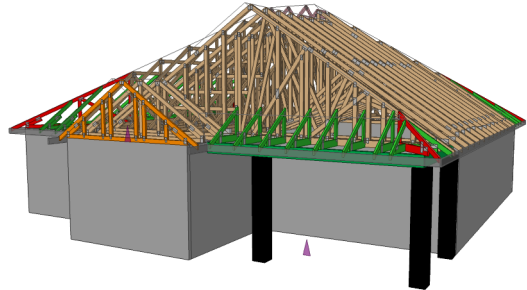




Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: Walk In Customer

Model: 24222 Adrian

THE PLACEMENT PLAN NOTES:

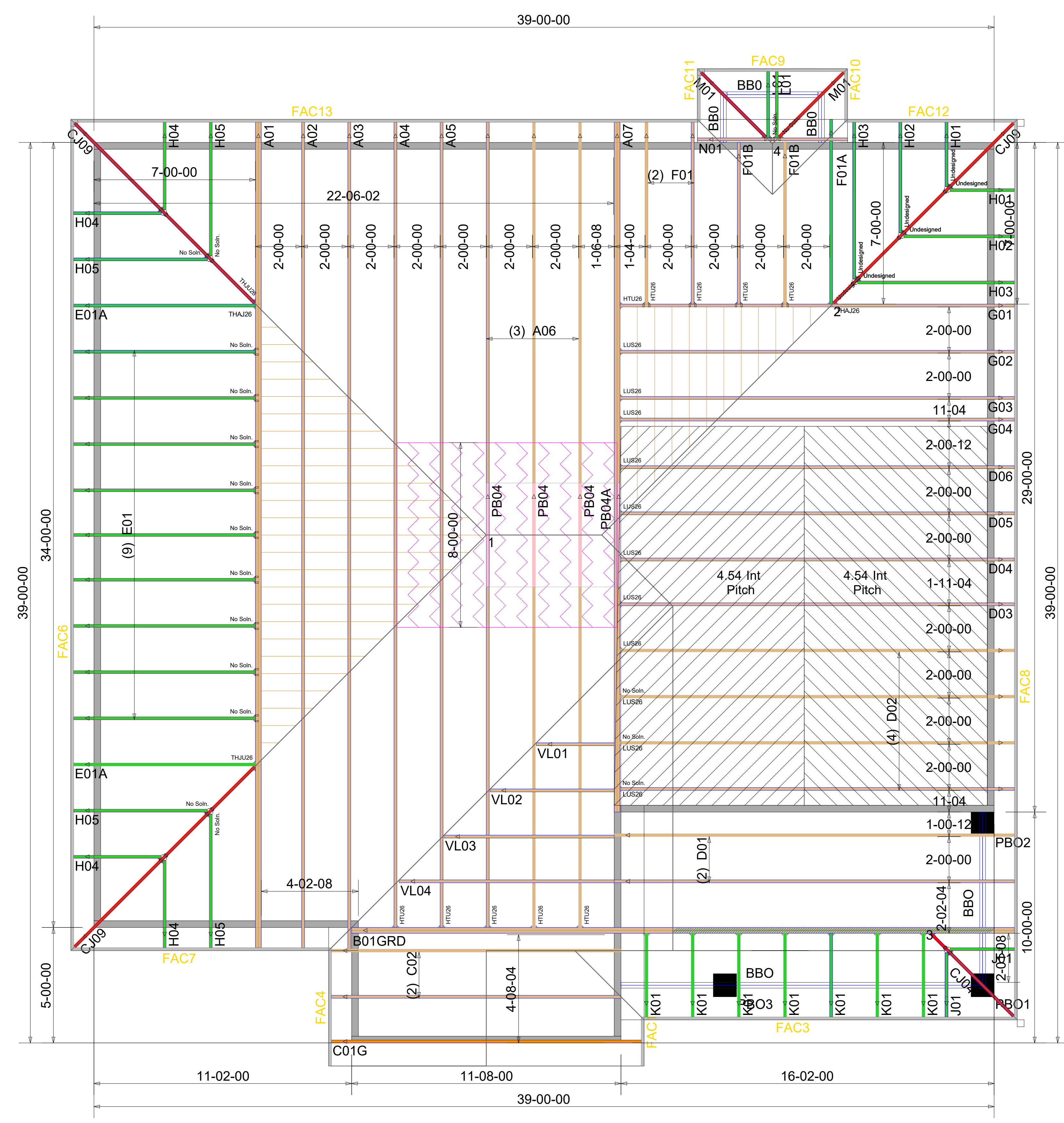
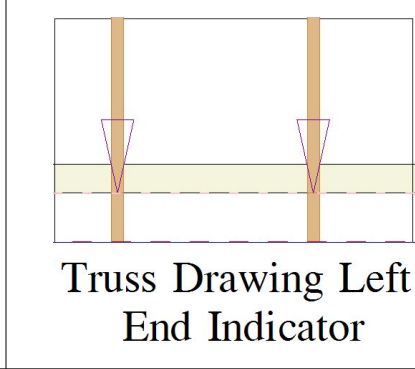
1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

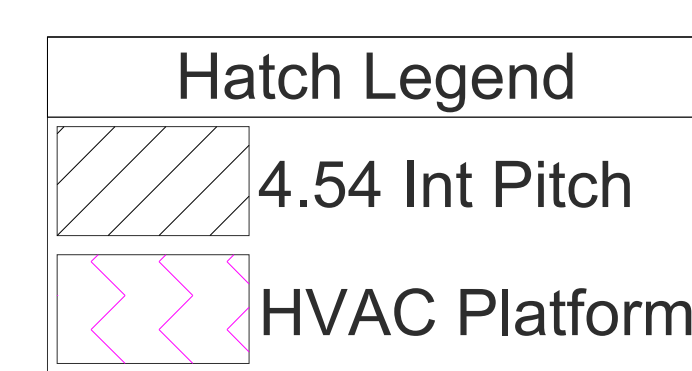
Date: _____

General Notes: ** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION. ** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS. ** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT. ** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE. ** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS. ** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH. ** ALL UPLIFT CONNECTORS SHOWN WITHIN THESE DOCUMENTS ARE RECOMMENDATIONS ONLY. PER ANSI/TPI 1, ALL UPLIFT CONNECTORS ARE THE RESPONSIBILITY OF THE BLDG DESIGNER AND OR CONTRACTOR.



Truss Connector Total List		
Manuf	Product	Qty
Simpson	HTU26	10
Simpson	LG72	5
Simpson	LUS26	11
Simpson	One H2.5A	79
Simpson	THJU26	3



Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Trusses" available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53179



Walk In Customer
5076 Old 421-Prime Estate
Reno.-Roof-24222 Adrian
ROOF PLACEMENT PLAN

Scale: **NTS**
 Date: **3/26/2025**
 Designer: **Gladys Rivas**
 Project Number: **25030187-01**
 Sheet Number:
1/1

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS. ** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES. ** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

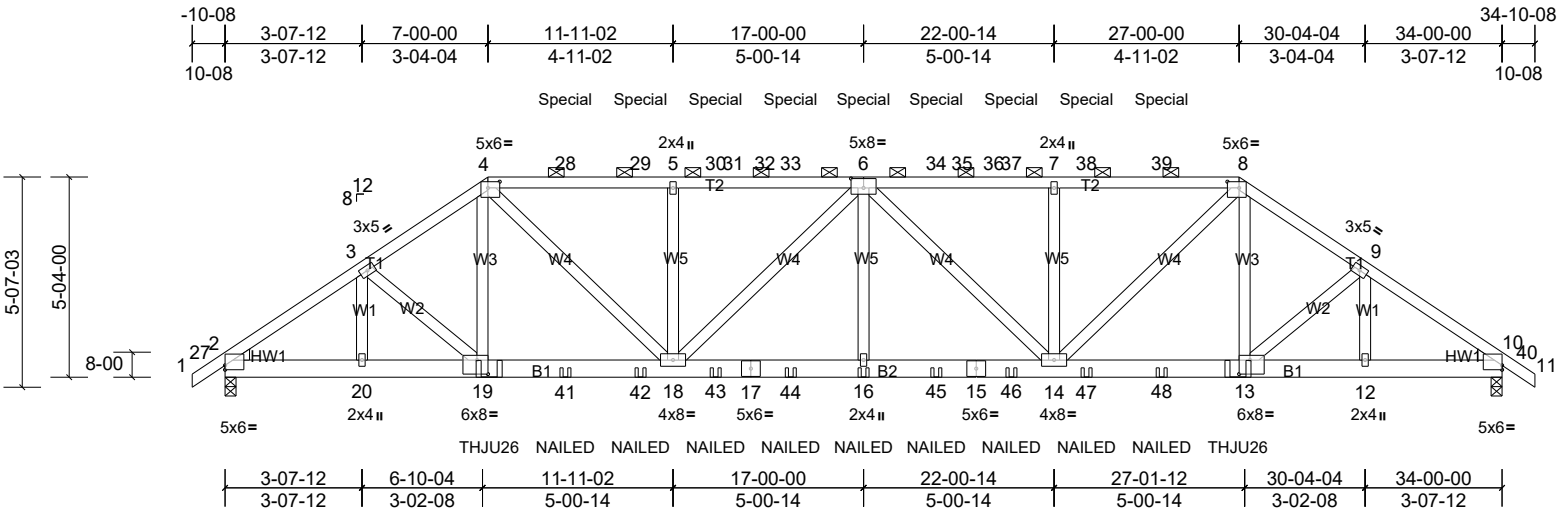
Job 25030187-01	Truss A01	Truss Type Hip Girder	Qty 1	Ply 2	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:01:58

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ID:qP8ymAfV5Exhva4I4rYiAHzXfxh-K3zTsQx2nHppyrJh8FJ9qthn43L6wLcVgBPY7TzX250



Scale = 1:61.6

Plate Offsets (X, Y): [2:Edge,1-06], [4:3-12,2-00], [6:4-00,3-00], [8:3-12,2-00], [10:Edge,1-06], [13:3-08,4-08], [19:3-08,4-08]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.23	16	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.92	Vert(CT)	-0.36	16	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.56	Horz(CT)	0.10	10	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 462 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-7-3 oc purlins, except
2-0-0 oc purlins (3-5-10 max.): 4-8.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=3917/3-08, (min. 2-06), 10=3917/3-08, (min. 2-06)
Max Horiz 2=124 (LC 11)
Max Uplift 2=-647 (LC 12), 10=-647 (LC 13)
Max Grav 2=4004 (LC 37), 10=4004 (LC 37)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-5967/982, 3-4=-6225/1066, 4-28=-7045/1156, 28-29=-7045/1156, 5-29=-7045/1156, 5-30=-7045/1156, 30-31=-7045/1156, 31-32=-7045/1156, 32-33=-7045/1156, 6-33=-7045/1156, 6-34=-7045/1156, 34-35=-7045/1156, 35-36=-7045/1156, 36-37=-7045/1156, 7-37=-7045/1156, 7-38=-7045/1156, 38-39=-7045/1156, 8-39=-7045/1156, 8-9=-6225/1067, 9-10=-5967/983
BOT CHORD 2-20=-819/4845, 19-20=-819/4845, 19-41=-833/5136, 41-42=-833/5136, 18-42=-833/5136, 18-43=-1174/7692, 17-43=-1174/7692, 17-44=-1174/7692, 16-44=-1174/7692, 16-45=-1174/7692, 15-45=-1174/7692, 15-46=-1174/7692, 14-46=-1174/7692, 14-47=-730/5136, 47-48=-730/5136, 13-48=-730/5136, 12-13=-736/4845, 10-12=-736/4845
WEBS 3-20=-537/115, 3-19=-178/541, 4-19=-200/1260, 4-18=-448/2708, 5-18=-1020/346, 6-18=-943/186, 6-16=0/437, 6-14=-943/185, 7-14=-1020/346, 8-14=-448/2708, 8-13=-201/1260, 9-13=-179/542, 9-12=-537/114

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 9-00 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 9-00 oc.
Web connected as follows: 2x4 - 1 row at 9-00 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

Job 25030187-01	Truss A01	Truss Type Hip Girder	Qty 1	Ply 2	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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- 11) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 10 and 2. This connection is for uplift only and does not consider lateral forces.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Use Simpson Strong-Tie THJU26 (SGL & SGL LC 2-PLY) or equivalent at 7-0-6 from the left end to connect truss(es) E01A (1 ply 2x4 SP), CJ09 (1 ply 2x4 SP) to front face of bottom chord.
- 14) Use Simpson Strong-Tie THJU26 (SGL & SGL RC 2-PLY) or equivalent at 26-11-10 from the left end to connect truss(es) E01A (1 ply 2x4 SP), CJ09 (1 ply 2x4 SP) to front face of bottom chord.
- 15) Fill all nail holes where hanger is in contact with lumber.
- 16) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- 17) LGT2 Hurricane ties must have two studs in line below the truss.
- 18) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 273 lb down and 100 lb up at 9-0-12, 273 lb down and 100 lb up at 11-0-12, 273 lb down and 100 lb up at 13-0-12, 273 lb down and 100 lb up at 15-0-12, 273 lb down and 95 lb up at 17-0-0, 273 lb down and 100 lb up at 18-11-4, 273 lb down and 100 lb up at 20-11-4, and 273 lb down and 100 lb up at 22-11-4, and 273 lb down and 100 lb up at 24-11-4 on top chord. The design/selection of such connection device (s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-4=-60, 4-8=-60, 8-11=-60, 21-24=-20

Concentrated Loads (lb)

Vert: 19=-1062, 6=-236, 16=-85, 13=-1062, 28=-236, 29=-236, 30=-236, 33=-236, 34=-236, 37=-236, 38=-236, 39=-236, 41=-85, 42=-85, 43=-85, 44=-85, 45=-85, 46=-85, 47=-85, 48=-85

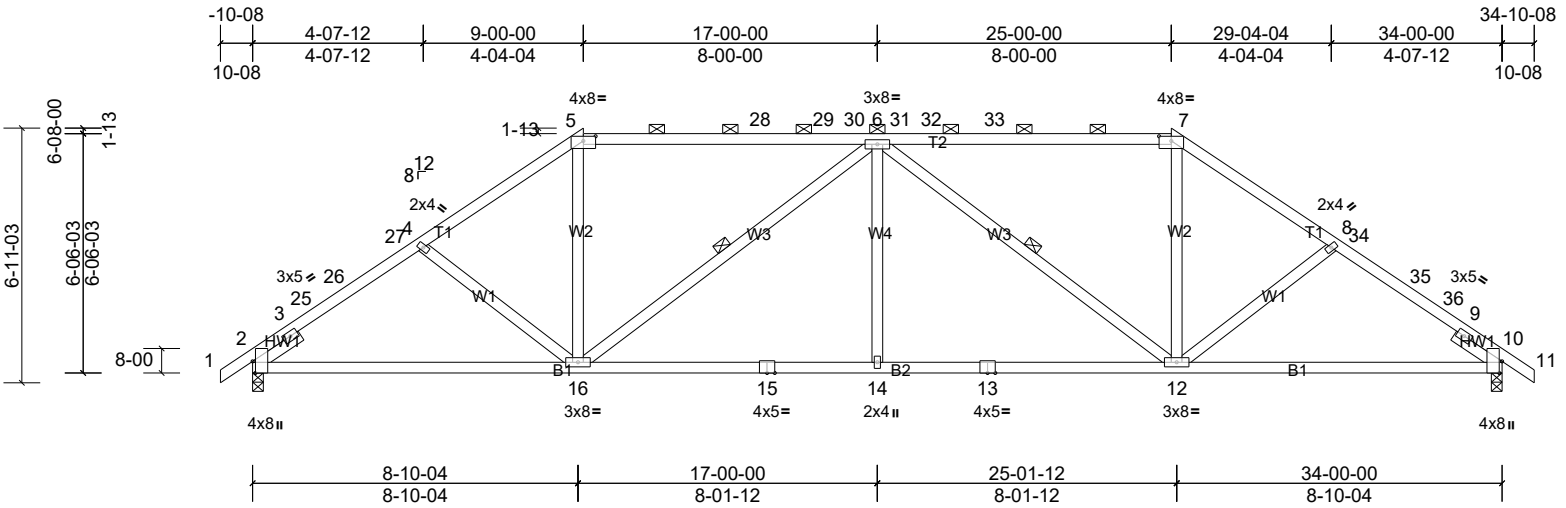
Job 25030187-01	Truss A02	Truss Type Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:63

Plate Offsets (X, Y): [2:3-13,Edge], [5:4-00,1-09], [7:4-00,1-09], [10:3-13,Edge]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.19	14-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.94	Vert(CT)	-0.34	14-16	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.50	Horz(CT)	0.11	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 186 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* T2:2x4 SP 2400F 2.0E
 BOT CHORD 2x4 SP No.2 *Except* B2:2x4 SP No.1
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-1-15 oc purlins, except
 2-0-0 oc purlins (5-4-0 max.): 5-7.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
 WEBS 1 Row at midpt 6-16, 6-12

REACTIONS

(lb/size) 2=1413/3-08, (min. 1-13), 10=1413/3-08, (min. 1-13)
 Max Horiz 2=153 (LC 13)
 Max Uplift 2=-156 (LC 14), 10=-156 (LC 15)
 Max Grav 2=1560 (LC 5), 10=1560 (LC 6)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1373/0, 3-25=-2437/218, 25-26=-2428/228, 26-27=-2388/239, 4-27=-2387/241, 4-5=-2363/225, 5-28=-1971/231, 28-29=-1971/231, 29-30=-1973/230, 6-30=-1973/230, 6-31=-1973/230, 31-32=-1973/230, 32-33=-1971/231, 7-33=-1971/231, 7-8=-2363/225, 8-34=-2387/242, 34-35=-2388/239, 35-36=-2428/228, 9-36=-2437/218, 9-10=-1122/0
 BOT CHORD 2-16=-209/1960, 15-16=-172/2667, 14-15=-172/2667, 13-14=-172/2667, 12-13=-172/2667, 10-12=-107/1960
 WEBS 4-16=-306/141, 5-16=-10/880, 6-16=-949/184, 6-14=0/434, 6-12=-949/184, 7-12=-9/880, 8-12=-306/141

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-10-8 to 2-6-5, Interior (1) 2-6-5 to 4-2-5, Exterior(2R) 4-2-5 to 13-9-11, Interior (1) 13-9-11 to 20-2-5, Exterior(2R) 20-2-5 to 29-9-11, Interior (1) 29-9-11 to 31-5-11, Exterior(2E) 31-5-11 to 34-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

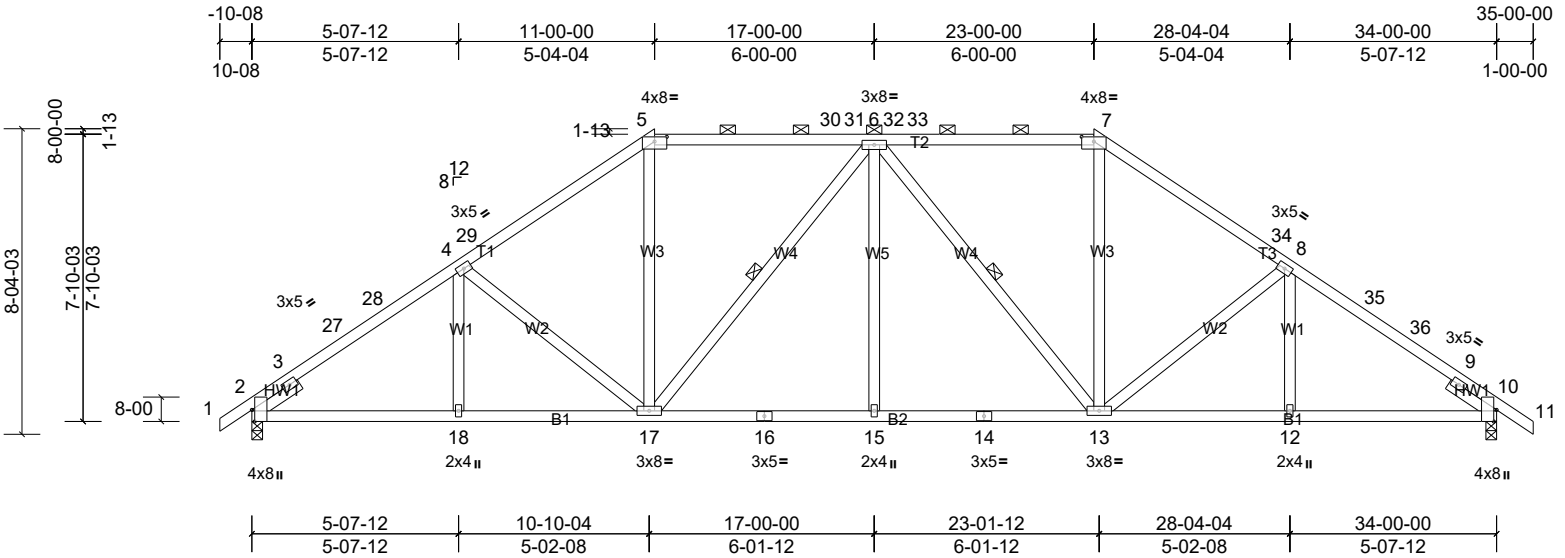
Job 25030187-01	Truss A03	Truss Type Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:63.2

Plate Offsets (X, Y): [2:3-13,Edge], [5:4-00,1-09], [7:4-00,1-09], [10:3-13,Edge]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.14	15-17	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.89	Vert(CT)	-0.25	15-17	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.40	Horz(CT)	0.10	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 207 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-9-9 oc purlins, except 2-0-0 oc purlins (3-10-0 max.): 5-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-17, 6-13

