

** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS.

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing consult "Bracing of Wood Trusses" available from the Truss Plate Institute, 583 D'Onofrio Drive; Madison, WI 53179

CARTER Lumber

Walk In Customer

5076 Old 421-Prime Estate
Reno.-Roof-24222 Adrian

ROOF PLACEMENT PLAN

Scale:	*NTS*
Date:	3/26/2025
Designer:	Gladys Rivas
Project Number:	**25030187-01**
Sheet Number:	**1/1**

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A01	Hip Girder	1	2	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:01:58 — Page: 1

ID:qP8ymAfV5Exhva4I4rYiAHzXfxh-K3zTsQx2nHppyrJh8FJ9qthn43L6wLCvgBPY7TzX25O

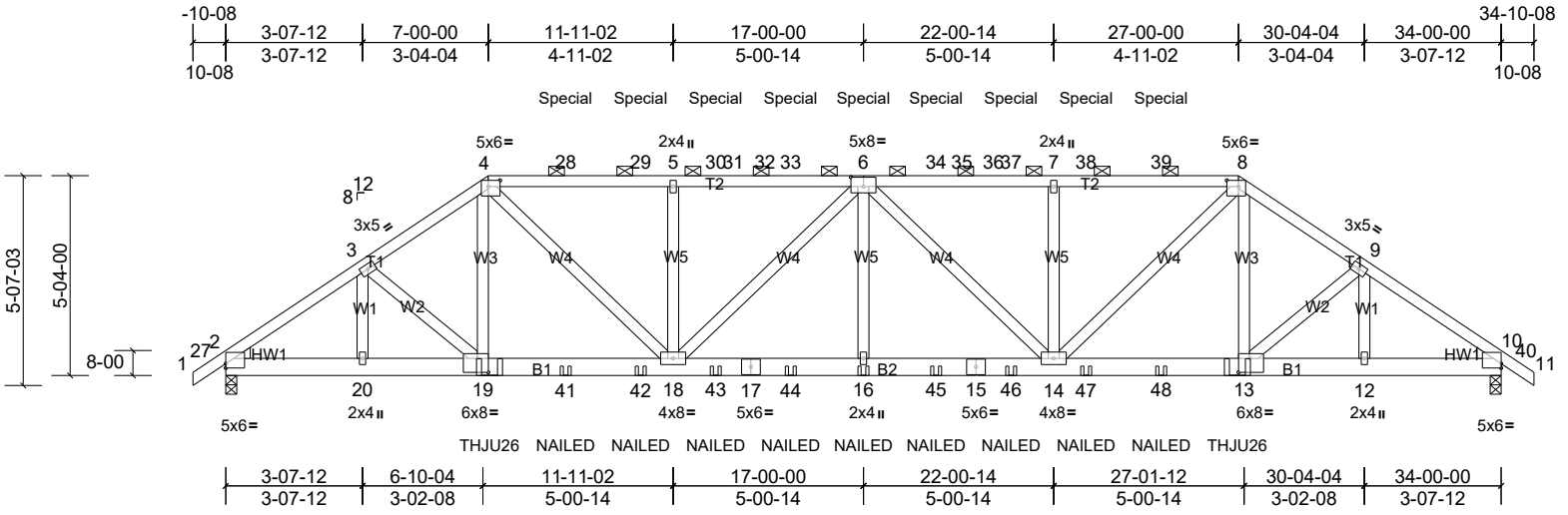


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Plate Offsets (X, Y): [2:Edge,1-06], [4:3-12,2-00], [6:4-00,3-00], [8:3-12,2-00], [10:Edge,1-06], [13:3-08,4-08], [19:3-08,4-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.23	16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.92	Vert(CT)	-0.36	16	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.56	Horz(CT)	0.10	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 462 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3 Right: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-7-3 oc purlins, except 2-0-0 oc purlins (3-5-10 max.): 4-8.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=3917/3-08, (min. 2-06), 10=3917/3-08, (min. 2-06)
Max Horiz 2=124 (LC 11)
Max Uplift 2=-647 (LC 12), 10=-647 (LC 13)
Max Grav 2=4004 (LC 37), 10=4004 (LC 37)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-5967/982, 3-4=-6225/1066, 4-28=-7045/1156, 28-29=-7045/1156, 5-29=-7045/1156, 5-30=-7045/1156, 30-31=-7045/1156, 31-32=-7045/1156, 32-33=-7045/1156, 6-33=-7045/1156, 6-34=-7045/1156, 34-35=-7045/1156, 35-36=-7045/1156, 36-37=-7045/1156, 7-37=-7045/1156, 7-38=-7045/1156, 38-39=-7045/1156, 8-39=-7045/1156, 8-9=-6225/1067, 9-10=-5967/983

BOT CHORD 2-20=-819/4845, 19-20=-819/4845, 19-41=-833/5136, 41-42=-833/5136, 18-42=-833/5136, 18-43=-1174/7692, 17-43=-1174/7692, 17-44=-1174/7692, 16-44=-1174/7692, 16-45=-1174/7692, 15-45=-1174/7692, 15-46=-1174/7692, 14-46=-1174/7692, 14-47=-730/5136, 47-48=-730/5136, 13-48=-730/5136, 12-13=-736/4845, 10-12=-736/4845

WEBS 3-20=-537/115, 3-19=-178/541, 4-19=-200/1260, 4-18=-448/2708, 5-18=-1020/346, 6-18=-943/186, 6-16=0/437, 6-14=-943/185, 7-14=-1020/346, 8-14=-448/2708, 8-13=-201/1260, 9-13=-179/542, 9-12=-537/114

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2x4 - 1 row at 9-00 oc.
   Bottom chords connected as follows: 2x6 - 2 rows staggered at 9-00 oc.
   Web connected as follows: 2x4 - 1 row at 9-00 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
6) Unbalanced snow loads have been considered for this design.
7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
8) Provide adequate drainage to prevent water ponding.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

Continued on page 2

11) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10 and 2. This connection is for uplift only and does not consider lateral forces.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
13) Use Simpson Strong-Tie THJU26 (SGL & SGL LC 2-PLY) or equivalent at 7-0-6 from the left end to connect truss(es) E01A (1 ply 2x4 SP), CJ09 (1 ply 2x4 SP) to front face of bottom chord.
14) Use Simpson Strong-Tie THJU26 (SGL & SGL RC 2-PLY) or equivalent at 26-11-10 from the left end to connect truss(es) E01A (1 ply 2x4 SP), CJ09 (1 ply 2x4 SP) to front face of bottom chord.
15) Fill all nail holes where hanger is in contact with lumber.
16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
17) LGT2 Hurricane ties must have two studs in line below the truss.
18) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 273 lb down and 100 lb up at 9-0-12, 273 lb down and 100 lb up at 11-0-12, 273 lb down and 100 lb up at 13-0-12, 273 lb down and 100 lb up at 15-0-12, 273 lb down and 95 lb up at 17-0-0, 273 lb down and 100 lb up at 18-11-4, 273 lb down and 100 lb up at 20-11-4, and 273 lb down and 100 lb up at 22-11-4, and 273 lb down and 100 lb up at 24-11-4 on top chord. The design/selection of such connection device (s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-4=-60, 4-8=-60, 8-11=-60, 21-24=-20
   Concentrated Loads (lb)
   Vert: 19=-1062, 6=-236, 16=-85, 13=-1062, 28=-236, 29=-236, 30=-236, 33=-236, 34=-236, 37=-236, 38=-236, 39=-236, 41=-85, 42=-85, 43=-85, 44=-85, 45=-85, 46=-85, 47=-85, 48=-85

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A02	Hip	1	1	Job Reference (optional)

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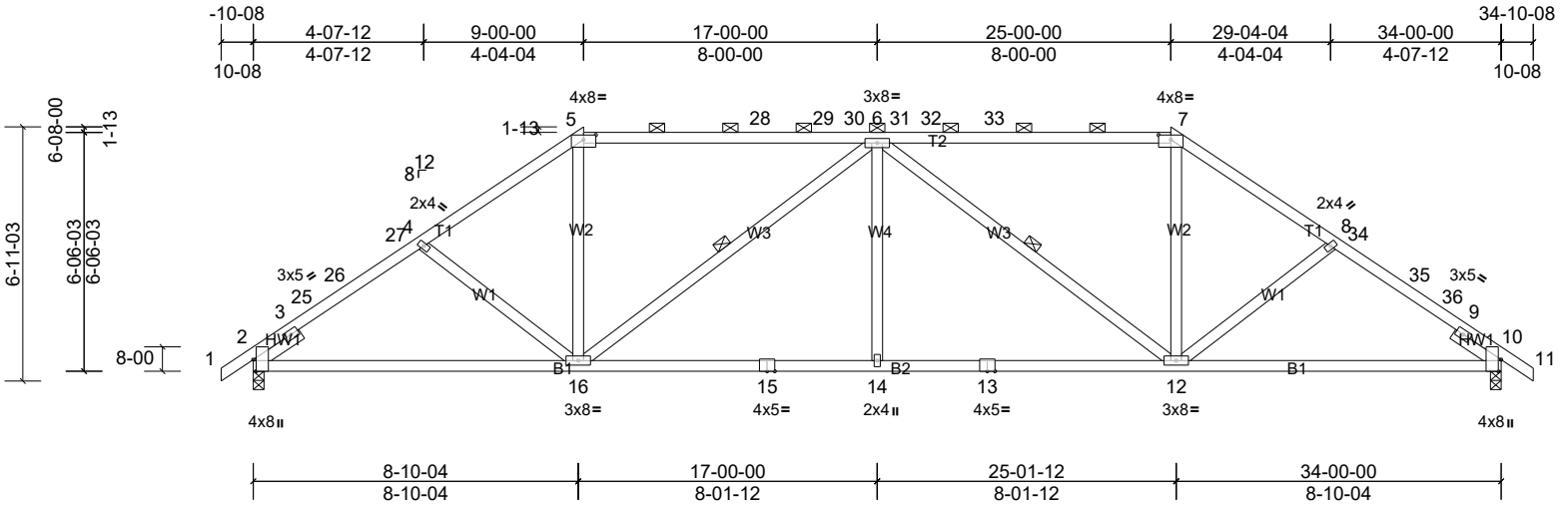

Scale = 1:63

Plate Offsets (X, Y): [2:3-13,Edge], [5:4-00,1-09], [7:4-00,1-09], [10:3-13,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.19	14-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.94	Vert(CT)	-0.34	14-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.11	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 186 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T2:2x4 SP 2400F 2.0E
BOT CHORD	2x4 SP No.2 *Except* B2:2x4 SP No.1
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=1413/3-08, (min. 1-13), 10=1413/3-08, (min. 1-13)
Max Horiz 2=153 (LC 13)
Max Uplift 2=-156 (LC 14), 10=-156 (LC 15)
Max Grav 2=1560 (LC 5), 10=1560 (LC 6)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-1-15 oc purlins, except 2-0-0 oc purlins (5-4-0 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	1 Row at midpt 6-16, 6-12

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1373/0, 3-25=-2437/218, 25-26=-2428/228, 26-27=-2388/239, 4-27=-2387/241, 4-5=-2363/225, 5-28=-1971/231, 28-29=-1971/231, 29-30=-1973/230, 6-30=-1973/230, 6-31=-1973/230, 31-32=-1973/230, 32-33=-1971/231, 7-33=-1971/231, 7-8=-2363/225, 8-34=-2387/242, 34-35=-2388/239, 35-36=-2428/228, 9-36=-2437/218, 9-10=-1122/0
BOT CHORD	2-16=-209/1960, 15-16=-172/2667, 14-15=-172/2667, 13-14=-172/2667, 12-13=-172/2667, 10-12=-107/1960
WEBS	4-16=-306/141, 5-16=-10/880, 6-16=-949/184, 6-14=0/434, 6-12=-949/184, 7-12=-9/880, 8-12=-306/141

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 4-2-5, Exterior(2R) 4-2-5 to 13-9-11, Interior (1) 13-9-11 to 20-2-5, Exterior(2R) 20-2-5 to 29-9-11, Interior (1) 29-9-11 to 31-5-11, Exterior(2E) 31-5-11 to 34-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A03	Hip	1	1	Job Reference (optional)

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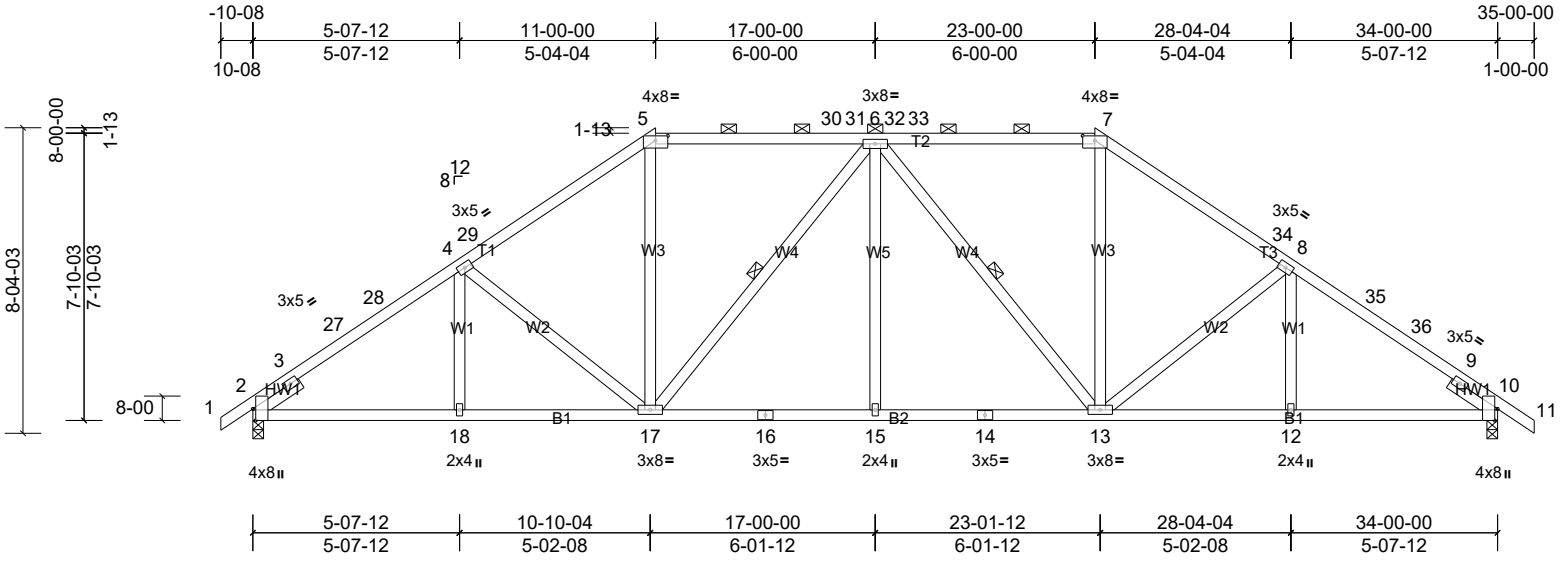

Scale = 1:63.2

Plate Offsets (X, Y): [2:3-13,Edge], [5:4-00,1-09], [7:4-00,1-09], [10:3-13,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.14	15-17	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.89	Vert(CT)	-0.25	15-17	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(CT)	0.10	10	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 207 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=1412/3-08, (min. 1-14), 10=1420/3-08, (min. 1-14)
Max Horiz 2=-185 (LC 12)
Max Uplift 2=-152 (LC 14), 10=-155 (LC 15)
Max Grav 2=1589 (LC 47), 10=1595 (LC 47)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-9-9 oc purlins, except 2-0-0 oc purlins (3-10-0 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-17, 6-13

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1324/0, 3-27=-2510/195, 27-28=-2438/197, 4-28=-2376/216, 4-29=-2148/204, 5-29=-2094/236, 5-30=-1758/239, 30-31=-1760/239, 6-31=-1760/239, 6-32=-1760/238, 32-33=-1760/238, 7-33=-1758/239, 7-34=-2094/235, 8-34=-2148/203, 8-35=-2375/215, 35-36=-2471/197, 9-36=-2508/194, 9-10=-1050/0
BOT CHORD	2-18=-204/2007, 17-18=-199/2007, 16-17=-77/2050, 15-16=-77/2050, 14-15=-77/2050, 13-14=-77/2050, 12-13=-73/2005, 10-12=-73/2005
WEBS	4-17=-487/160, 5-17=-33/820, 6-17=-577/145, 6-15=0/349, 6-13=-577/145, 7-13=-33/820, 8-13=-486/159

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 6-2-5, Exterior(2R) 6-2-5 to 15-9-11, Interior (1) 15-9-11 to 18-2-5, Exterior(2R) 18-2-5 to 27-9-11, Interior (1) 27-9-11 to 31-7-3 Exterior(2E) 31-7-3 to 35-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A04	Hip	1	1	Job Reference (optional)

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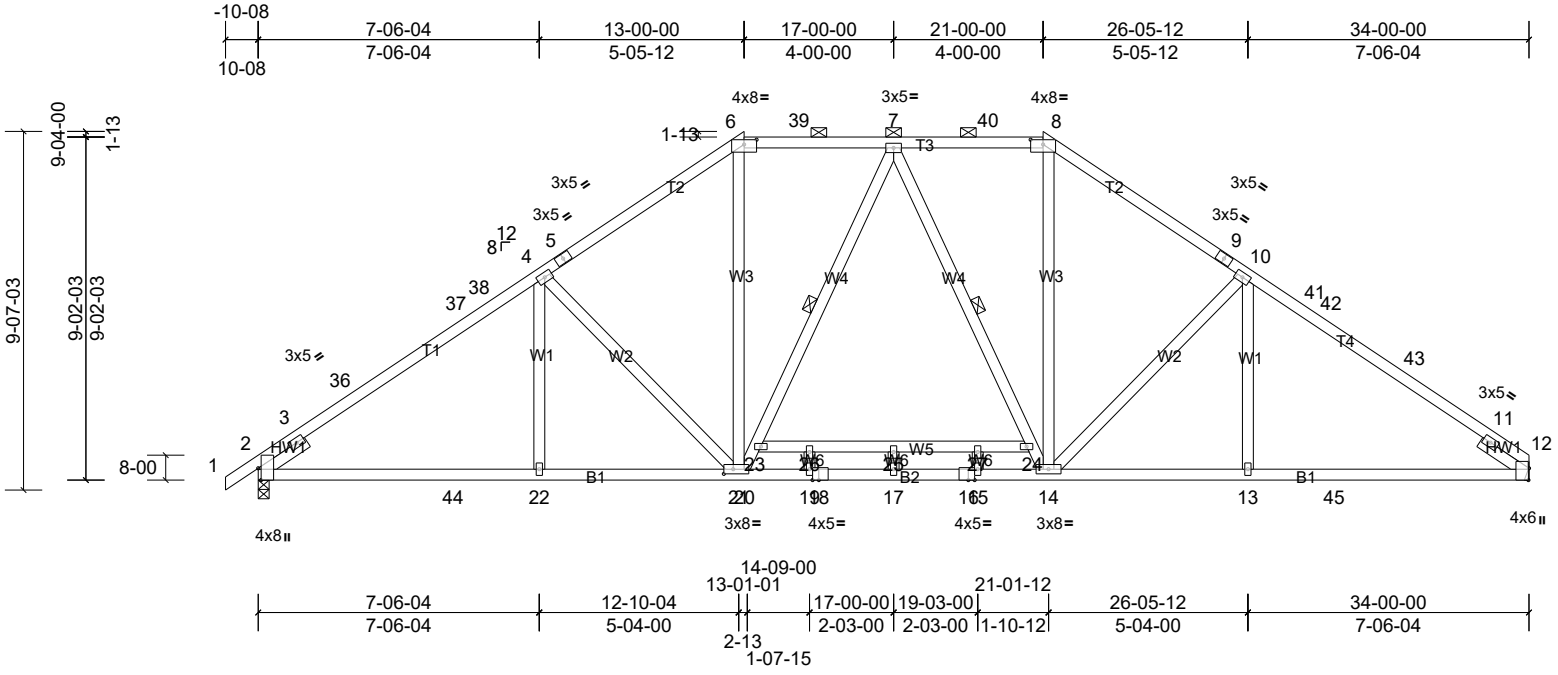

