# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

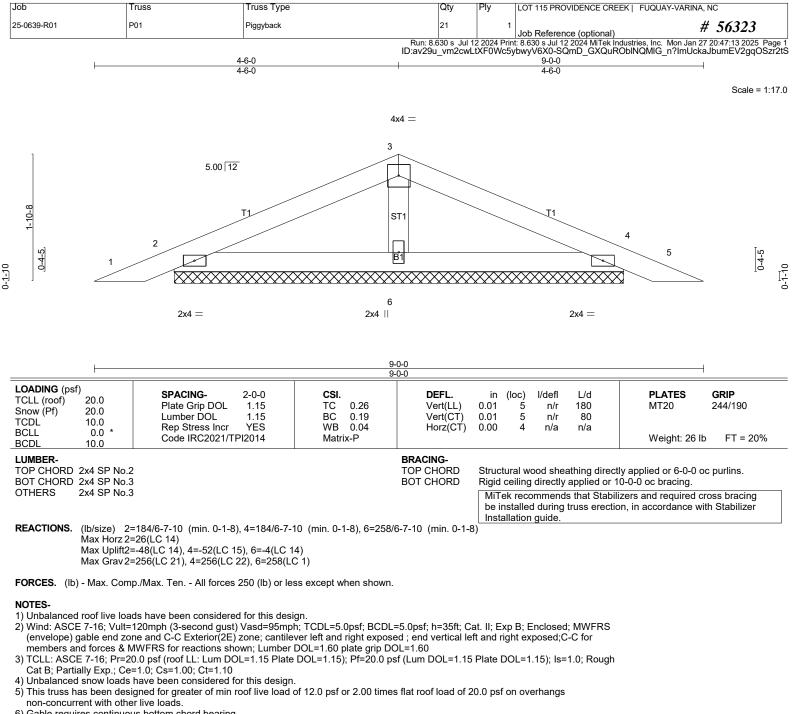
AST #: 56323 JOB: 25-0639-R01 JOB NAME: LOT 115 PROVIDENCE CREEK Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 35 These truss designs comply with IRC 2018 as well as IRC 2021. 19 Truss Design(s)

Trusses:

P01, R01, R02, R02A, R03, R04, R05, R06, R07, R09, R10, R11, R12, R13, V01, V02, V03,



# Warning !--- Verify design parameters and read notes before use.



- Gable requires continuous bottom chord bearing.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

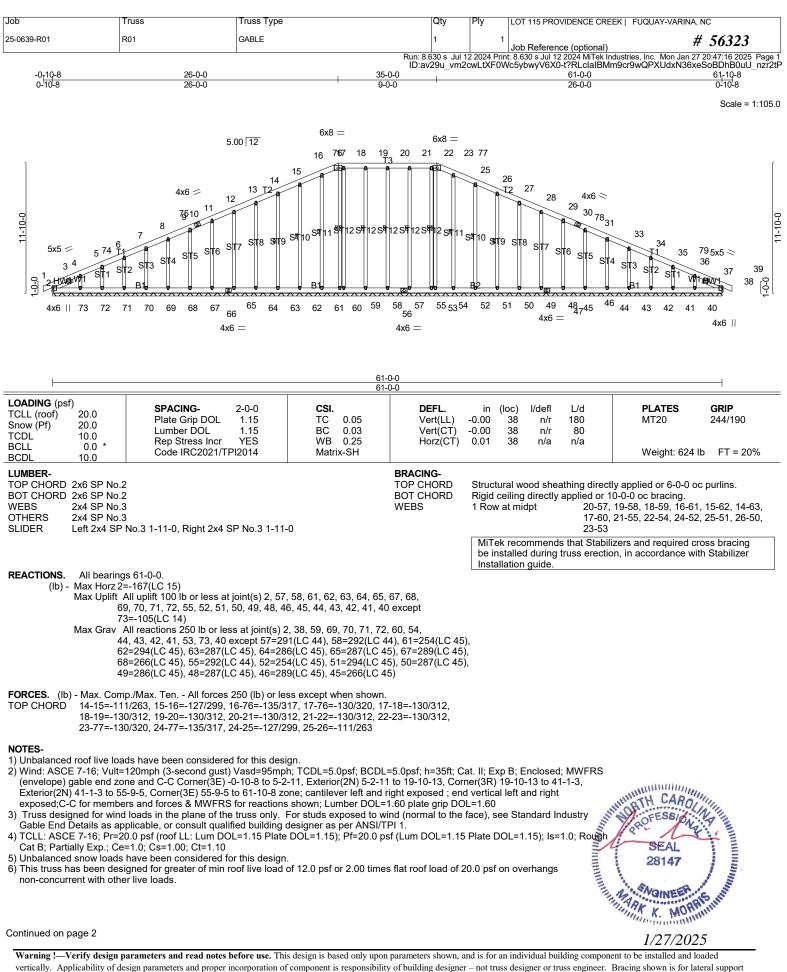
8)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 48 lb uplift at joint 2, 52 lb uplift at joint 4 and 4 lb uplift at joint 6.

10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building desianer.

# LOAD CASE(S) Standard





#### Continued on page 2

Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

1/27/2025

Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUA	AY-VARINA, NC
25-0639-R01	R01	GABLE	1	1	Job Reference (optional)	# 56323
					t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wc5ybwyV6X0-LB?jqebxygu0E?k7_6	

7) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing. 8) Provide adequate drainage to prevent water ponding.

9) All plates are 2x4 MT20 unless otherwise indicated.

10) Gable requires continuous bottom chord bearing.

11) Gable studs spaced at 2-0-0 oc.

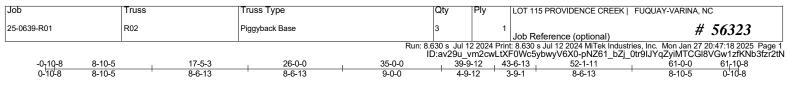
12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

13) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

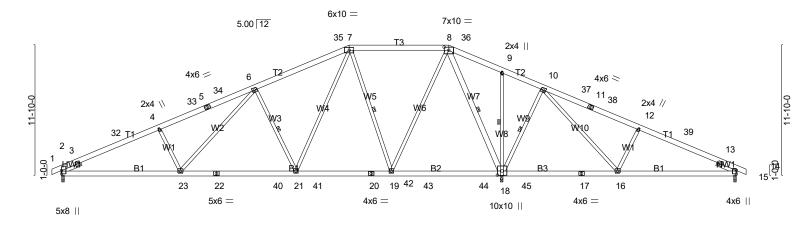
14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 57, 58, 61, 62, 63, 64, 65, 67, 68, 69, 70, 71, 72, 55, 52, 51, 50, 49, 48, 46, 45, 44, 43, 42, 41, 40 except (jt=lb) 73=105.