REACTIONS (lb/size) 2=1412/3-08, (min. 1-14), 10=1420/3-08, (min. 1-14)
Max Horiz 2=-185 (LC 12)
Max Uplift 2=-152 (LC 14), 10=-155 (LC 15)
Max Grav 2=1589 (LC 47), 10=1595 (LC 47)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1324/0, 3-27=-2510/195, 27-28=-2438/197, 4-28=-2376/216, 4-29=-2148/204, 5-29=-2094/236, 5-30=-1758/239, 30-31=-1760/239, 6-31=-1760/239, 6-32=-1760/238, 32-33=-1760/238, 7-33=-1758/239, 7-34=-2094/235, 8-34=-2148/203, 8-35=-2375/215, 35-36=-2471/197, 9-36=-2508/194, 9-10=-1050/0
BOT CHORD 2-18=-204/2007, 17-18=-199/2007, 16-17=-77/2050, 15-16=-77/2050, 14-15=-77/2050, 13-14=-77/2050, 12-13=-73/2005, 10-12=-73/2005
WEBS 4-17=-487/160, 5-17=-33/820, 6-17=-577/145, 6-15=0/349, 6-13=-577/145, 7-13=-33/820, 8-13=-486/159

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 6-2-5, Exterior(2R) 6-2-5 to 15-9-11, Interior (1) 15-9-11 to 18-2-5, Exterior(2R) 18-2-5 to 27-9-11, Interior (1) 27-9-11 to 31-7-3 Exterior(2E) 31-7-3 to 35-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

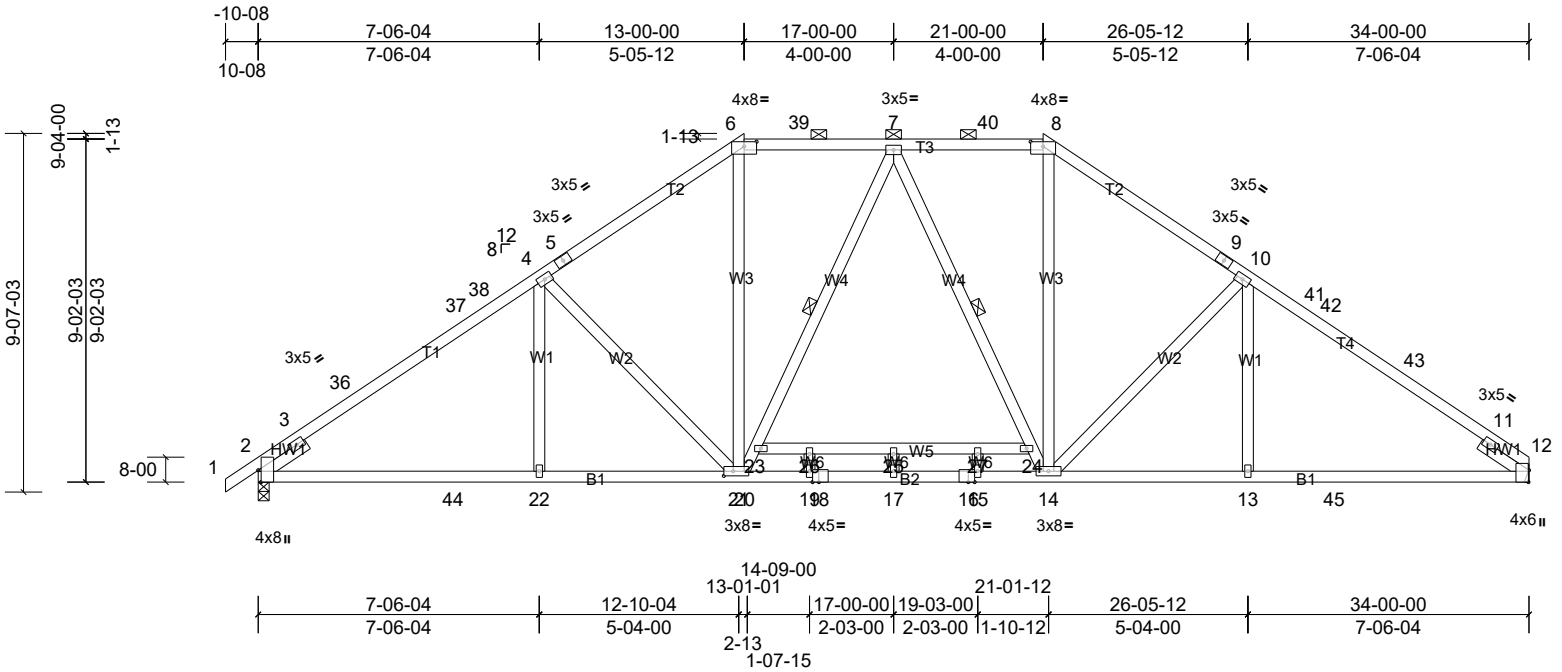
Job 25030187-01	Truss A04	Truss Type Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:61.9

Plate Offsets (X, Y): [2:3-13,Edge], [6:4-00,1-09], [8:4-00,1-09], [12:3-13,Edge], [16:2-00,Edge], [18:2-00,Edge], [21:3-00,1-08]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.70	Vert(LL)	-0.15	17	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-0.40	17	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.71	Horz(CT)	0.10	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 219 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-8-3 oc purlins, except
2-0-0 oc purlins (4-4-8 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 7-14, 7-20

REACTIONS (lb/size) 2=1513/3-08, (min. 2-00), 12=1459/ Mechanical, (min. 1-08)
Max Horiz 2=211 (LC 11)
Max Uplift 2=-47 (LC 14), 12=-30 (LC 15)
Max Grav 2=1721 (LC 47), 12=1676 (LC 47)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1352/0, 3-36=-2673/11, 36-37=-2582/19, 37-38=-2545/31, 4-38=-2496/44, 4-5=-2238/33, 5-6=-2109/64, 6-39=-1769/95, 7-39=-1770/94, 7-40=-1769/96, 8-40=-1768/96, 8-9=-2108/66, 9-10=-2237/35, 10-41=-2499/45, 41-42=-2548/32, 42-43=-2586/19, 11-43=-2676/17, 11-12=-1109/0
BOT CHORD 2-44=-269/2118, 22-44=-60/2118, 21-22=-60/2118, 20-21=0/1679, 19-20=0/1724, 18-19=0/1724, 17-18=0/1724, 16-17=0/1724, 15-16=0/1724, 14-15=0/1724, 13-14=0/2121, 13-45=0/2121, 12-45=0/2121
WEBS 6-21=0/870, 8-14=0/871, 10-14=-628/228, 10-13=0/252, 4-21=-623/229, 7-24=-285/160, 14-24=-334/105, 20-23=-349/108, 7-23=-290/161

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 8-2-5, Exterior(2R) 8-2-5 to 25-9-11, Interior (1) 25-9-11 to 30-7-3, Exterior(2E) 30-7-3 to 34-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 12.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Job 25030187-01	Truss A04	Truss Type Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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LOAD CASE(S) Standard

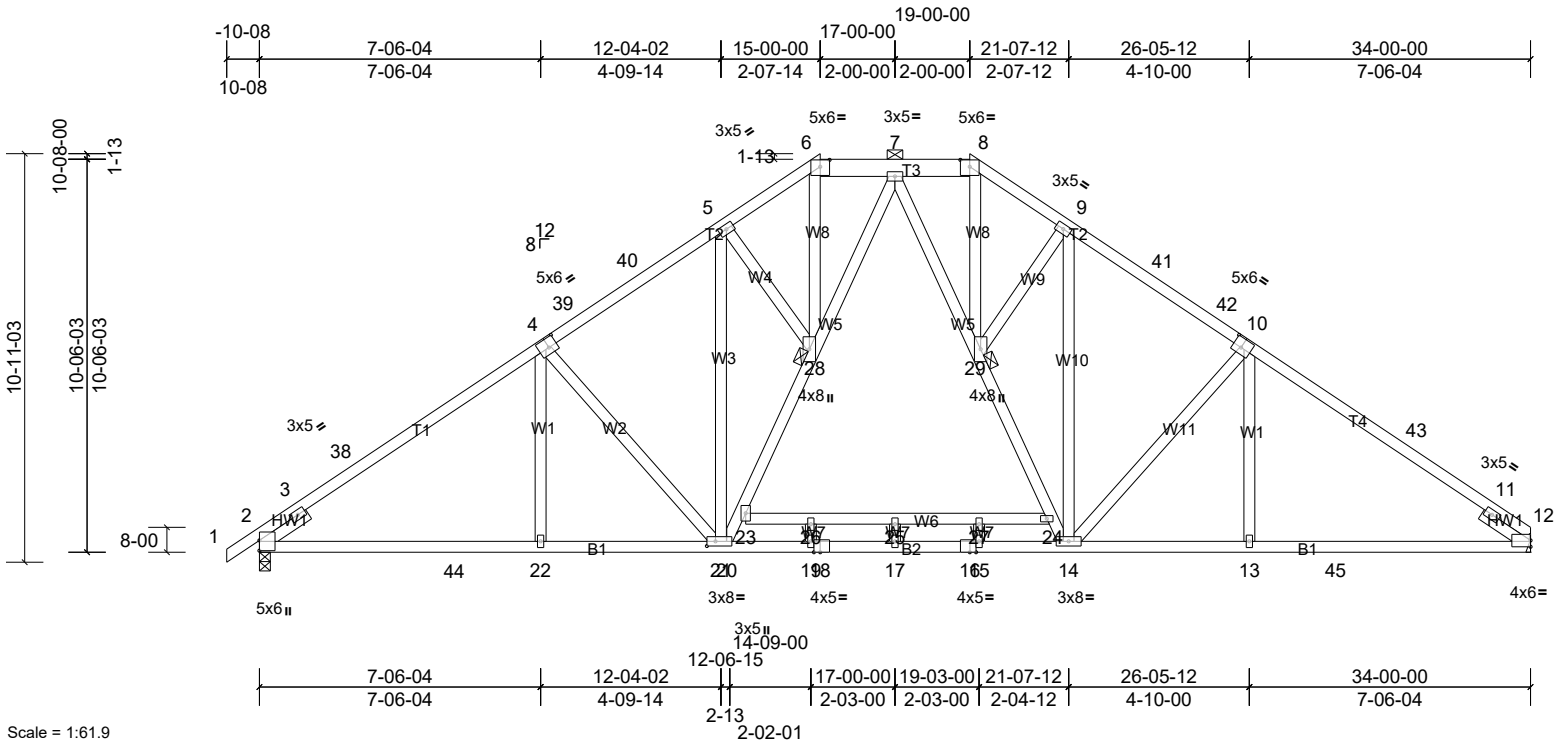
Job 25030187-01	Truss A05	Truss Type Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:61.9

Plate Offsets (X, Y): [4:2-12,3-00], [6:3-00,2-03], [8:3-00,2-03], [10:2-12,3-00], [12:Edge,2-01], [16:2-00,Edge], [18:2-00,Edge], [21:2-12,1-08]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.17	17	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.93	Vert(CT)	-0.49	17	>838	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.88	Horz(CT)	0.09	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 253 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* T3:2x6 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* W5:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00
REACTIONS (lb/size) 2=1513/3-08, (min. 2-02), 12=1459/ Mechanical, (min. 1-08)
Max Horiz 2=242 (LC 11)
Max Uplift 2=42 (LC 14), 12=24 (LC 15)
Max Grav 2=1791 (LC 51), 12=1741 (LC 53)

BRACING
TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except
2-0-0 oc purlins (5-8-8 max.): 6-8.
BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
JOINTS 1 Brace at Jt(s): 28, 29

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1363/0, 3-38=-2738/1, 4-38=-2644/35, 4-39=-2378/13, 39-40=-2273/15, 5-40=-2271/41, 5-6=-2051/113, 6-7=-1652/115, 7-8=-1649/116, 8-9=-2048/114, 9-41=-2276/42, 41-42=-2279/17, 10-42=-2384/14, 10-43=-2646/36, 11-43=-2738/8, 11-12=-1121/0
BOT CHORD 2-44=-297/2169, 22-44=-69/2164, 20-21=0/1818, 19-20=0/1466, 18-19=0/1466, 17-18=0/1466, 16-17=0/1466, 15-16=0/1466, 14-15=0/1466, 13-14=0/2166, 13-45=0/2171, 12-45=0/2171
WEBS 9-14=0/296, 10-14=-538/232, 5-21=0/284, 4-21=-542/232, 20-23=-121/679, 23-28=-62/695, 7-28=-259/160, 7-29=-269/156, 24-29=-66/671, 14-24=-105/652, 6-28=-18/981, 8-29=-20/978, 9-29=-415/17, 5-28=-402/16

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-5, Interior (1) 2-6-5 to 10-2-5, Exterior(2R) 10-2-5 to 23-9-11, Interior (1) 23-9-11 to 30-7-3, Exterior(2E) 30-7-3 to 34-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 24 lb uplift at joint 12.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Job 25030187-01	Truss A05	Truss Type Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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LOAD CASE(S) Standard

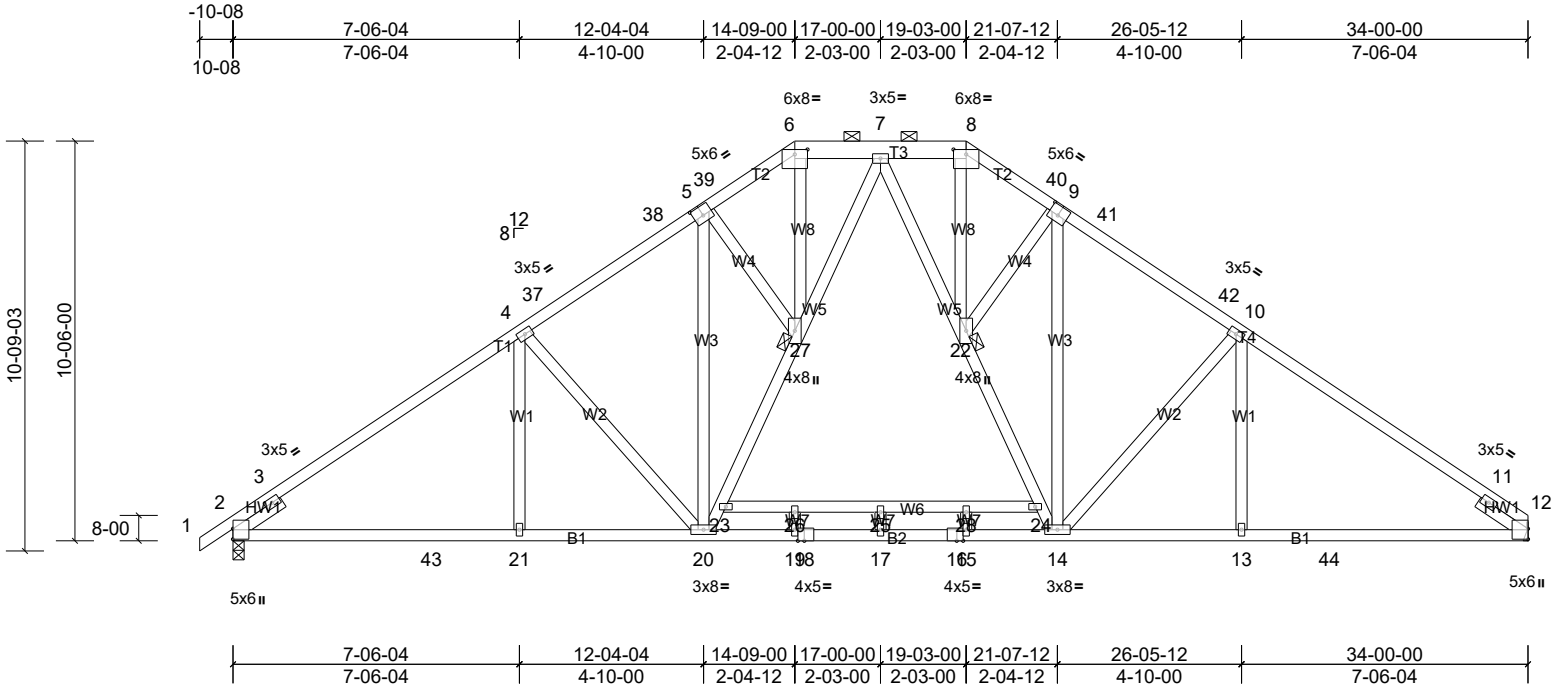
Job 25030187-01	Truss A06	Truss Type Piggyback Base	Qty 3	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:60.8

Plate Offsets (X, Y): [5:3-00,3-00], [6:4-00,1-09], [8:4-00,1-09], [9:3-00,3-00], [16:2-00,Edge], [18:2-00,Edge]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	-0.18	17	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.96	Vert(CT)	-0.51	17	>800	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.09	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 252 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* T3:2x6 SP No.2

BOT CHORD 2x4 SP No.2

WEBS 2x4 SP No.3 *Except* W5:2x4 SP No.2

SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

REACTIONS (lb/size) 2=1513/3-08, (min. 2-02), 12=1459/ Mechanical, (min. 1-08)

Max Horiz 2=240 (LC 11)

Max Uplift 2=-22 (LC 14), 12=-5 (LC 15)

Max Grav 2=1778 (LC 51), 12=1728 (LC 53)

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except

BOT CHORD 2-0-0 oc purlins (5-7-8 max.): 6-8.

JOINTS Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Brace at Jt(s): 22, 27

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1404/0, 3-4=-2830/52, 4-37=-2470/45, 37-38=-2362/47, 5-38=-2293/74, 5-39=-2103/124, 6-39=-2084/139, 6-7=-1733/134, 7-8=-1733/135, 8-40=-2084/139, 9-40=-2103/124, 9-41=-2294/74, 41-42=-2362/47, 10-42=-2470/46, 10-11=-2832/54, 11-12=-1152/0

BOT CHORD 2-43=-282/2237, 21-43=-45/2237, 20-21=-45/2237, 19-20=0/1537, 18-19=0/1537, 17-18=0/1537, 16-17=0/1537, 15-16=0/1537, 14-15=0/1537, 13-14=0/2239, 13-44=0/2239, 12-44=0/2239

WEBS 8-22=-30/1009, 5-20=0/303, 4-20=-556/229, 9-14=0/304, 10-14=-559/231, 20-23=-116/634, 23-27=-70/667, 22-24=-69/666, 14-24=-111/629, 6-27=-30/1009, 5-27=-489/17, 9-22=-489/17

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-6-0, Interior (1) 2-6-0 to 11-4-8, Exterior(2R) 11-4-8 to 22-7-8, Interior (1) 22-7-8 to 30-7-8, Exterior(2E) 30-7-8 to 34-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 12.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

Job 25030187-01	Truss A06	Truss Type Piggyback Base	Qty 3	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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LOAD CASE(S) Standard

Job 25030187-01	Truss A07	Truss Type Piggyback Base Girder	Qty 1	Ply 2	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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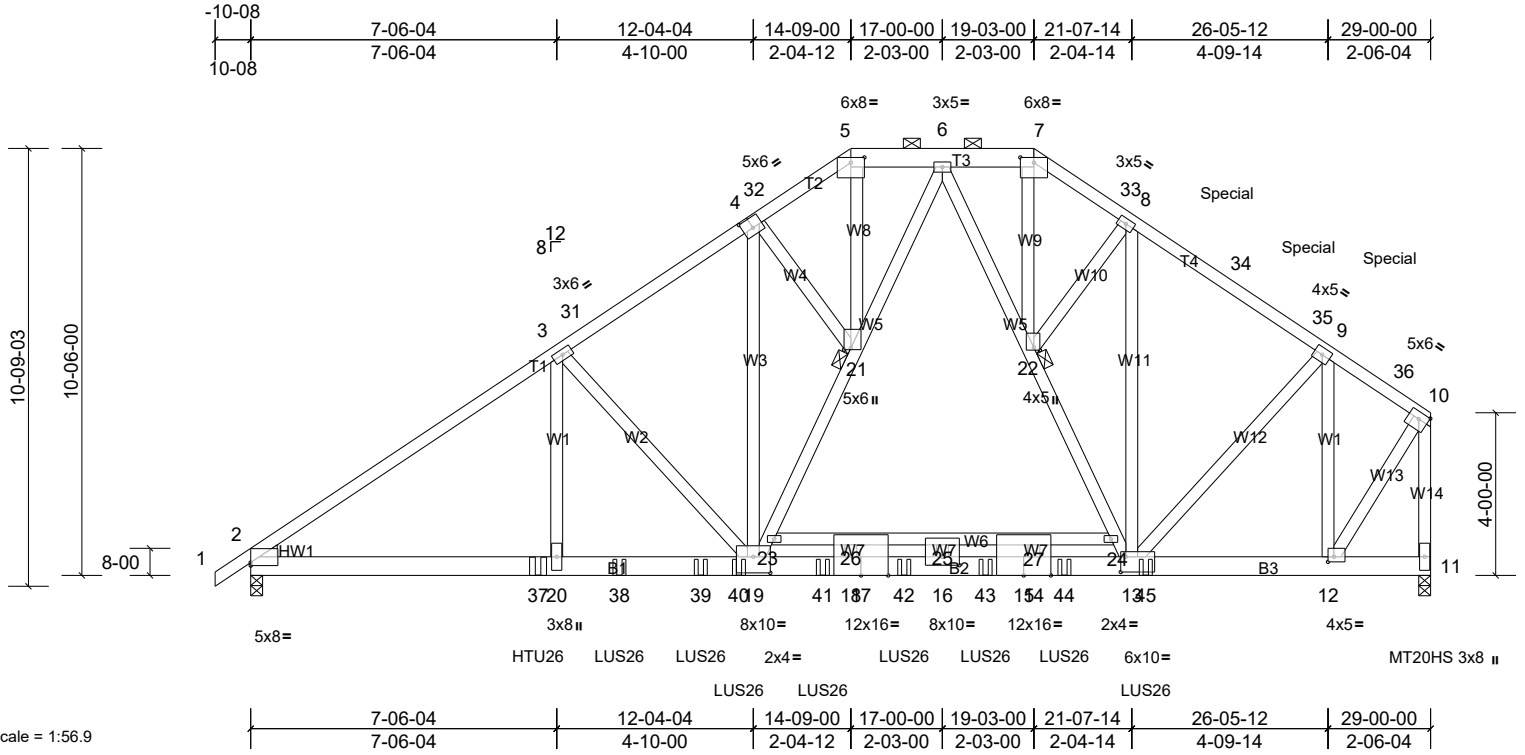