Scale = 1:61.9

Plate Offsets (X, Y): [2:3-13,Edge], [6:4-00,1-09], [8:4-00,1-09], [12:3-13,Edge], [16:2-00,Edge], [18:2-00,Edge], [21:3-00,1-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.15	17	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-0.40	17	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(CT)	0.10	12	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 219 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=1513/3-08, (min. 2-00), 12=1459/ Mechanical, (min. 1-08)
Max Horiz 2=211 (LC 11)
Max Uplift 2=-47 (LC 14), 12=-30 (LC 15)
Max Grav 2=1721 (LC 47), 12=1676 (LC 47)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-8-3 oc purlins, except 2-0-0 oc purlins (4-4-8 max.): 6-8.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 7-14, 7-20

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1352/0, 3-36=-2673/11, 36-37=-2582/19, 37-38=-2545/31, 4-38=-2496/44, 4-5=-2238/33, 5-6=-2109/64, 6-39=-1769/95, 7-39=-1770/94, 7-40=-1769/96, 8-40=-1768/96, 8-9=-2108/66, 9-10=-2237/35, 10-41=-2499/45, 41-42=-2548/32, 42-43=-2586/19, 11-43=-2676/17, 11-12=-1109/0

BOT CHORD 2-44=-269/2118, 22-44=-60/2118, 21-22=-60/2118, 20-21=0/1679, 19-20=0/1724, 18-19=0/1724, 17-18=0/1724, 16-17=0/1724, 15-16=0/1724, 14-15=0/1724, 13-14=0/2121, 13-45=0/2121, 12-45=0/2121

WEBS 6-21=0/870, 8-14=0/871, 10-14=-628/228, 10-13=0/252, 4-21=-623/229, 7-24=-285/160, 14-24=-334/105, 20-23=-349/108, 7-23=-290/161

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 8-2-5, Exterior(2R) 8-2-5 to 25-9-11, Interior (1) 25-9-11 to 30-7-3, Exterior(2E) 30-7-3 to 34-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
7) Provide adequate drainage to prevent water ponding.
8) All plates are 2x4 MT20 unless otherwise indicated.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
11) Refer to girder(s) for truss to truss connections.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 12.
13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A04	Hip	1	1	Job Reference (optional)

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A05	Hip	1	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:01:59 — Page: 1

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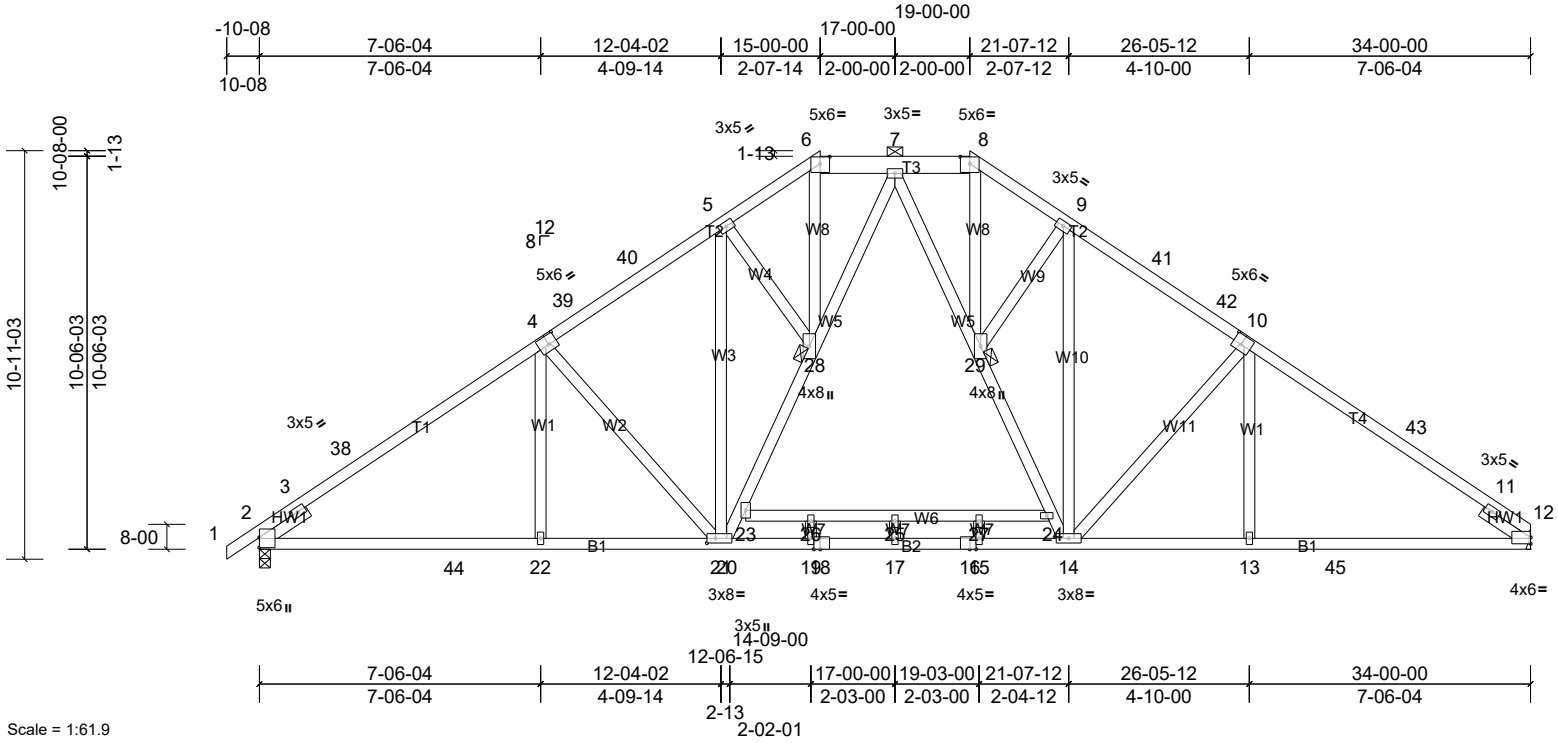

Scale = 1:61.9

Plate Offsets (X, Y): [4:2-12,3-00], [6:3-00,2-03], [8:3-00,2-03], [10:2-12,3-00], [12:Edge,2-01], [16:2-00,Edge], [18:2-00,Edge], [21:2-12,1-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.17	17	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.49	17	>838	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(CT)	0.09	12	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 253 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T3:2x6 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* W5:2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=1513/3-08, (min. 2-02), 12=1459/ Mechanical, (min. 1-08)
Max Horiz 2=242 (LC 11)
Max Uplift 2=-42 (LC 14), 12=-24 (LC 15)
Max Grav 2=1791 (LC 51), 12=1741 (LC 53)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (5-8-8 max.): 6-8.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
JOINTS	1 Brace at Jt(s): 28, 29

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1363/0, 3-38=-2738/1, 4-38=-2644/35, 4-39=-2378/13, 39-40=-2273/15, 5-40=-2271/41, 5-6=-2051/113, 6-7=-1652/115, 7-8=-1649/116, 8-9=-2048/114, 9-41=-2276/42, 41-42=-2279/17, 10-42=-2384/14, 10-43=-2646/36, 11-43=-2738/8, 11-12=-1121/0
BOT CHORD	2-44=-297/2169, 22-44=-69/2169, 21-22=-69/2164, 20-21=0/1818, 19-20=0/1466, 18-19=0/1466, 17-18=0/1466, 16-17=0/1466, 15-16=0/1466, 14-15=0/1466, 13-14=0/2166, 13-45=0/2171, 12-45=0/2171
WEBS	9-14=0/296, 10-14=-538/232, 5-21=0/284, 4-21=-542/232, 20-23=-121/679, 23-28=-62/695, 7-28=-259/160, 7-29=-269/156, 24-29=-66/671, 14-24=-105/652, 6-28=-18/981, 8-29=-20/978, 9-29=-415/17, 5-28=-402/16

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 10-2-5, Exterior(2R) 10-2-5 to 23-9-11, Interior (1) 23-9-11 to 30-7-3, Exterior(2E) 30-7-3 to 34-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
7) Provide adequate drainage to prevent water ponding.
8) All plates are 2x4 MT20 unless otherwise indicated.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
11) Refer to girder(s) for truss to truss connections.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 12.
13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A05	Hip	1	1	Job Reference (optional)

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A06	Piggyback Base	3	1	Job Reference (optional)

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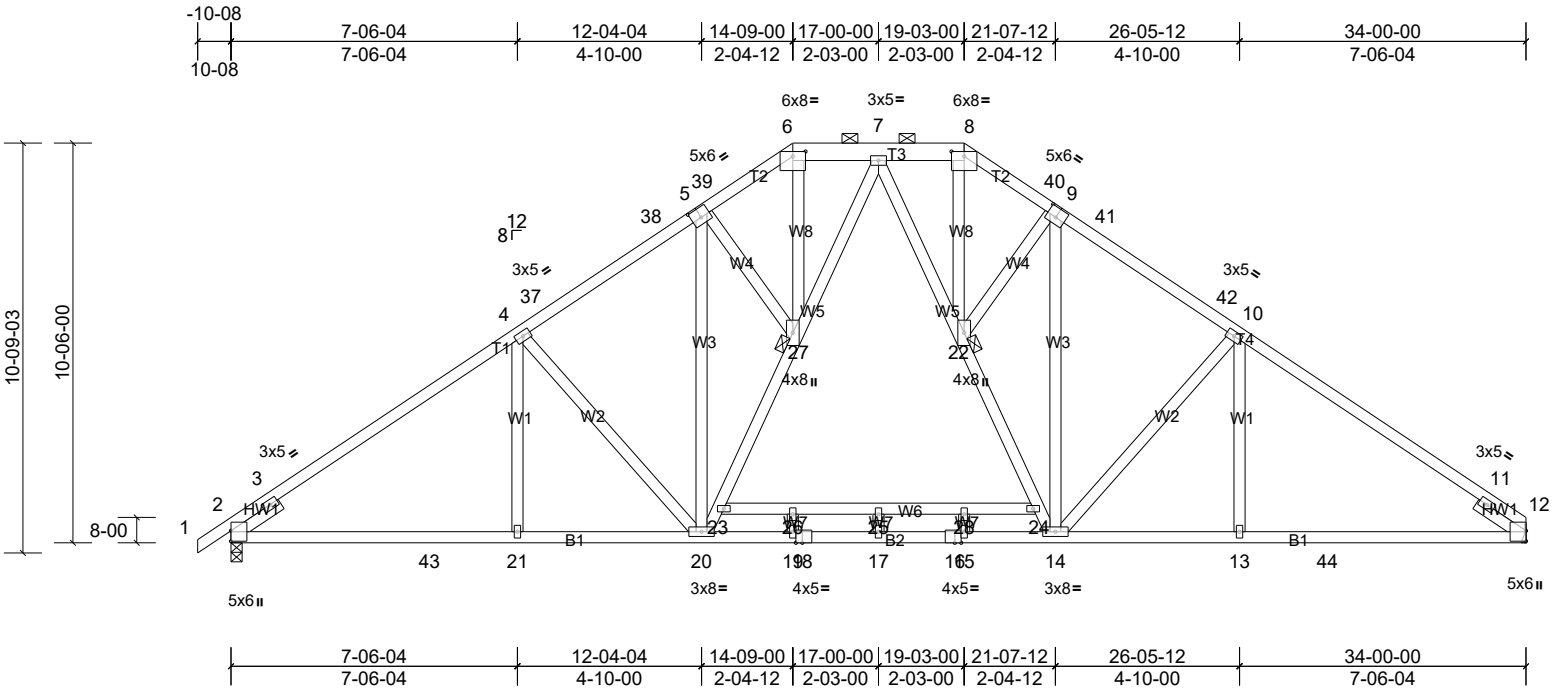

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Plate Offsets (X, Y): [5:3-00,3-00], [6:4-00,1-09], [8:4-00,1-09], [9:3-00,3-00], [16:2-00,Edge], [18:2-00,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.18	17	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.51	17	>800	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.09	12	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 252 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T3:2x6 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* W5:2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=1513/3-08, (min. 2-02), 12=1459/ Mechanical, (min. 1-08)
Max Horiz 2=240 (LC 11)
Max Uplift 2=-22 (LC 14), 12=-5 (LC 15)
Max Grav 2=1778 (LC 51), 12=1728 (LC 53)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except 2-0-0 oc purlins (5-7-8 max.): 6-8.
BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
JOINTS	1 Brace at Jt(s): 22, 27

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1404/0, 3-4=-2830/52, 4-37=-2470/45, 37-38=-2362/47, 5-38=-2293/74, 5-39=-2103/124, 6-39=-2084/139, 6-7=-1733/134, 7-8=-1733/135, 8-40=-2084/139, 9-40=-2103/124, 9-41=-2294/74, 41-42=-2362/47, 10-42=-2470/46, 10-11=-2832/54, 11-12=-1152/0
BOT CHORD	2-43=-282/2237, 21-43=-45/2237, 20-21=-45/2237, 19-20=0/1537, 18-19=0/1537, 17-18=0/1537, 16-17=0/1537, 15-16=0/1537, 14-15=0/1537, 13-14=0/2239, 13-44=0/2239, 12-44=0/2239
WEBS	8-22=-30/1009, 5-20=0/303, 4-20=-556/229, 9-14=0/304, 10-14=-559/231, 20-23=-116/634, 23-27=-70/667, 22-24=-69/666, 14-24=-111/629, 6-27=-30/1009, 5-27=-489/17, 9-22=-489/17

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-0, Interior (1) 2-6-0 to 11-4-8, Exterior(2R) 11-4-8 to 22-7-8, Interior (1) 22-7-8 to 30-7-8, Exterior(2E) 30-7-8 to 34-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
7) Provide adequate drainage to prevent water ponding.
8) All plates are 2x4 MT20 unless otherwise indicated.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
11) Refer to girder(s) for truss to truss connections.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 12.
13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Continued on page 2

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A06	Piggyback Base	3	1	Job Reference (optional)

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	A07	Piggyback Base Girder	1	2	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:00 — Page: 1

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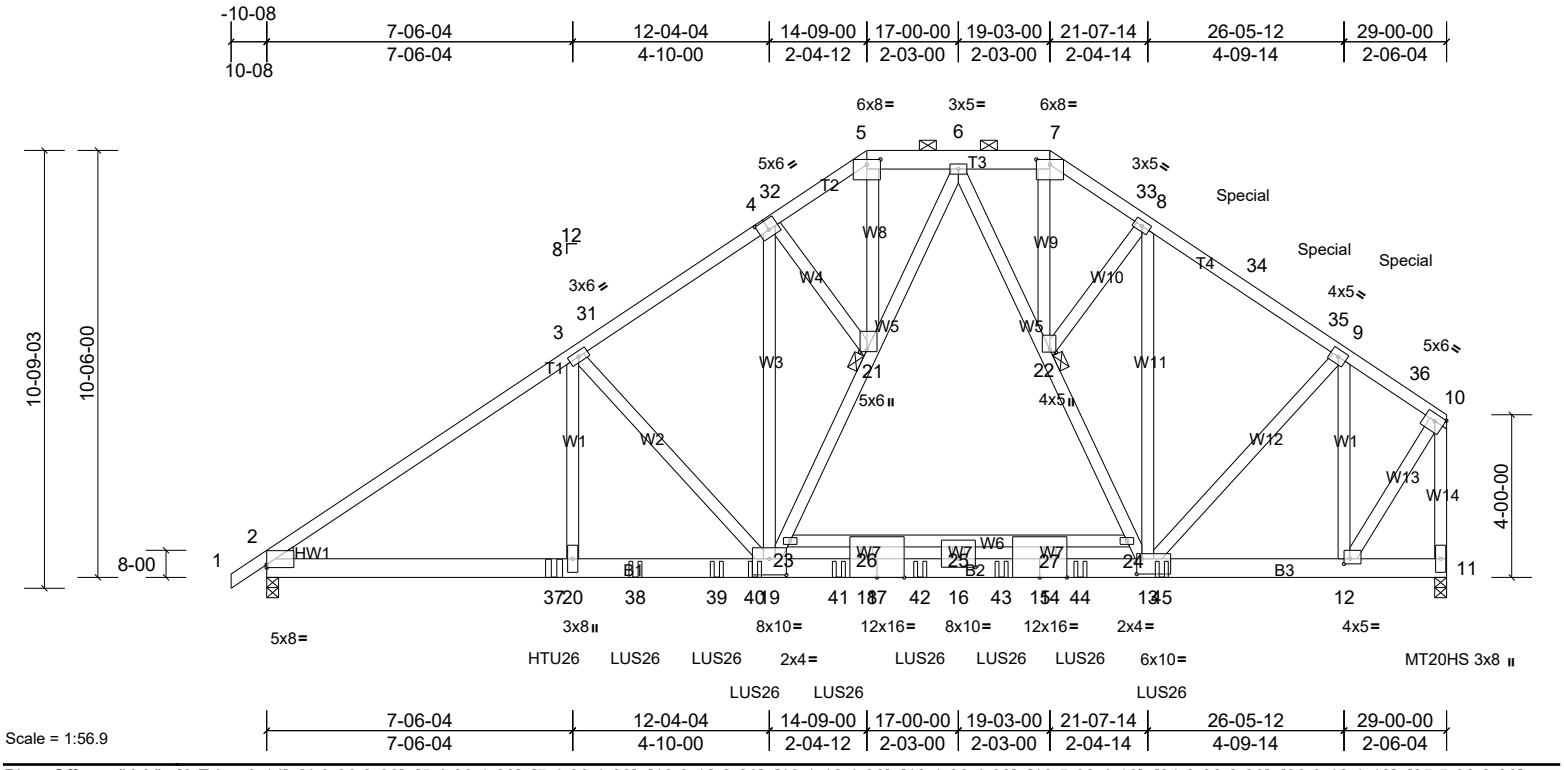

Plate Offsets (X, Y): [2:Edge,0-14], [4:3-00,3-00], [5:4-00,1-09], [7:4-00,1-09], [10:2-12,2-00], [12:1-12,1-08], [13:1-08,4-08], [19:5-00,4-12], [21:0-08,2-00], [22:0-12,1-12], [25:5-00,2-08]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.25	16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.76	Vert(CT)	-0.46	16-18	>756	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	0.70	Horz(CT)	0.05	11	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 530 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T3:2x6 SP No.2, T4:2x4 SP No.1, T1:2x4 SP 2400F 2.0E
BOT CHORD	2x6 SP 2400F 2.0E
WEBS	2x4 SP No.3 *Except* W13,W5,W6:2x4 SP No.2
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-7.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS	1 Brace at Jt(s): 21, 22

REACTIONS (lb/size) 2=5690/3-08, (min. 2-07), 11=6348/3-08, (min. 2-11)
Max Horiz 2=280 (LC 11)
Max Uplift 2=-964 (LC 12), 11=-1108 (LC 12)
Max Grav 2=5820 (LC 37), 11=6497 (LC 37)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-9296/1560, 3-31=-7592/1252, 4-31=-7476/1280, 4-32=-6248/1124, 5-32=-6226/1139, 5-6=-5238/984, 6-7=-3905/746, 7-33=-4732/884, 8-33=-4751/870, 8-34=-5357/925, 34-35=-5767/995, 9-35=-6104/1063, 9-36=-2967/547, 10-36=-3366/622, 10-11=-6151/1078

BOT CHORD 2-37=-1337/7610, 20-37=-1337/7610, 20-38=-1337/7610, 38-39=-1337/7610, 39-40=-1337/7610, 19-40=-1337/7610, 19-41=-717/4478, 18-41=-717/4478, 17-18=-717/4478, 17-42=-717/4478, 16-42=-717/4478, 16-43=-717/4478, 15-43=-717/4478, 14-15=-717/4478, 14-44=-717/4478, 13-44=-717/4478, 13-45=-479/2691, 12-45=-479/2691

WEBS 8-13=-141/871, 4-19=-298/1820, 3-20=-408/2094, 3-19=-2058/552, 9-12=-4892/847, 9-13=-392/2835, 10-12=-830/4897, 19-23=-687/3905, 21-23=-683/3926, 6-21=-288/1687, 6-22=-1820/347, 22-24=-193/433, 13-24=-224/394, 5-21=-583/3332, 4-21=-1704/300, 7-22=-498/2592, 8-22=-1236/238

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 7-00 oc, 2x6 - 2 rows staggered at 9-00 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 5-00 oc.
Web connected as follows: 2x4 - 1 row at 9-00 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
6) Unbalanced snow loads have been considered for this design.
7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
8) 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
9) Provide adequate drainage to prevent water ponding.
10) All plates are MT20 plates unless otherwise indicated.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Continued on page 2