LOAD CASE(S) Standard





Scale = 1:104.1



		29-9-12	39-9-12	50-3-2	61-0-0
		8-7-8	10-0-0	10-5-6	10-8-14
· · / ·	0-5-0,0-3-7], [18:0-4-8,0-5-0]		Τ		
OADING (psf)           CLL (roof)         20.0           snow (Pf)         20.0           CDL         10.0           GCLL         0.0 *           GCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.84 BC 0.83 WB 0.93 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d -0.31 21-23 >999 240 -0.47 21-23 >999 180 0.05 18 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 466 lb         FT = 20%
UMBER- OP CHORD 2x6 SP N OT CHORD 2x6 SP N VEBS 2x4 SP N W7: 2x6	o.2 o.3 *Except*	0	BRACING- TOP CHORD BOT CHORD WEBS	MiTek recommends that Stabi	
Max Hór Max Upl	2=1412/0-3-8 (min. 0-1-14), 14=461/0- 2=-167(LC 15) f2=-222(LC 14), 14=-145(LC 15), 18=-2: /2=1577(LC 39), 14=583(LC 55), 18=41	32(LC 11)	112/0-3-8 (min. 0-		
OP CHORD 2-3=-97 5-34=-2 8-36=0 11-38= OT CHORD 2-23=-4 20-41= 18-44= VEBS 4-23=-4	omp./Max. Ten All forces 250 (lb) or le: 5/1, 3-32=-2865/374, 4-32=-2774/390, 4 613/395, 6-34=-2510/412, 6-35=-1782/3 1500, 9-36=0/1429, 9-10=-26/1462, 10-3 286/363, 12-38=-388/332, 12-39=-432/3 33/2561, 22-23=-250/1951, 22-40=-250/ 26/1001, 20-42=-26/1001, 19-42=-26/10 256/220, 18-45=-905/158, 17-45=-905/1 21/229, 6-23=-131/727, 6-21=-1190/324 38/1835, 8-18=-2747/243, 10-18=-1061/ 47/97	-33=-2709/391, 5-33=-2 41, 7-35=-1533/343, 7- 37=-256/493, 11-37=-28 15, 13-39=-582/270, 13 1951, 21-40=-250/1951 01, 19-43=-256/220, 43 58, 16-17=-905/158, 14 , 7-21=-217/1520, 7-19	2628/394, 8=-646/242, 10/374, 1-14=-402/2 , 21-41=-26/1001 1-44=-256/220, 1-16=-249/483 =-1292/235,		
) Wind: ASCE 7-16; Vu (envelope) gable end 43-6-6 to 55-9-5 Exte	oads have been considered for this desi It=120mph (3-second gust) Vasd=95mp zone and C-C Exterior(2E) -0-10-8 to 5- rior(2E) 55-9-5 to 61-10-8 zone; cantilev & MWFRS for reactions shown; Lumber =20.0 psf (roof LL: Lum DOL=1.15 Plate Ce=1.0; Cs=1.00; Ct=1.10 ds have been considered for this design esigned for greater of min roof live load ther live loads. a span truss requires extreme care and e s, see Guide to Good Practice for Handli 3CA and TPI. The building owner or the sign and inspection of the temporary insi ek assumes no responsibility for truss m inage to prevent water ponding.	h; TCDL=5.0psf; BCDL 2-11, Interior(1) 5-2-11 per left and right expose	to 17-4-8, Exterior	(2R) 17-4-8 to 43-6-6, Interior(1)	SEAL 28147

Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-VARINA,	NC		
25-0639-R01	R02	Piggyback Base	3	1	Job Reference (optional)	# 56323		
Run: 8.630 s_Jul 12 2024 Print: 8.630 s_Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:18 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-pNZ61_bZj_0tr9IJYqZyiMTCGl8VGw1zfKNb3fzr2tN								

8) All plates are 5x5 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

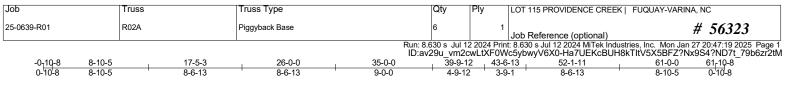
11) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

12) Provide metal plate or equivalent at bearing(s) 18 to support reaction shown.

13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=222, 14=145, 18=232.

LOAD CASE(S) Standard

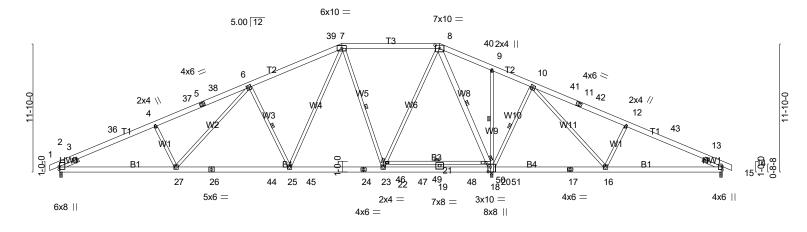




Scale = 1:106.0

TO MARDINI MORA

1/27/2025



	<u>  10-8-14</u>   10-8-14		29-9-12 8-7-8	<u>35-0-0</u> <u>39-9-</u> 5-2-4 <u>4-9-</u>		50-3-2 10-5-6	<u>61-0-0</u> 10-8-14				
Plate Offs		5-0,0-3-7], [18:0-5-4,0-2-8]	0-7-0	5-2-4 4-9-	12	10-5-6	10-0-14				
LOADING TCLL (roc Snow (Pf) TCDL BCLL BCDL	of) 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.85 BC 0.87 WB 0.93 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	-0.33 25-27	l/defl L/d >999 240 >957 180 n/a n/a	PLATES MT20 Weight: 480 II	<b>GRIP</b> 244/190 p FT = 20%			
LUMBER-	DRD 2x6 SP No.2 DRD 2x6 SP No.2 B3: 2x4 SP 2x4 SP No.3 W8: 2x6 SP	2 *Except* No.2, B2: 2x6 SP No.1 3 *Except*	-0	BRACING- TOP CHORD BOT CHORD WEBS	Rigid ceiling o 6-0-0 oc brac 1 Row at mid MiTek recor	directly applied or 6 ing: 20-22 pt 6-25, 7 mmends that Stabili during truss erection	ly applied or 2-7-0 oc 5-0-0 oc bracing. Exc 7-23, 8-20, 10-18, 9-1 izers and required cr on, in accordance wit	8 oss bracing			
REACTIO	REACTIONS. (lb/size)       2=1429/0-3-8 (min. 0-1-14), 14=448/0-3-8 (min. 0-1-8), 18=3298/0-3-8 (min. 0-3-2)         Max Horz 2=-167(LC 15)       Max Uplift2=-212(LC 14), 14=-148(LC 15), 18=-144(LC 11)         Max Grav 2=1593(LC 39), 14=572(LC 55), 18=4541(LC 45)										
TOP CHC	FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown.         TOP CHORD       2-3=-984/0, 3-36=-2959/350, 4-36=-2867/366, 4-37=-2803/367, 5-37=-2723/371, 5-38=-2707/372, 6-38=-2604/388, 6-39=-1877/316, 7-39=-1631/318, 7-8=-774/211, 8-40=0/1545, 9-40=0/1475, 9-10=-21/1508, 10-41=-228/556, 11-41=-252/438, 11-42=-258/426, 12-42=-360/394, 12-43=-404/377, 13-43=-554/330, 13-14=-402/0										
BOT CHC	24-45=-1/1 19-48=-24 18-51=-95 4-27=-418 22-23=-11	7/1831, 8-22=-96/2043, 8-20=-2957/2	23-47=-24/423, 19-47= 1-49=-326/9, 21-50=-3 53, 14-16=-304/457 3, 7-25=-224/1483, 7-23 200, 18-20=-3174/181,	24/423, 26/9, 20-50=-326/9 3=-1264/251,	9,						
2) Wind: A (envelo 43-6-6 membe 3) TCLL: A Cat B; I 4) Unbala 5) This tru	22-23=-117/1831, 8-22=-96/2043, 8-20=-2957/200, 18-20=-3174/181, 10-18=-1056/270, 10-16=-162/1061, 12-16=-602/250, 19-21=-323/0, 9-18=-348/96										

#### Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-VARINA, NC			
25-0639-R01	R02A	Piggyback Base	6	1	Job Reference (optional) # 56323			
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:19 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-Ha7UEKcBUH8kTltV5X5BFZ?Nx9S4?ND7t_79b6zr2tM								

6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.

7) Provide adequate drainage to prevent water ponding.

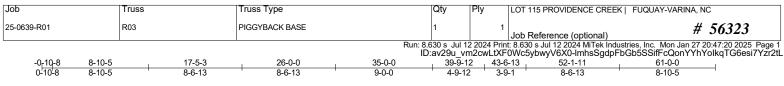
8) All plates are 5x5 MT20 unless otherwise indicated.

9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

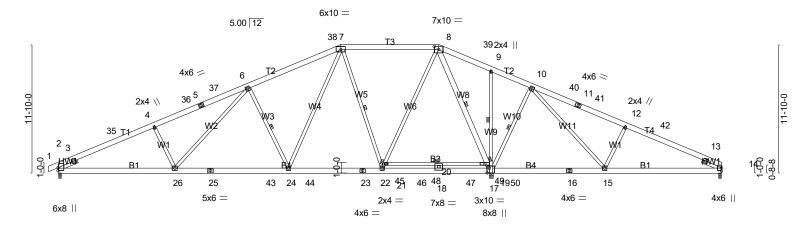
- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 18 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide metal plate or equivalent at bearing(s) 18 to support reaction shown.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=212, 14=148, 18=144.

LOAD CASE(S) Standard



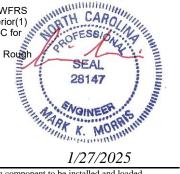


Scale = 1:105.9



F	<u>10-8-14</u> 10-8-14		<u>21-2-4</u> 10-5-6	29-9-12 8-7-8	<u>35-0-0</u> <u>39-9</u> <u>5-2-4</u> <u>4-9-</u>		50-3-2 10-5-6		<u>61-0-0</u> 10-8-14	
Plate Offsets (X	(,Y) [8:0-5-	0,0-3-7], [17:0-5-4	,0-2-8]							
Snow (Pf) ´ TCDL BCLL	20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip D Lumber DOL Rep Stress I Code IRC20	1.15 ncr YES	<b>CSI.</b> TC 0.85 BC 0.87 WB 0.93 Matrix-MSH	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.33 24-26 -0.50 24-26 0.06 17	l/defl >999 >956 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 478	<b>GRIP</b> 244/190 Ib FT = 20%
WEBS 2 V	2x6 SP No.2 33: 2x4 SP N 2x4 SP No.3 V8: 2x6 SP [	lo.2, B2: 2x6 SP N *Except* DSS			BRACING- TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly applied or 2-7-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. Except: 6-0-0 oc bracing: 19-21 1 Row at midpt 6-24, 7-22, 8-19, 10-17, 9-17 MiTek recommends that Stabilizers and required cross bracing				
SLIDER L	.eft 2x4 SP N	lo.3 1-11-0, Right	2x4 SP No.3 1-1	1-0		be installe	d during t		on, in accordance w	
Installation guide.         Installation guide.         REACTIONS. (lb/size) 2=1430/0-3-8 (min. 0-1-14), 14=397/0-3-8 (min. 0-1-8), 17=3295/0-3-8 (min. 0-3-2)         Max Horz 2=172(LC 14)         Max Uplift2=-211(LC 14), 14=-129(LC 15), 17=-147(LC 11)         Max Grav 2=1594(LC 39), 14=520(LC 55), 17=4539(LC 45)         FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.										
TOP CHORD	2-3=-985/0, 5-37=-2709 8-39=0/154 11-41=-267	3-35=-2961/349, 4 /370, 6-37=-2606/3 0, 9-39=0/1470, 9- /418, 12-41=-369/3	4-35=-2869/365, 387, 6-38=-1879 10=-24/1504, 1( 391, 12-42=-413	4-36=-2806/366, 5-36 //314, 7-38=-1629/316, )-40=-237/547, 11-40=- //374, 13-42=-563/326,	=-2725/369, 7-8=-773/208, 261/430, 13-14=-411/0					
	23-44=-5/11 18-47=-28/4 17-50=-948	102, 23-45=-5/110 118, 17-47=-28/41 /143, 16-50=-948/	2, 22 <sup>-</sup> 45=-5/1102 3, 21-48=-326/9, 143, 15-16=-948	1/2042, 24-43=-231/20 2, 22-46=-28/418, 18-44 20-48=-326/9, 20-49=- /143, 14-15=-301/466	6=-28/418, -326/9, 19-49=-326/					
	WEBS 4-26=-418/230, 6-26=-132/726, 6-24=-1190/323, 7-24=-224/1483, 7-22=-1262/252, 21-22=-118/1830, 8-21=-97/2041, 8-19=-2954/203, 17-19=-3171/184, 10-17=-1057/270, 10-15=-163/1064, 12-15=-604/251, 18-20=-323/0, 9-17=-348/97									
NOTES- 1) Unbalanced r 2) Wind: ASCE (envelope) ga 43-6-6 to 54- members and 3) TCLL: ASCE	roof live load 7-16; Vult=1 able end zon 10-13, Exteri d forces & M' 7-16; Pr=20	is have been consi 20mph (3-second e and C-C Exterio ior(2E) 54-10-13 to WFRS for reaction .0 psf (roof LL: Lur	dered for this de gust) Vasd=95n r(2E) -0-10-8 to o 61-0-0 zone; ca s shown; Lumbe n DOL=1.15 Pla	esign. 1ph; TCDL=5.0psf; BCI 5-2-11, Interior(1) 5-2-1 antilever left and right e er DOL=1.60 plate grip tte DOL=1.15); Pf=20.0	DL=5.0psf; h=35ft; C 1 to 17-4-8, Exterio xposed ; end vertica DOL=1.60 psf (Lum DOL=1.1	Cat. II; Exp B; E r(2R) 17-4-8 to al left and right 5 Plate DOL=1	Enclosed; 5 43-6-6, l t exposed 1.15); ls= <sup>2</sup>	MWFRS nterior(1) ;C-C for	TH CARO	

- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
  5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



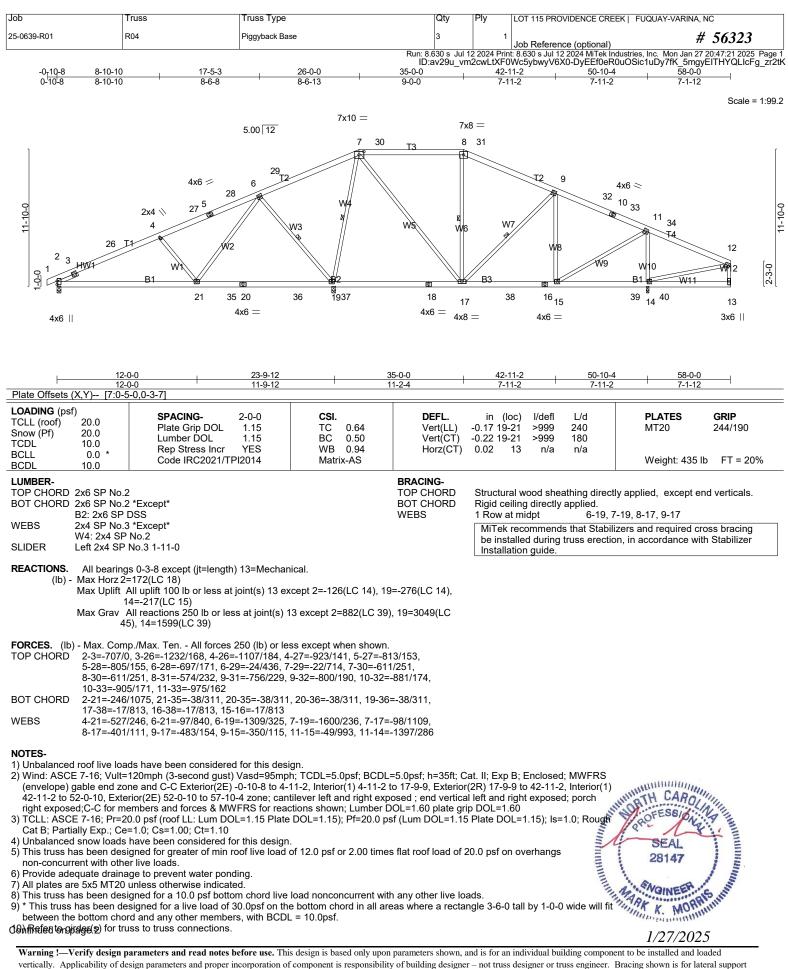
#### Continued on page 2

Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-	VARINA, NC		
25-0639-R01	R03	PIGGYBACK BASE	1	1	Job Reference (optional)	# 56323		
Run: 8.630 s. Jul 12 2024 Print: 8.630 s. Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:20 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-ImhsSgdpFbGb5SSifFcQonYYhYoIkqTG6esi7Yzr2tL								

- 6) WARNING: This long span truss requires extreme care and experience for proper and safe handling and erection. For general handling and erection guidance, see Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses ("BCSI"), jointly produced by SBCA and TPI. The building owner or the owner's authorized agent shall contract with a qualified registered design professional for the design and inspection of the temporary installation restraint/bracing and the permanent individual truss member restraint/bracing. MiTek assumes no responsibility for truss manufacture, handling, erection, or bracing.
- 7) Provide adequate drainage to prevent water ponding.
- 8) All plates are 5x5 MT20 unless otherwise indicated.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 11) Bearing at joint(s) 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 12) Provide metal plate or equivalent at bearing(s) 17 to support reaction shown.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=211, 14=129, 17=147.

LOAD CASE(S) Standard





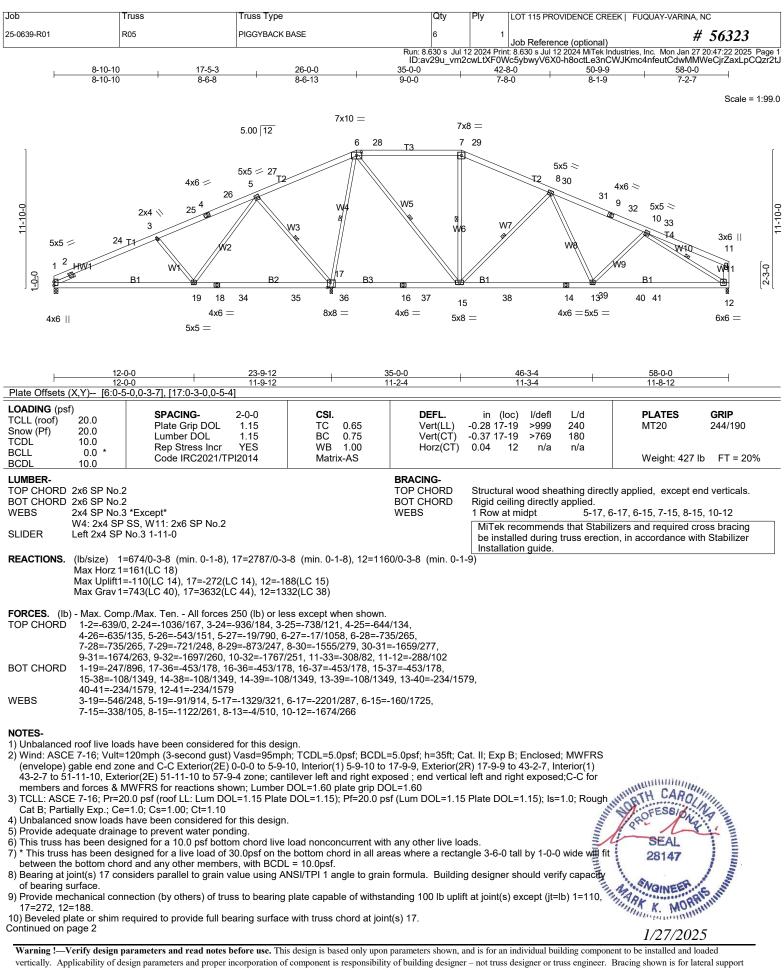
Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK	FUQUAY-VARINA, NC		
25-0639-R01	R04	Piggyback Base	3	1	Job Reference (optional)	# 56323		
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:21 2025 Page 2 ID:av29u vm2cwLtXF0Wc5vbwvV6X0-DvEEf0eR0uOSic1uDv7fK 5mgvEITHYQLIcFg zr2tK								

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 2=126, 19=276, 14=217.