Plate Offsets (X, Y): [2:Edge,0-14], [4:3-00,3-00], [5:4-00,1-09], [7:4-00,1-09], [10:2-12,2-00], [12:1-12,1-08], [13:1-08,4-08], [19:5-00,4-12], [21:0-08,2-00], [22:0-12,1-12], [25:5-00,2-08]

Loading	(psf)	Spacing	1-11-04	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.25	16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.46	16-18	>756	180	MT20HS	187/143
TCDL	10.0	Rep Stress Incr	NO	WB	Horz(CT)	0.05	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 530 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* T3:2x6 SP No.2, T4:2x4 SP No.1, T1:2x4 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* W13,W5,W6:2x4 SP No.2
WEDGE Left: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-4-5 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.); 5-7.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 21, 22

REACTIONS (lb/size) 2=5690/3-08, (min. 2-07), 11=6348/3-08, (min. 2-11)
Max Horiz 2=280 (LC 11)
Max Uplift 2=-964 (LC 12), 11=-1108 (LC 12)
Max Grav 2=5820 (LC 37), 11=6497 (LC 37)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-9296/1560, 3-31=-7592/1252, 4-31=-7476/1280, 4-32=-6248/1124, 5-32=-6226/1139, 5-6=-5238/984, 6-7=-3905/746, 7-33=-4732/884, 8-33=-4751/870, 8-34=-5357/925, 34-35=-5767/995, 9-35=-6104/1063, 9-36=-2967/547, 10-36=-3366/622, 10-11=-6151/1078
BOT CHORD 2-37=-1337/7610, 20-37=-1337/7610, 20-38=-1337/7610, 38-39=-1337/7610, 39-40=-1337/7610, 19-40=-1337/7610, 19-41=-717/4478, 18-41=-717/4478, 17-18=-717/4478, 17-42=-717/4478, 16-42=-717/4478, 16-43=-717/4478, 15-43=-717/4478, 14-15=-717/4478, 14-44=-717/4478, 13-44=-717/4478, 13-45=-479/2691, 12-45=-479/2691
WEBS 8-13=-141/871, 4-19=-298/1820, 3-20=-408/2094, 3-19=-2058/552, 9-12=-4892/847, 9-13=-392/2835, 10-12=-830/4897, 19-23=-687/3905, 21-23=-683/3926, 6-21=-288/1687, 6-22=-1820/347, 22-24=-193/433, 13-24=-224/394, 5-21=-583/3332, 4-21=-1704/300, 7-22=-498/2592, 8-22=-1236/238

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 7-00 oc, 2x6 - 2 rows staggered at 9-00 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 5-00 oc.
Web connected as follows: 2x4 - 1 row at 9-00 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TC LL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 200.0lb AC unit load placed on the bottom chord, 17-0-0 from left end, supported at two points, 5-0-0 apart.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

Job 25030187-01	Truss A07	Truss Type Piggyback Base Girder	Qty 1	Ply 2	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 13) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 11. This connection is for uplift only and does not consider lateral forces.
- 14) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 15) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss) or equivalent at 7-0-12 from the left end to connect truss(es) G01 (1 ply 2x6 SP) to back face of bottom chord.
- 16) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-12 oc max. starting at 9-0-12 from the left end to 22-0-0 to connect truss(es) G02 (1 ply 2x4 SP), G03 (1 ply 2x4 SP), G04 (1 ply 2x4 SP), D06 (1 ply 2x4 SP), D05 (1 ply 2x4 SP), D04 (1 ply 2x4 SP), D03 (1 ply 2x4 SP), D02 (1 ply 2x4 SP) to back face of bottom chord.
- 17) Fill all nail holes where hanger is in contact with lumber.
- 18) LGT2 Hurricane ties must have two studs in line below the truss.
- 19) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 637 lb down and 175 lb up at 24-0-0, and 637 lb down and 175 lb up at 26-0-0, and 638 lb down and 174 lb up at 28-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-5=-58, 5-7=-58, 7-10=-58, 11-28=-19

Concentrated Loads (lb)

Vert: 18=-100, 14=-100, 34=-591, 35=-591, 36=-595, 37=-2124, 38=-732, 39=-656, 40=-645, 41=-723, 42=-784, 43=-784, 44=-698, 45=-629

Job 25030187-01	Truss B01GRD	Truss Type Common Girder	Qty 1	Ply 2	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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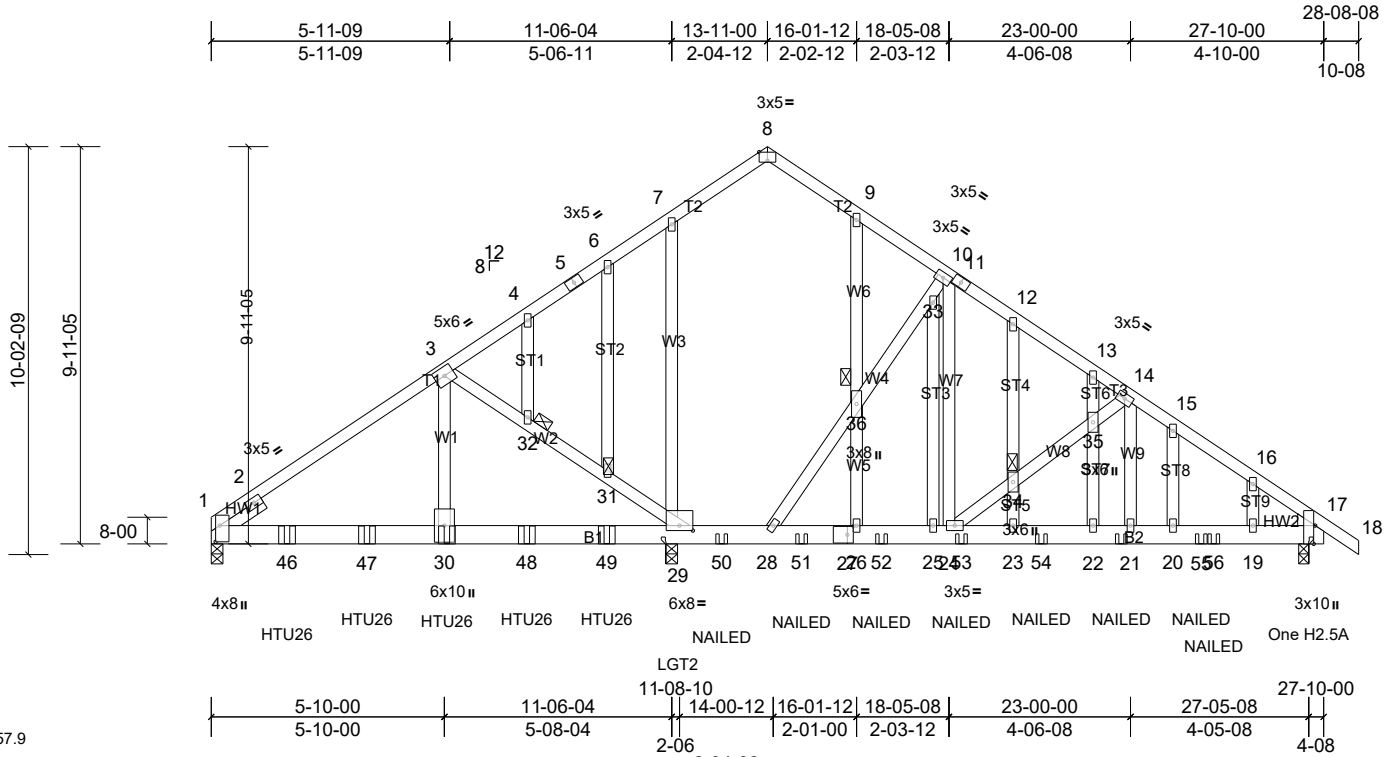


Plate Offsets (X, Y): [1:4-14,1-04], [8:2-08,Edge], [17:5-08,Edge], [27:1-12,2-08], [29:4-00,1-08]

Loading	(psf)	Spacing	1-11-04	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	Vert(LL)	-0.07	30-39	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.65	Vert(CT)	-0.13	30-39	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.69	Horz(CT)	0.03	17	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 460 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP 2400F 2.0E *Except* B2:2x6 SP No.2
WEBS 2x4 SP No.3 *Except* W1:2x4 SP No.2
OTHERS 2x4 SP No.3
WEDGE Right: 2x4 SP 2400F 2.0E
SLIDER Left 2x4 SP No.3 -- 1-02-13

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-2-11 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
JOINTS 1 Brace at Jt(s): 31, 32, 34, 36

REACTIONS (lb/size) 1=4374/3-08, (min. 1-14), 17=2556/3-00, (min. 1-09),
29=6129/3-08, (min. 2-10)
Max Horiz 1=-220 (LC 36)
Max Uplift 1=-55 (LC 12), 17=-535 (LC 13), 29=-567 (LC 12)
Max Grav 1=4535 (LC 22), 17=2640 (LC 20), 29=6320 (LC 22)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-4459/47, 2-3=-4932/28, 6-7=-352/100, 8-9=-255/84, 9-10=-391/112, 10-11=-1312/360, 11-12=-1409/360,
12-13=-1505/345, 13-14=-1534/315, 14-15=-2451/562, 15-16=-2599/549, 16-17=-2620/515
BOT CHORD 1-46=-119/4189, 46-47=-119/4189, 30-47=-119/4189, 30-48=-119/4189, 48-49=-119/4189, 29-49=-119/4189,
28-51=-136/1192, 27-51=-136/1192, 26-27=-136/1192, 26-52=-136/1192, 26-52=-136/1192, 25-52=-136/1192, 24-25=-136/1192,
24-53=-366/2059, 23-53=-366/2059, 23-54=-366/2059, 22-54=-366/2059, 21-22=-366/2059, 20-21=-366/2059,
20-55=-366/2059, 55-56=-366/2059, 19-56=-366/2059, 17-19=-366/2059
WEBS 28-36=-1794/579, 33-36=-1749/561, 10-33=-1460/479, 10-24=-331/1208, 24-34=-1189/338, 34-35=-1151/330,
14-35=-1103/317, 14-21=-226/796, 3-32=-4823/179, 31-32=-4854/184, 29-31=-5085/206, 6-31=-390/36, 25-33=-106/371,
7-29=-73/258, 3-30=-22/5169

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 9-00 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 6-00 oc.
Web connected as follows: 2x4 - 1 row at 9-00 oc, Except member 3-30 2x4 - 1 row at 5-00 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

Job 25030187-01	Truss B01GRD	Truss Type Common Girder	Qty 1	Ply 2	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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- 9) All plates are 2x4 MT20 unless otherwise indicated.
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 13) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 1 and 17. This connection is for uplift only and does not consider lateral forces.
- 14) LGT2 Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 29. This connection is for uplift only and does not consider lateral forces.
- 15) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-10-12 from the left end to 9-10-12 to connect truss(es) A04 (1 ply 2x4 SP), A05 (1 ply 2x4 SP), A06 (1 ply 2x4 SP) to back face of bottom chord.
- 16) Fill all nail holes where hanger is in contact with lumber.
- 17) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- 18) LGT2 Hurricane ties must have two studs in line below the truss.

LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-8=-58, 17-18=-58, 28-37=-19

Concentrated Loads (lb)

Vert: 21=-74, 30=-1614, 46=-1569, 47=-1624, 48=-1614, 49=-1614, 50=-74, 51=-74, 52=-74, 53=-74, 54=-74, 55=-74, 56=-108

Trapezoidal Loads (lb/ft)

Vert: 8=-58-to-9=-83 (F=-25), 9=-83 (F=-25)-to-10=-110 (F=-52), 10=-110 (F=-52)-to-11=-113 (F=-55), 11=-113 (F=-55)-to-12=-128 (F=-70), 12=-128 (F=-70)-to-13=-151 (F=-92), 13=-151 (F=-92)-to-14=-161 (F=-103), 14=-161 (F=-103)-to-15=-173 (F=-115), 15=-173 (F=-115)-to-16=-196 (F=-138), 16=-196 (F=-138)-to-17=-216 (F=-158), 17=-216 (F=-158)-to-18=-216 (F=-158), 18=-216 (F=-158)-to-19=-216 (F=-158), 19=-216 (F=-158)-to-20=-212 (F=-154), 20=-212 (F=-154)-to-21=-123 (F=-103), 21=-123 (F=-103)-to-22=-112 (F=-92), 22=-112 (F=-92)-to-23=-89 (F=-70), 23=-89 (F=-70)-to-24=-71 (F=-52), 24=-71 (F=-52)-to-25=-66 (F=-47), 25=-66 (F=-47)-to-26=-45 (F=-25), 26=-45 (F=-25)-to-27=-42 (F=-23), 27=-42 (F=-23)-to-28=-19 (F=0), 28=-19 (F=0)-to-29=-19 (F=-10), 29=-19 (F=-10)-to-30=-1614 (F=-151), 30=-1614 (F=-151)-to-31=-1614 (F=-151), 31=-1614 (F=-151)-to-32=-1614 (F=-151), 32=-1614 (F=-151)-to-33=-1614 (F=-151), 33=-1614 (F=-151)-to-34=-1614 (F=-151), 34=-1614 (F=-151)-to-35=-1614 (F=-151), 35=-1614 (F=-151)-to-36=-1614 (F=-151), 36=-1614 (F=-151)-to-37=-1614 (F=-151), 37=-1614 (F=-151)-to-38=-1614 (F=-151), 38=-1614 (F=-151)-to-39=-1614 (F=-151), 39=-1614 (F=-151)-to-40=-1614 (F=-151), 40=-1614 (F=-151)-to-41=-1614 (F=-151), 41=-1614 (F=-151)-to-42=-212 (F=-154), 42=-212 (F=-154)-to-43=-173 (F=-115), 43=-173 (F=-115)-to-44=-209 (F=-151), 44=-209 (F=-151)-to-45=-170 (F=-115), 45=-170 (F=-115)-to-46=-1569 (F=-115), 46=-1569 (F=-115)-to-47=-1624 (F=-115), 47=-1624 (F=-115)-to-48=-1614 (F=-115), 48=-1614 (F=-115)-to-49=-1614 (F=-115), 49=-1614 (F=-115)-to-50=-74 (F=-115), 50=-74 (F=-115)-to-51=-74 (F=-115), 51=-74 (F=-115)-to-52=-74 (F=-115), 52=-74 (F=-115)-to-53=-74 (F=-115), 53=-74 (F=-115)-to-54=-97 (F=-78), 54=-97 (F=-78)-to-55=-143 (F=-123), 55=-143 (F=-123)-to-56=-146 (F=-127), 56=-146 (F=-127)-to-57=-146 (F=-127), 57=-146 (F=-127)-to-58=-157 (F=-138), 58=-157 (F=-138)-to-59=-157 (F=-138), 59=-157 (F=-138)-to-60=-157 (F=-138), 60=-157 (F=-138)-to-61=-157 (F=-138), 61=-157 (F=-138)-to-62=-157 (F=-138), 62=-157 (F=-138)-to-63=-157 (F=-138), 63=-157 (F=-138)-to-64=-157 (F=-138), 64=-157 (F=-138)-to-65=-157 (F=-138), 65=-157 (F=-138)-to-66=-157 (F=-138), 66=-157 (F=-138)-to-67=-157 (F=-138), 67=-157 (F=-138)-to-68=-157 (F=-138), 68=-157 (F=-138)-to-69=-157 (F=-138), 69=-157 (F=-138)-to-70=-157 (F=-138), 70=-157 (F=-138)-to-71=-157 (F=-138), 71=-157 (F=-138)-to-72=-157 (F=-138), 72=-157 (F=-138)-to-73=-157 (F=-138), 73=-157 (F=-138)-to-74=-157 (F=-138), 74=-157 (F=-138)-to-75=-157 (F=-138), 75=-157 (F=-138)-to-76=-157 (F=-138), 76=-157 (F=-138)-to-77=-157 (F=-138), 77=-157 (F=-138)-to-78=-157 (F=-138), 78=-157 (F=-138)-to-79=-157 (F=-138), 79=-157 (F=-138)-to-80=-157 (F=-138), 80=-157 (F=-138)-to-81=-157 (F=-138), 81=-157 (F=-138)-to-82=-157 (F=-138), 82=-157 (F=-138)-to-83=-157 (F=-138), 83=-157 (F=-138)-to-84=-157 (F=-138), 84=-157 (F=-138)-to-85=-157 (F=-138), 85=-157 (F=-138)-to-86=-157 (F=-138), 86=-157 (F=-138)-to-87=-157 (F=-138), 87=-157 (F=-138)-to-88=-157 (F=-138), 88=-157 (F=-138)-to-89=-157 (F=-138), 89=-157 (F=-138)-to-90=-157 (F=-138), 90=-157 (F=-138)-to-91=-157 (F=-138), 91=-157 (F=-138)-to-92=-157 (F=-138), 92=-157 (F=-138)-to-93=-157 (F=-138), 93=-157 (F=-138)-to-94=-157 (F=-138), 94=-157 (F=-138)-to-95=-157 (F=-138), 95=-157 (F=-138)-to-96=-157 (F=-138), 96=-157 (F=-138)-to-97=-157 (F=-138), 97=-157 (F=-138)-to-98=-157 (F=-138), 98=-157 (F=-138)-to-99=-157 (F=-138), 99=-157 (F=-138)-to-100=-157 (F=-138)

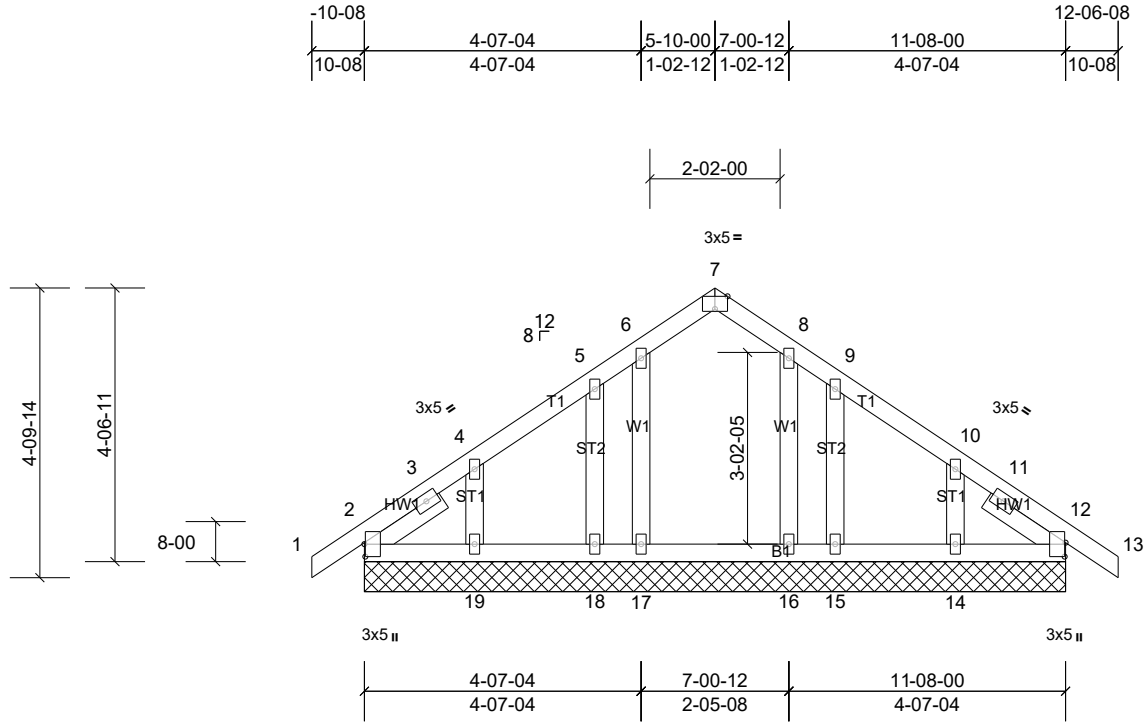
Job 25030187-01	Truss C01G	Truss Type Common Supported Gable	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:38.5

Plate Offsets (X, Y): [2:2-08,0-03], [7:2-08,Edge], [12:2-13,0-03]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	12	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 68 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 OTHERS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 11-08-00.

(lb) - Max Horiz 2=-102 (LC 12), 20=-102 (LC 12)
 Max Uplift All uplift 100 (lb) or less at joint(s) 2, 14, 15, 18, 19, 20
 Max Grav All reactions 250 (lb) or less at joint(s) 2, 12, 14, 15, 16, 17, 18, 19, 20, 24

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 1-10-0, Exterior(2N) 1-10-0 to 2-10-0, Corner(3R) 2-10-0 to 8-10-0, Exterior(2N) 8-10-0 to 9-6-8, Corner(3E) 9-6-8 to 12-6-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 19, 15, 14.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 12, 24.