12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
13) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.
14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
15) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent at 7-0-12 from the left end to connect truss(es) G01 (1 ply 2x6 SP) to back face of bottom chord.
16) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-12 oc max. starting at 9-0-12 from the left end to 22-0-0 to connect truss(es) G02 (1 ply 2x4 SP), G03 (1 ply 2x4 SP), G04 (1 ply 2x4 SP), D06 (1 ply 2x4 SP), D05 (1 ply 2x4 SP), D04 (1 ply 2x4 SP), D03 (1 ply 2x4 SP), D02 (1 ply 2x4 SP) to back face of bottom chord.
17) Fill all nail holes where hanger is in contact with lumber.
18) LGT2 Hurricane ties must have two studs in line below the truss.
19) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 637 lb down and 175 lb up at 24-0-0, and 637 lb down and 175 lb up at 26-0-0, and 638 lb down and 174 lb up at 28-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-5=-58, 5-7=-58, 7-10=-58, 11-28=-19
   Concentrated Loads (lb)
   Vert: 18=-100, 14=-100, 34=-591, 35=-591, 36=-595, 37=-2124, 38=-732, 39=-656, 40=-645, 41=-723, 42=-784, 43=-784, 44=-698, 45=-629

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	B01GRD	Common Girder	1	2	Job Reference (optional)

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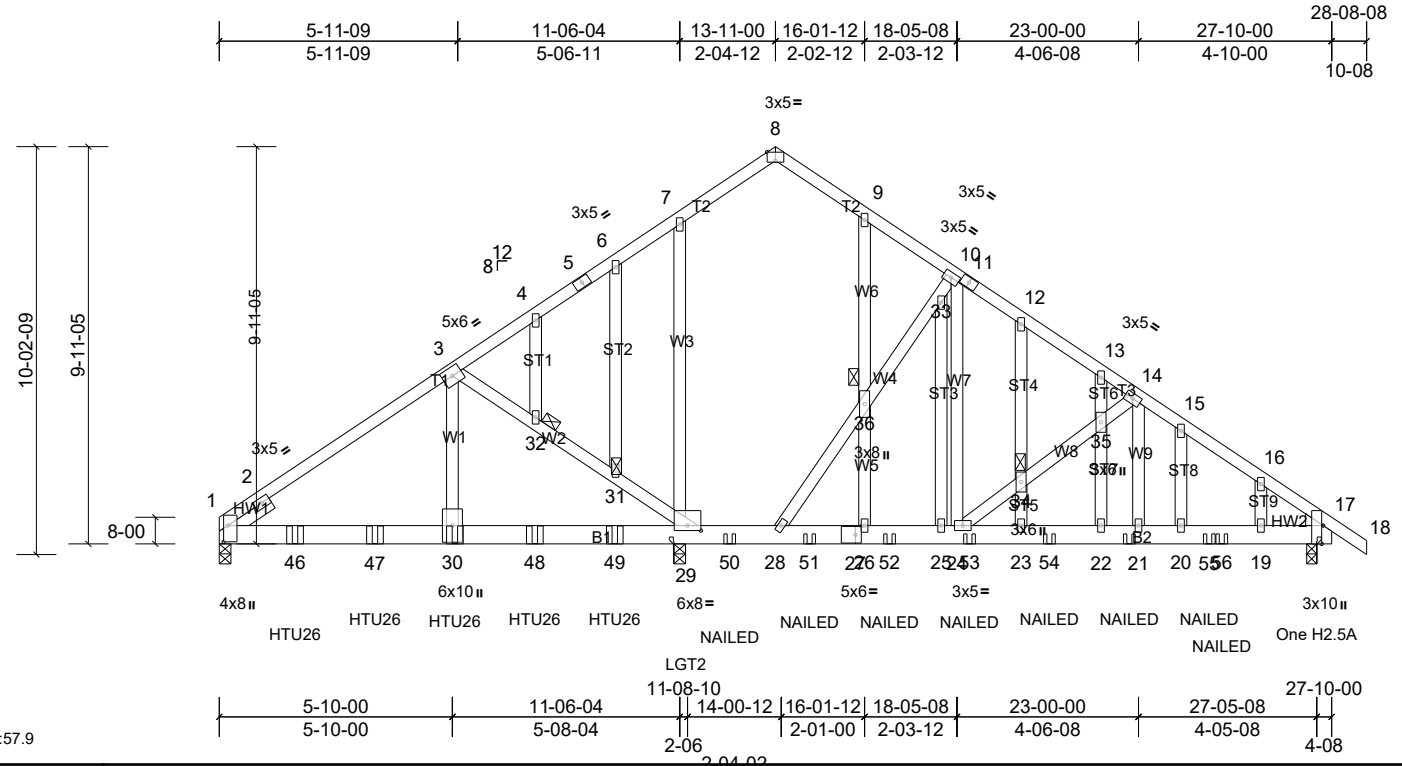


Scale = 1:57.9

Plate Offsets (X, Y): [1:4-14,1-04], [8:2-08,Edge], [17:5-08,Edge], [27:1-12,2-08], [29:4-00,1-08]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	Vert(LL)	-0.07	30-39	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.65	Vert(CT)	-0.13	30-39	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.69	Horz(CT)	0.03	17	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 460 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP 2400F 2.0E *Except* B2:2x6 SP No.2
WEBS	2x4 SP No.3 *Except* W1:2x4 SP No.2
OTHERS	2x4 SP No.3
WEDGE	Right: 2x4 SP 2400F 2.0E
SLIDER	Left 2x4 SP No.3 -- 1-02-13

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-2-11 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS	1 Brace at Jt(s): 31, 32, 34, 36

REACTIONS (lb/size) 1=4374/3-08, (min. 1-14), 17=2556/3-00, (min. 1-09), 29=6129/3-08, (min. 2-10)
Max Horiz 1=-220 (LC 36)
Max Uplift 1=-55 (LC 12), 17=-535 (LC 13), 29=-567 (LC 12)
Max Grav 1=4535 (LC 22), 17=2640 (LC 20), 29=6320 (LC 22)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-4459/47, 2-3=-4932/28, 6-7=-352/100, 8-9=-255/84, 9-10=-391/112, 10-11=-1312/360, 11-12=-1409/360, 12-13=-1505/345, 13-14=-1534/315, 14-15=-2451/562, 15-16=-2599/549, 16-17=-2620/515

BOT CHORD 1-46=-119/4189, 46-47=-119/4189, 30-47=-119/4189, 30-48=-119/4189, 48-49=-119/4189, 29-49=-119/4189, 28-51=-136/1192, 27-51=-136/1192, 26-27=-136/1192, 26-52=-136/1192, 25-52=-136/1192, 24-25=-136/1192, 24-53=-366/2059, 23-53=-366/2059, 23-54=-366/2059, 22-54=-366/2059, 21-22=-366/2059, 20-21=-366/2059, 20-55=-366/2059, 55-56=-366/2059, 19-56=-366/2059, 17-19=-366/2059

WEBS 28-36=-1794/579, 33-36=-1749/561, 10-33=-1460/479, 10-24=-331/1208, 24-34=-1189/338, 34-35=-1151/330, 14-35=-1103/317, 14-21=-226/796, 3-32=-4823/179, 31-32=-4854/184, 29-31=-5085/206, 6-31=-390/36, 25-33=-106/371, 7-29=-73/258, 3-30=-22/5169

NOTES

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
   Top chords connected as follows: 2x4 - 1 row at 9-00 oc.
   Bottom chords connected as follows: 2x6 - 2 rows staggered at 6-00 oc.
   Web connected as follows: 2x4 - 1 row at 9-00 oc, Except member 3-30 2x4 - 1 row at 5-00 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Unbalanced roof live loads have been considered for this design.
4) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
5) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
6) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
7) Unbalanced snow loads have been considered for this design.
8) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

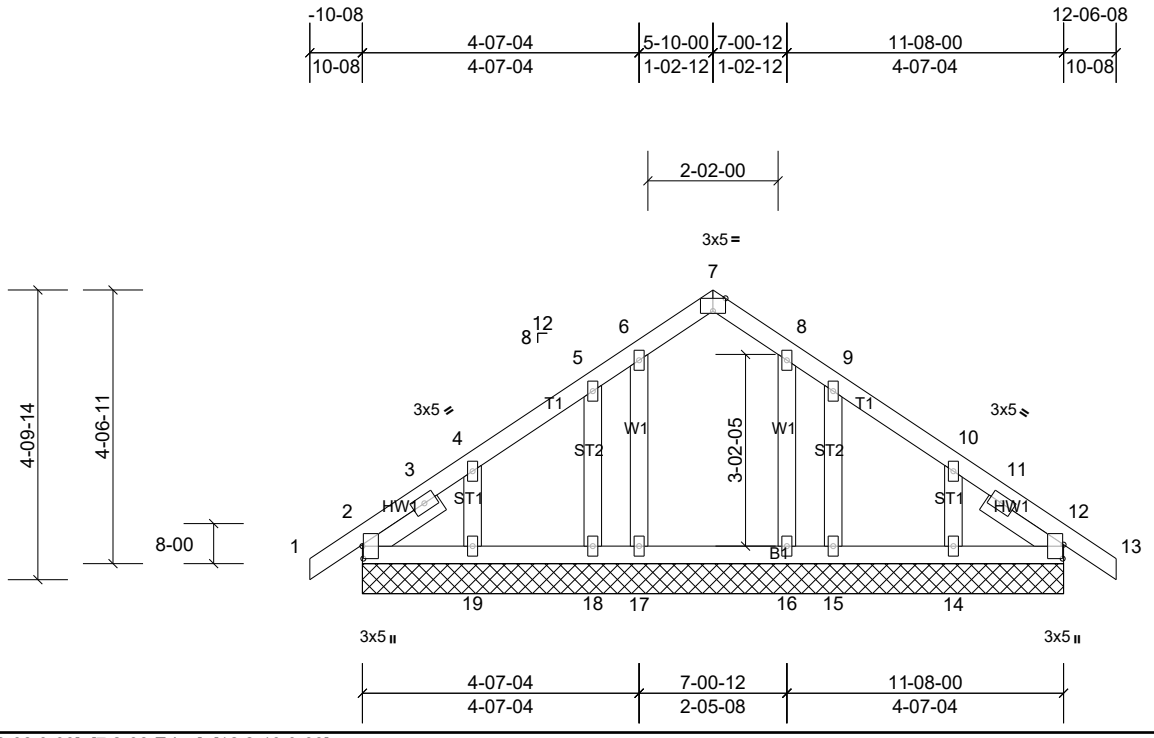
Continued on page 2

9) All plates are 2x4 MT20 unless otherwise indicated.
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 17. This connection is for uplift only and does not consider lateral forces.
14) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 29. This connection is for uplift only and does not consider lateral forces.
15) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-10-12 from the left end to 9-10-12 to connect truss(es) A04 (1 ply 2x4 SP), A05 (1 ply 2x4 SP), A06 (1 ply 2x4 SP) to back face of bottom chord.
16) Fill all nail holes where hanger is in contact with lumber.
17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
18) LGT2 Hurricane ties must have two studs in line below the truss.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-8=-58, 17-18=-58, 28-37=-19
   Concentrated Loads (lb)
   Vert: 21=-74, 30=-1614, 46=-1569, 47=-1624, 48=-1614, 49=-1614, 50=-74, 51=-74, 52=-74, 53=-74, 54=-74, 55=-74, 56=-108
   Trapezoidal Loads (lb/ft)
   Vert: 8=-58-to-9=-83 (F=-25), 9=-83 (F=-25)-to-10=-110 (F=-52), 10=-110 (F=-52)-to-11=-113 (F=-55), 11=-113 (F=-55)-to-12=-128 (F=-70), 12=-128 (F=-70)-to-13=-151 (F=-92), 13=-151 (F=-92)-to-14=-161 (F=-103), 14=-161 (F=-103)-to-15=-173 (F=-115), 15=-173 (F=-115)-to-16=-196 (F=-138), 16=-196 (F=-138)-to-44=-209 (F=-151), 44=-209 (F=-151)-to-42=-212 (F=-154), 42=-212 (F=-154)-to-17=-216 (F=-158), 28=-19 (F=0)-to-51=-29 (F=-10), 51=-29 (F=-10)-to-27=-42 (F=-23), 27=-42 (F=-23)-to-26=-45 (F=-25), 26=-45 (F=-25)-to-52=-52 (F=-32), 52=-52 (F=-32)-to-25=-66 (F=-47), 25=-66 (F=-47)-to-24=-71 (F=-52), 24=-71 (F=-52)-to-53=-74 (F=-55), 53=-74 (F=-55)-to-23=-89 (F=-70), 23=-89 (F=-70)-to-54=-97 (F=-78), 54=-97 (F=-78)-to-22=-112 (F=-92), 22=-112 (F=-92)-to-21=-123 (F=-103), 21=-123 (F=-103)-to-20=-135 (F=-115), 20=-135 (F=-115)-to-55=-143 (F=-123), 55=-143 (F=-123)-to-56=-146 (F=-127), 56=-146 (F=-127)-to-19=-157 (F=-138), 19=-157 (F=-138)-to-45=-170 (F=-151), 45=-170 (F=-151)-to-43=-173 (F=-154), 43=-173 (F=-154)-to-41=-177 (F=-158)

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	C01G	Common Supported Gable	1	1	Job Reference (optional)



Scale = 1:38.5

Plate Offsets (X, Y): [2:2-08,0-03], [7:2-08,Edge], [12:2-13,0-03]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	12	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 68 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 11-08-00.
(lb) - Max Horiz 2=-102 (LC 12), 20=-102 (LC 12)
Max Uplift All uplift 100 (lb) or less at joint(s) 2, 14, 15, 18, 19, 20
Max Grav All reactions 250 (lb) or less at joint(s) 2, 12, 14, 15, 16, 17, 18, 19, 20, 24

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 1-10-0, Exterior(2N) 1-10-0 to 2-10-0, Corner(3R) 2-10-0 to 8-10-0, Exterior(2N) 8-10-0 to 9-6-8, Corner(3E) 9-6-8 to 12-6-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) All plates are 2x4 MT20 unless otherwise indicated.
8) Gable requires continuous bottom chord bearing.
9) Gable studs spaced at 2-0-0 oc.
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 19, 15, 14.
13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 12, 24.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	C02	Common	2	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:02 — Page: 1

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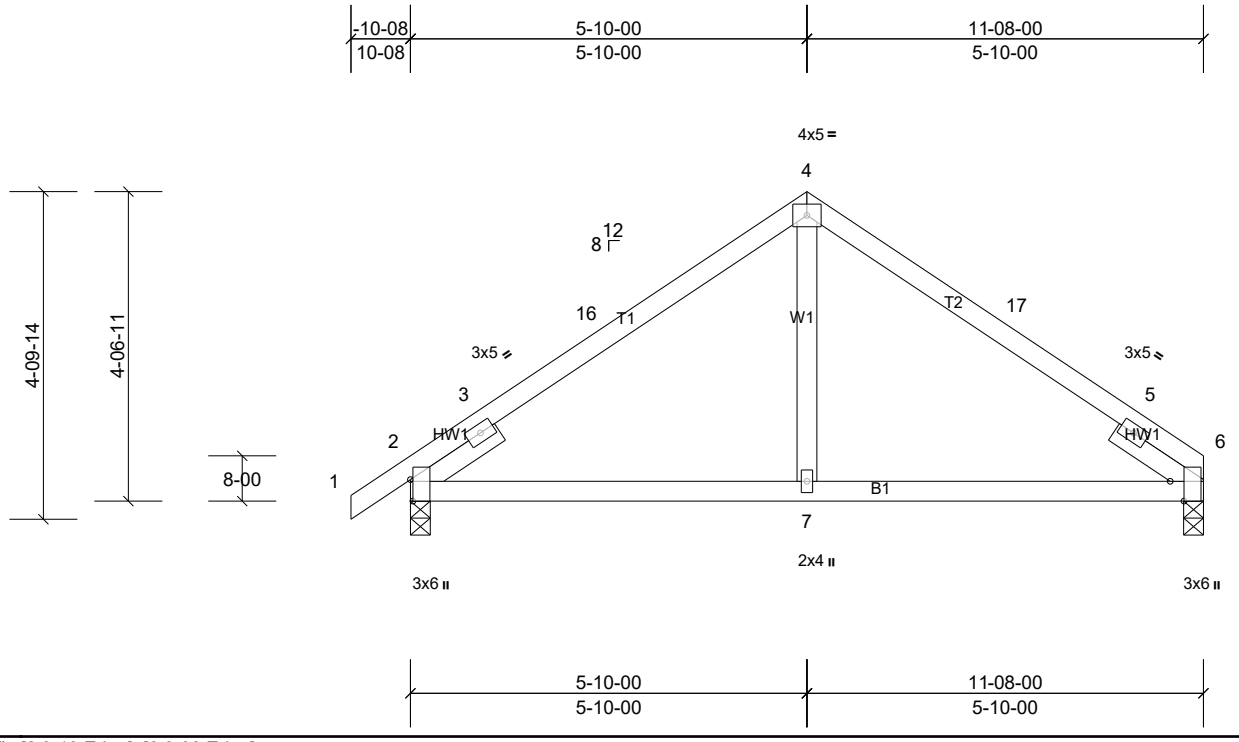

Scale = 1:34

Plate Offsets (X, Y): [2:3-13,Edge], [6:3-08,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.65	Vert(LL)	-0.07	7-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.09	7-10	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 51 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=521/3-08, (min. 1-08), 6=465/3-08, (min. 1-08)
Max Horiz 2=100 (LC 11)
Max Uplift 2=-56 (LC 14), 6=-39 (LC 15)
Max Grav 2=619 (LC 21), 6=564 (LC 22)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-11-7 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-284/29, 3-16=-513/116, 4-16=-504/136, 4-17=-504/135, 5-17=-508/117, 5-6=-273/0
BOT CHORD	2-7=-163/397, 6-7=-15/396
WEBS	4-7=0/257

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-10-0, Exterior(2R) 2-10-0 to 8-8-0, Exterior(2E) 8-8-0 to 11-8-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	CJ04	Diagonal Hip Girder	1	1	Job Reference (optional)

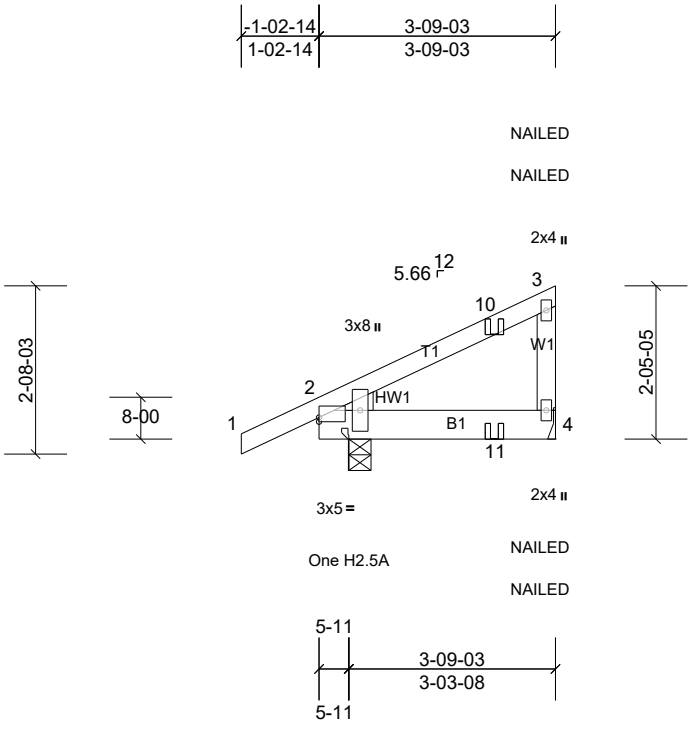

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Plate Offsets (X, Y): [2:Edge,0-13]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	0.00	4-9	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	0.00	4-9	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 21 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-9-3 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/4-04, (min. 1-08), 4=97/ Mechanical, (min. 1-08)
Max Horiz 2=82 (LC 11)
Max Uplift 2=-46 (LC 12), 4=-31 (LC 9)
Max Grav 2=395 (LC 19), 4=135 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
10) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-60, 4-5=-20

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	CJ09	Diagonal Hip Girder	3	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:02 — Page: 1