12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard



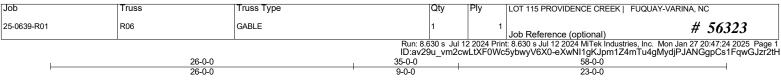


Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-VARIN	IA, NC		
25-0639-R01	R05	PIGGYBACK BASE	6	1	Job Reference (optional)	# 56323		
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:22 2025 Page 2 ID:av29u vm2cwLtXF0Wc5ybwyV6X0-h8octLe3nCWJKmc4nfeutCdwMMWeCjrZaxLpCQzr2tJ								

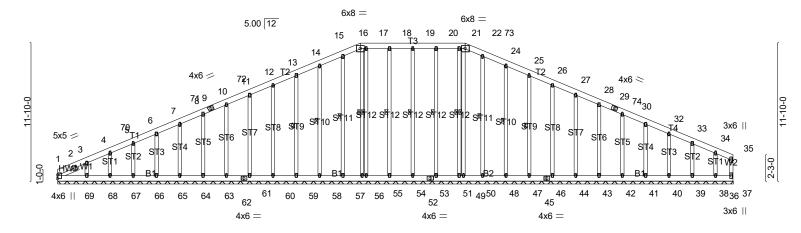
NOTES-11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard









			-0-0 -0-0		
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.10 BC 0.04 WB 0.25 Matrix-SH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d n/a - n/a 999 n/a - n/a 999 0.00 36 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 603 lb         FT = 20%
LUMBER- TOP CHORD 2x6 SP No. BOT CHORD 2x6 SP No. WEBS 2x4 SP No. OTHERS 2x4 SP No. SLIDER Left 2x4 SP	2 3		BRACING- TOP CHORD BOT CHORD WEBS	end verticals. Rigid ceiling directly applied o 1 Row at midpt 19-5 16-5 22-4 MiTek recommends that Stal	3, 18-54, 17-55, 15-57, 14-58, 13-59, 6, 20-51, 21-50, 23-48, 24-47, 25-46,
FORCES. (lb) - Max. Con TOP CHORD 10-72=-68 14-15=-12 19-20=-11		2, 41, 40, 39, 38, 37 exc , 55, 65, 66, 67, 68, 56, C 43), 54=292(LC 43), 5 6(LC 44), 61=287(LC 44) 5(LC 44), 47=294(LC 44) 9(LC 44), 41=266(LC 44) ss except when shown. 12-13=-97/335, 13-14=- 11, 17-18=-131/410, 18 11, 22-73=-130/423, 23	xept 69=-103(LC 50, 40, 57=255(LC 44), 4), 63=289(LC 44), 4), 46=287(LC 44), 4) -112/369, -19=-131/410, -73=-135/422,		
5) Unbalanced snow loads	ads have been considered for this des =120mph (3-second gust) Vasd=95mp one and C-C Corner(3E) 0-0-0 to 5-9-1 er(3E) 52-0-10 to 57-10-4 zone; cantilu MWFRS for reactions shown; Lumber d loads in the plane of the truss only. oplicable, or consult qualified building 20.0 psf (roof LL: Lum DOL=1.15 Plate =1.0; Cs=1.00; Ct=1.10 c have been considered for this design age to prevent water ponding.		=5.0psf; h=35ft; C o 20-2-6, Corner(3 ed ; end vertical le 0L=1.60 nd (normal to the Pl 1. f (Lum DOL=1.15	at. II; Exp B; Enclosed; MWFRS 3R) 20-2-6 to 40-6-0, Exterior(2N eft and right exposed;C-C for face), see Standard Industry 5 Plate DOL=1.15); Is=1.0; Roug	South SEAL 28147 1/27/2025

•	Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-VAI	RINA, NC		
2	25-0639-R01	R06	GABLE	1	1	Job Reference (optional)	# 56323		
_	Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:25 2025 Page 2 ID:av29u vm2cwLtXF0Wc5ybwyV6X0-6jUIVNhy47vuBDLfSoBbUgFa2ZiVPGS?GvaTpIzr2tG								

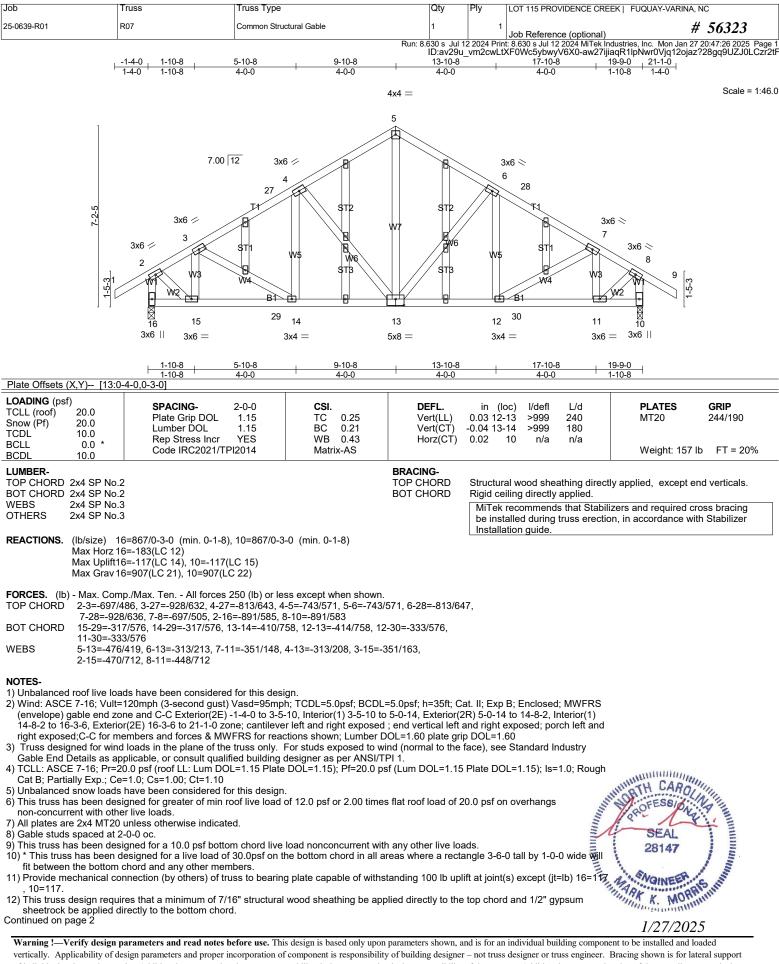
10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

11) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 53, 54, 57, 58, 59, 60, 61, 63, 64, 65, 66, 67, 68, 51, 48, 47, 46, 44, 43, 42, 41, 40, 39, 38, 37 except (jt=lb) 69=103.