LOAD CASE(S) Standard

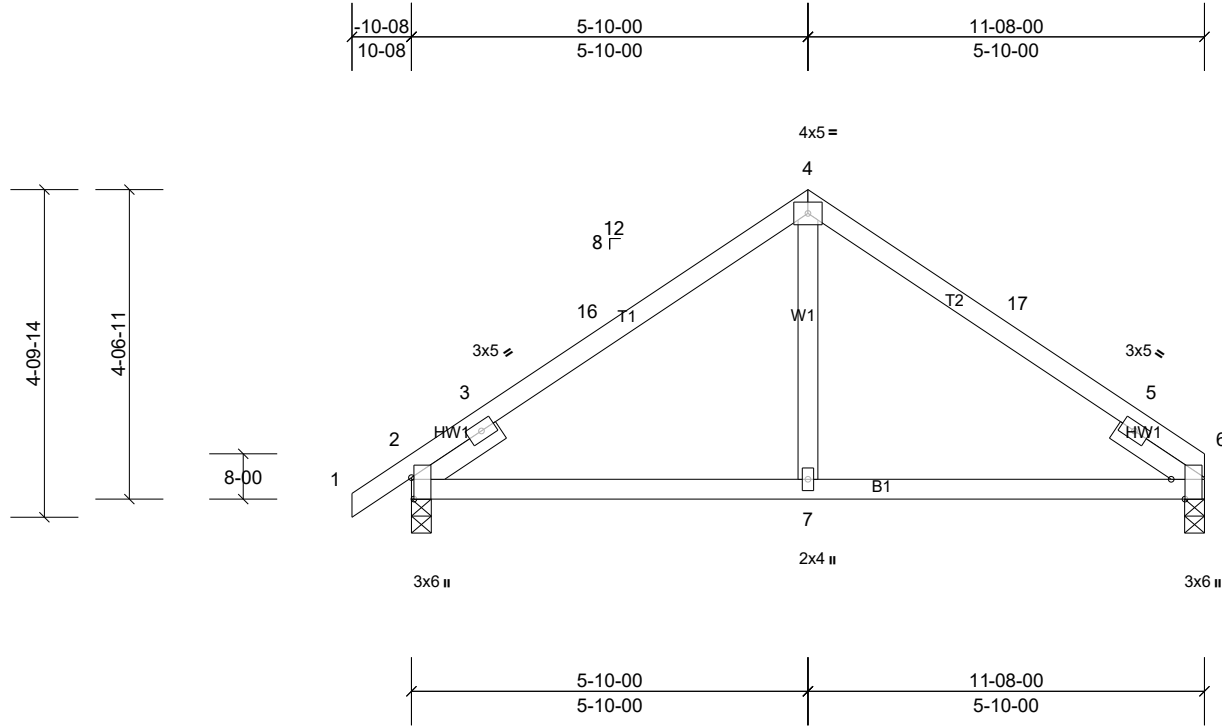
Job 25030187-01	Truss C02	Truss Type Common	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:34

Plate Offsets (X, Y): [2:3-13,Edge], [6:3-08,Edge]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.65	Vert(LL)	-0.07	7-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.53	Vert(CT)	-0.09	7-10	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.10	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 51 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 5-11-7 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=521/3-08, (min. 1-08), 6=465/3-08, (min. 1-08)
 Max Horiz 2=100 (LC 11)
 Max Uplift 2=-56 (LC 14), 6=-39 (LC 15)
 Max Grav 2=619 (LC 21), 6=564 (LC 22)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-284/29, 3-16=-513/116, 4-16=-504/136, 4-17=-504/135, 5-17=-508/117, 5-6=-273/0
 BOT CHORD 2-7=-163/397, 6-7=-15/396
 WEBS 4-7=0/257

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-10-0, Exterior(2R) 2-10-0 to 8-8-0, Exterior(2E) 8-8-0 to 11-8-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 6 and 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

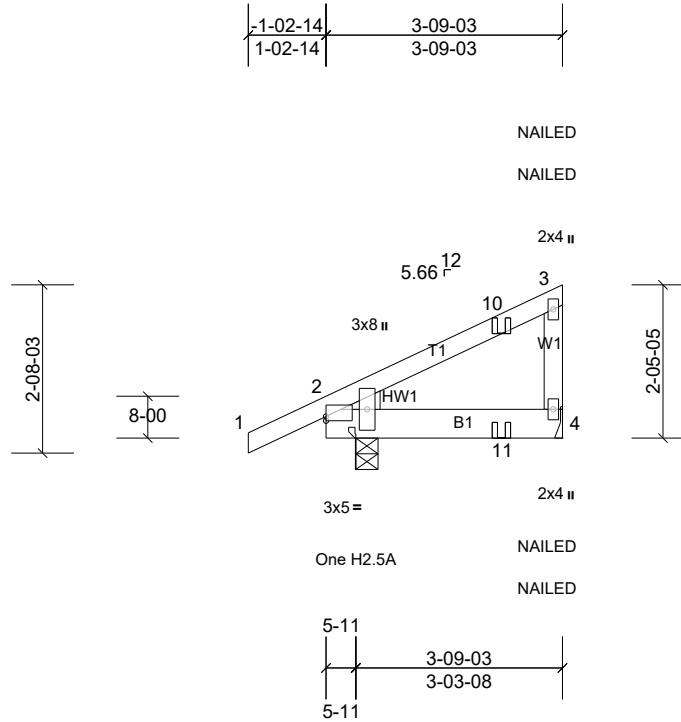
Job 25030187-01	Truss CJ04	Truss Type Diagonal Hip Girder	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:02

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Scale = 1:36.8

Plate Offsets (X, Y): [2:Edge,0-13]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	0.00	4-9	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	0.00	4-9	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							
BCDL	10.0										Weight: 21 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-9-3 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=267/4-04, (min. 1-08), 4=97/ Mechanical, (min. 1-08)
Max Horiz 2=82 (LC 11)
Max Uplift 2=-46 (LC 12), 4=-31 (LC 9)
Max Grav 2=395 (LC 19), 4=135 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 31 lb uplift at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidelines.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=-60, 4-5=-20

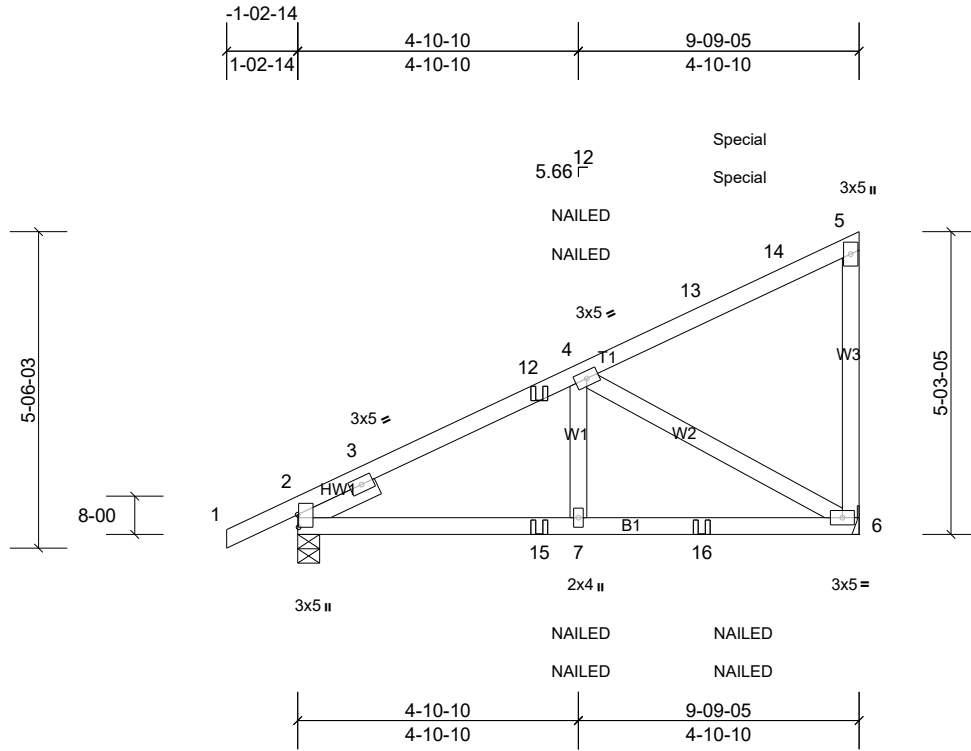
Job 25030187-01	Truss CJ09	Truss Type Diagonal Hip Girder	Qty 3	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:40.3

Plate Offsets (X, Y): [2:2-10,0-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.04	6-7	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.54	Vert(CT)	-0.07	6-7	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.43	Horz(CT)	0.01	6	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 53 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.2 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-8-11 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=605/4-09, (min. 1-08), 6=673/ Mechanical, (min. 1-08)

Max Horiz 2=171 (LC 11)
 Max Uplift 2=-92 (LC 12), 6=-137 (LC 9)
 Max Grav 2=605 (LC 1), 6=714 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-404/38, 3-12=-772/131, 4-12=-700/101, 5-6=-251/84
 BOT CHORD 2-15=-178/686, 7-15=-178/686, 7-16=-178/686, 6-16=-178/686
 WEBS 4-6=-755/197

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 137 lb uplift at joint 6.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- "NAILED" indicates 3-10d (0.148"x3") or 2-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 162 lb down and 87 lb up at 7-0-7, and 162 lb down and 87 lb up at 7-0-7 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (lb/ft)
 Vert: 1-5=-60, 6-8=-20
 Concentrated Loads (lb)
 Vert: 12=-50, 13=-245, 15=-32, 16=-106

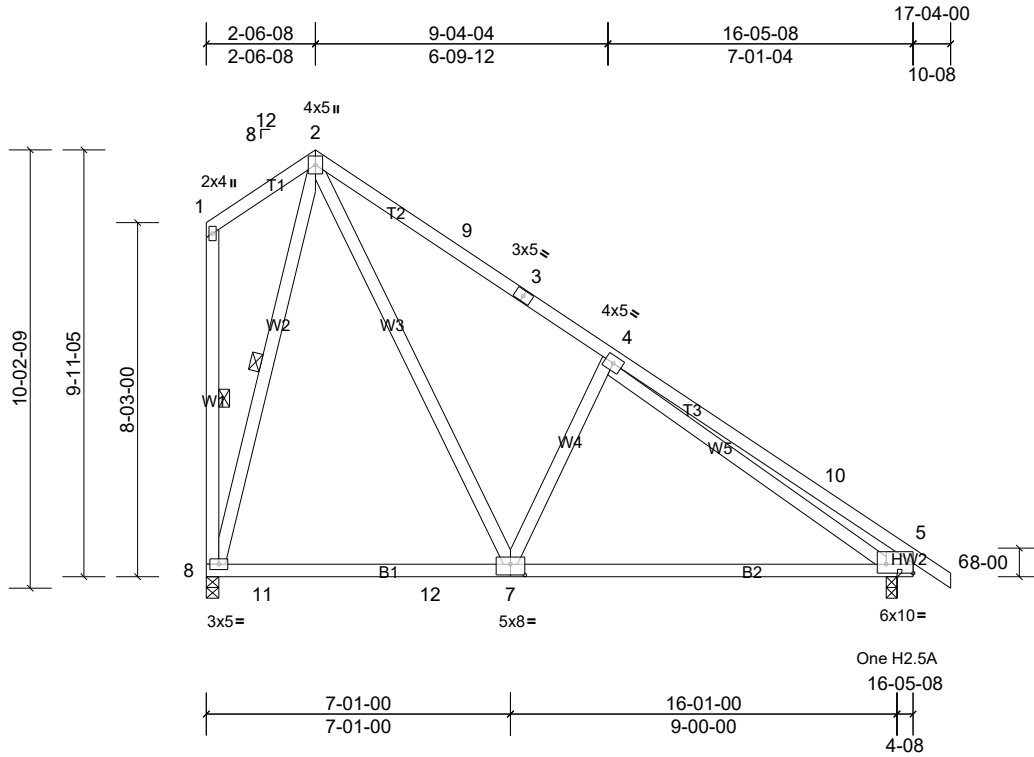
Job 25030187-01	Truss D01	Truss Type Common	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:53.9

Plate Offsets (X, Y): [5:Edge,2-08], [7:4-00,3-00]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.76	Vert(LL)	0.20	5-7	>982	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.84	Vert(CT)	-0.37	5-7	>532	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 118 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* W3:2x4 SP No.2
 SLIDER Right 2x4 SP No.2 -- 7-08

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-5-3 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 1-8, 2-8

REACTIONS (lb/size) 5=709/3-00, (min. 1-08), 8=646/3-08, (min. 1-08)
 Max Horiz 8=-323 (LC 12)
 Max Uplift 5=-69 (LC 15), 8=-155 (LC 10)
 Max Grav 5=764 (LC 6), 8=739 (LC 6)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-9=-672/187, 3-9=-681/166, 3-4=-784/145, 4-10=-830/111, 5-10=-887/78
 BOT CHORD 8-11=-100/272, 11-12=-100/272, 7-12=-100/272, 5-7=-25/673
 WEBS 2-7=-347/770, 4-7=-493/262, 2-8=-666/417

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 11-6-4 to 13-11-0, Exterior(2R) 13-11-0 to 16-11-0, Interior (1) 16-11-0 to 25-8-8, Exterior(2E) 25-8-8 to 28-8-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 8 and 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

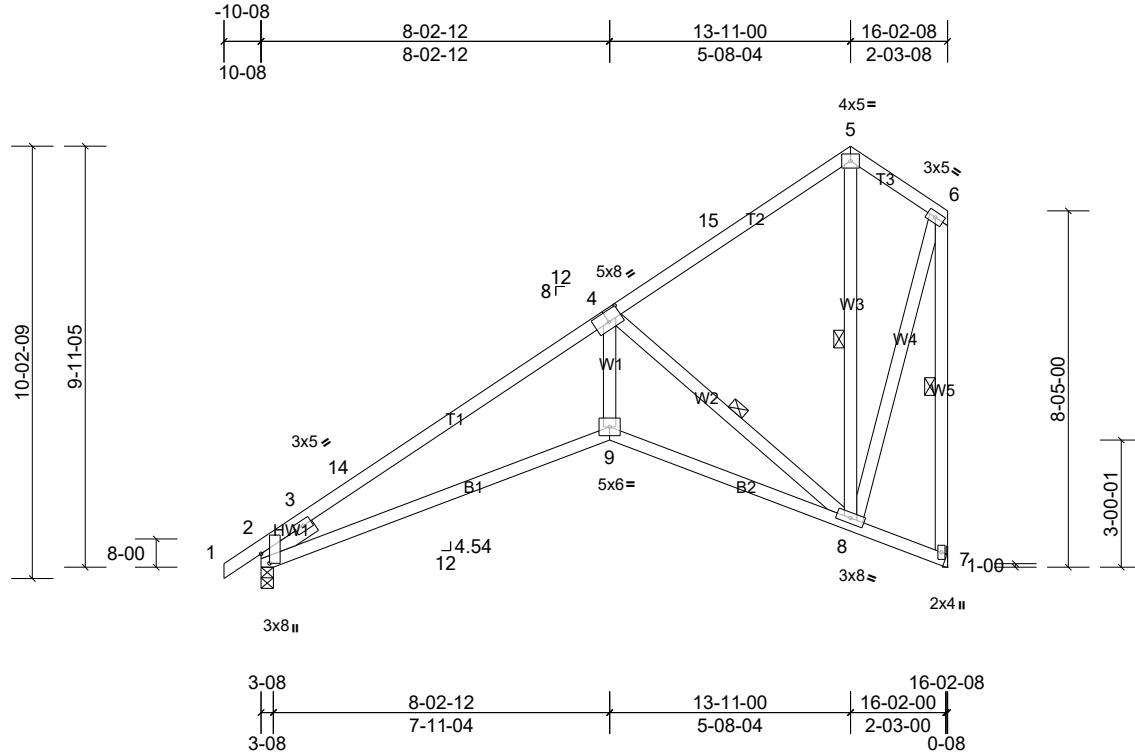
Job 25030187-01	Truss D02	Truss Type Scissor	Qty 4	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:54.6

Plate Offsets (X, Y): [2:2-11,2-07], [4:4-00,3-00]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	0.12	9-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.25	9-12	>784	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.42	Horz(CT)	0.10	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 109 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-3-1 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 8-4-14 oc bracing.
 WEBS 1 Row at midpt 6-7, 5-8, 4-8

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
 Max Horiz 2=329 (LC 14)
 Max Uplift 2=-26 (LC 14), 7=-161 (LC 14)
 Max Grav 2=711 (LC 21), 7=649 (LC 21)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-790/0, 3-14=-1432/216, 4-14=-1342/256, 4-15=-318/6, 6-7=-712/157
 BOT CHORD 2-9=-449/1305, 8-9=-450/1340
 WEBS 6-8=-133/577, 4-9=-248/1060, 4-8=-1402/486

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 10-11-0, Exterior(2R) 10-11-0 to 13-11-0, Exterior(2E) 13-11-0 to 16-0-12 zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 161 lb uplift at joint 7.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

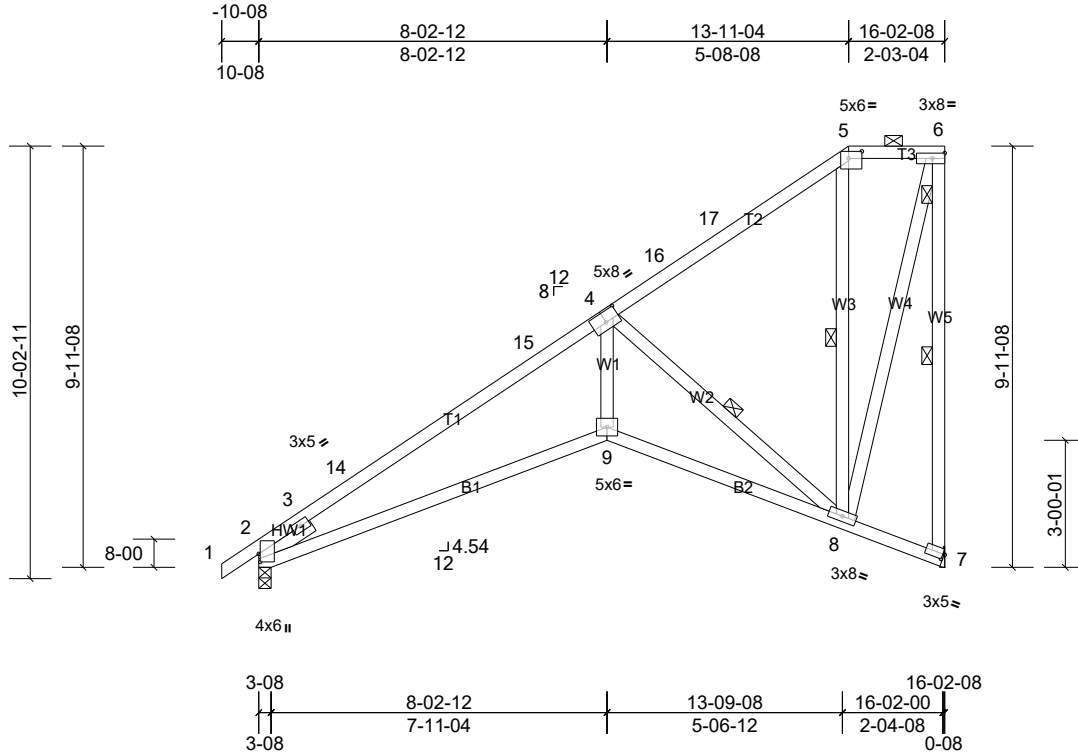
Job 25030187-01	Truss D03	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:54.7

Plate Offsets (X, Y): [2:2-04,0-07], [4:4-00,3-00], [5:3-12,2-00], [7:0-09,1-08]

Loading	(psf)	Spacing	1-11-04	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	0.12	9-12	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.24	9-12	>808	180		
TCDL	10.0	Rep Stress Incr	YES	WB	Horz(CT)	0.13	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 112 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-1-3 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 6-7, 5-8, 4-8

REACTIONS (lb/size) 2=675/3-08, (min. 1-08), 7=621/ Mechanical, (min. 1-08)
 Max Horiz 2=339 (LC 13)
 Max Uplift 2=-59 (LC 14), 7=-122 (LC 14)
 Max Grav 2=755 (LC 38), 7=716 (LC 38)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-864/0, 3-14=-1625/153, 14-15=-1535/166, 4-15=-1442/191, 4-16=-392/114, 16-17=-279/128, 5-17=-260/148, 6-7=-816/127
 BOT CHORD 2-9=-339/1677, 8-9=-313/1724
 WEBS 5-8=-272/143, 6-8=-121/882, 4-9=-148/1316, 4-8=-1700/351

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 10-11-4, Exterior(2R) 10-11-4 to 13-11-4, Exterior(2E) 13-11-4 to 16-0-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 122 lb uplift at joint 7.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

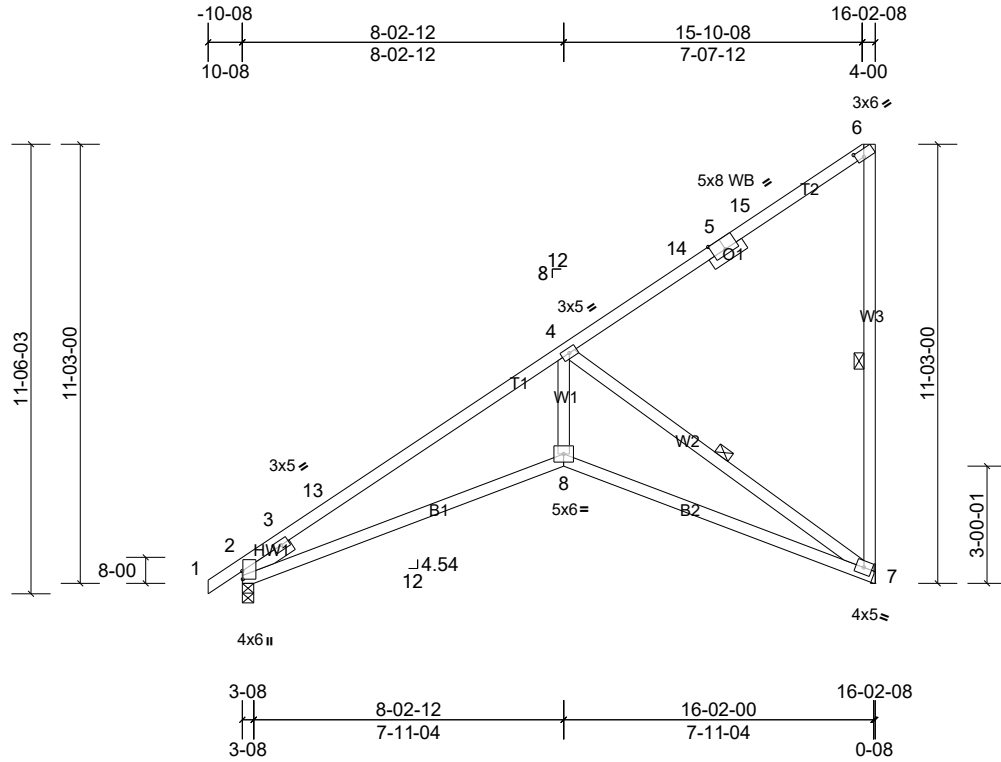
Job 25030187-01	Truss D04	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:59.3