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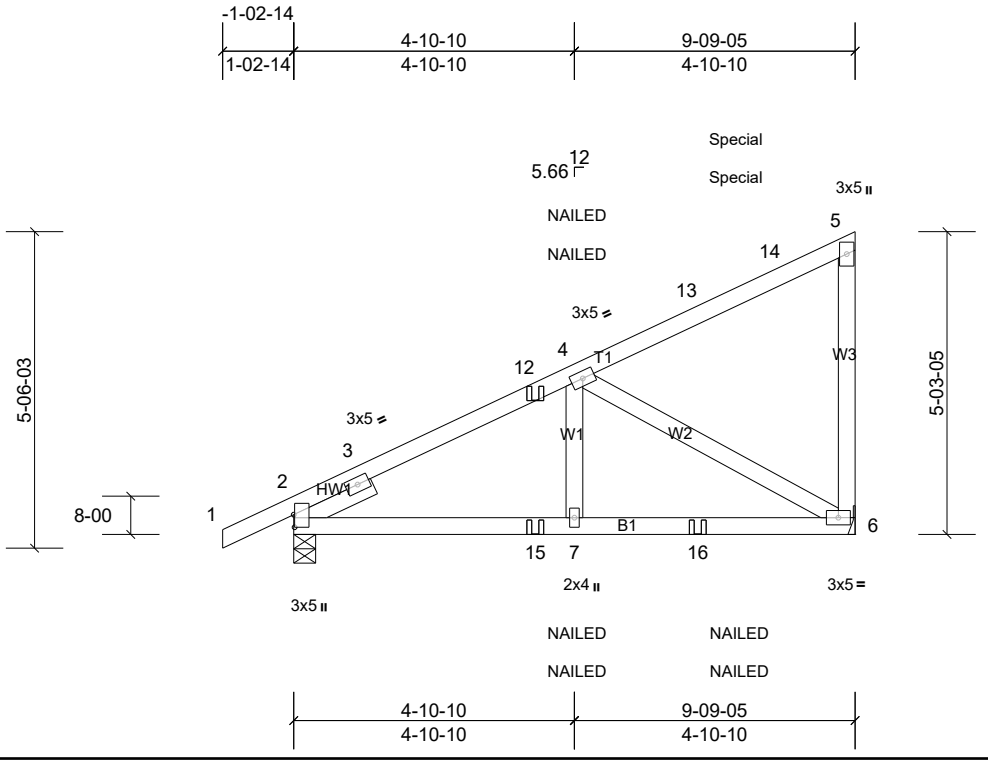

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Plate Offsets (X, Y): [2:2-10,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.04	6-7	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.07	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 53 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.2 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-8-11 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=605/4-09, (min. 1-08), 6=673/ Mechanical, (min. 1-08)
Max Horiz 2=171 (LC 11)
Max Uplift 2=-92 (LC 12), 6=-137 (LC 9)
Max Grav 2=605 (LC 1), 6=714 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-404/38, 3-12=-772/131, 4-12=-700/101, 5-6=-251/84
BOT CHORD	2-15=-178/686, 7-15=-178/686, 7-16=-178/686, 6-16=-178/686
WEBS	4-6=-755/197

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 6.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
10) "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidlines.
11) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 162 lb down and 87 lb up at 7-0-7, and 162 lb down and 87 lb up at 7-0-7 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-5=-60, 6-8=-20
   Concentrated Loads (lb)
   Vert: 12=-50, 13=-245, 15=-32, 16=-106

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	D01	Common	2	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:02 — Page: 1

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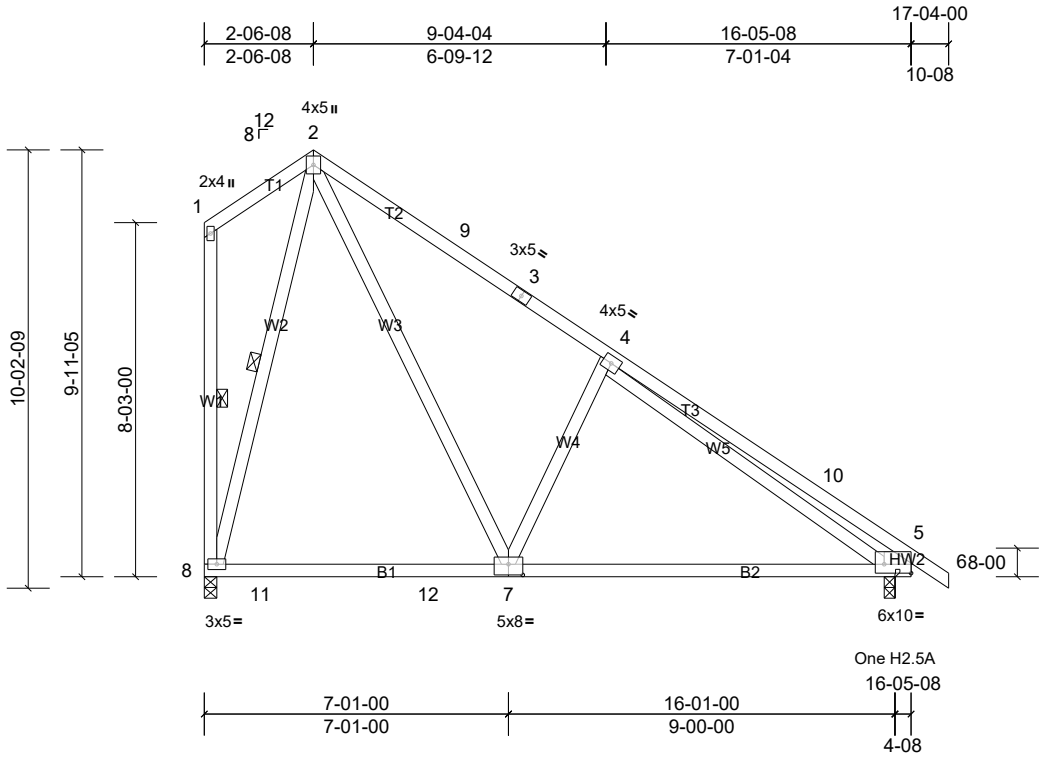

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Plate Offsets (X, Y): [5:Edge,2-08], [7:4-00,3-00]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	0.20	5-7	>982	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.84	Vert(CT)	-0.37	5-7	>532	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 118 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* W3:2x4 SP No.2
SLIDER	Right 2x4 SP No.2 -- 7-08

REACTIONS (lb/size) 5=709/3-00, (min. 1-08), 8=646/3-08, (min. 1-08)
Max Horiz 8=-323 (LC 12)
Max Uplift 5=-69 (LC 15), 8=-155 (LC 10)
Max Grav 5=764 (LC 6), 8=739 (LC 6)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-5-3 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 1-8, 2-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-9=-672/187, 3-9=-681/166, 3-4=-784/145, 4-10=-830/111, 5-10=-887/78
BOT CHORD	8-11=-100/272, 11-12=-100/272, 7-12=-100/272, 5-7=-25/673
WEBS	2-7=-347/770, 4-7=-493/262, 2-8=-666/417

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 11-6-4 to 13-11-0, Exterior(2R) 13-11-0 to 16-11-0, Interior (1) 16-11-0 to 25-8-8, Exterior(2E) 25-8-8 to 28-8-8 zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	D02	Scissor	4	1	Job Reference (optional)

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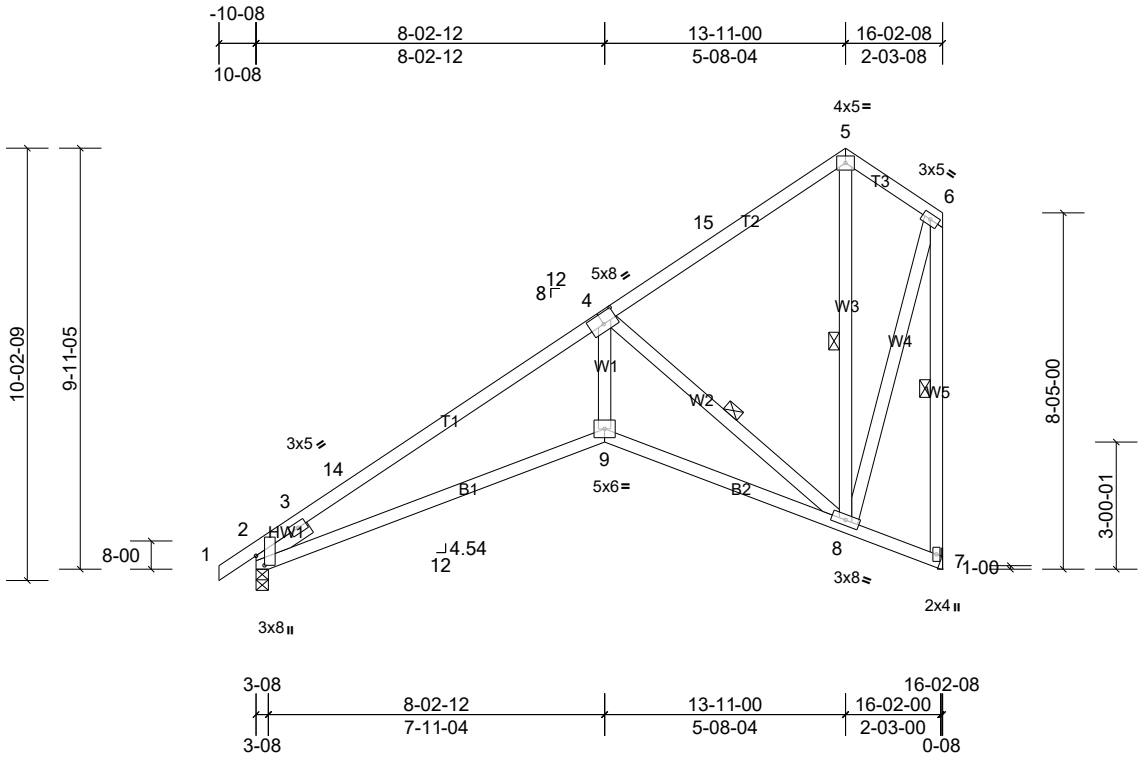


Scale = 1:54.6

Plate Offsets (X, Y): [2:2-11,2-07], [4:4-00,3-00]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	0.12	9-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.25	9-12	>784	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.10	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 109 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=329 (LC 14)
Max Uplift 2=-26 (LC 14), 7=-161 (LC 14)
Max Grav 2=711 (LC 21), 7=649 (LC 21)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-3-1 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 8-4-14 oc bracing.
WEBS	1 Row at midpt 6-7, 5-8, 4-8

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

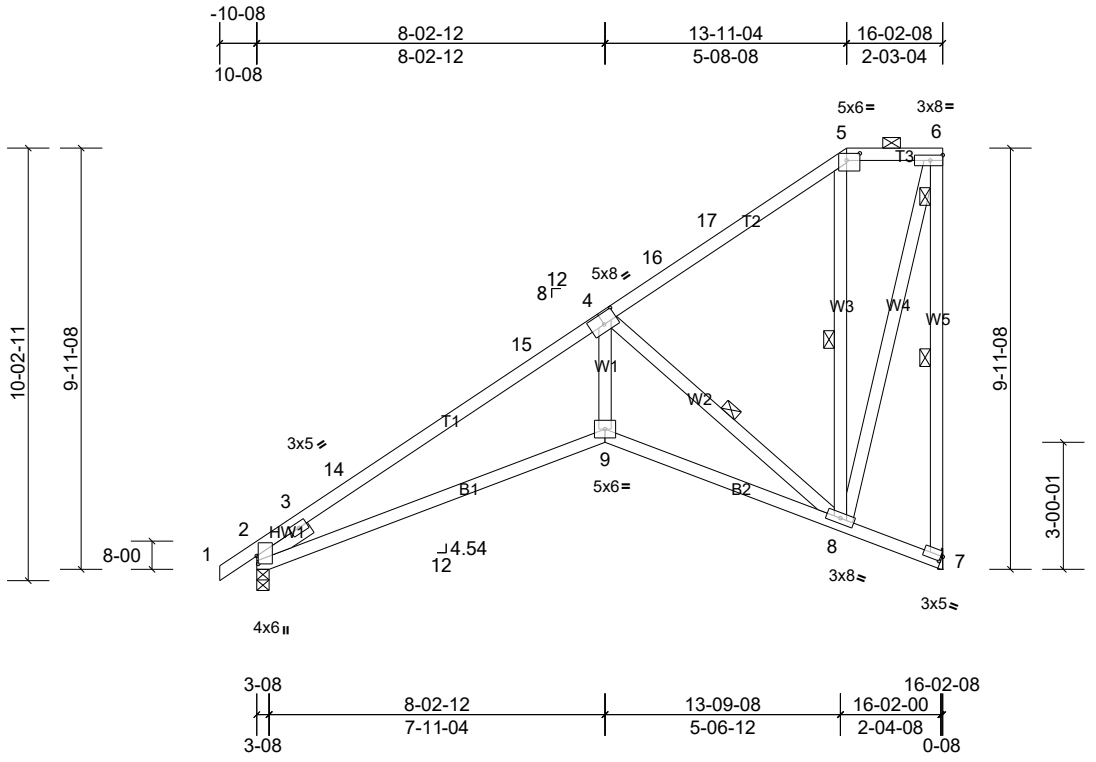
TOP CHORD	2-3=-790/0, 3-14=-1432/216, 4-14=-1342/256, 4-15=-318/6, 6-7=-712/157
BOT CHORD	2-9=-449/1305, 8-9=-450/1340
WEBS	6-8=-133/577, 4-9=-248/1060, 4-8=-1402/486

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 10-11-0, Exterior(2R) 10-11-0 to 13-11-0, Exterior(2E) 13-11-0 to 16-0-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) Refer to girder(s) for truss to truss connections.
9) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 7.
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	D03	Half Hip	1	1	Job Reference (optional)



Scale = 1:54.7

Plate Offsets (X, Y): [2:2-04,0-07], [4:4-00,3-00], [5:3-12,2-00], [7:0-09,1-08]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	1.00	Vert(LL)	0.12	9-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.24	9-12	>808	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.13	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 112 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-1-3 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7, 5-8, 4-8

REACTIONS (lb/size) 2=675/3-08, (min. 1-08), 7=621/ Mechanical, (min. 1-08)
Max Horiz 2=339 (LC 13)
Max Uplift 2=-59 (LC 14), 7=-122 (LC 14)
Max Grav 2=755 (LC 38), 7=716 (LC 38)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-864/0, 3-14=-1625/153, 14-15=-1535/166, 4-15=-1442/191, 4-16=-392/114, 16-17=-279/128, 5-17=-260/148, 6-7=-816/127
BOT CHORD	2-9=-339/1677, 8-9=-313/1724
WEBS	5-8=-272/143, 6-8=-121/882, 4-9=-148/1316, 4-8=-1700/351

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 10-11-4, Exterior(2R) 10-11-4 to 13-11-4, Exterior(2E) 13-11-4 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Refer to girder(s) for truss to truss connections.
10) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 7.
12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	D04	Half Hip	1	1	Job Reference (optional)

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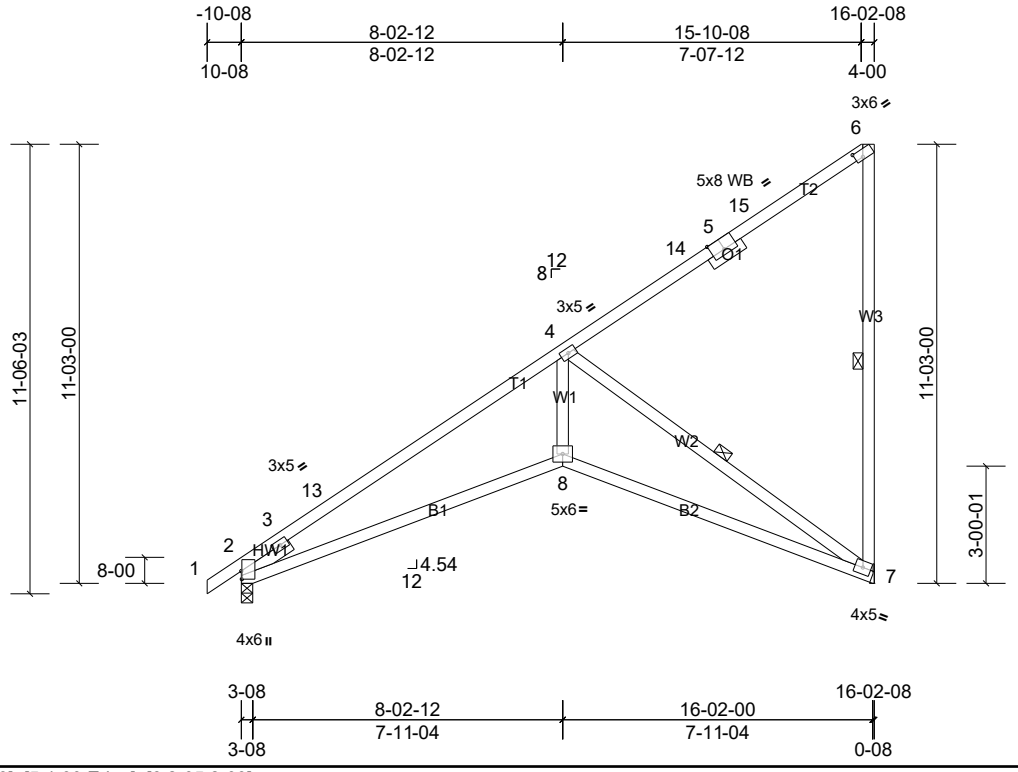


Scale = 1:59.3

Plate Offsets (X, Y): [2:2-08,0-03], [5:4-00,Edge], [6:2-05,2-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.16	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.73	Vert(CT)	-0.33	7-8	>583	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.14	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 94 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* W3:2x4 SP No.1
OTHERS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=400 (LC 13)
Max Uplift 2=-47 (LC 14), 7=-174 (LC 14)
Max Grav 2=724 (LC 21), 7=803 (LC 21)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-1-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-6-13 oc bracing.
WEBS	1 Row at midpt 6-7, 4-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

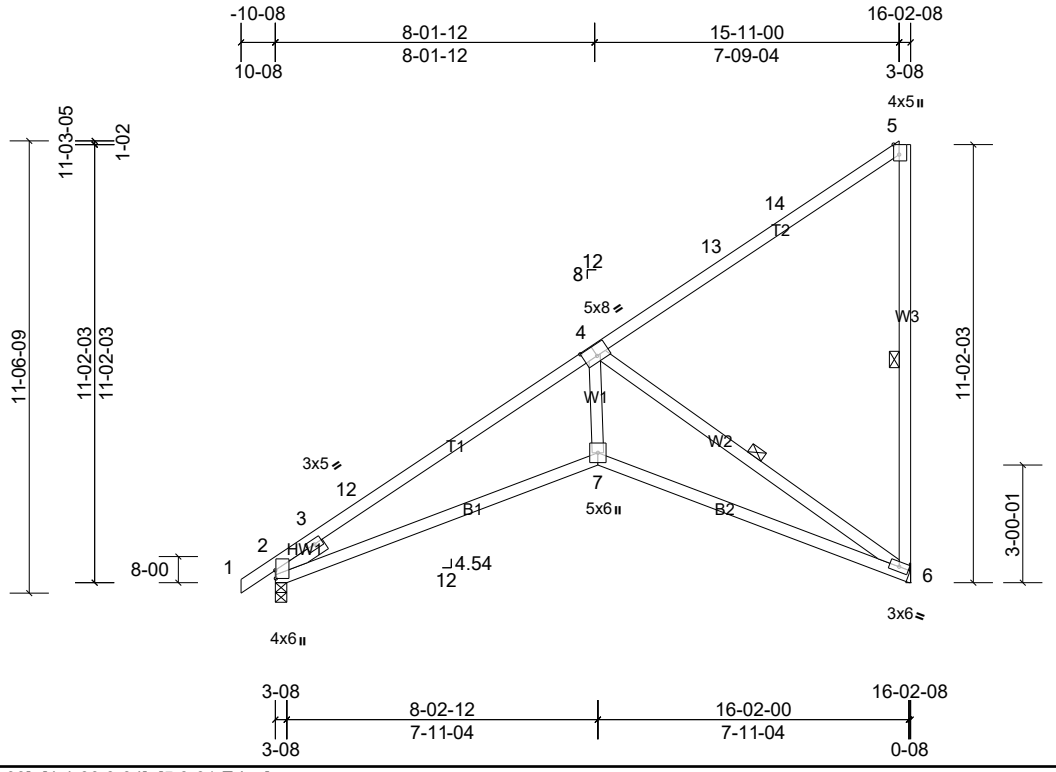
TOP CHORD	2-3=-884/0, 3-13=-1491/147, 4-13=-1275/188, 4-14=-275/176, 6-7=-319/103
BOT CHORD	2-8=-360/1583, 7-8=-325/1581
WEBS	4-8=-125/1245, 4-7=-1671/406

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 7.
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	D05	Half Hip	1	1	Job Reference (optional)