LOAD CASE(S) Standard



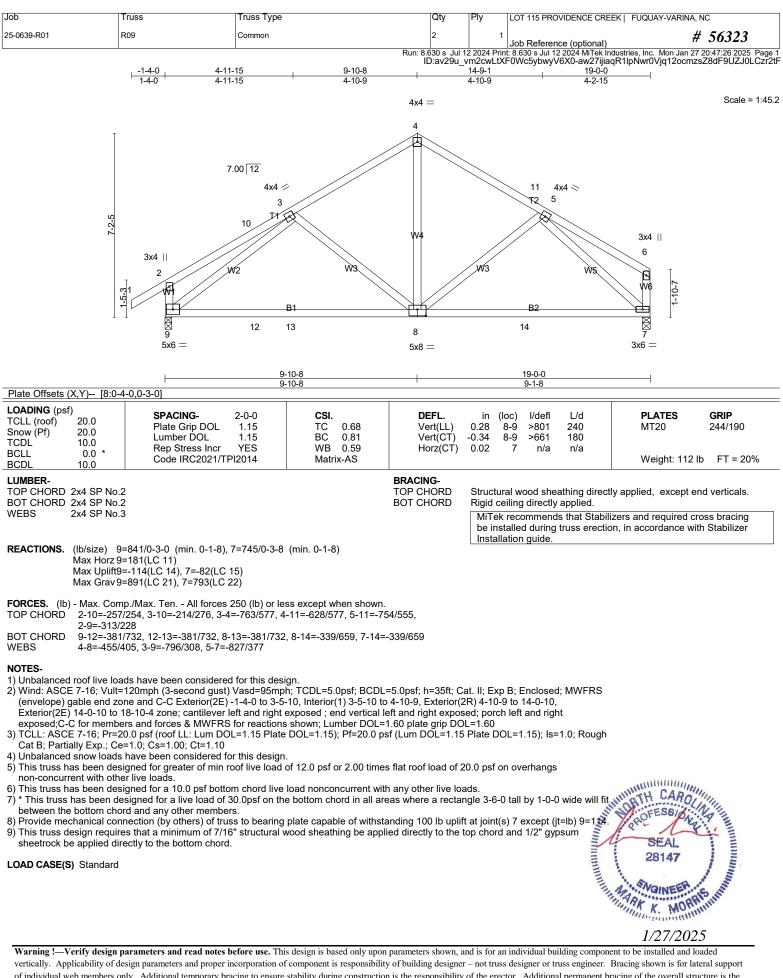


vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

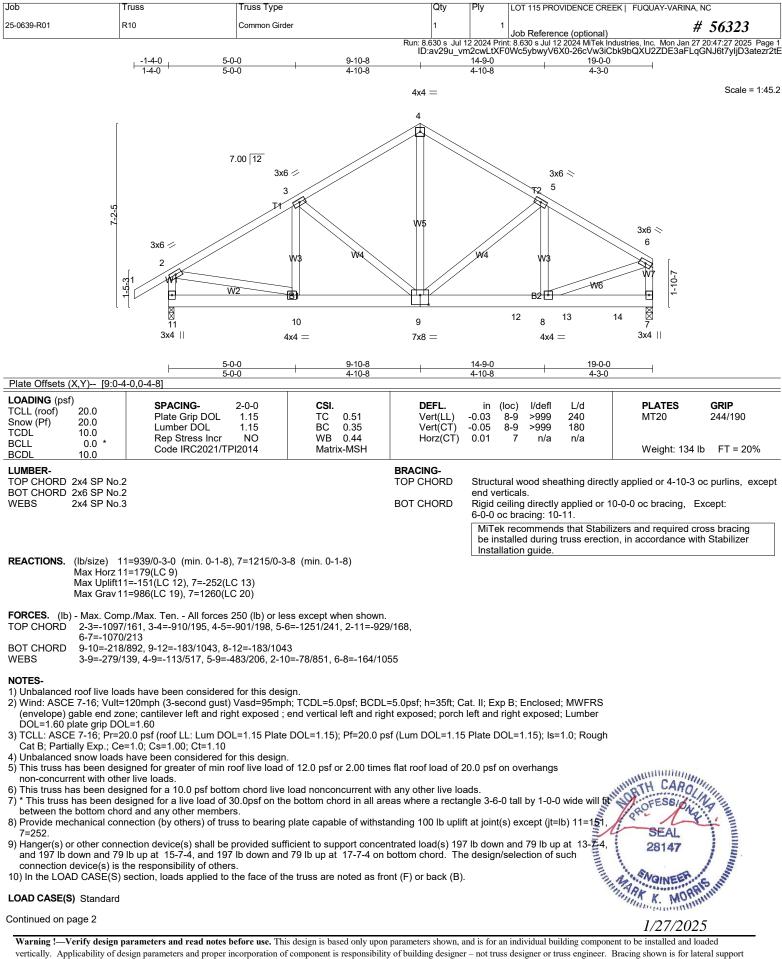
Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-VARINA	, NC		
25-0639-R01	R07	Common Structural Gable	1	1	Job Reference (optional)	# 56323		
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:26 2025 Page 2 ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-aw27ijiaqR1IpNwr0Vjq12ojaz?28gq9UZJ0LCzr2tF								

LOAD CASE(S) Standard





vertically. Applicability of design parameters and road notes before tact. This design is black only upon parameters shown, and is for an individual building component to be instance and black upon the vertically. Applicability of black design are readed and black upon the vertical structure is the responsibility of the vertical design are readed and black upon the vertical design are readed and the vertical design and the vertical design are readed are readed and the vertical design are readed are readed and the vertical design are readed are rea



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

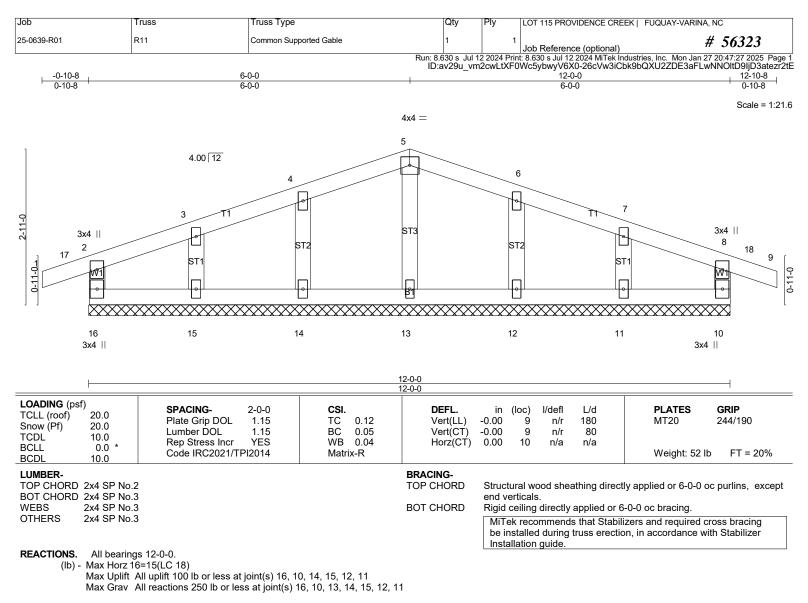
ſ	Job	Truss	Truss Type	Qty	Ply	LOT 115 PROVIDENCE CREEK   FUQUAY-VARINA, NC
	25-0639-R01	R10	Common Girder	1	1	Job Reference (optional) # 56323
	Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Mon Jan 27 20:47:27 2025 P ID:av29u_vm2cwLtXF0Wc5ybwyV6X0-26cVw3iCbk9bQXU2ZDE3aFLqGNJ6t7yIjD3ate					

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-2=-60, 2-4=-60, 4-6=-60, 7-11=-20