Plate Offsets (X, Y): [2:2-08,0-03], [5:4-00,Edge], [6:2-05,2-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.16	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.73	Vert(CT)	-0.33	7-8	>583	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.80	Horz(CT)	0.14	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 94 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* W3:2x4 SP No.1
OTHERS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 3-1-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 9-6-13 oc bracing.
WEBS 1 Row at midpt 6-7, 4-7

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=400 (LC 13)
Max Uplift 2=-47 (LC 14), 7=-174 (LC 14)
Max Grav 2=724 (LC 21), 7=803 (LC 21)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-884/0, 3-13=-1491/147, 4-13=-1275/188, 4-14=-275/176, 6-7=-319/103
BOT CHORD 2-8=-360/1583, 7-8=-325/1581
WEBS 4-8=-125/1245, 4-7=-1671/406

- NOTES**
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 3) Unbalanced snow loads have been considered for this design.
 - 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 7) Refer to girder(s) for truss to truss connections.
 - 8) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 7.
 - 10) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

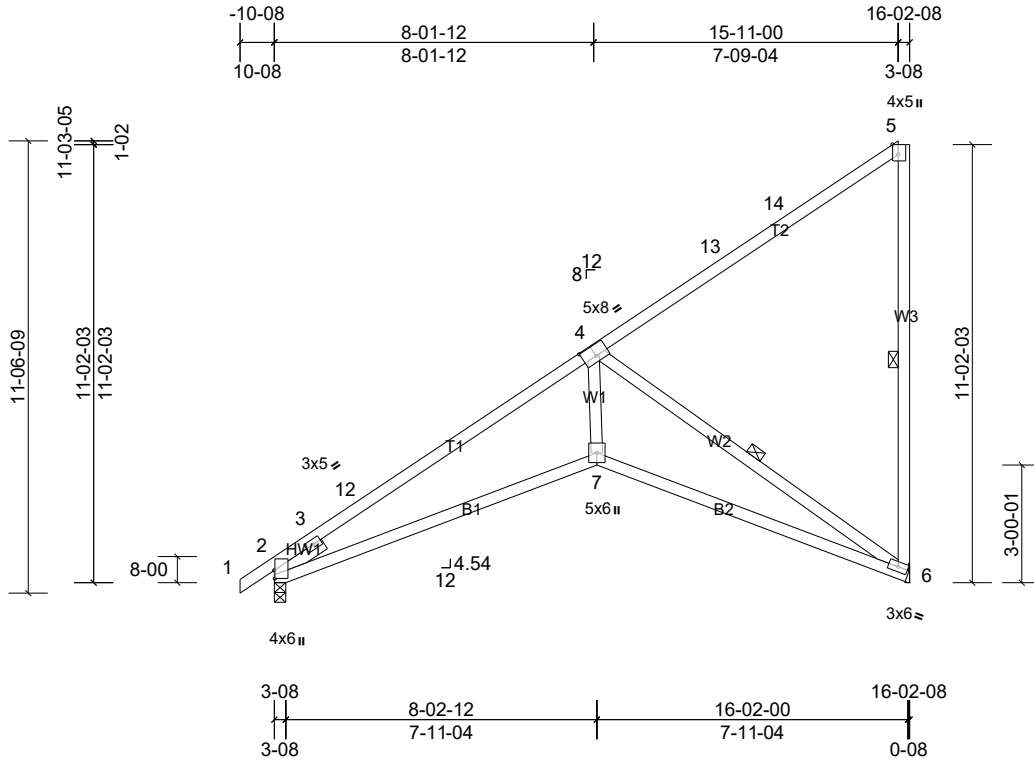
Job 25030187-01	Truss D05	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
Carter Components, Sanford, NC, user					Job Reference (optional)

Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:03

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ID:9dOTXBdLtlBxYJGQdjqzX4iK-9DKk7T?pn7aygmmrVWQZ48woZURKK_Qn27ssL6zX25l



Scale = 1:59

Plate Offsets (X, Y): [2:2-08,0-03], [4:4-00,3-04], [5:3-01,Edge]

Loading	(psf)	Spacing	2-00-00	CSI	0.90	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.90	Vert(LL)	-0.16	6-7	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.74	Vert(CT)	-0.33	6-7	>576	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.15	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 92 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1 *Except* T1:2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3 *Except* W3:2x4 SP No.1
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
 Rigid ceiling directly applied or 9-5-9 oc bracing.
 1 Row at midpt 5-6, 4-6

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 6=641/ Mechanical, (min. 1-08)
 Max Horiz 2=400 (LC 13)
 Max Uplift 2=-47 (LC 14), 6=-174 (LC 14)
 Max Grav 2=724 (LC 21), 6=803 (LC 21)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-878/0, 3-12=-1496/152, 4-12=-1283/192, 4-13=-277/164, 5-6=-325/105
 BOT CHORD 2-7=-360/1491, 6-7=-332/1537
 WEBS 4-7=-128/1196, 4-6=-1606/413

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 174 lb uplift at joint 6.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

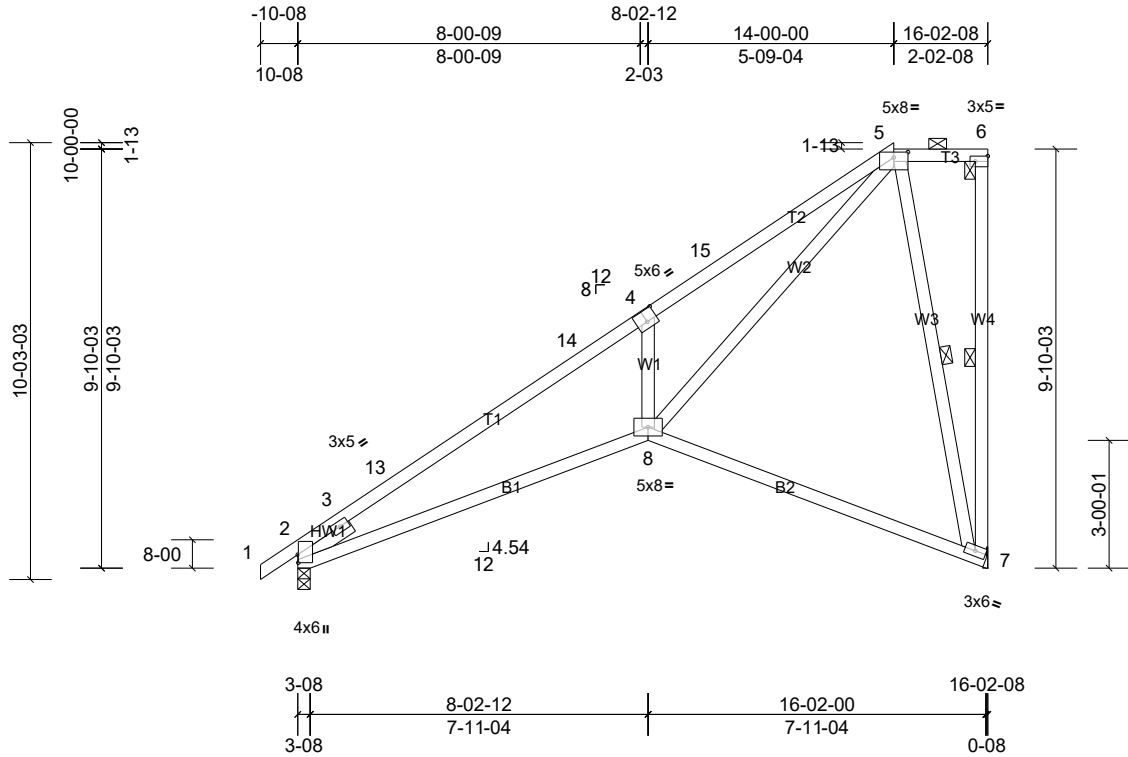
Job 25030187-01	Truss D06	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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ID:VW4fletqGph7GDC4h?ymYzX4jJ-9DKk7T?pN7aygmmrVWQZ48ws8UTrk0pn27ssl6zX25l



Scale = 1:54.4

Plate Offsets (X, Y): [2:2-04,0-03], [4:3-00,3-04], [5:4-00,1-09], [6:Edge,1-08]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.16	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.33	7-8	>588	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.68	Horz(CT)	0.13	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 103 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3 *Except* W4:2x4 SP No.2
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 3-7-2 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6. Rigid ceiling directly applied or 10-0-0 oc bracing.
1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=346 (LC 13)
Max Uplift 2=-62 (LC 14), 7=-126 (LC 14)
Max Grav 2=779 (LC 38), 7=742 (LC 38)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-878/0, 3-13=-1642/155, 13-14=-1527/168, 4-14=-1422/195, 4-15=-1681/355, 5-15=-1545/390
BOT CHORD 2-8=-341/1637, 7-8=-152/324
WEBS 5-8=-386/1831, 5-7=-902/216, 4-8=-556/301

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 9-9-1, Exterior(2R) 9-9-1 to 14-0-0, Exterior(2E) 14-0-0 to 16-0-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 7.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

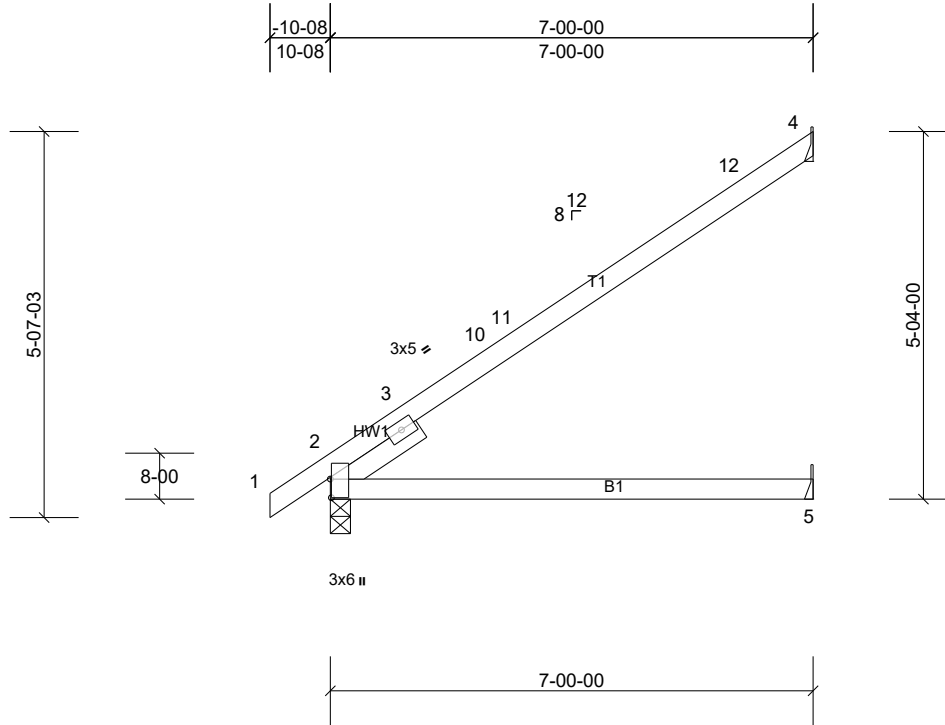
Job 25030187-01	Truss E01	Truss Type Jack-Open	Qty 9	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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ID:TB rVA0TryRVO06MLFZ3AUKzXgGa-dPu6Kp0R8QiplwL12DxodLT_Tul63dhxHnbQtzX25H



Scale = 1:33.6

Plate Offsets (X, Y): [2:3-04,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.17	5-8	>481	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.77	Vert(CT)	-0.31	5-8	>270	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.06	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=333/3-08, (min. 1-08), 4=185/ Mechanical, (min. 1-08), 5=89/ Mechanical, (min. 1-08)
 Max Horiz 2=181 (LC 14)
 Max Uplift 2=-1 (LC 14), 4=-104 (LC 14)
 Max Grav 2=396 (LC 21), 4=296 (LC 21), 5=128 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-521/306
 BOT CHORD 2-5=-364/331

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-8-5, Exterior(2R) 2-8-5 to 6-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

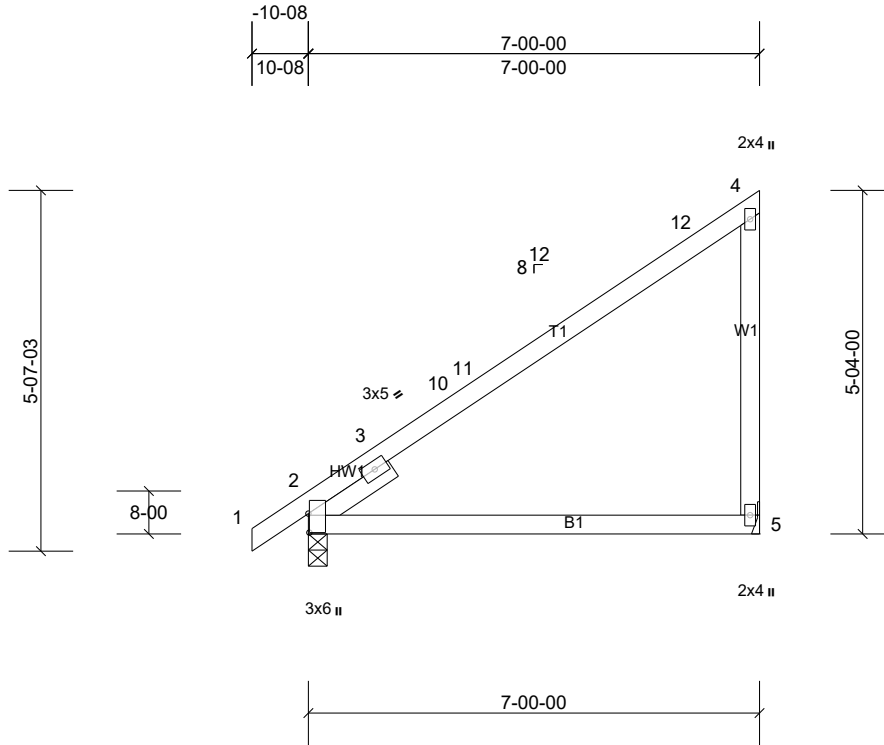
Job 25030187-01	Truss E01A	Truss Type Jack-Closed	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:04

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ID:TB rVA0TryRVO06MLFZ3AUKzXgGa-dPu6Kp0R8QiplwL12DxodLT_wumG3dhxHnbQtZzX25H



Scale = 1:35.9

Plate Offsets (X, Y): [2:3-09,0-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.17	5-8	>495	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.76	Vert(CT)	-0.29	5-8	>279	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.06	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 34 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=330/3-08, (min. 1-08), 5=271/ Mechanical, (min. 1-08)

Max Horiz 2=181 (LC 13)
 Max Uplift 2=-25 (LC 14), 5=-69 (LC 14)
 Max Grav 2=394 (LC 21), 5=396 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-452/299, 4-5=-293/100
 BOT CHORD 2-5=-239/369

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-7-5, Exterior(2R) 2-7-5 to 6-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

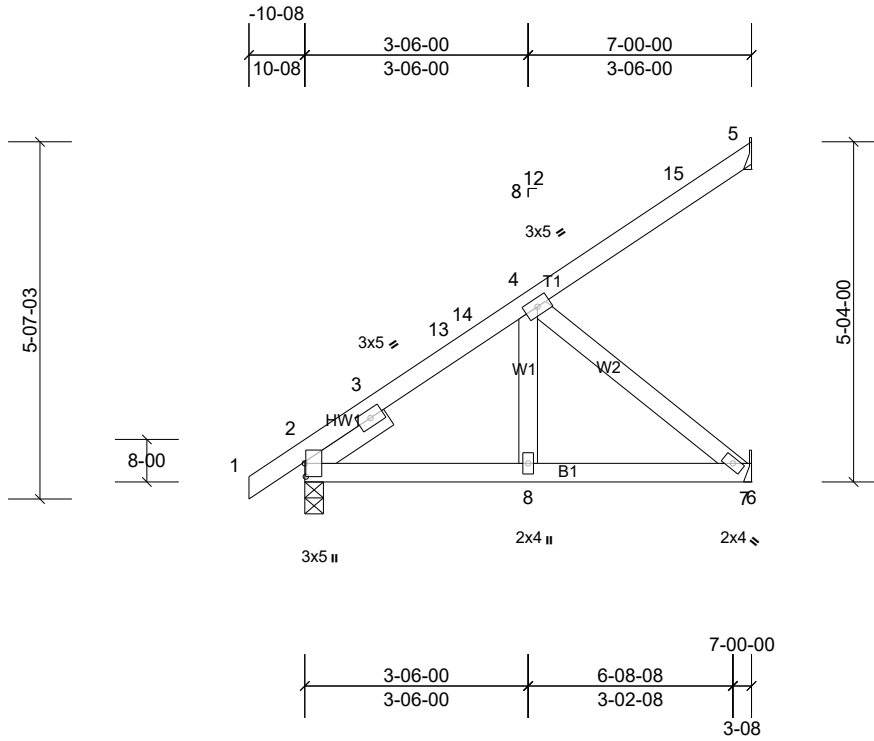
Job 25030187-01	Truss F01	Truss Type Jack-Partial	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:04

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Scale = 1:36.3

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	0.00	7-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.01	7-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.00	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 37 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=332/3-08, (min. 1-08), 5=83/ Mechanical, (min. 1-08), 7=194/
 Mechanical, (min. 1-08)
 Max Horiz 2=181 (LC 14)
 Max Uplift 2=-1 (LC 14), 5=-42 (LC 14), 7=-51 (LC 14)
 Max Grav 2=395 (LC 21), 5=143 (LC 21), 7=261 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-13=-348/0, 13-14=-309/0, 4-14=-281/0
 BOT CHORD 2-8=-169/289, 7-8=-134/289
 WEBS 4-7=-369/171

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 2-8-5, Exterior(2R) 2-8-5 to 6-11-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 5 and 51 lb uplift at joint 7.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

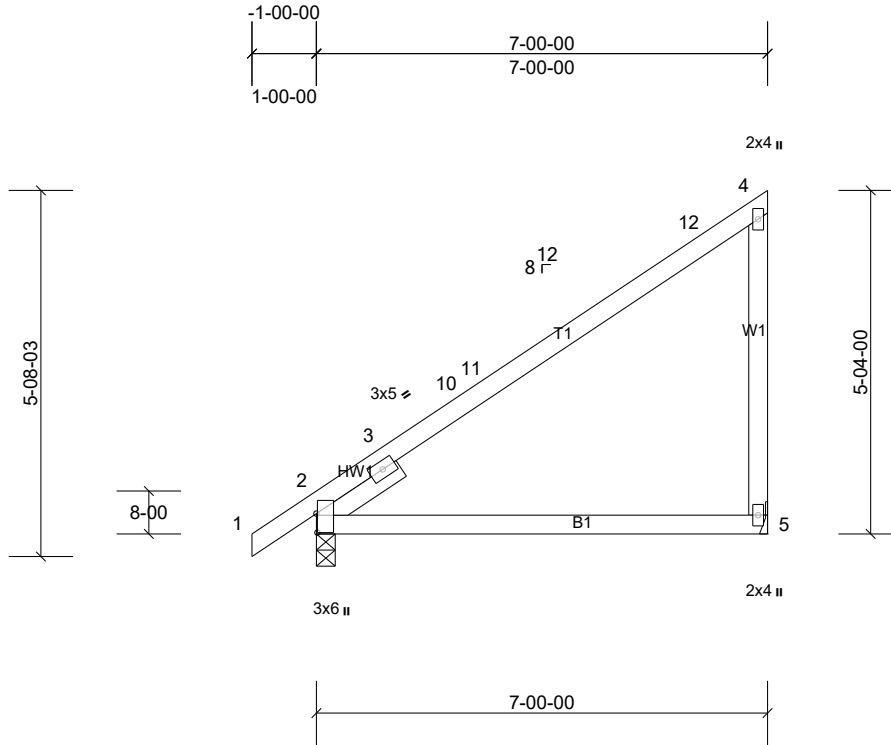
Job 25030187-01	Truss F01A	Truss Type Jack-Closed	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:35.9

Plate Offsets (X, Y): [2:3-09,0-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.86	Vert(LL)	-0.17	5-8	>498	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.75	Vert(CT)	-0.29	5-8	>281	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.06	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 34 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-3-13 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=339/3-08, (min. 1-08), 5=270/ Mechanical, (min. 1-08)

Max Horiz 2=183 (LC 13)
 Max Uplift 2=-28 (LC 14), 5=-69 (LC 14)
 Max Grav 2=402 (LC 21), 5=395 (LC 21)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-460/307, 4-5=-292/99
 BOT CHORD 2-5=-232/364

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -1-0-0 to 2-0-0, Interior (1) 2-0-0 to 2-7-5, Exterior(2R) 2-7-5 to 6-10-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at jt(s) 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

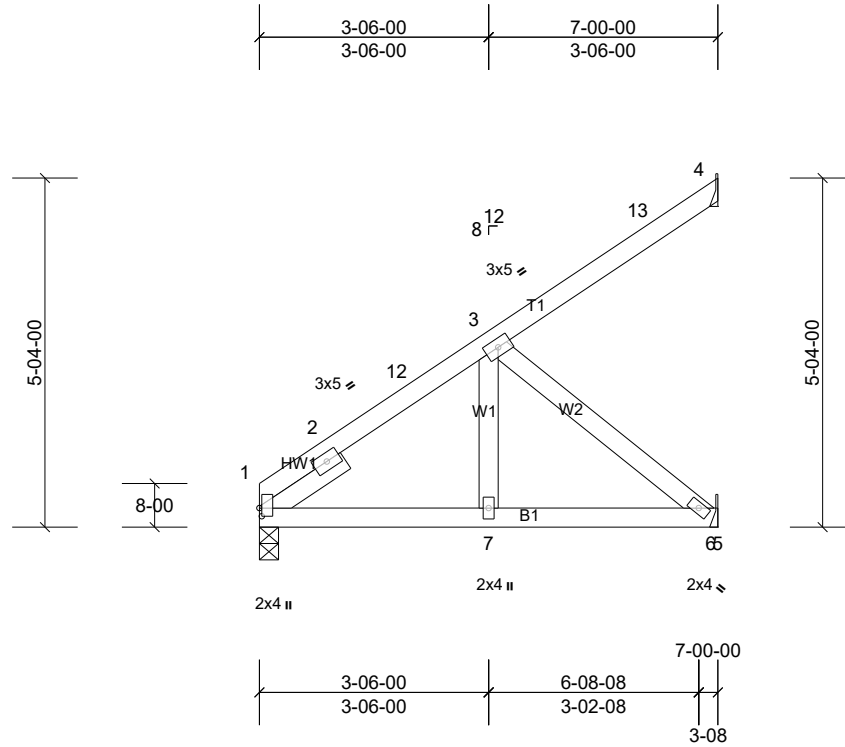
Job 25030187-01	Truss F01B	Truss Type Jack-Partial	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:04

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Scale = 1:35.3

Plate Offsets (X, Y): [1:1-08,0-07]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.25	Vert(LL)	0.01	7-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.12	Vert(CT)	-0.01	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.14	Horz(CT)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 35 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=276/3-08, (min. 1-08), 4=82/ Mechanical, (min. 1-08), 6=198/ Mechanical, (min. 1-08)
 Max Horiz 1=163 (LC 14)
 Max Uplift 4=-42 (LC 14), 6=-53 (LC 14)
 Max Grav 1=339 (LC 20), 4=142 (LC 20), 6=265 (LC 20)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-12=-355/0, 3-12=-286/0
 BOT CHORD 1-7=-184/295, 6-7=-136/295
 WEBS 3-6=-376/173

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 42 lb uplift at joint 4 and 53 lb uplift at joint 6.