Scale = 1:59

Plate Offsets (X, Y): [2:2-08,0-03], [4:4-00,3-04], [5:3-01,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.16	6-7	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.74	Vert(CT)	-0.33	6-7	>576	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.15	6	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 92 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.1 *Except* T1:2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* W3:2x4 SP No.1
SLIDER	Left 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 6=641/ Mechanical, (min. 1-08)
Max Horiz 2=400 (LC 13)
Max Uplift 2=-47 (LC 14), 6=-174 (LC 14)
Max Grav 2=724 (LC 21), 6=803 (LC 21)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 9-5-9 oc bracing.
WEBS	1 Row at midpt 5-6, 4-6

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-878/0, 3-12=-1496/152, 4-12=-1283/192, 4-13=-277/164, 5-6=-325/105
BOT CHORD	2-7=-360/1491, 6-7=-332/1537
WEBS	4-7=-128/1196, 4-6=-1606/413

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 6.
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	D06	Half Hip	1	1	Job Reference (optional)

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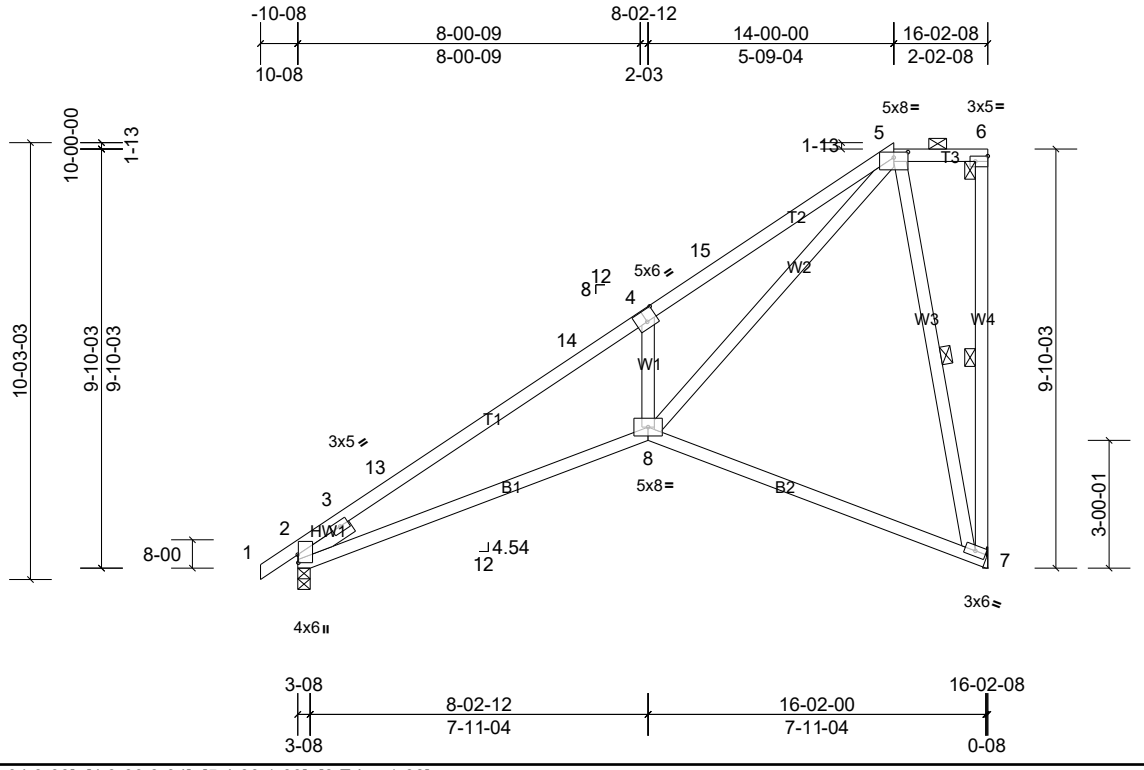

Scale = 1:54.4

Plate Offsets (X, Y): [2:2-04,0-03], [4:3-00,3-04], [5:4-00,1-09], [6:Edge,1-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.16	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.33	7-8	>588	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(CT)	0.13	7	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 103 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3 *Except* W4:2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-7-2 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=346 (LC 13)
Max Uplift 2=-62 (LC 14), 7=-126 (LC 14)
Max Grav 2=779 (LC 38), 7=742 (LC 38)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-878/0, 3-13=-1642/155, 13-14=-1527/168, 4-14=-1422/195, 4-15=-1681/355, 5-15=-1545/390
BOT CHORD	2-8=-341/1637, 7-8=-152/324
WEBS	5-8=-386/1831, 5-7=-902/216, 4-8=-556/301

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 9-9-1, Exterior(2R) 9-9-1 to 14-0-0, Exterior(2E) 14-0-0 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Refer to girder(s) for truss to truss connections.
10) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 7.
12) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	E01	Jack-Open	9	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:04 — Page: 1

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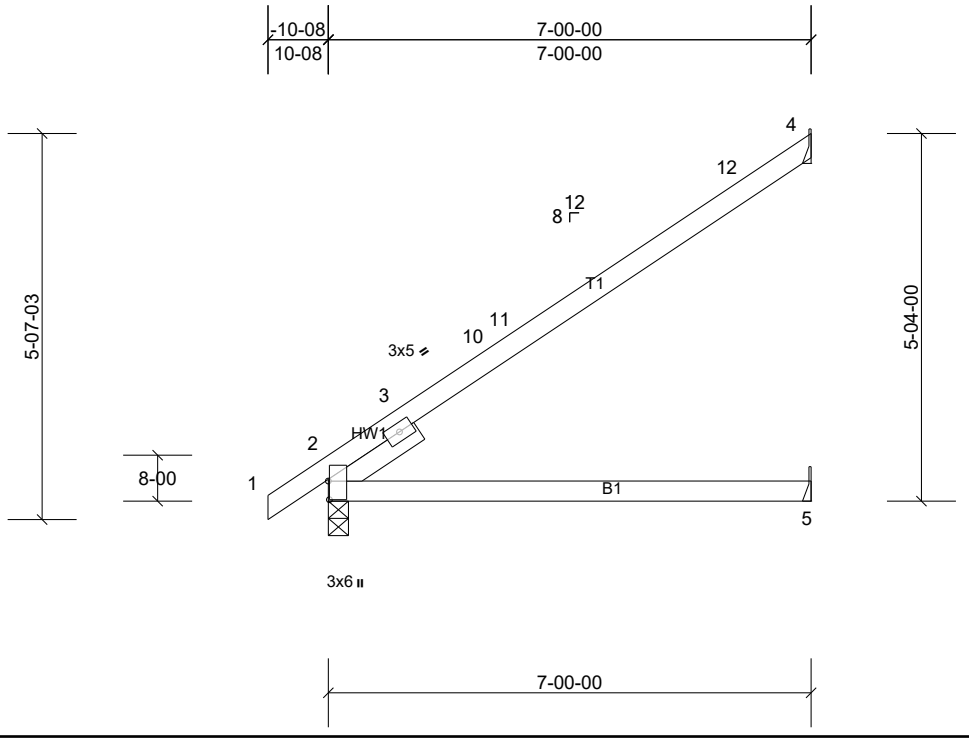

Scale = 1:33.6

Plate Offsets (X, Y): [2:3-04,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.17	5-8	>481	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.77	Vert(CT)	-0.31	5-8	>270	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.06	4	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=333/3-08, (min. 1-08), 4=185/ Mechanical, (min. 1-08), 5=89/ Mechanical, (min. 1-08)
Max Horiz 2=181 (LC 14)
Max Uplift 2=-1 (LC 14), 4=-104 (LC 14)
Max Grav 2=396 (LC 21), 4=296 (LC 21), 5=128 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-521/306
BOT CHORD 2-5=-364/331

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-8-5, Exterior(2R) 2-8-5 to 6-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	E01A	Jack-Closed	2	1	Job Reference (optional)

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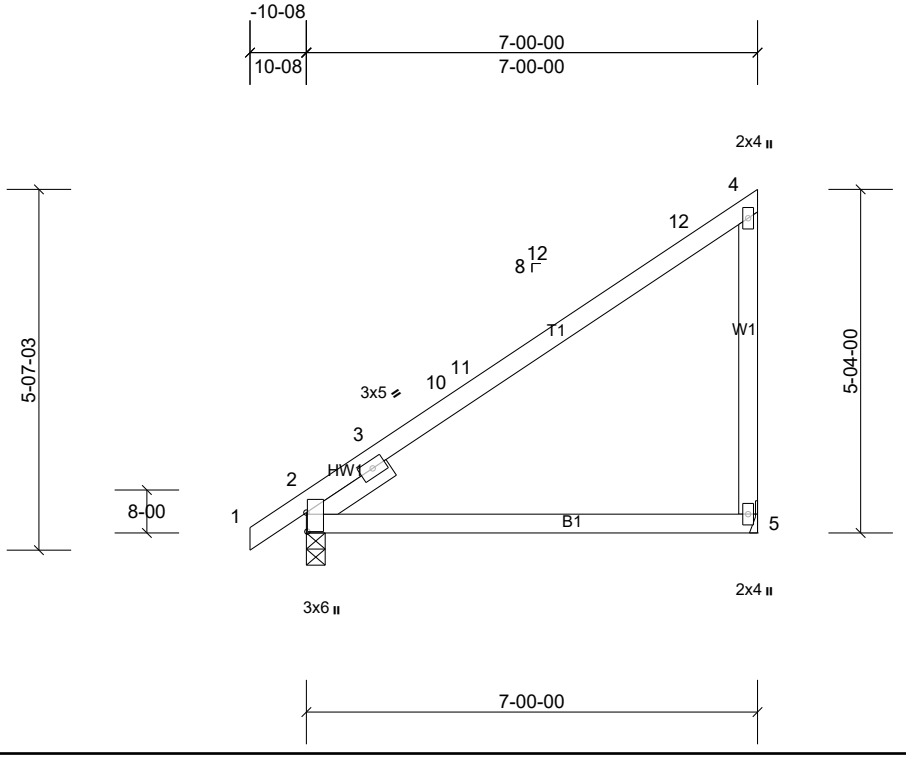


Scale = 1:35.9

Plate Offsets (X, Y): [2:3-09,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.17	5-8	>495	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.76	Vert(CT)	-0.29	5-8	>279	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.06	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 34 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=330/3-08, (min. 1-08), 5=271/ Mechanical, (min. 1-08)
Max Horiz 2=181 (LC 13)
Max Uplift 2=-25 (LC 14), 5=-69 (LC 14)
Max Grav 2=394 (LC 21), 5=396 (LC 21)

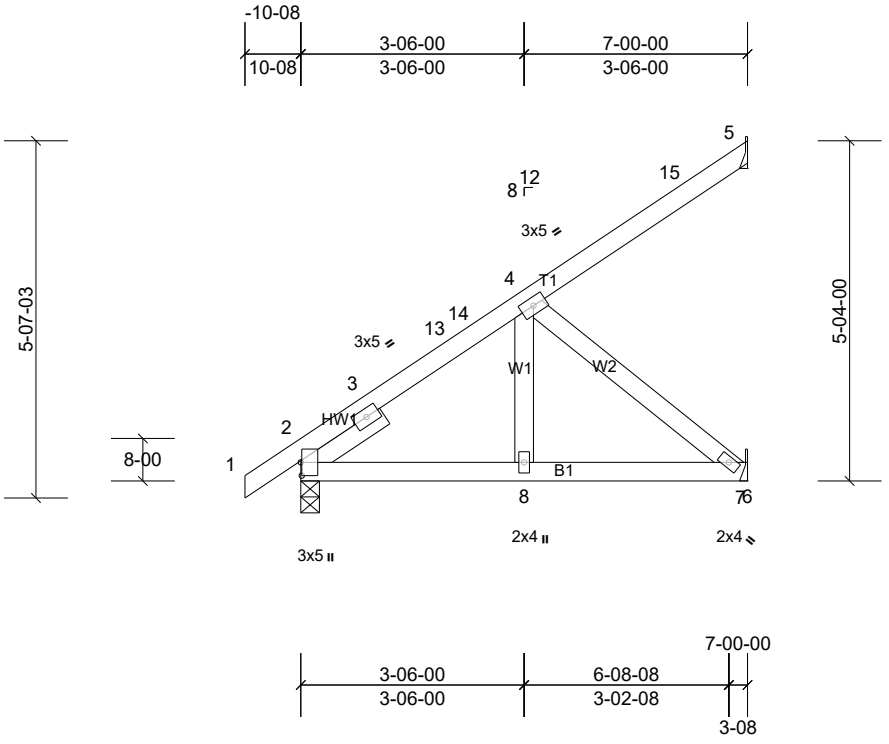
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-452/299, 4-5=-293/100
BOT CHORD 2-5=-239/369

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-7-5, Exterior(2R) 2-7-5 to 6-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 5.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	F01	Jack-Partial	2	1	Job Reference (optional)



Scale = 1:36.3

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	0.00	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.01	7-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.00	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 37 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=332/3-08, (min. 1-08), 5=83/ Mechanical, (min. 1-08), 7=194/ Mechanical, (min. 1-08)
Max Horiz 2=181 (LC 14)
Max Uplift 2=-1 (LC 14), 5=-42 (LC 14), 7=-51 (LC 14)
Max Grav 2=395 (LC 21), 5=143 (LC 21), 7=261 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	3-13=-348/0, 13-14=-309/0, 4-14=-281/0
BOT CHORD	2-8=-169/289, 7-8=-134/289
WEBS	4-7=-369/171

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-8-5, Exterior(2R) 2-8-5 to 6-11-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 5 and 51 lb uplift at joint 7.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	F01A	Jack-Closed	1	1	Job Reference (optional)

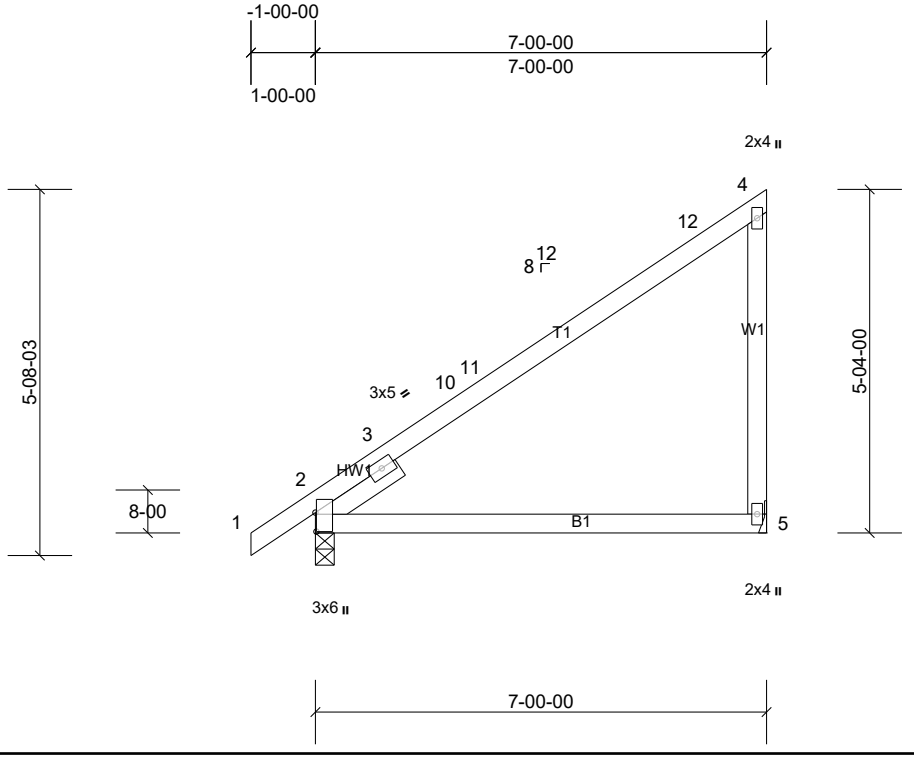


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Plate Offsets (X, Y): [2:3-09,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.17	5-8	>498	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.75	Vert(CT)	-0.29	5-8	>281	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.06	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 34 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.1
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-3-13 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=339/3-08, (min. 1-08), 5=270/ Mechanical, (min. 1-08)
Max Horiz 2=183 (LC 13)
Max Uplift 2=-28 (LC 14), 5=-69 (LC 14)
Max Grav 2=402 (LC 21), 5=395 (LC 21)

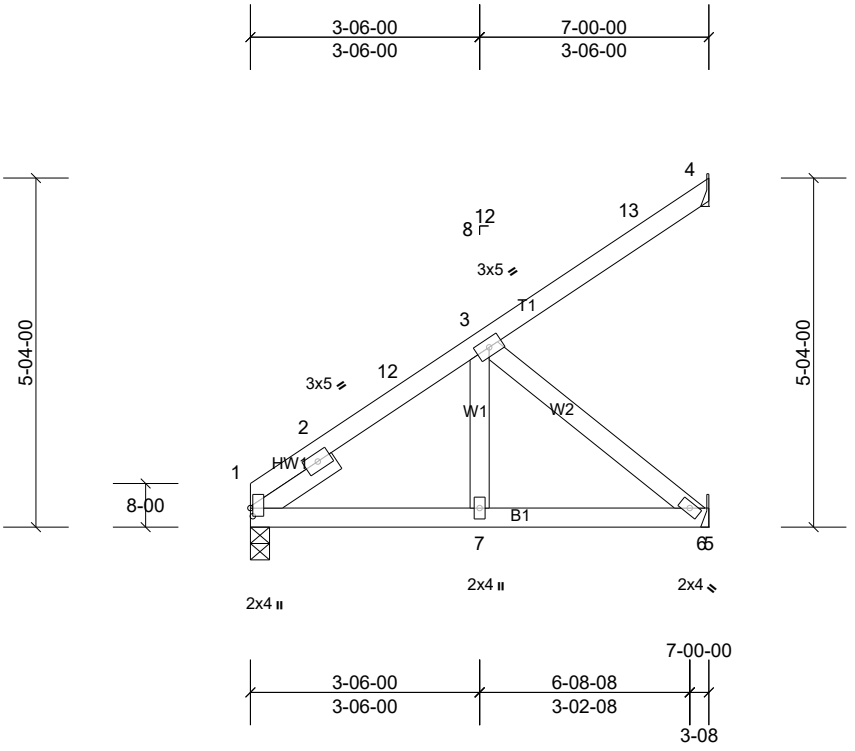
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-460/307, 4-5=-292/99
BOT CHORD 2-5=-232/364

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 2-7-5, Exterior(2R) 2-7-5 to 6-10-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 5.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	F01B	Jack-Partial	2	1	Job Reference (optional)



Scale = 1:35.3

Plate Offsets (X, Y): [1:1-08,0-07]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	0.01	7-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.01	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 35 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=276/3-08, (min. 1-08), 4=82/ Mechanical, (min. 1-08), 6=198/ Mechanical, (min. 1-08)
Max Horiz 1=163 (LC 14)
Max Uplift 4=-42 (LC 14), 6=-53 (LC 14)
Max Grav 1=339 (LC 20), 4=142 (LC 20), 6=265 (LC 20)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-12=-355/0, 3-12=-286/0
BOT CHORD 1-7=-184/295, 6-7=-136/295
WEBS 3-6=-376/173

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 4 and 53 lb uplift at joint 6.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	G01	Half Hip Girder	1	1	Job Reference (optional)