Concentrated Loads (lb) Vert: 12=-190(F) 13=-190(F) 14=-190(F)





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Corner(3R) 4-0-0 to 8-0-0, Corner(3E) 8-0-0 to 12-10-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

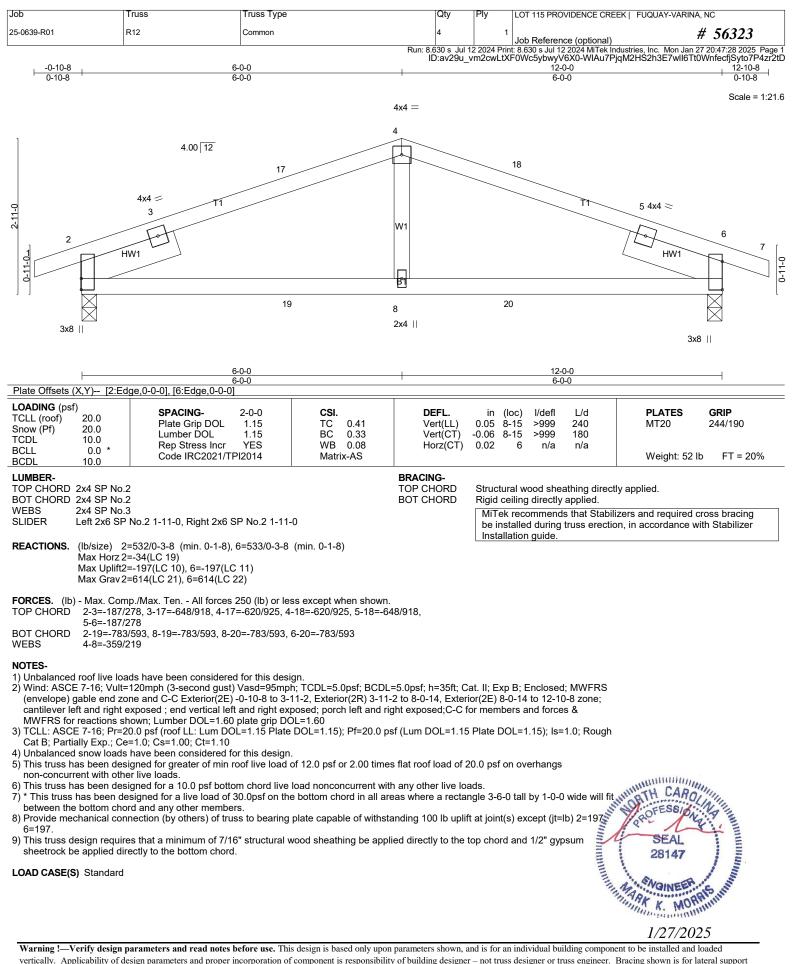
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B: Partially Exp.: Ce=1.0: Cs=1.00: Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.

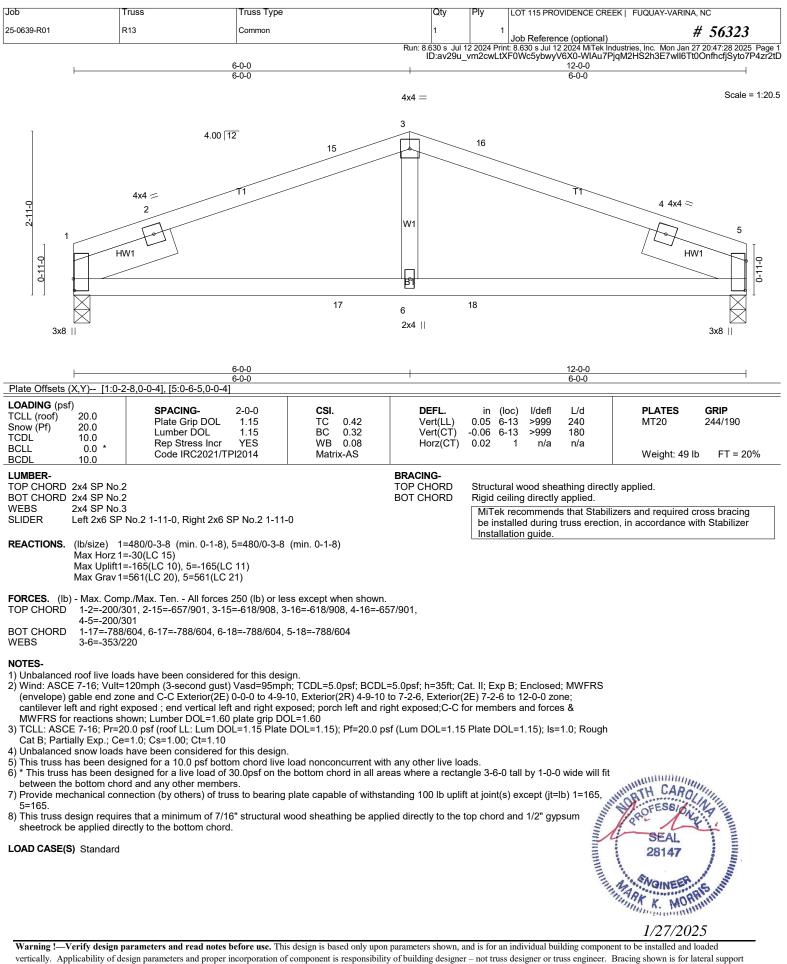
- 12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide with the bottom chord and any other members.
  13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 the standing 100 the stand