LOAD CASE(S) Standard

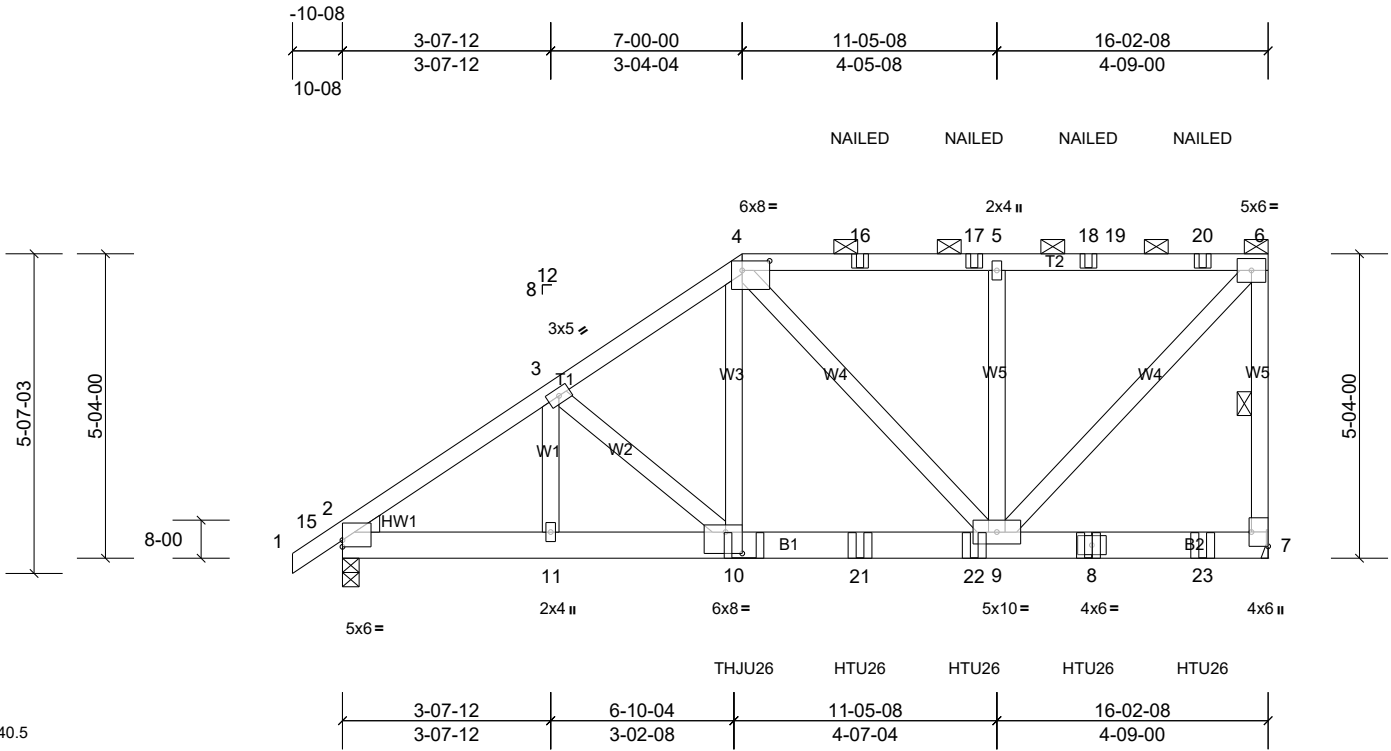
Job 25030187-01	Truss G01	Truss Type Half Hip Girder	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:40.5

Plate Offsets (X, Y): [2:Edge,1-06], [4:5-12,2-00], [7:Edge,3-08], [10:3-08,4-08]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.63	Vert(LL)	-0.06	9-10	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.71	Vert(CT)	-0.10	9-10	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.92	Horz(CT)	0.02	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 115 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3

BRACING
TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 3-5-11 oc purlins, except end verticals, and 2-0-0 oc purlins (4-1-7 max.): 4-6. Rigid ceiling directly applied or 10-0-0 oc bracing.
1 Row at midpt 6-7

REACTIONS (lb/size) 2=1628/3-08, (min. 2-02), 7=2071/ Mechanical, (min. 1-08)
Max Horiz 2=184 (LC 11)
Max Uplift 2=-315 (LC 12), 7=-449 (LC 9)
Max Grav 2=1776 (LC 34), 7=2143 (LC 33)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-2484/461, 3-4=-2364/509, 4-16=-1543/365, 16-17=-1543/365, 5-17=-1543/365, 5-18=-1543/365, 18-19=-1543/365, 19-20=-1543/365, 6-20=-1543/365, 6-7=-1866/403
BOT CHORD 2-11=-420/1980, 10-11=-420/1980, 10-21=-425/1847, 21-22=-425/1847, 9-22=-425/1847
WEBS 4-10=-269/1406, 4-9=-542/119, 5-9=-614/178, 6-9=-467/2232

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 449 lb uplift at joint 7.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Use Simpson Strong-Tie THJU26 (SGL & SGL LC 1-PLY) or equivalent at 7-0-6 from the left end to connect truss(es) F01A (1 ply 2x4 SP), CJ09 (1 ply 2x4 SP) to front face of bottom chord.
 - Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 9-0-12 from the left end to 15-0-12 to connect truss(es) F01B (1 ply 2x4 SP), F01 (1 ply 2x4 SP) to front face of bottom chord.
 - Fill all nail holes where hanger is in contact with lumber.
 - "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.

Job 25030187-01	Truss G01	Truss Type Half Hip Girder	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (lb/ft)
 - Vert: 1-4=-60, 4-6=-60, 7-12=-20
- Concentrated Loads (lb)
 - Vert: 8=-241, 10=-1061, 16=-82, 17=-82, 18=-83, 20=-83, 21=-245, 22=-245, 23=-241

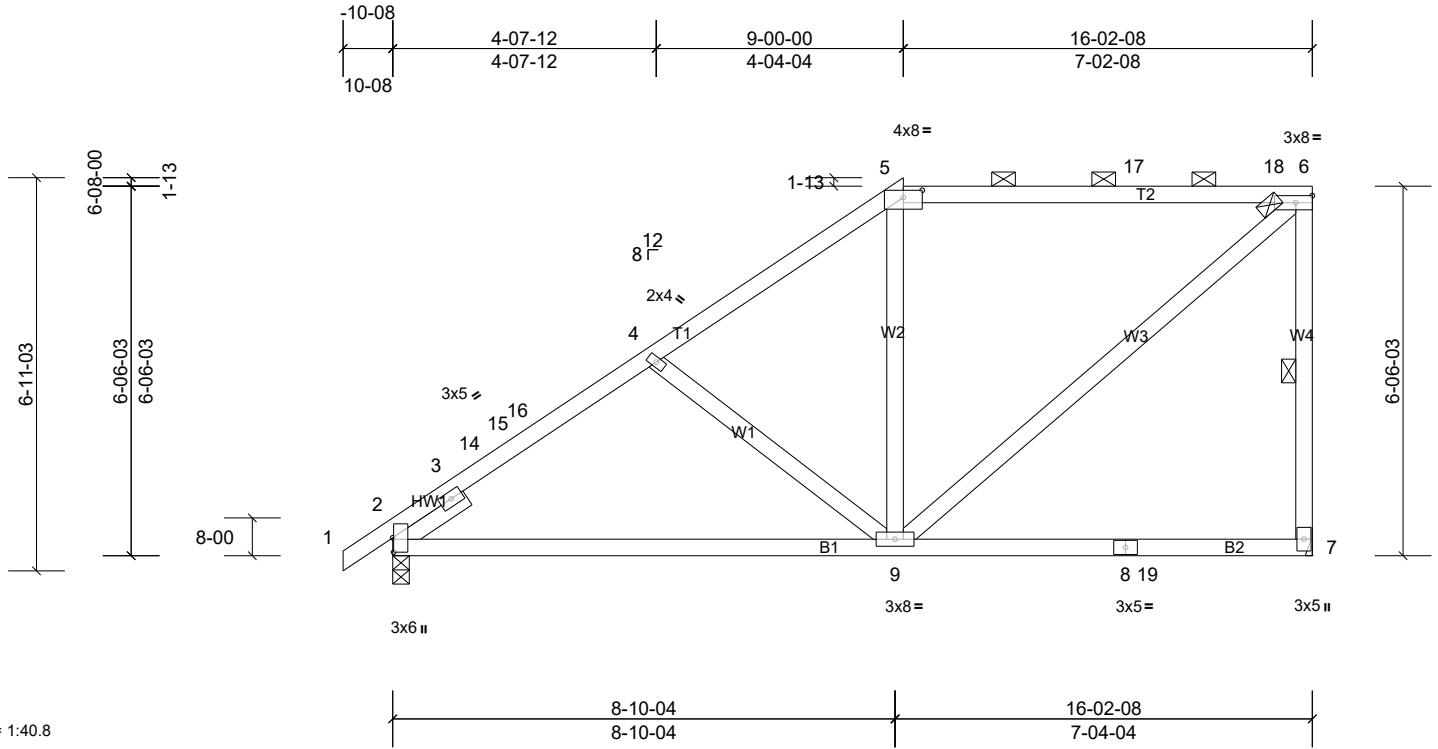
Job 25030187-01	Truss G02	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:40.8

Plate Offsets (X, Y): [2:3-01,0-03], [5:4-00,1-09]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.98	Vert(LL)	-0.09	9-12	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.67	Vert(CT)	-0.19	9-12	>999	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.27	Horz(CT)	0.01	2	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 94 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2 *Except* T2:2x4 SP No.1
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 5-9-8 oc purlins, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 5-6. Rigid ceiling directly applied or 10-0-0 oc bracing.
1 Row at midpt 6-7

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=228 (LC 13)
Max Uplift 2=-72 (LC 14), 7=-105 (LC 11)
Max Grav 2=838 (LC 44), 7=802 (LC 39)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-838/0, 3-14=-980/129, 14-15=-960/132, 15-16=-960/136, 4-16=-877/152, 4-5=-740/143, 5-17=-553/161, 17-18=-553/161, 6-18=-553/160, 6-7=-786/135
BOT CHORD 2-9=-214/971
WEBS 4-9=-386/161, 6-9=-82/786

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 4-6-5, Exterior(2R) 4-6-5 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 7.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

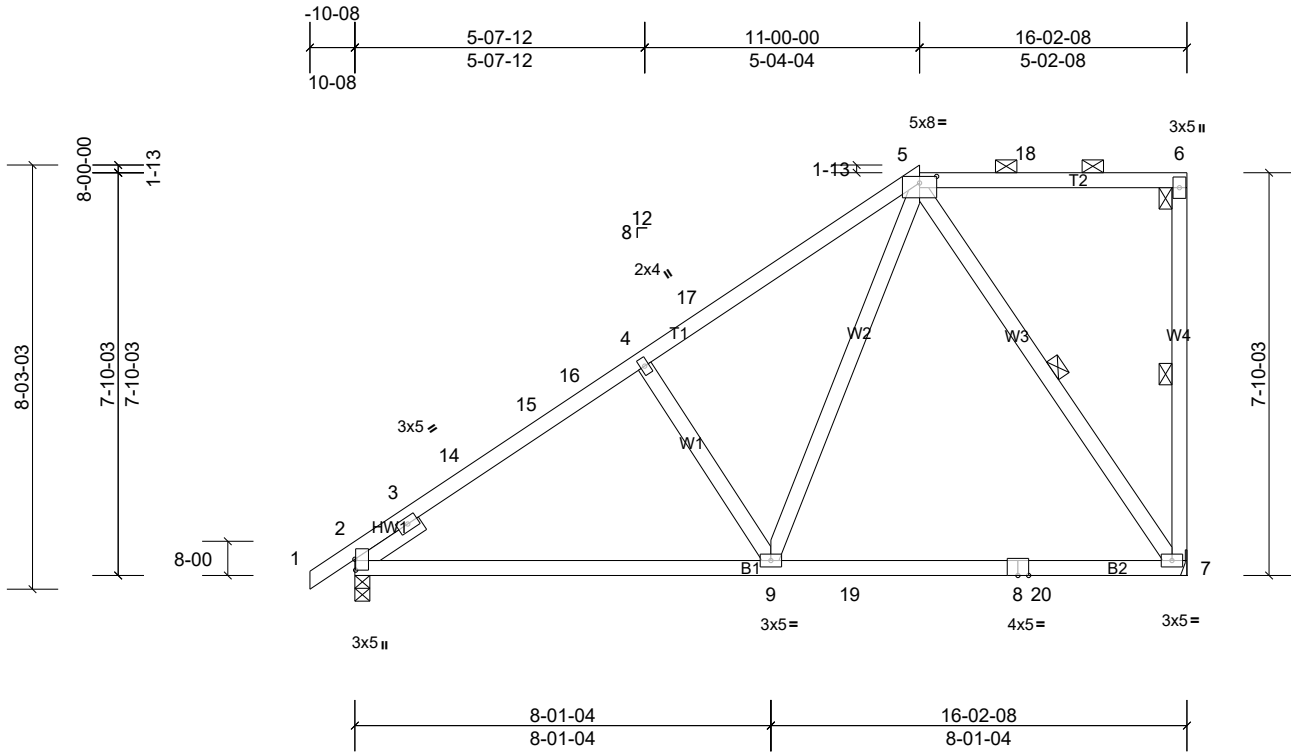
Job 25030187-01	Truss G03	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:45.1

Plate Offsets (X, Y): [2:2-09,0-03], [5:4-00,1-09]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.69	Vert(LL)	-0.20	7-9	>983	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.73	Vert(CT)	-0.30	7-9	>647	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.32	Horz(CT)	0.01	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 98 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-10-4 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
Max Horiz 2=275 (LC 13)
Max Uplift 2=-73 (LC 14), 7=-105 (LC 11)
Max Grav 2=857 (LC 44), 7=748 (LC 39)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-663/0, 3-14=-977/102, 14-15=-950/113, 15-16=-923/125, 4-16=-864/132, 4-17=-853/139, 5-17=-731/171
BOT CHORD 2-9=-223/995, 9-19=-116/507, 8-19=-116/507, 8-20=-116/507, 7-20=-116/507
WEBS 4-9=-418/202, 5-9=-84/753, 5-7=-762/109

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 6-9-1, Exterior(2R) 6-9-1 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 105 lb uplift at joint 7.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

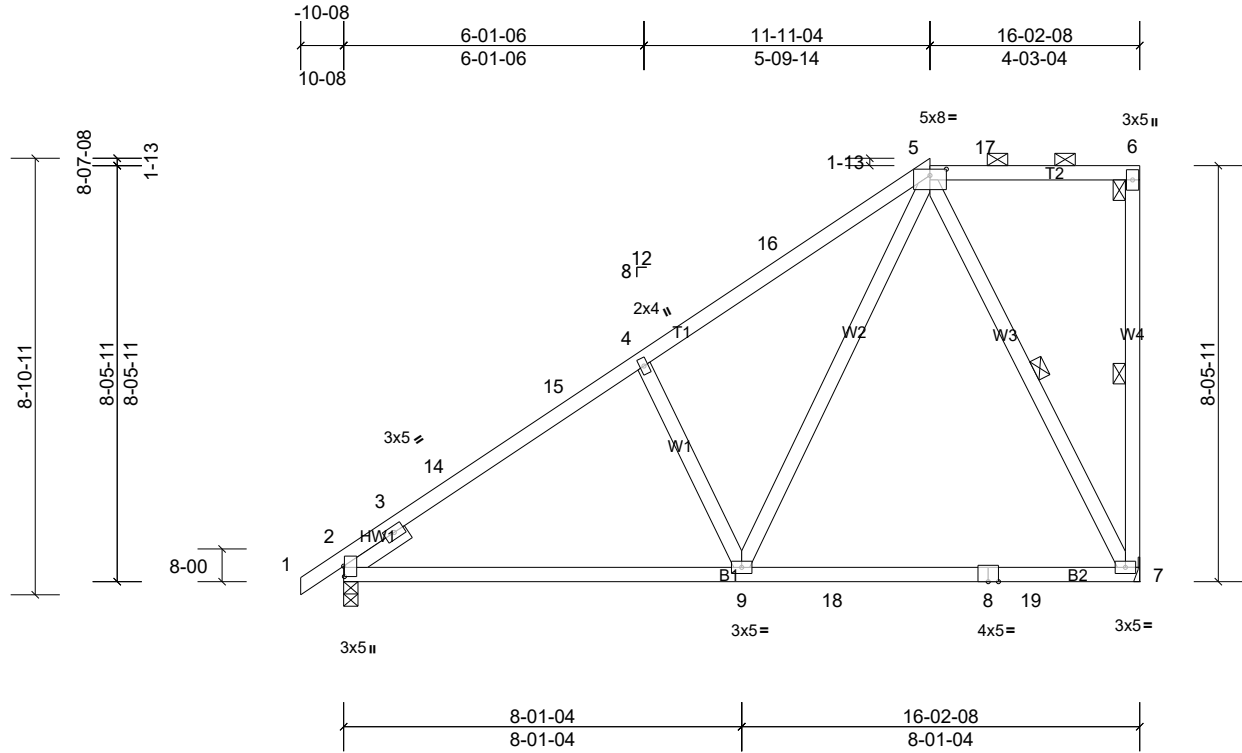
Job 25030187-01	Truss G04	Truss Type Half Hip	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian
Carter Components, Sanford, NC, user					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:47.1

Plate Offsets (X, Y): [2:2-09,0-03], [5:4-00,1-09]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.81	Vert(LL)	-0.21	7-9	>931	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.74	Vert(CT)	-0.31	7-9	>624	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.01	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 101 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 4-11-11 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
 Rigid ceiling directly applied or 10-0-0 oc bracing.
 1 Row at midpt 6-7, 5-7

REACTIONS (lb/size) 2=696/3-08, (min. 1-08), 7=641/ Mechanical, (min. 1-08)
 Max Horiz 2=298 (LC 13)
 Max Uplift 2=-71 (LC 14), 7=-106 (LC 11)
 Max Grav 2=854 (LC 44), 7=753 (LC 44)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-657/0, 3-14=-993/101, 14-15=-945/114, 4-15=-857/134, 4-16=-905/179, 5-16=-771/198
 BOT CHORD 2-9=-236/1034, 9-18=-113/456, 8-18=-113/456, 8-19=-113/456, 7-19=-113/456
 WEBS 4-9=-490/224, 5-9=-125/876, 5-7=-824/123

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 8-11-4, Exterior(2R) 8-11-4 to 13-0-12, Exterior(2E) 13-0-12 to 16-0-12 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 106 lb uplift at jt(s) 2.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

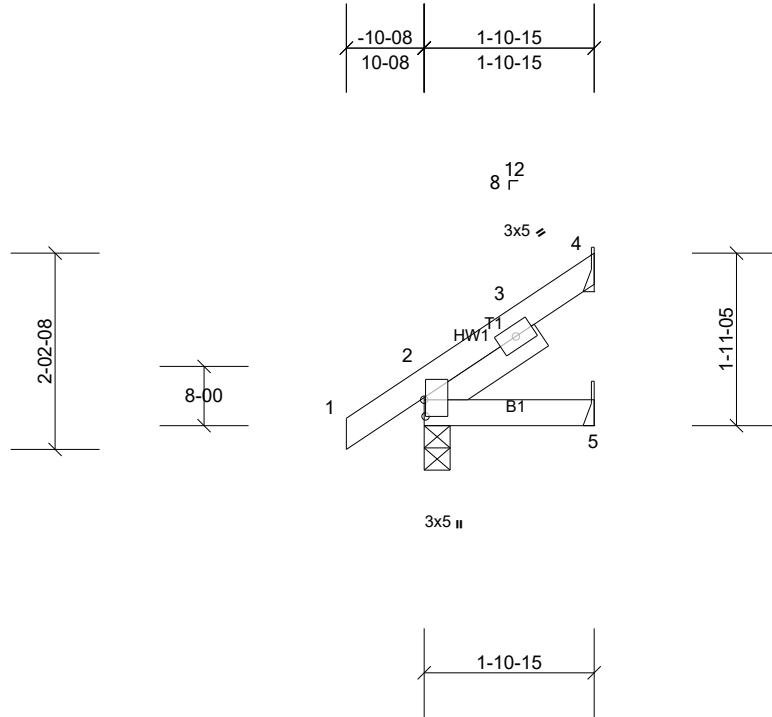
Job 25030187-01	Truss H01	Truss Type Jack-Open	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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ID:jpau1n08pmdDMn9xlRMBgKzXgEa-Zn?sIV1hg2yXXDVPaezGimYWchd8XXADk54WYrZx25F