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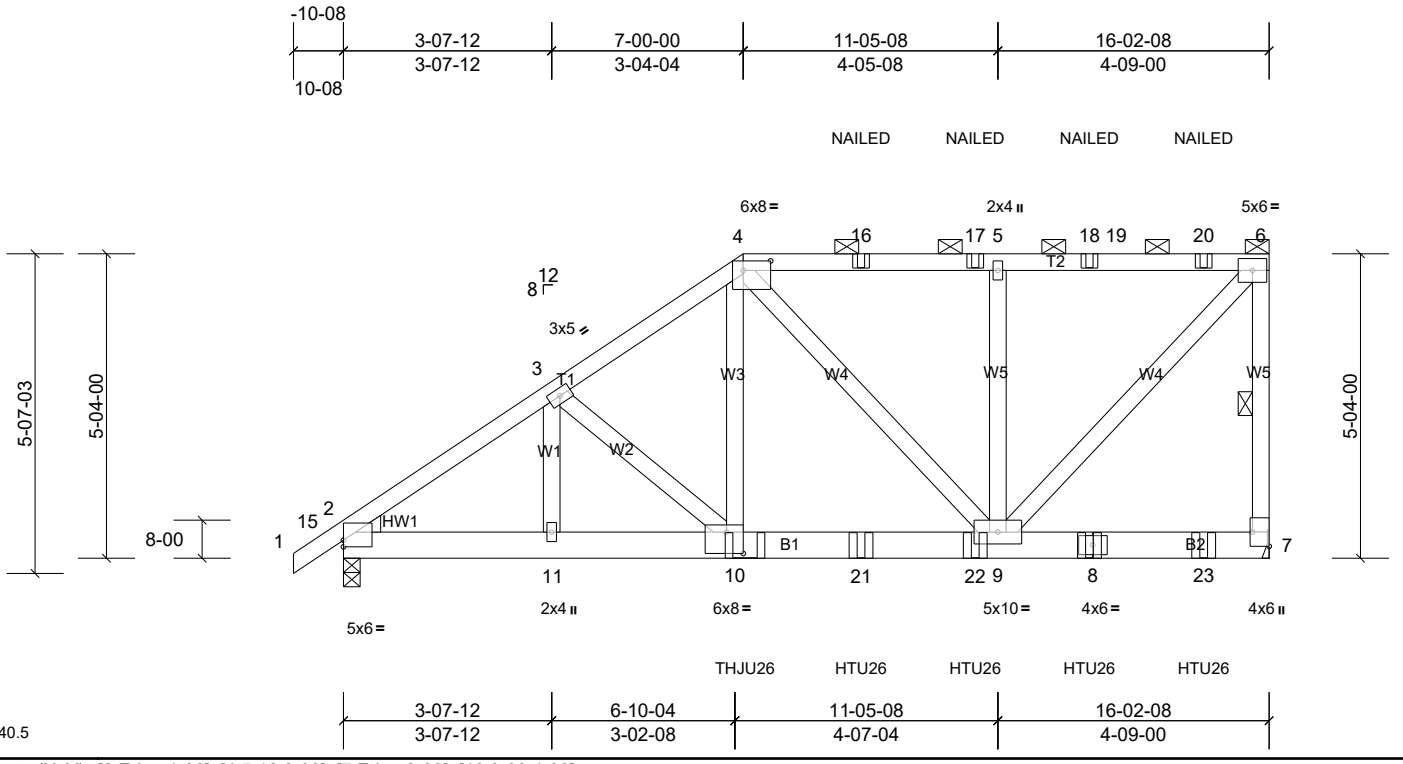

Scale = 1:40.5

Plate Offsets (X, Y): [2:Edge,1-06], [4:5-12,2-00], [7:Edge,3-08], [10:3-08,4-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.06	9-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.71	Vert(CT)	-0.10	9-10	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.92	Horz(CT)	0.02	7	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 115 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.3
WEDGE	Left: 2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-5-11 oc purlins, except end verticals, and 2-0-0 oc purlins (4-1-7 max.): 4-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7

REACTIONS (lb/size) 2=1628/3-08, (min. 2-02), 7=2071/ Mechanical, (min. 1-08)
Max Horiz 2=184 (LC 11)
Max Uplift 2=-315 (LC 12), 7=-449 (LC 9)
Max Grav 2=1776 (LC 34), 7=2143 (LC 33)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-2484/461, 3-4=-2364/509, 4-16=-1543/365, 16-17=-1543/365, 5-17=-1543/365, 5-18=-1543/365, 18-19=-1543/365, 19-20=-1543/365, 6-20=-1543/365, 6-7=-1866/403
BOT CHORD	2-11=-420/1980, 10-11=-420/1980, 10-21=-425/1847, 21-22=-425/1847, 9-22=-425/1847
WEBS	4-10=-269/1406, 4-9=-542/119, 5-9=-614/178, 6-9=-467/2232

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 7.
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
13) Use Simpson Strong-Tie THJU26 (SGL & SGL LC 1-PLY) or equivalent at 7-0-6 from the left end to connect truss(es) F01A (1 ply 2x4 SP), CJ09 (1 ply 2x4 SP) to front face of bottom chord.
14) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 9-0-12 from the left end to 15-0-12 to connect truss(es) F01B (1 ply 2x4 SP), F01 (1 ply 2x4 SP) to front face of bottom chord.
15) Fill all nail holes where hanger is in contact with lumber.
16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

Continued on page 2

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-4=-60, 4-6=-60, 7-12=-20
   Concentrated Loads (lb)
   Vert: 8=-241, 10=-1061, 16=-82, 17=-82, 18=-83, 20=-83, 21=-245, 22=-245, 23=-241

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	G02	Half Hip	1	1	Job Reference (optional)

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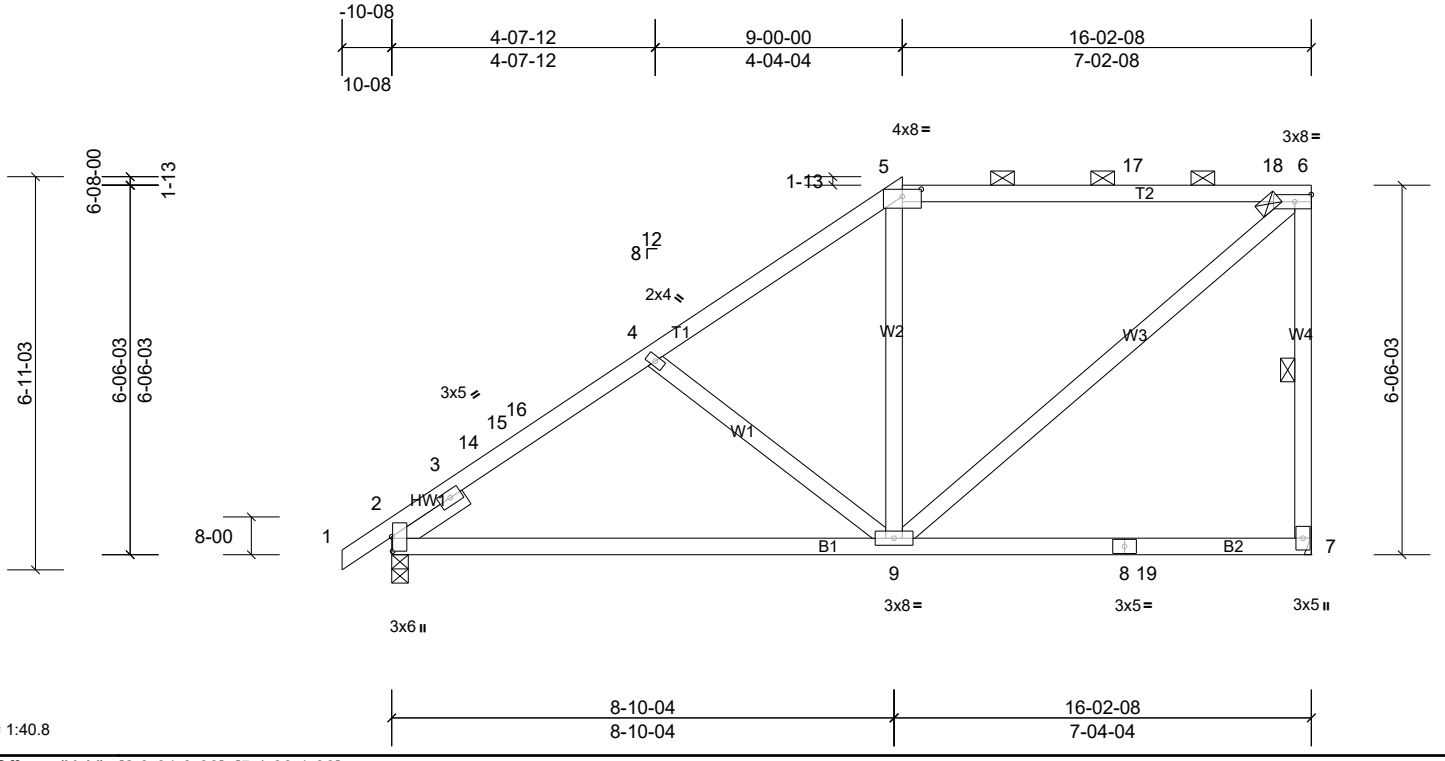

Scale = 1:40.8

8-10-04 / 8-10-04 — 16-02-08 / 7-04-04

Plate Offsets (X, Y): [2:3-01,0-03], [5:4-00,1-09]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.98	Vert(LL)	-0.09	9-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.19	9-12	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 94 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2 *Except* T2:2x4 SP No.1
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-9-8 oc purlins, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=228 (LC 13)
Max Uplift 2=-72 (LC 14), 7=-105 (LC 11)
Max Grav 2=838 (LC 44), 7=802 (LC 39)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-838/0, 3-14=-980/129, 14-15=-960/132, 15-16=-960/136, 4-16=-877/152, 4-5=-740/143, 5-17=-553/161, 17-18=-553/161, 6-18=-553/160, 6-7=-786/135
BOT CHORD	2-9=-214/971
WEBS	4-9=-386/161, 6-9=-82/786

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 4-6-5, Exterior(2R) 4-6-5 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 7.
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	G03	Half Hip	1	1	Job Reference (optional)

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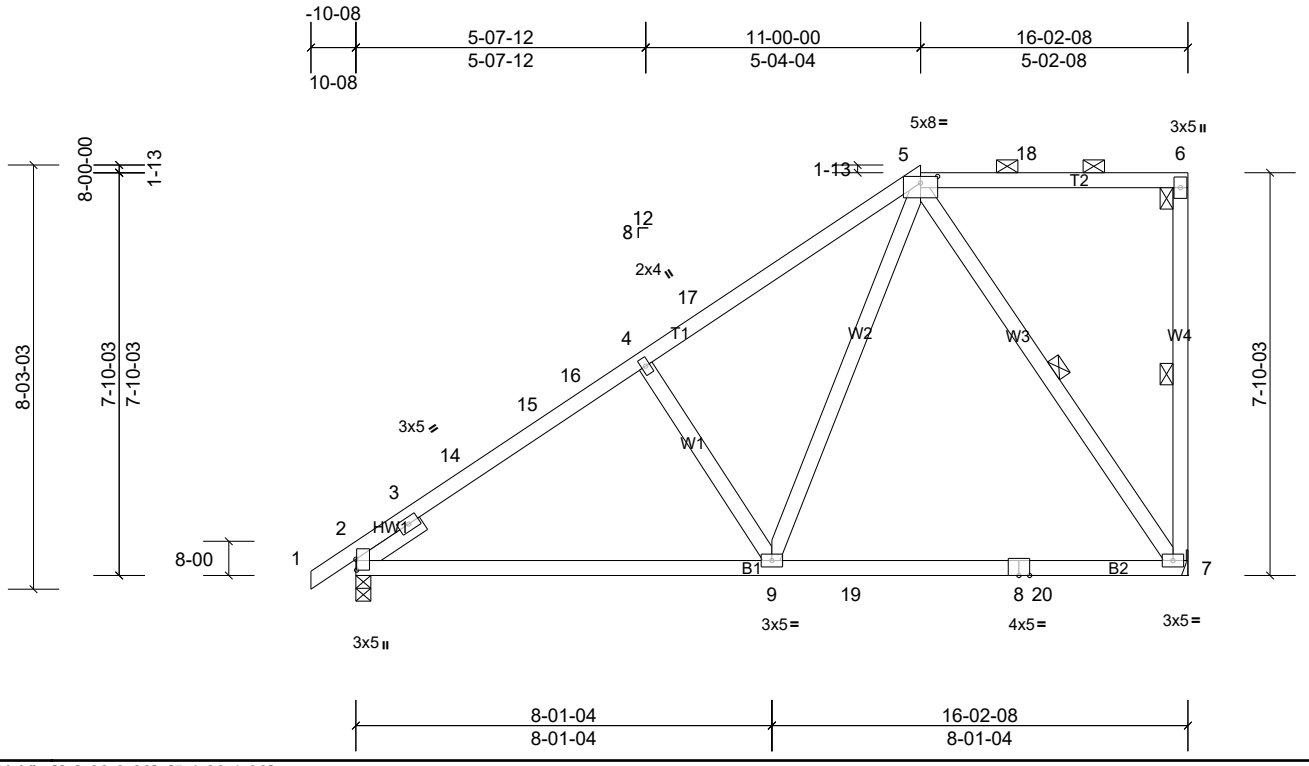

Scale = 1:45.1

8-01-04 / 8-01-04 — 16-02-08 / 8-01-04

Plate Offsets (X, Y): [2:2-09,0-03], [5:4-00,1-09]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.69	Vert(LL)	-0.20	7-9	>983	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.73	Vert(CT)	-0.30	7-9	>647	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.01	7	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 98 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=275 (LC 13)
Max Uplift 2=-73 (LC 14), 7=-105 (LC 11)
Max Grav 2=857 (LC 44), 7=748 (LC 39)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-10-4 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7, 5-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-663/0, 3-14=-977/102, 14-15=-950/113, 15-16=-923/125, 4-16=-864/132, 4-17=-853/139, 5-17=-731/171
BOT CHORD	2-9=-223/995, 9-19=-116/507, 8-19=-116/507, 8-20=-116/507, 7-20=-116/507
WEBS	4-9=-418/202, 5-9=-84/753, 5-7=-762/109

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 6-9-1, Exterior(2R) 6-9-1 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 7.
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	G04	Half Hip	1	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:05 — Page: 1

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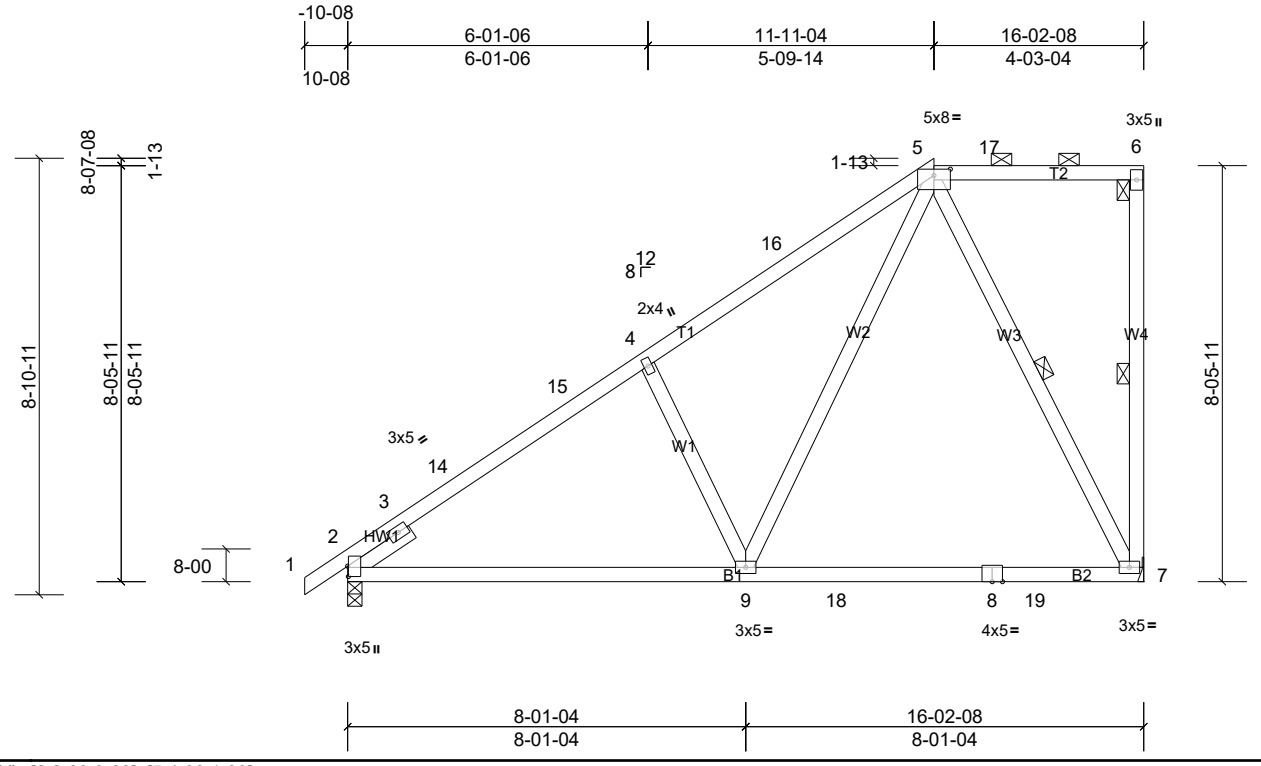

Scale = 1:47.1

Plate Offsets (X, Y): [2:2-09,0-03], [5:4-00,1-09]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.81	Vert(LL)	-0.21	7-9	>931	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.74	Vert(CT)	-0.31	7-9	>624	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 101 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=298 (LC 13)
Max Uplift 2=-71 (LC 14), 7=-106 (LC 11)
Max Grav 2=854 (LC 44), 7=753 (LC 44)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-11-11 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	1 Row at midpt 6-7, 5-7

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-657/0, 3-14=-993/101, 14-15=-945/114, 4-15=-857/134, 4-16=-905/179, 5-16=-771/198
BOT CHORD	2-9=-236/1034, 9-18=-113/456, 8-18=-113/456, 8-19=-113/456, 7-19=-113/456
WEBS	4-9=-490/224, 5-9=-125/876, 5-7=-824/123

NOTES

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 8-11-4, Exterior(2R) 8-11-4 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) Provide adequate drainage to prevent water ponding.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
9) Refer to girder(s) for truss to truss connections.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at joint 7.
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	H01	Jack-Open	2	1	Job Reference (optional)

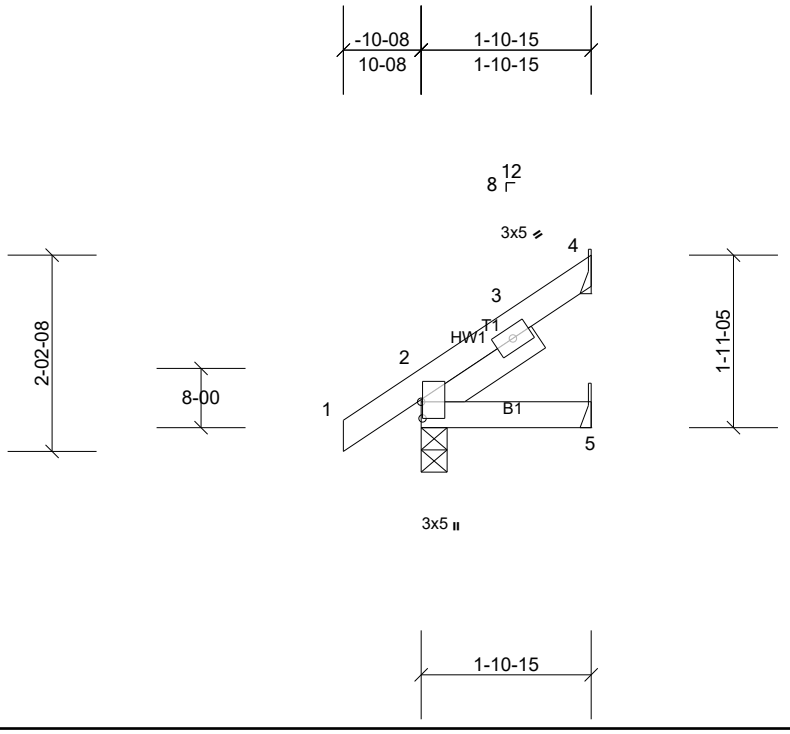

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Plate Offsets (X, Y): [2:2-04,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	0.00	8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 10 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=141/3-08, (min. 1-08), 4=43/ Mechanical, (min. 1-08), 5=21/ Mechanical, (min. 1-08)
Max Horiz 2=65 (LC 14)
Max Uplift 2=-8 (LC 14), 4=-32 (LC 14), 5=-1 (LC 14)
Max Grav 2=203 (LC 21), 4=62 (LC 21), 5=31 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 5 and 32 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	H02	Jack-Open	2	1	Job Reference (optional)

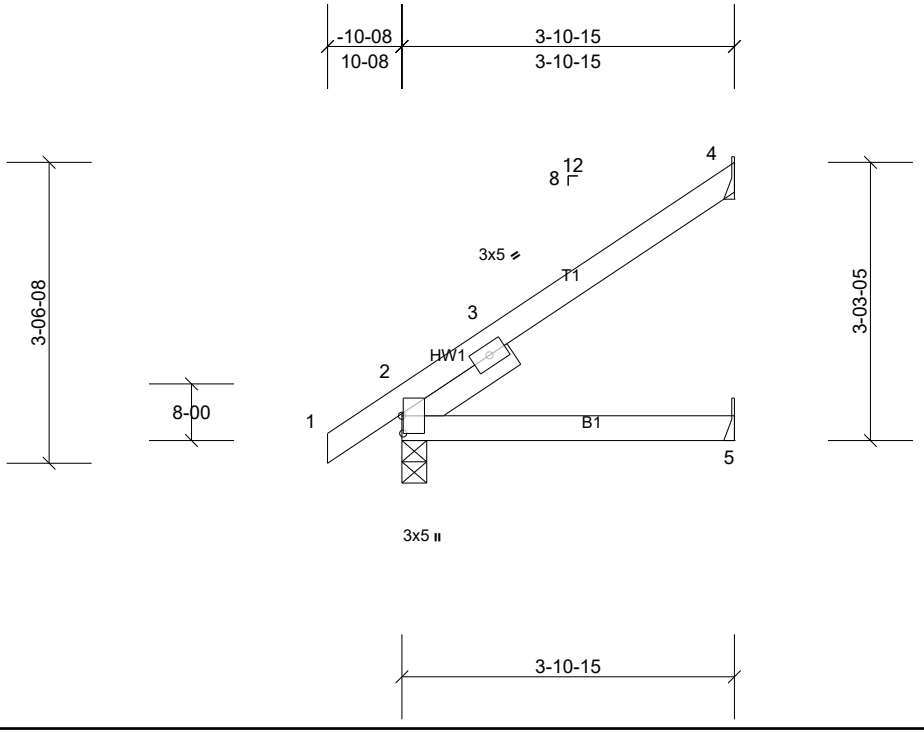

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Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	-0.02	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.03	5-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 17 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

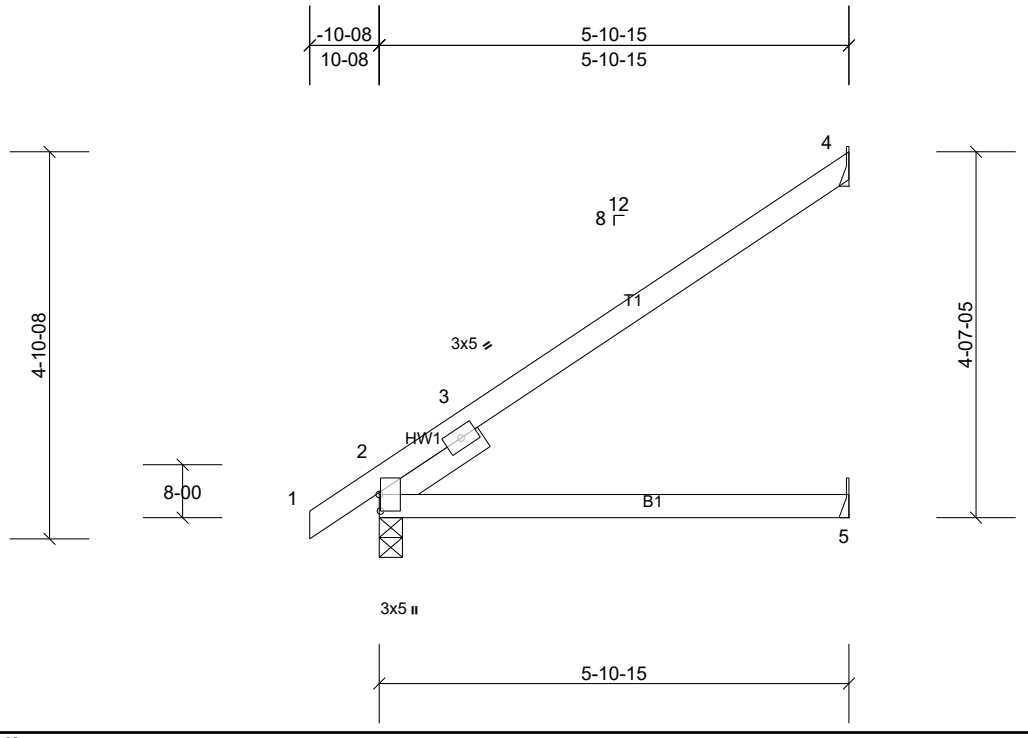
REACTIONS (lb/size) 2=212/3-08, (min. 1-08), 4=98/ Mechanical, (min. 1-08), 5=50/ Mechanical, (min. 1-08)
Max Horiz 2=112 (LC 14)
Max Uplift 2=-3 (LC 14), 4=-64 (LC 14)
Max Grav 2=329 (LC 21), 4=159 (LC 21), 5=70 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	H03	Jack-Open	2	1	Job Reference (optional)



Scale = 1:29.1

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.11	5-8	>662	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.18	5-8	>397	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.04	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 23 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

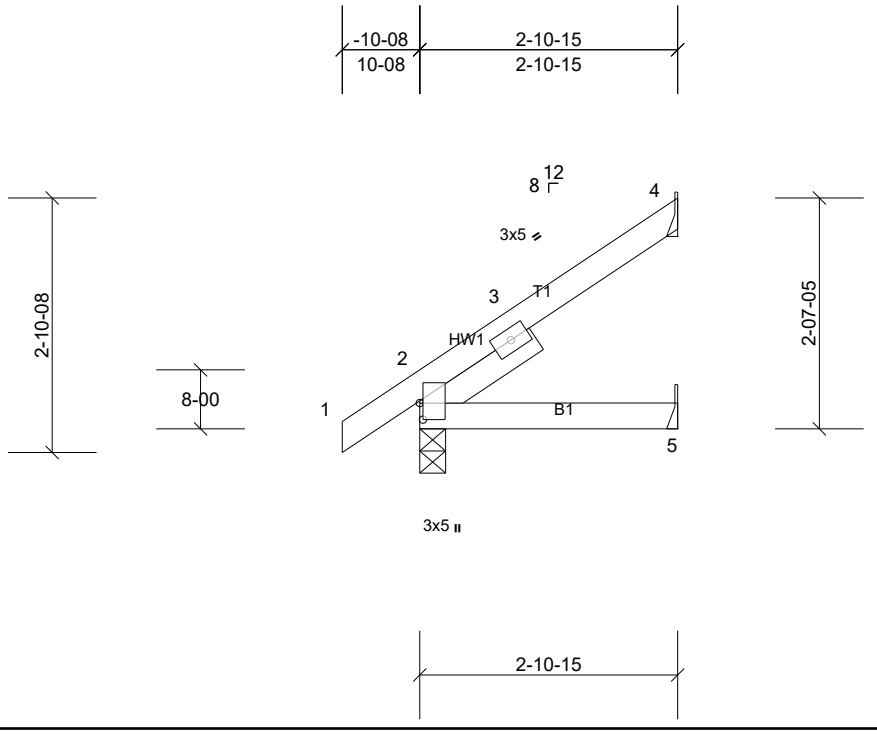
REACTIONS (lb/size) 2=290/3-08, (min. 1-08), 4=153/ Mechanical, (min. 1-08), 5=78/ Mechanical, (min. 1-08)
Max Horiz 2=161 (LC 14)
Max Uplift 4=-98 (LC 14)
Max Grav 2=372 (LC 21), 4=250 (LC 21), 5=109 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-410/257
BOT CHORD 2-5=-295/289

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 4.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	H04	Jack-Open	4	1	Job Reference (optional)



Scale = 1:26.1

Plate Offsets (X, Y): [2:2-04,0-07]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	0.00	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	-0.01	5-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 14 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=175/3-08, (min. 1-08), 4=70/ Mechanical, (min. 1-08), 5=36/ Mechanical, (min. 1-08)
Max Horiz 2=88 (LC 14)
Max Uplift 2=-6 (LC 14), 4=-48 (LC 14)
Max Grav 2=261 (LC 21), 4=109 (LC 21), 5=51 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	H05	Jack-Open	4	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:06 — Page: 1

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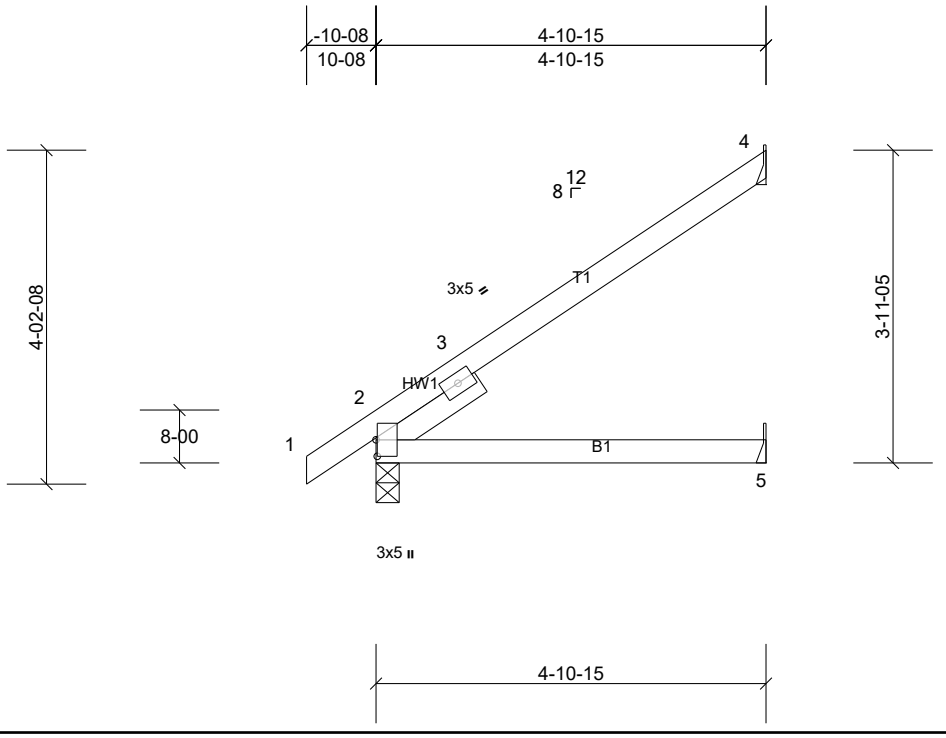

Scale = 1:29.1

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.56	Vert(LL)	-0.05	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.44	Vert(CT)	-0.09	5-8	>677	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.02	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 20 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-10-15 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=251/3-08, (min. 1-08), 4=125/ Mechanical, (min. 1-08), 5=64/ Mechanical, (min. 1-08)
Max Horiz 2=137 (LC 14)
Max Uplift 2=-1 (LC 14), 4=-81 (LC 14)
Max Grav 2=351 (LC 21), 4=207 (LC 21), 5=90 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-285/134

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	J01	Jack-Open	2	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:06 — Page: 1

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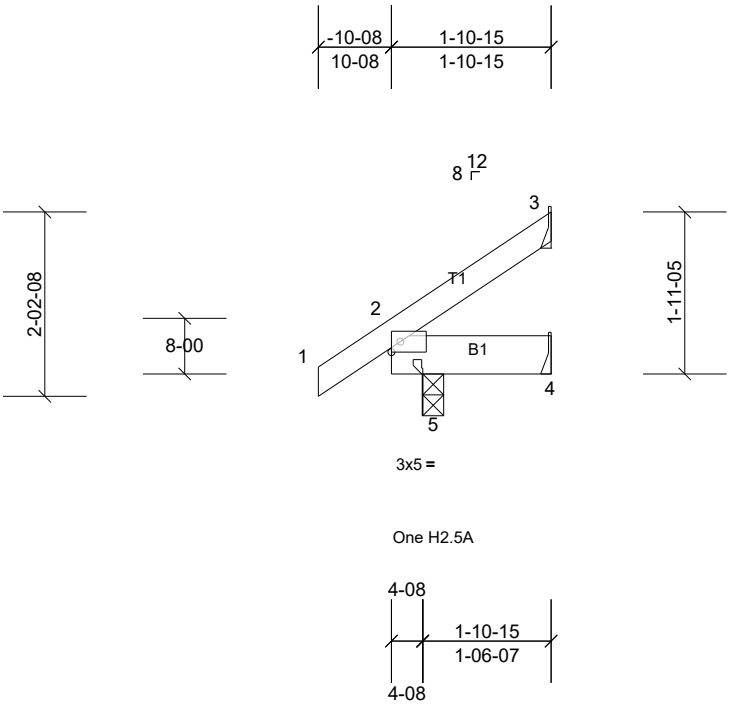


Scale = 1:27.7

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 10 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

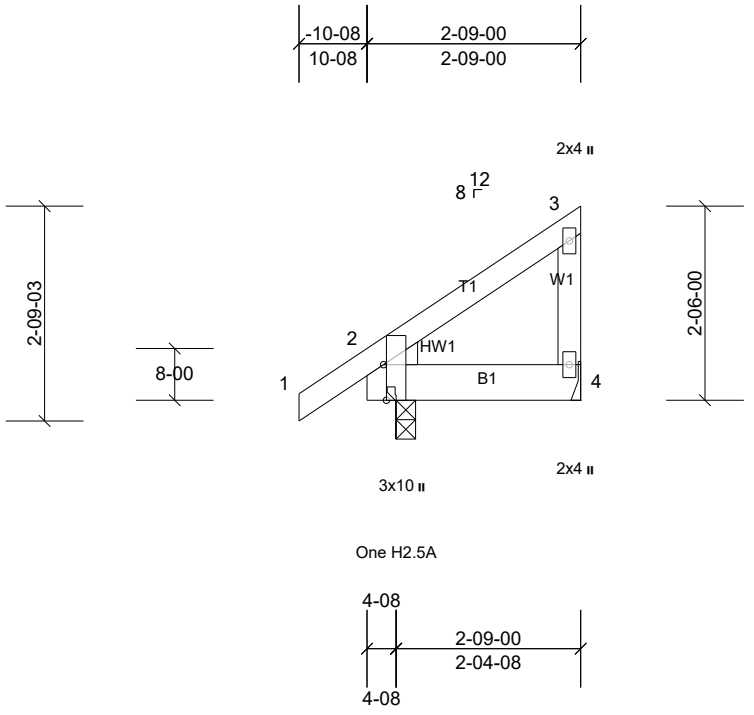
REACTIONS (lb/size) 3=34/ Mechanical, (min. 1-08), 4=-21/ Mechanical, (min. 1-08), 5=191/3-00, (min. 1-08)
Max Horiz 5=65 (LC 14)
Max Uplift 3=-28 (LC 14), 4=-38 (LC 21), 5=-15 (LC 14)
Max Grav 3=52 (LC 21), 4=13 (LC 7), 5=275 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 4 and 28 lb uplift at joint 3.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	K01	Jack-Closed	7	1	Job Reference (optional)



Scale = 1:29.8

Plate Offsets (X, Y): [2:5-08,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	4-9	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	n/a	-	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 16 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x6 SP No.2
WEBS	2x4 SP No.3
WEDGE	Left: 2x4 SP 2400F 2.0E

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-9-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=193/3-00, (min. 1-08), 4=68/ Mechanical, (min. 1-08)
Max Horiz 2=78 (LC 13)
Max Uplift 2=-26 (LC 14), 4=-46 (LC 11)
Max Grav 2=287 (LC 21), 4=94 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; porch left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 4.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	L01	Jack-Open	2	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:07 — Page: 1

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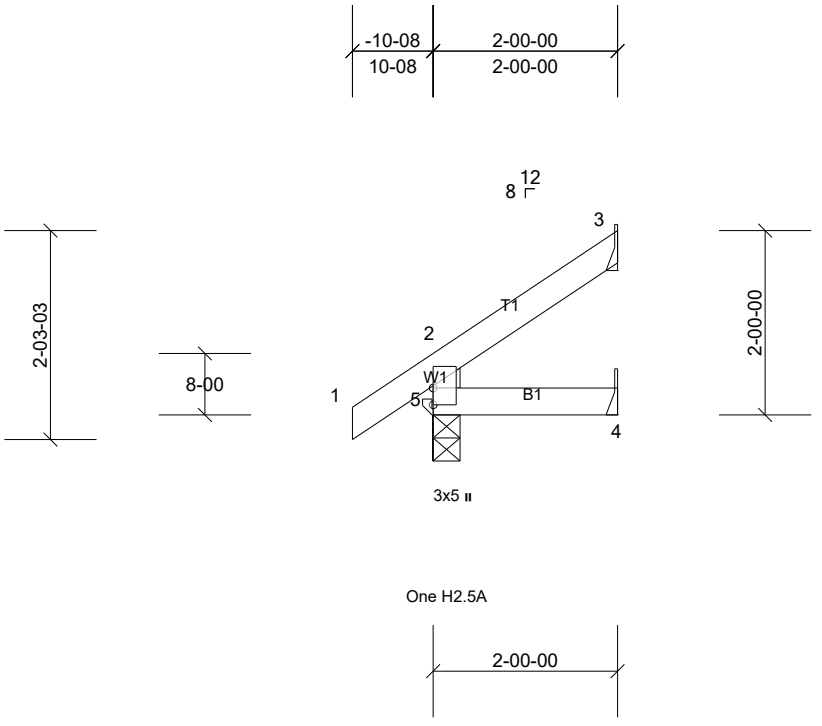


One H2.5A

Scale = 1:25.1

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 9 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 3=41/ Mechanical, (min. 1-08), 4=16/ Mechanical, (min. 1-08), 5=152/3-08, (min. 1-08)
Max Horiz 5=61 (LC 14)
Max Uplift 3=-35 (LC 14), 5=-11 (LC 14)
Max Grav 3=62 (LC 21), 4=33 (LC 7), 5=221 (LC 21)

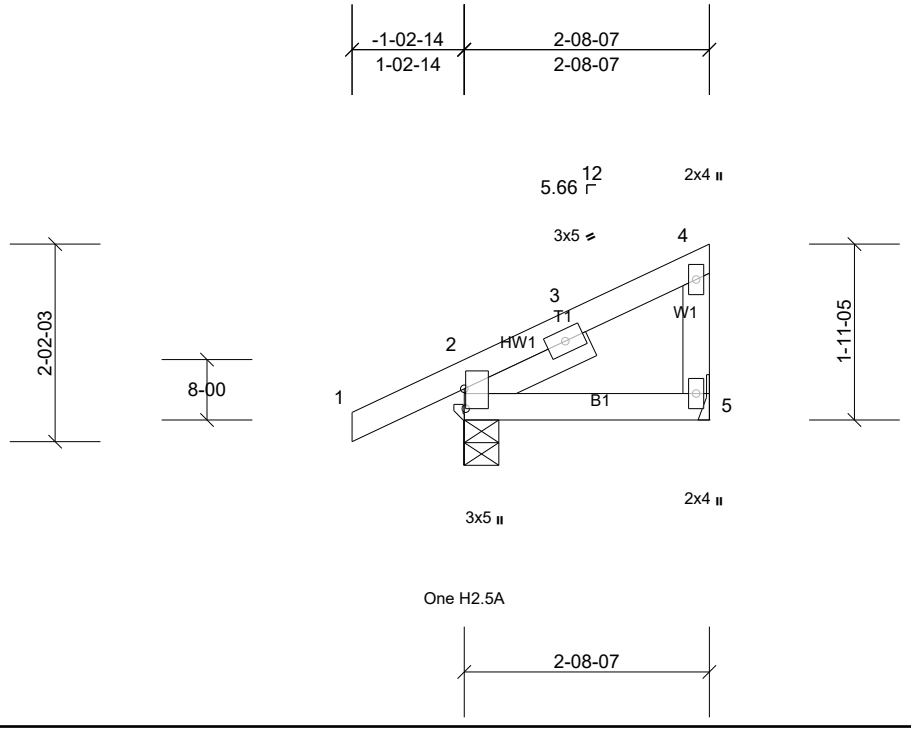
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 3.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	M01	Diagonal Hip Girder	2	1	Job Reference (optional)



Scale = 1:25.5

Plate Offsets (X, Y): [2:2-10,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	0.00	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	0.00	5-8	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 15 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD	Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=195/4-09, (min. 1-08), 5=84/ Mechanical, (min. 1-08)
Max Horiz 2=65 (LC 11)
Max Uplift 2=-37 (LC 12), 5=-19 (LC 12)
Max Grav 2=280 (LC 19), 5=114 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Refer to girder(s) for truss to truss connections.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 5.
9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	N01	Hip Girder	1	1	Job Reference (optional)

Carter Components, Sanford, NC, user — Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:07 — Page: 1