Scale = 1:26

Plate Offsets (X, Y): [2:2-04,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.08	Vert(LL)	0.00	8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							Weight: 10 lb	FT = 20%
BCDL	10.0											

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=141/3-08, (min. 1-08), 4=43/ Mechanical, (min. 1-08), 5=21/ Mechanical, (min. 1-08)
 Max Horiz 2=65 (LC 14)
 Max Uplift 2=-8 (LC 14), 4=-32 (LC 14), 5=-1 (LC 14)
 Max Grav 2=203 (LC 21), 4=62 (LC 21), 5=31 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 5 and 32 lb uplift at joint 4.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

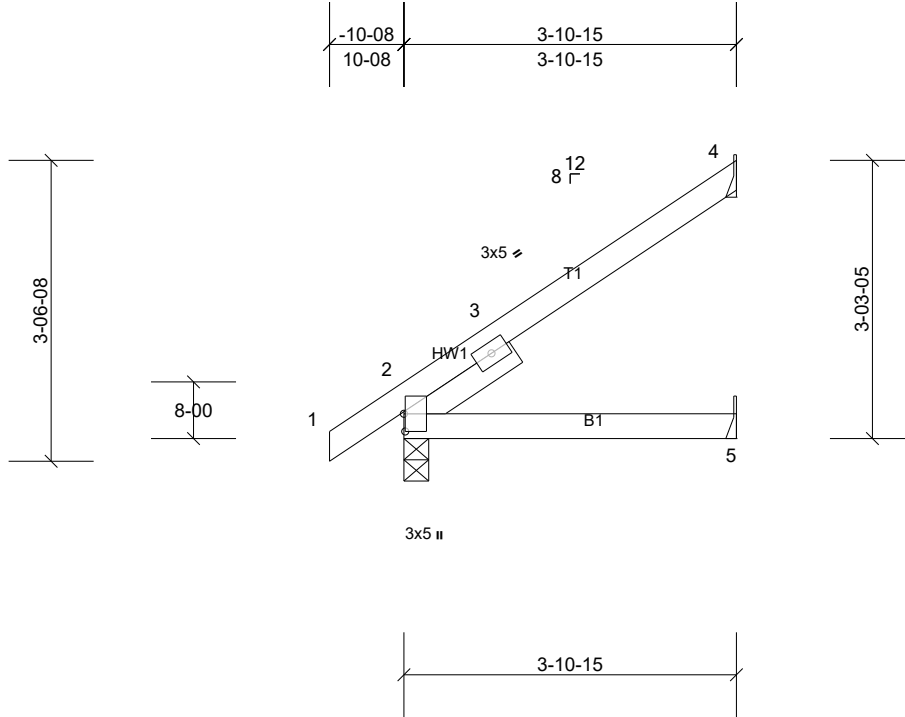
Job 25030187-01	Truss H02	Truss Type Jack-Open	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:27.2

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	-0.02	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.25	Vert(CT)	-0.03	5-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.01	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 17 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-10-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=212/3-08, (min. 1-08), 4=98/ Mechanical, (min. 1-08), 5=50/ Mechanical, (min. 1-08)
 Max Horiz 2=112 (LC 14)
 Max Uplift 2=-3 (LC 14), 4=-64 (LC 14)
 Max Grav 2=329 (LC 21), 4=159 (LC 21), 5=70 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 64 lb uplift at joint 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

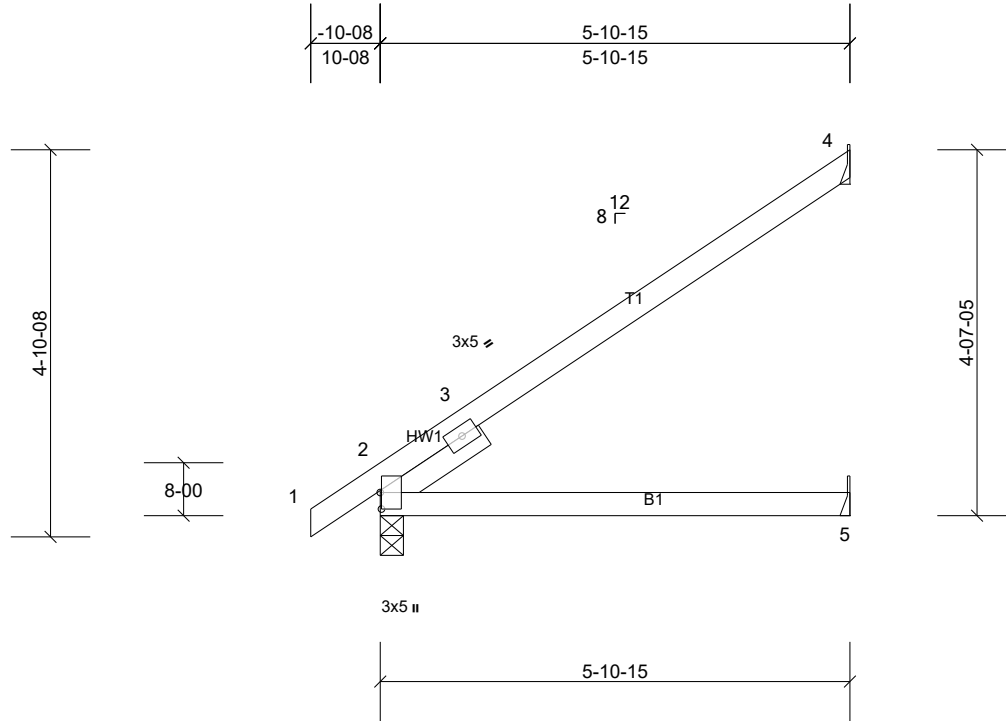
Job 25030187-01	Truss H03	Truss Type Jack-Open	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:29.1

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.11	5-8	>662	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.63	Vert(CT)	-0.18	5-8	>397	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.04	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 23 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 5-10-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=290/3-08, (min. 1-08), 4=153/ Mechanical, (min. 1-08), 5=78/ Mechanical, (min. 1-08)
 Max Horiz 2=161 (LC 14)
 Max Uplift 4=-98 (LC 14)
 Max Grav 2=372 (LC 21), 4=250 (LC 21), 5=109 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-410/257
 BOT CHORD 2-5=-295/289

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 98 lb uplift at joint 4.

LOAD CASE(S) Standard

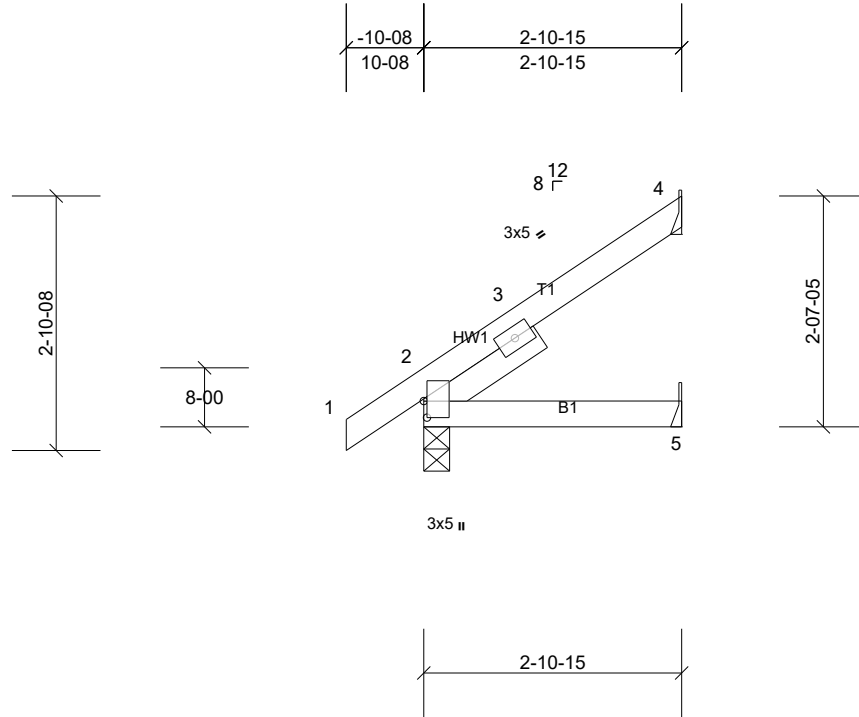
Job 25030187-01	Truss H04	Truss Type Jack-Open	Qty 4	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:26.1

Plate Offsets (X, Y): [2:2-04,0-07]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.15	Vert(LL)	0.00	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.11	Vert(CT)	-0.01	5-8	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 14 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-10-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=175/3-08, (min. 1-08), 4=70/ Mechanical, (min. 1-08), 5=36/
 Mechanical, (min. 1-08)
 Max Horiz 2=88 (LC 14)
 Max Uplift 2=-6 (LC 14), 4=-48 (LC 14)
 Max Grav 2=261 (LC 21), 4=109 (LC 21), 5=51 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

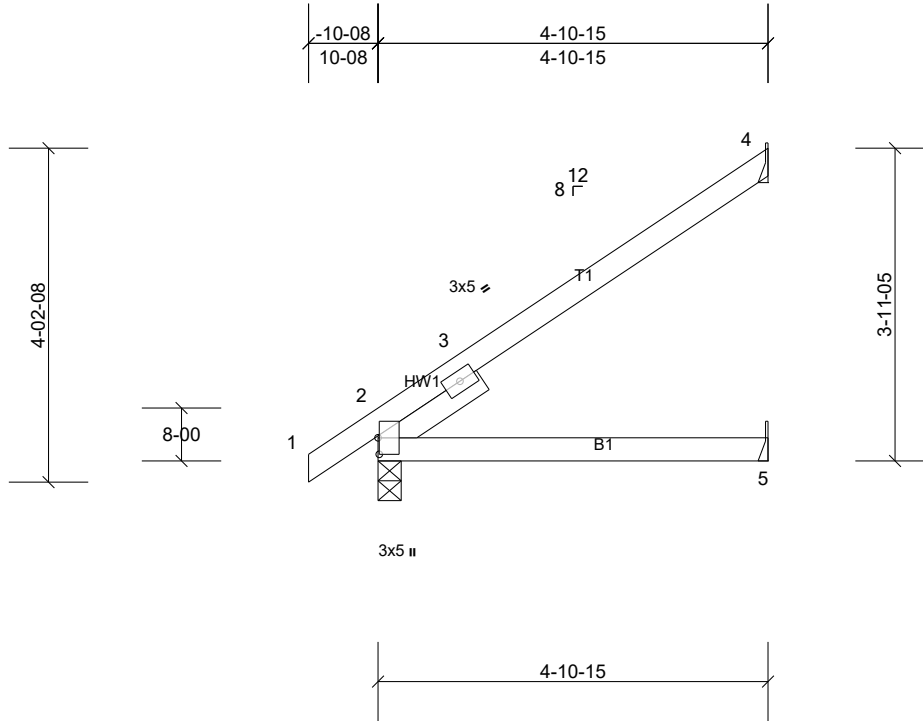
Job 25030187-01	Truss H05	Truss Type Jack-Open	Qty 4	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:29.1

Plate Offsets (X, Y): [2:2-08,0-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.56	Vert(LL)	-0.05	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.44	Vert(CT)	-0.09	5-8	>677	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.02	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 20 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-10-15 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=251/3-08, (min. 1-08), 4=125/ Mechanical, (min. 1-08), 5=64/ Mechanical, (min. 1-08)
 Max Horiz 2=137 (LC 14)
 Max Uplift 2=-1 (LC 14), 4=-81 (LC 14)
 Max Grav 2=351 (LC 21), 4=207 (LC 21), 5=90 (LC 7)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-3=-285/134

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 81 lb uplift at joint 4.
- 9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

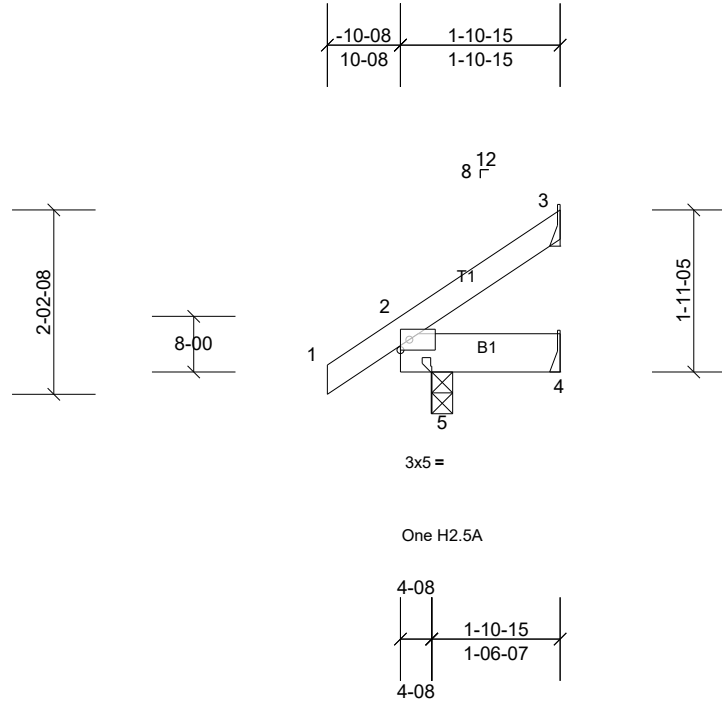
Job 25030187-01	Truss J01	Truss Type Jack-Open	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:27.7

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.07	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							Weight: 10 lb	FT = 20%
BCDL	10.0											

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x6 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 1-10-15 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=34/ Mechanical, (min. 1-08), 4=-21/ Mechanical, (min. 1-08),
5=191/3-00, (min. 1-08)
Max Horiz 5=65 (LC 14)
Max Uplift 3=-28 (LC 14), 4=-38 (LC 21), 5=-15 (LC 14)
Max Grav 3=52 (LC 21), 4=13 (LC 7), 5=275 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 4 and 28 lb uplift at joint 3.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

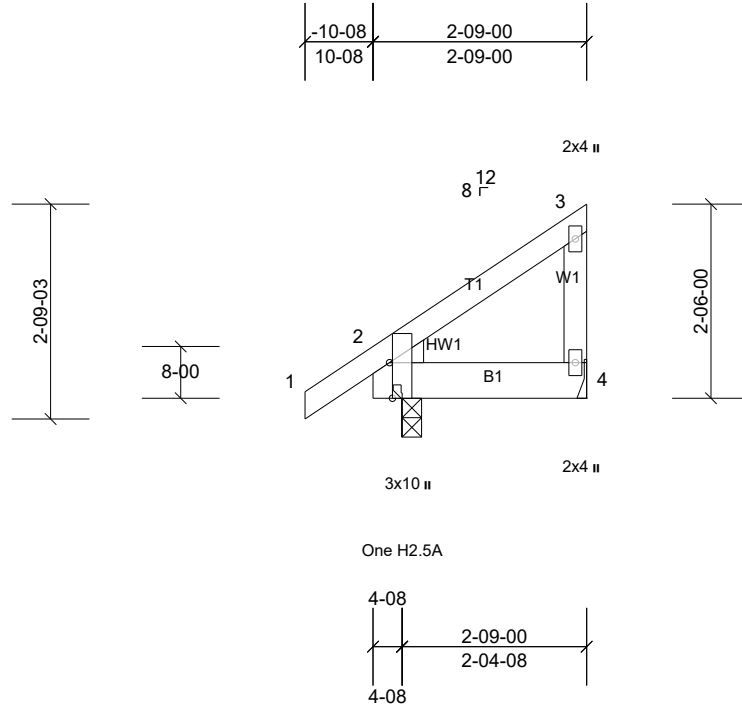
Job 25030187-01	Truss K01	Truss Type Jack-Closed	Qty 7	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:06

Page: 1

ID: oJ97anRXFjMdvmbQ6Wzdw?zXgCl-Zn?sIV1hg2yXXDVP AezGimYWRhd_XXADk54WyRzX25F



Scale = 1:29.8

Plate Offsets (X, Y): [2:5-08,Edge]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	4-9	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	n/a	-	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 16 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x6 SP No.2
 WEBS 2x4 SP No.3
 WEDGE Left: 2x4 SP 2400F 2.0E

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-9-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=193/3-00, (min. 1-08), 4=68/ Mechanical, (min. 1-08)
 Max Horiz 2=78 (LC 13)
 Max Uplift 2=-26 (LC 14), 4=-46 (LC 11)
 Max Grav 2=287 (LC 21), 4=94 (LC 21)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 4.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

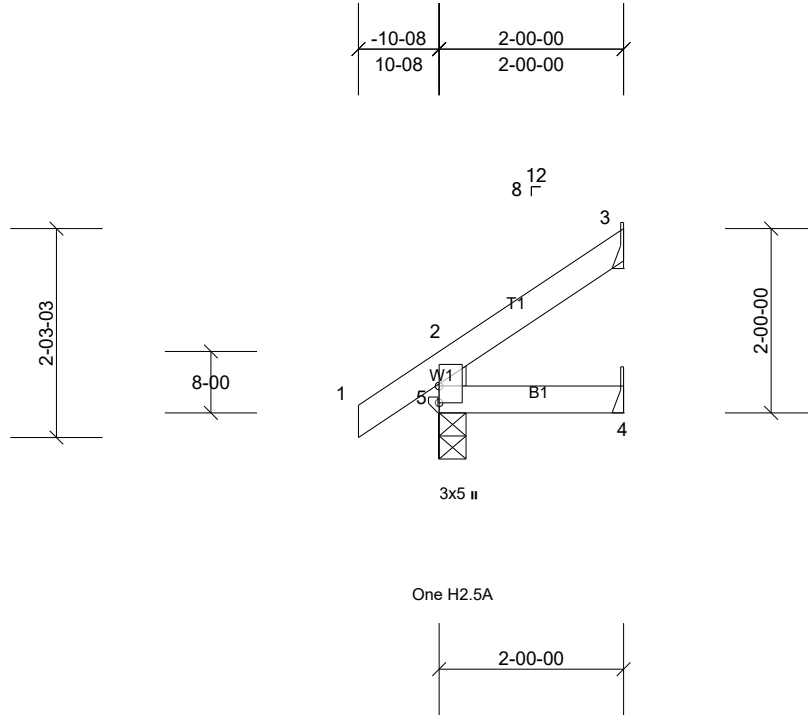
Job 25030187-01	Truss L01	Truss Type Jack-Open	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Wed Mar 26 15:02:07

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Scale = 1:25.1

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.04	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							Weight: 9 lb	FT = 20%
BCDL	10.0											

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 3=41/ Mechanical, (min. 1-08), 4=16/ Mechanical, (min. 1-08), 5=152/3-08, (min. 1-08)
Max Horiz 5=61 (LC 14)
Max Uplift 3=-35 (LC 14), 5=-11 (LC 14)
Max Grav 3=62 (LC 21), 4=33 (LC 7), 5=221 (LC 21)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 3.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 5. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

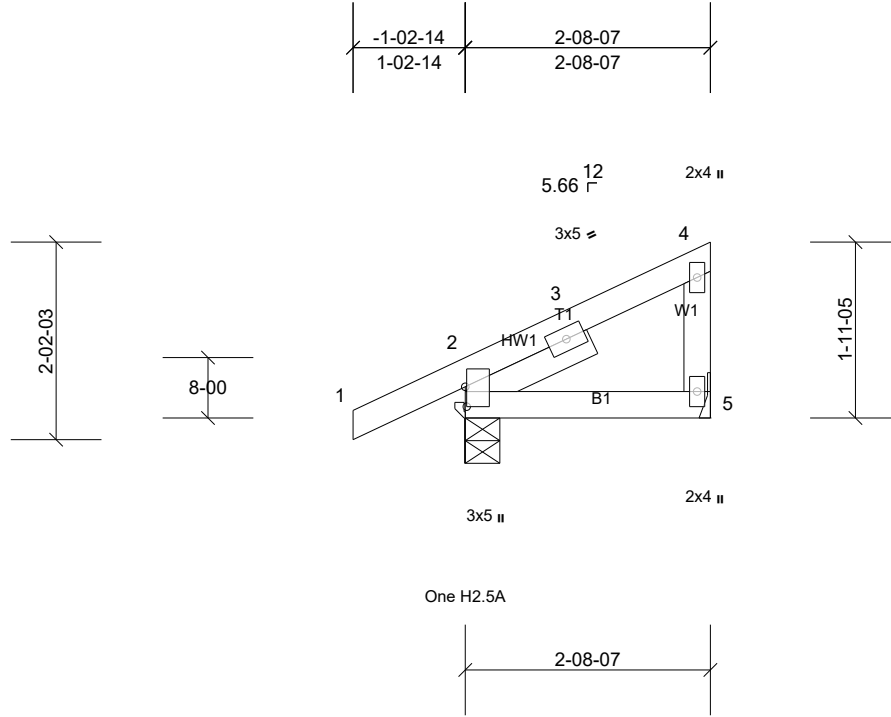
Job 25030187-01	Truss M01	Truss Type Diagonal Hip Girder	Qty 2	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:25.5

Plate Offsets (X, Y): [2:2-10,0-03]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	0.00	5-8	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.06	Vert(CT)	0.00	5-8	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.00	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 15 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD Structural wood sheathing directly applied or 2-8-7 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=195/4-09, (min. 1-08), 5=84/ Mechanical, (min. 1-08)
 Max Horiz 2=65 (LC 11)
 Max Uplift 2=-37 (LC 12), 5=-19 (LC 12)
 Max Grav 2=280 (LC 19), 5=114 (LC 19)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 5.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

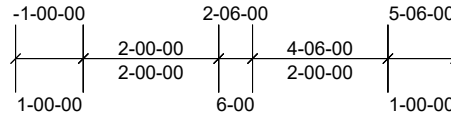
Job 25030187-01	Truss N01	Truss Type Hip Girder	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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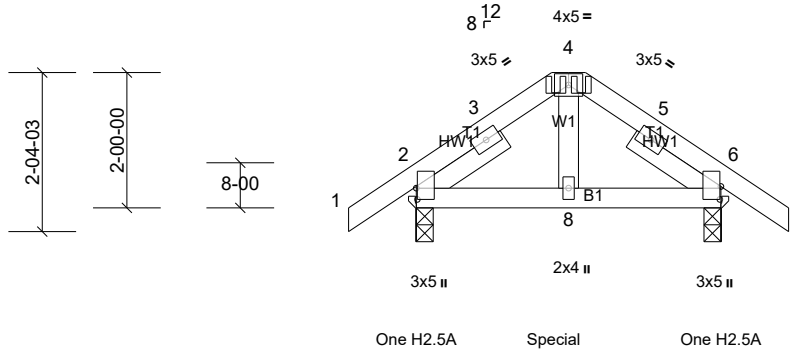
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NAILED

NAILED



Scale = 1:34.2

Plate Offsets (X, Y): [2:2-00,0-03], [6:2-05,0-03]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.12	Vert(LL)	0.00	8	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.10	Vert(CT)	0.00	8	>999	180	
TCDL	10.0	Rep Stress Incr	NO	WB	0.07	Horz(CT)	0.00	2	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							Weight: 25 lb FT = 20%
BCDL	10.0										

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 SLIDER Left 2x4 SP No.3 -- 1-06-00, Right 2x4 SP No.3 -- 1-06-00

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 4-6-0 oc purlins.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=333/3-00, (min. 1-08), 6=333/3-00, (min. 1-08)

Max Horiz 2=-49 (LC 10)
 Max Uplift 2=-86 (LC 12), 6=-86 (LC 13)
 Max Grav 2=411 (LC 19), 6=411 (LC 20)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-272/100, 4-5=-272/100

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed ; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 6. This connection is for uplift only and does not consider lateral forces.
- "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidelines.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 85 lb down and 36 lb up at 2-0-0, and 85 lb down and 36 lb up at 2-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (lb/ft)
 Vert: 1-4=-60, 4-7=-60, 9-13=-20
 Concentrated Loads (lb)
 Vert: 8=-171, 4=-14

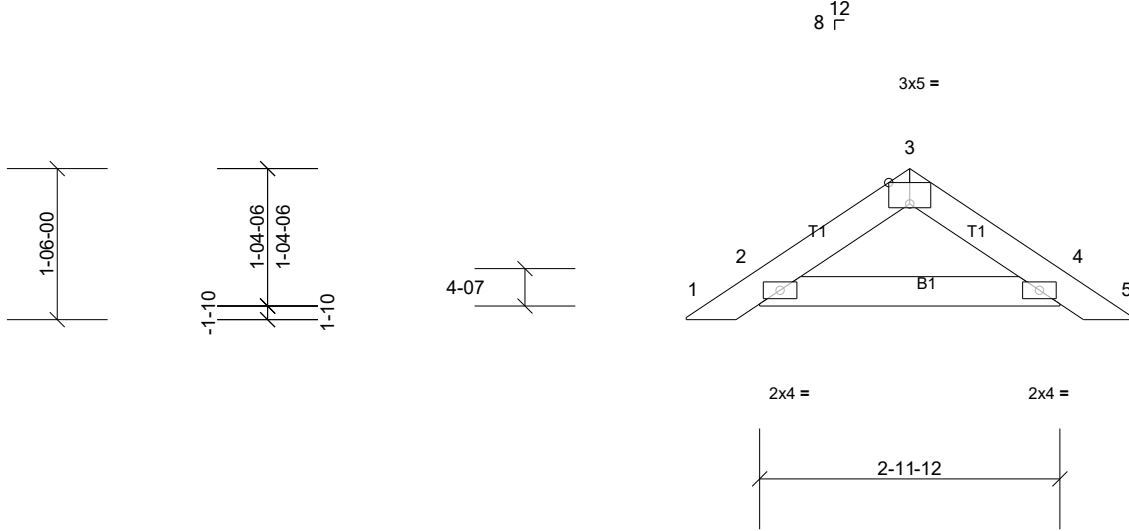
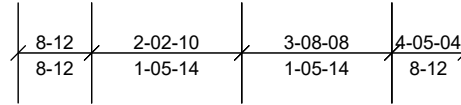
Job 25030187-01	Truss PB04	Truss Type Piggyback	Qty 3	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:23

Plate Offsets (X, Y): [3:2-08,Edge]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.05	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	n/a	-	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 12 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 4-6-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS All bearings 2-11-12.

(lb) - Max Horiz 2=31 (LC 13), 6=31 (LC 13)
Max Uplift All uplift 100 (lb) or less at joint(s) 2, 4, 6, 10
Max Grav All reactions 250 (lb) or less at joint(s) 2, 4, 6, 10

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 4. This connection is for uplift only and does not consider lateral forces.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard

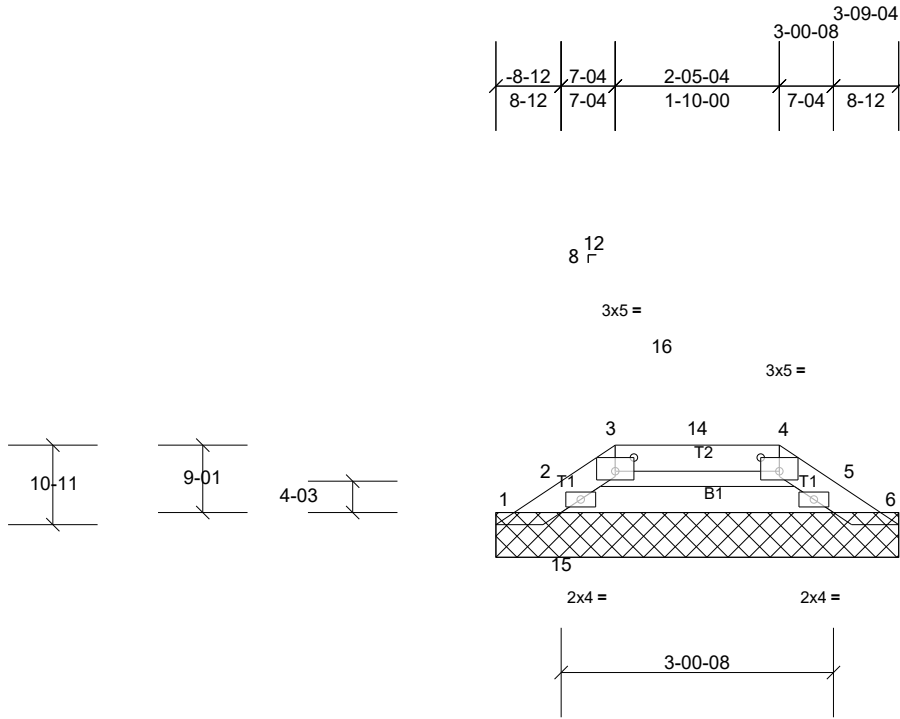
Job 25030187-01	Truss PB04A	Truss Type Piggyback	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:25.9

Plate Offsets (X, Y): [3:2-08,1-13], [4:2-08,1-13]

Loading	(psf)	Spacing	1-11-04	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.13	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.10	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 12 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2

BRACING

TOP CHORD Structural wood sheathing directly applied or 4-0-6 oc purlins, except 2-0-0 oc purlins: 3-4.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS All bearings 4-06-00.

(lb) - Max Horiz 1=-17 (LC 12)
Max Uplift All uplift 100 (lb) or less at joint(s) 1, 2, 6, 7
Max Grav All reactions 250 (lb) or less at joint(s) 1, 5, 6, 13 except 2=281 (LC 21), 7=281 (LC 21)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-3-2 to 1-4-0, Exterior(2R) 1-4-0 to 3-2-0, Exterior(2E) 3-2-0 to 4-2-14 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Provide adequate drainage to prevent water ponding.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral forces.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

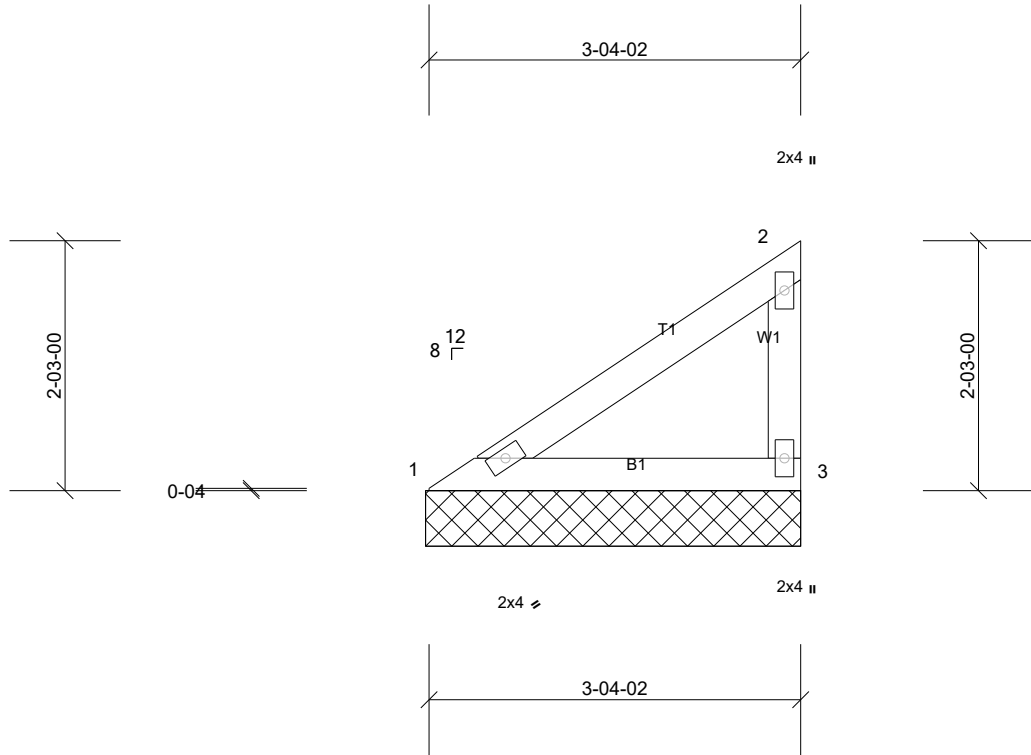
Job 25030187-01	Truss VL01	Truss Type Valley	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:20.8

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.22	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP							Weight: 13 lb	FT = 20%
BCDL	10.0											

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 3-4-8 oc purlins, except end verticals.
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=129/3-04-08, (min. 1-08), 3=129/3-04-08, (min. 1-08)
 Max Horiz 1=69 (LC 11)
 Max Uplift 1=-9 (LC 14), 3=-32 (LC 14)
 Max Grav 1=183 (LC 20), 3=183 (LC 20)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 9 lb uplift at joint 1 and 32 lb uplift at joint 3.
- 8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard

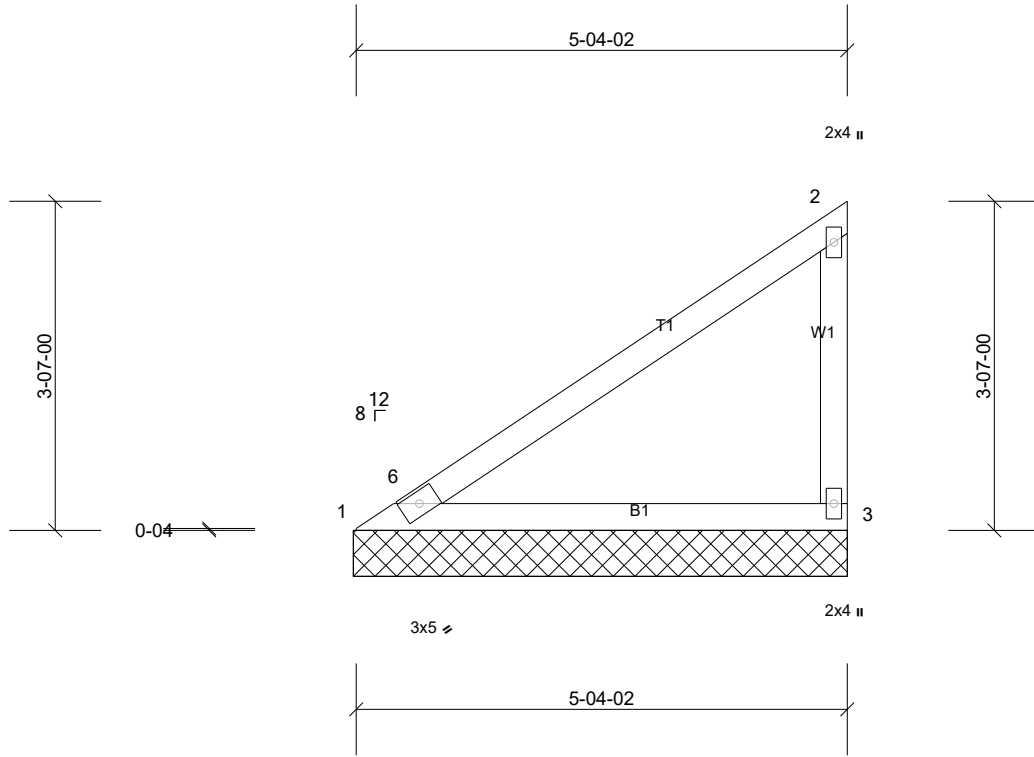
Job 25030187-01	Truss VL02	Truss Type Valley	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:25.2

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.61	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.01	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 21 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD

Structural wood sheathing directly applied or 5-4-2 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=209/5-04-08, (min. 1-08), 3=209/5-04-08, (min. 1-08)

Max Horiz 1=116 (LC 11)
 Max Uplift 1=-13 (LC 14), 3=-53 (LC 14)
 Max Grav 1=293 (LC 20), 3=315 (LC 20)

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-6=-440/63, 1-6=-420/64
 BOT CHORD 1-3=-94/368

NOTES

- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 53 lb uplift at joint 3 and 13 lb uplift at joint 1.
- 8) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1.

LOAD CASE(S) Standard

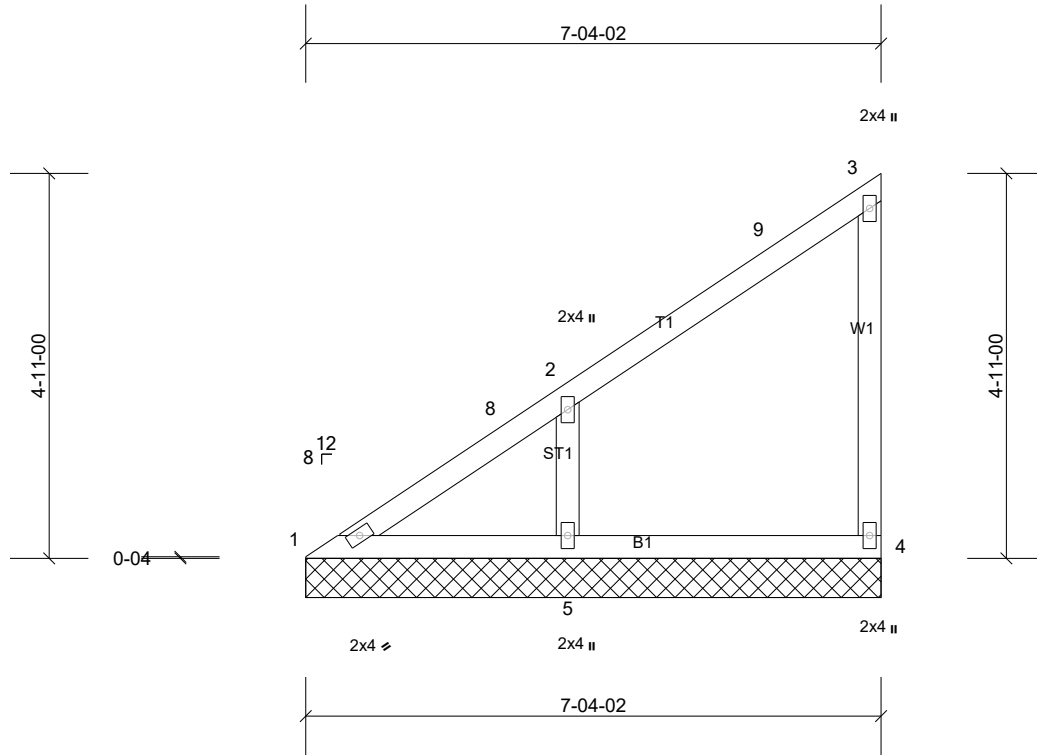
Job 25030187-01	Truss VL03	Truss Type Valley	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:29.5

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.34	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.13	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MP								
BCDL	10.0										Weight: 32 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6'-0" oc purlins, except end verticals.

Rigid ceiling directly applied or 10'-0" oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 1=95/7'-04"-02, (min. 1'-08), 4=120/7'-04"-02, (min. 1'-08), 5=360/7'-04"-02, (min. 1'-08)
Max Horiz 1=164 (LC 11)
Max Uplift 1=-4 (LC 10), 4=-33 (LC 11), 5=-115 (LC 14)
Max Grav 1=121 (LC 25), 4=188 (LC 20), 5=492 (LC 20)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-5=400/220

NOTES

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3'-06"-00 tall by 2'-00"-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 33 lb uplift at joint 4, 4 lb uplift at joint 1 and 115 lb uplift at joint 5.

LOAD CASE(S) Standard

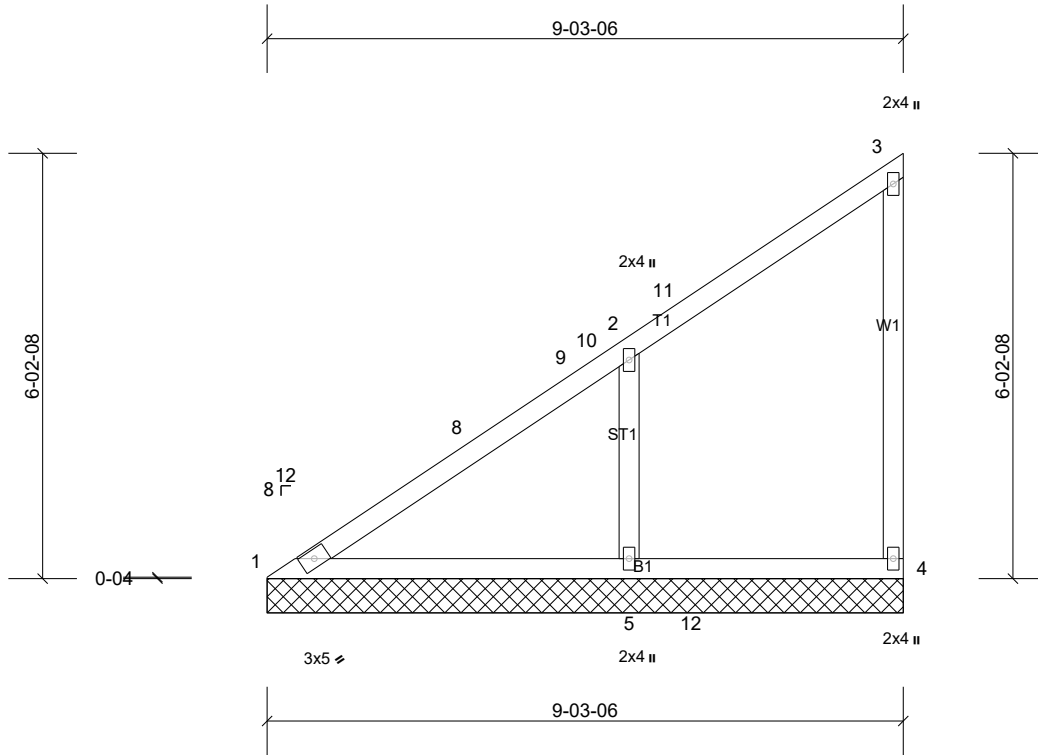
Job 25030187-01	Truss VL04	Truss Type Valley	Qty 1	Ply 1	5076 Old 421-Prime Estate Reno.-Roof-24222 Adrian Job Reference (optional)
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Scale = 1:33.8

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.26	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.11	Horiz(TL)	0.01	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 42 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6'-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10'-0-0 oc bracing.

REACTIONS (lb/size) 1=165/9-03-06, (min. 1-08), 4=95/9-03-06, (min. 1-08),
5=448/9-03-06, (min. 1-08)
Max Horiz 1=204 (LC 11)
Max Uplift 4=-37 (LC 11), 5=-130 (LC 14)
Max Grav 1=203 (LC 25), 4=179 (LC 5), 5=579 (LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-8=-302/190
BOT CHORD 1-5=-72/302
WEBS 2-5=-427/202

- NOTES**
- 1) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) 0-0-6 to 3-0-6, Interior (1) 3-0-6 to 4-11-1, Exterior(2R) 4-11-1 to 9-2-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - 3) Unbalanced snow loads have been considered for this design.
 - 4) Gable requires continuous bottom chord bearing.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 37 lb uplift at joint 4 and 130 lb uplift at joint 5.

LOAD CASE(S) Standard