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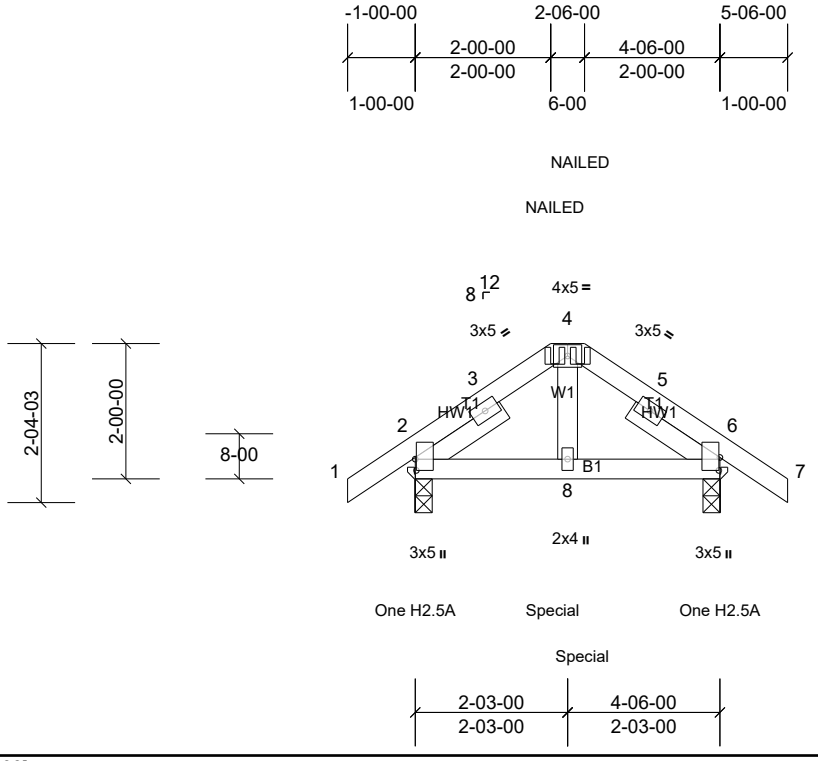


Scale = 1:34.2

Plate Offsets (X, Y): [2:2-00,0-03], [6:2-05,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	0.00	8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	0.00	8	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 25 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
SLIDER	Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=333/3-00, (min. 1-08), 6=333/3-00, (min. 1-08)
Max Horiz 2=-49 (LC 10)
Max Uplift 2=-86 (LC 12), 6=-86 (LC 13)
Max Grav 2=411 (LC 19), 6=411 (LC 20)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-272/100, 4-5=-272/100

BRACING

TOP CHORD	Structural wood sheathing directly applied or 4-6-0 oc purlins.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

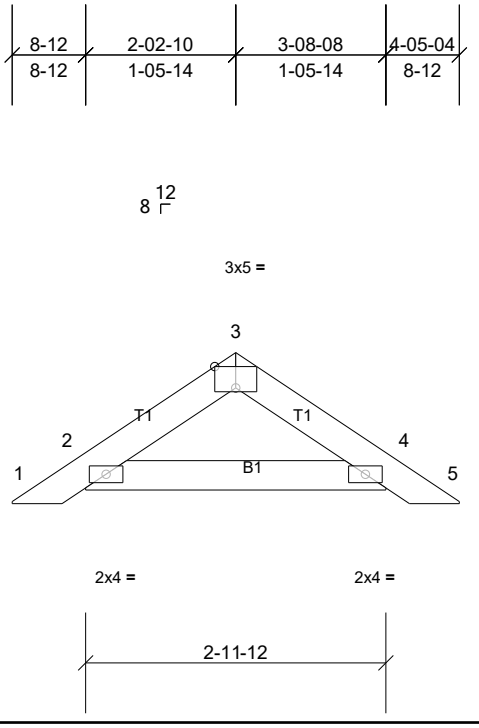
NOTES

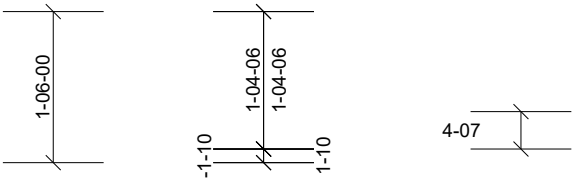
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
9) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.
10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 36 lb up at 2-0-0, and 85 lb down and 36 lb up at 2-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
   Uniform Loads (lb/ft)
   Vert: 1-4=-60, 4-7=-60, 9-13=-20
   Concentrated Loads (lb)
   Vert: 8=-171, 4=-14

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	PB04	Piggyback	3	1	Job Reference (optional)





Scale = 1:23

Plate Offsets (X, Y): [3:2-08,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	n/a	-	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 12 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-6-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 2-11-12.
(lb) - Max Horiz 2=31 (LC 13), 6=31 (LC 13)
Max Uplift All uplift 100 (lb) or less at joint(s) 2, 4, 6, 10
Max Grav All reactions 250 (lb) or less at joint(s) 2, 4, 6, 10

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
7) Gable requires continuous bottom chord bearing.
8) Gable studs spaced at 4-0-0 oc.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
11) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
12) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	PB04A	Piggyback	1	1	Job Reference (optional)

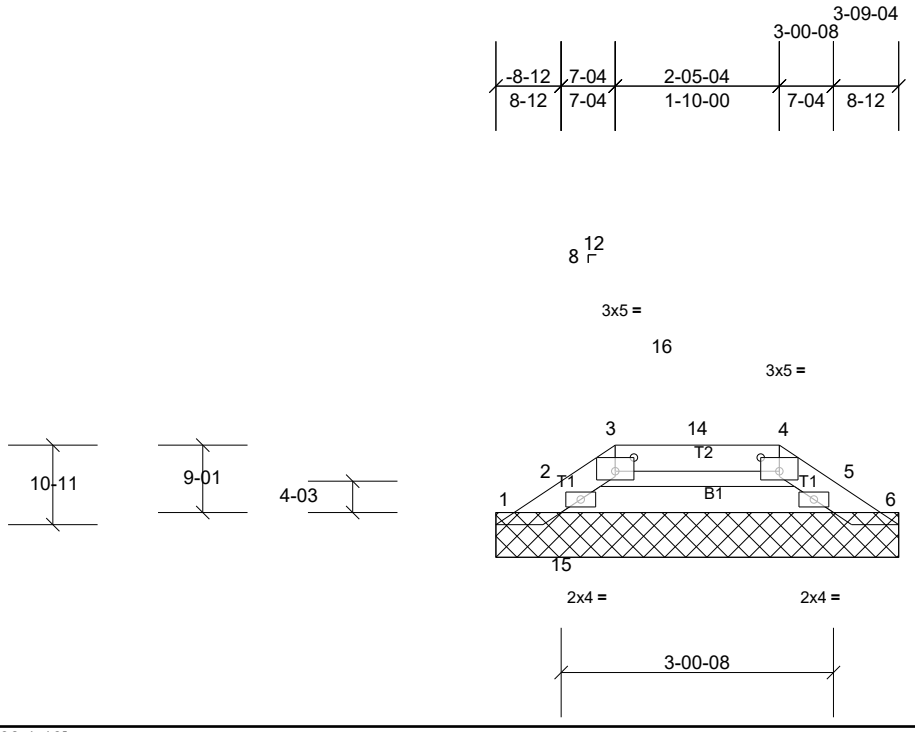


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Plate Offsets (X, Y): [3:2-08,1-13], [4:2-08,1-13]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.10	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	6	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 12 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

REACTIONS All bearings 4-06-00.
(lb) - Max Horiz 1=-17 (LC 12)
Max Uplift All uplift 100 (lb) or less at joint(s) 1, 2, 6, 7
Max Grav All reactions 250 (lb) or less at joint(s) 1, 5, 6, 13 except 2=281 (LC 21), 7=281 (LC 21)

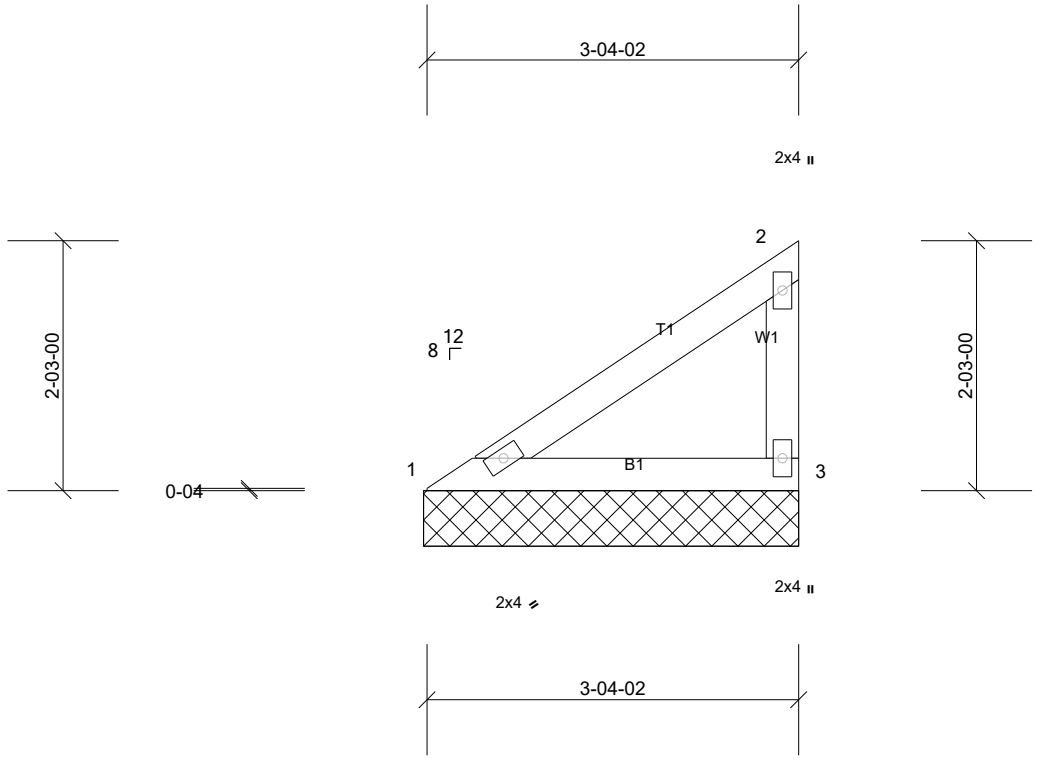
BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-6 oc purlins, except
2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-3-2 to 1-4-0, Exterior(2R) 1-4-0 to 3-2-0, Exterior(2E) 3-2-0 to 4-2-14 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
4) Unbalanced snow loads have been considered for this design.
5) Provide adequate drainage to prevent water ponding.
6) Gable requires continuous bottom chord bearing.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6.
10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	VL01	Valley	1	1	Job Reference (optional)



Scale = 1:20.8

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.22	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 13 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

REACTIONS (lb/size) 1=129/3-04-08, (min. 1-08), 3=129/3-04-08, (min. 1-08)
Max Horiz 1=69 (LC 11)
Max Uplift 1=-9 (LC 14), 3=-32 (LC 14)
Max Grav 1=183 (LC 20), 3=183 (LC 20)

BRACING

TOP CHORD	Structural wood sheathing directly applied or 3-4-8 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

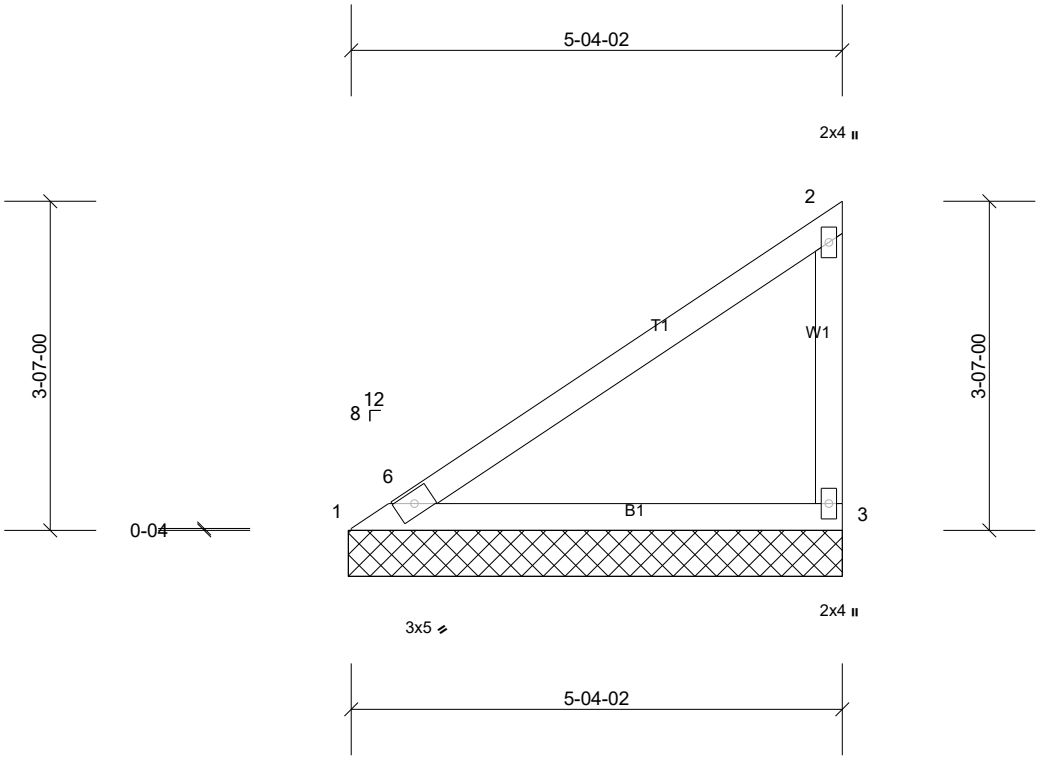
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1 and 32 lb uplift at joint 3.
8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	VL02	Valley	1	1	Job Reference (optional)



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Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.61	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.01	3	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 21 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 5-4-2 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=209/5-04-08, (min. 1-08), 3=209/5-04-08, (min. 1-08)
Max Horiz 1=116 (LC 11)
Max Uplift 1=-13 (LC 14), 3=-53 (LC 14)
Max Grav 1=293 (LC 20), 3=315 (LC 20)

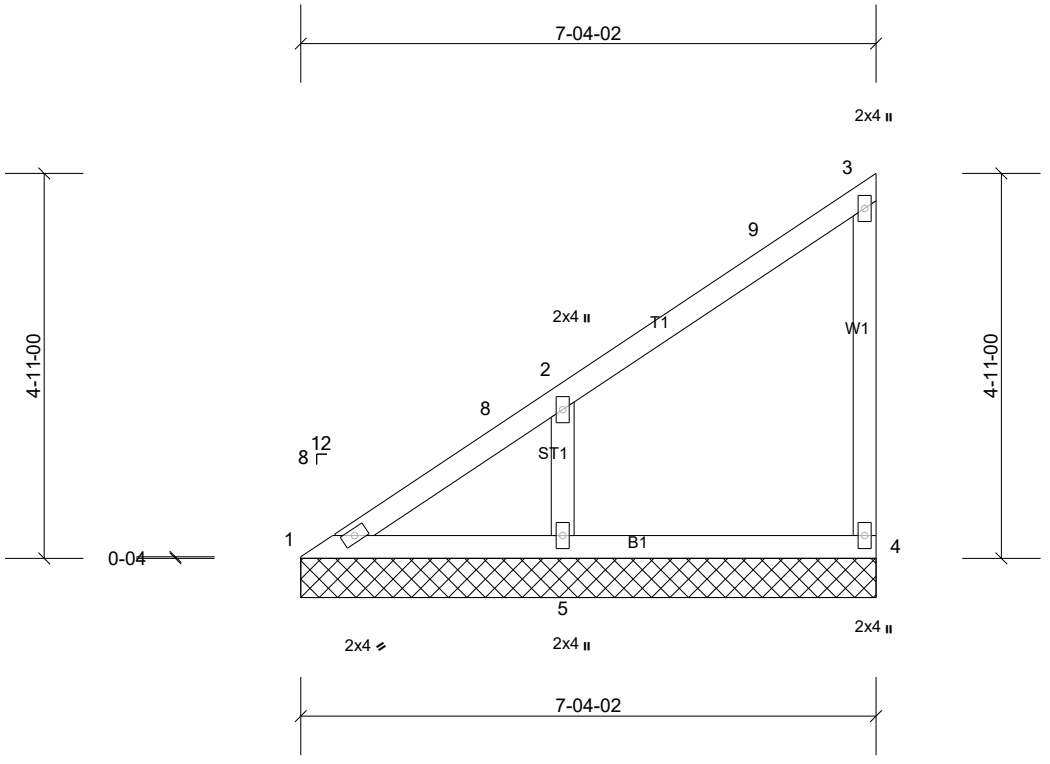
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-6=-440/63, 1-6=-420/64
BOT CHORD 1-3=-94/368

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 3 and 13 lb uplift at joint 1.
8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	VL03	Valley	1	1	Job Reference (optional)



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Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 32 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=95/7-04-02, (min. 1-08), 4=120/7-04-02, (min. 1-08), 5=360/7-04-02, (min. 1-08)
Max Horiz 1=164 (LC 11)
Max Uplift 1=-4 (LC 10), 4=-33 (LC 11), 5=-115 (LC 14)
Max Grav 1=121 (LC 25), 4=188 (LC 20), 5=492 (LC 20)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-5=-400/220

NOTES

1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 4, 4 lb uplift at joint 1 and 115 lb uplift at joint 5.

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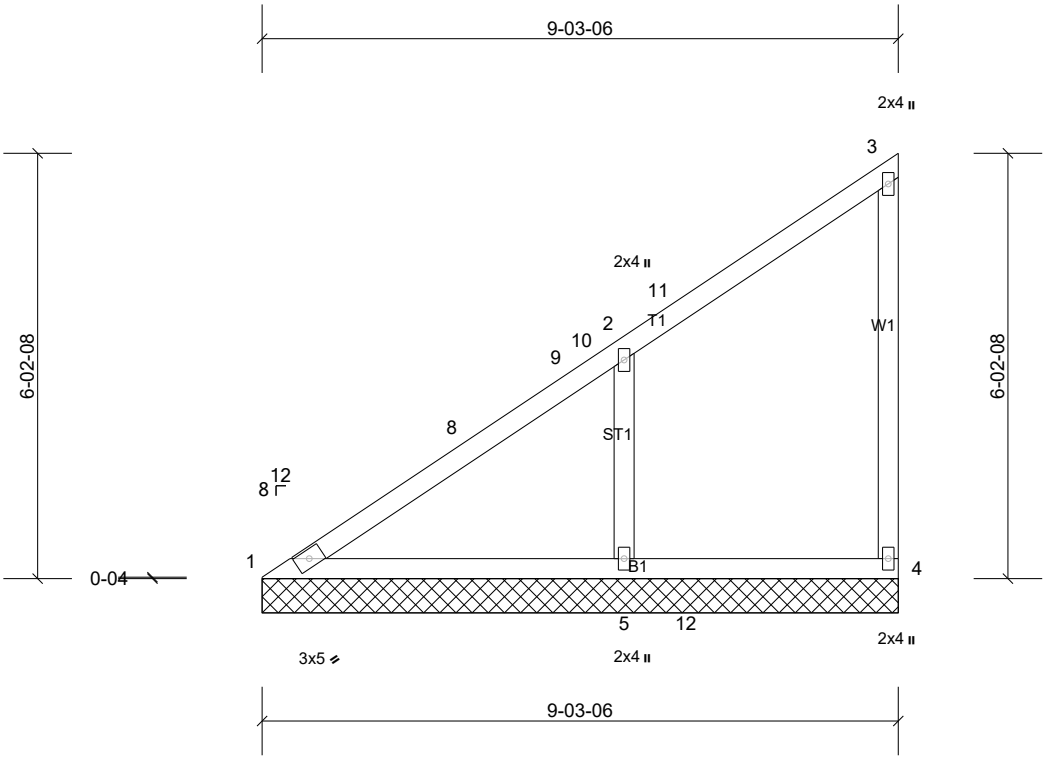
Job	Truss	Truss Type	Qty	Ply	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
25030187-01	VL04	Valley	1	1	Job Reference (optional)



Scale = 1:33.8

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.26	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horiz(TL)	0.01	4	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 42 lb	FT = 20%

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
WEBS	2x4 SP No.3
OTHERS	2x4 SP No.3

BRACING

TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=165/9-03-06, (min. 1-08), 4=95/9-03-06, (min. 1-08), 5=448/9-03-06, (min. 1-08)
Max Horiz 1=204 (LC 11)
Max Uplift 4=-37 (LC 11), 5=-130 (LC 14)
Max Grav 1=203 (LC 25), 4=179 (LC 5), 5=579 (LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-8=-302/190
BOT CHORD	1-5=-72/302
WEBS	2-5=-427/202

NOTES

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2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
3) Unbalanced snow loads have been considered for this design.
4) Gable requires continuous bottom chord bearing.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 4 and 130 lb uplift at joint 5.

LOAD CASE(S) Standard