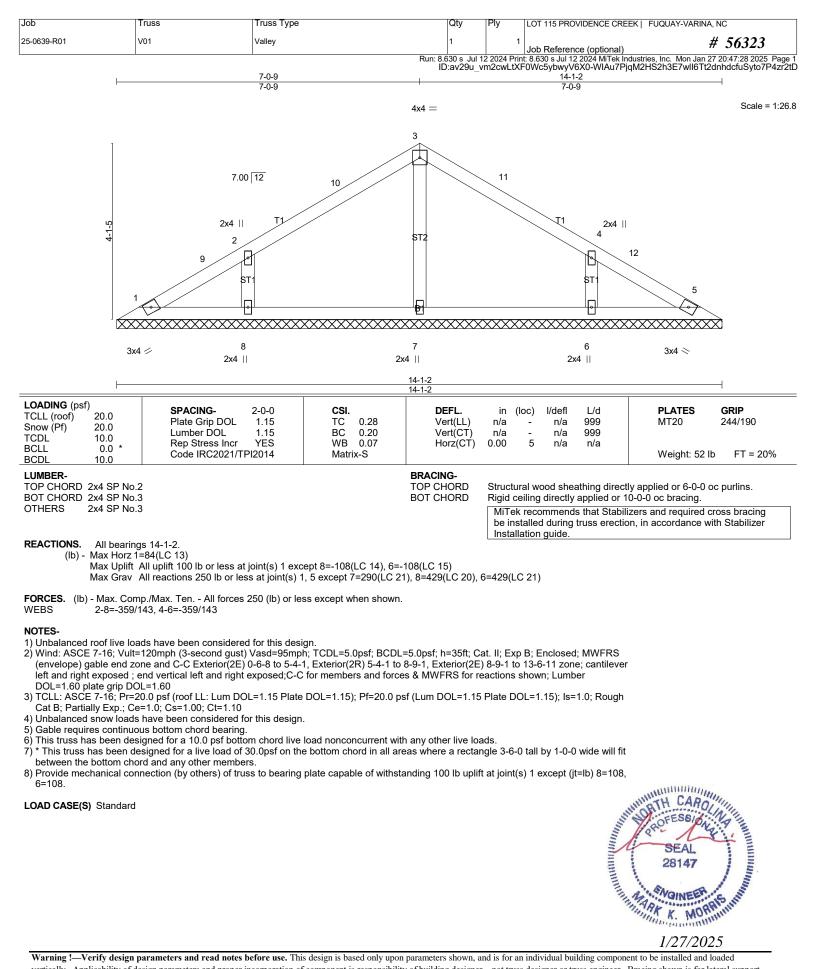
LOAD CASE(S) Standard

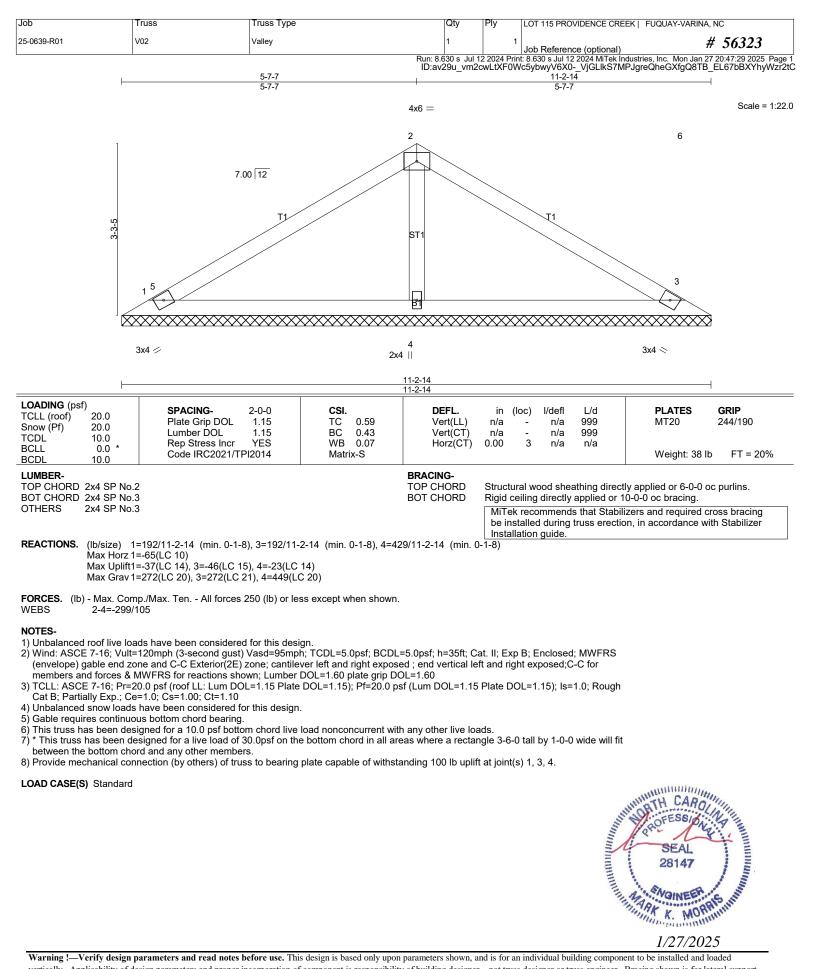


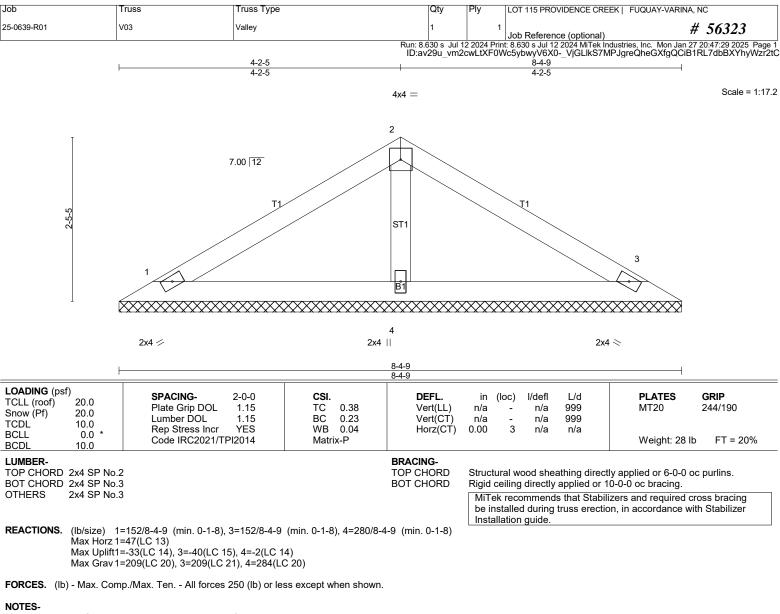
<sup>1)</sup> Unbalanced roof live loads have been considered for this design.











1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

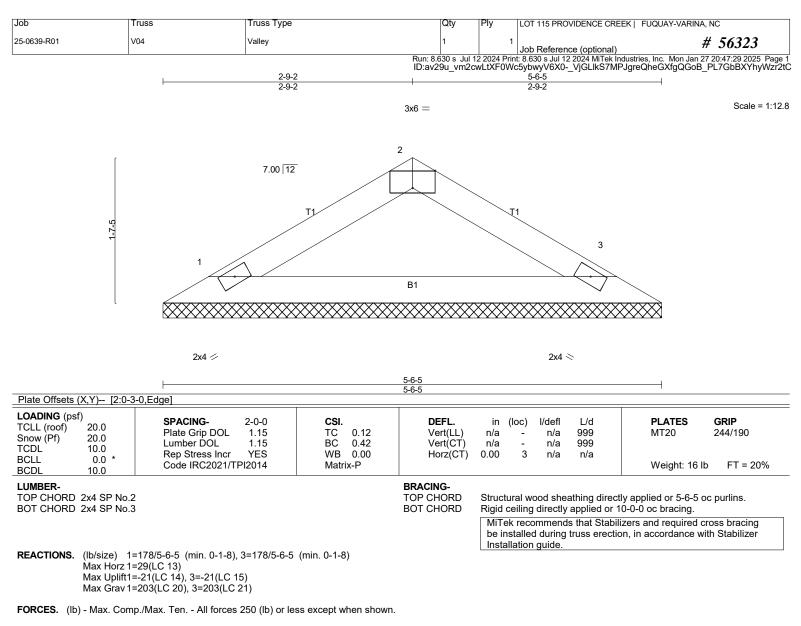
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

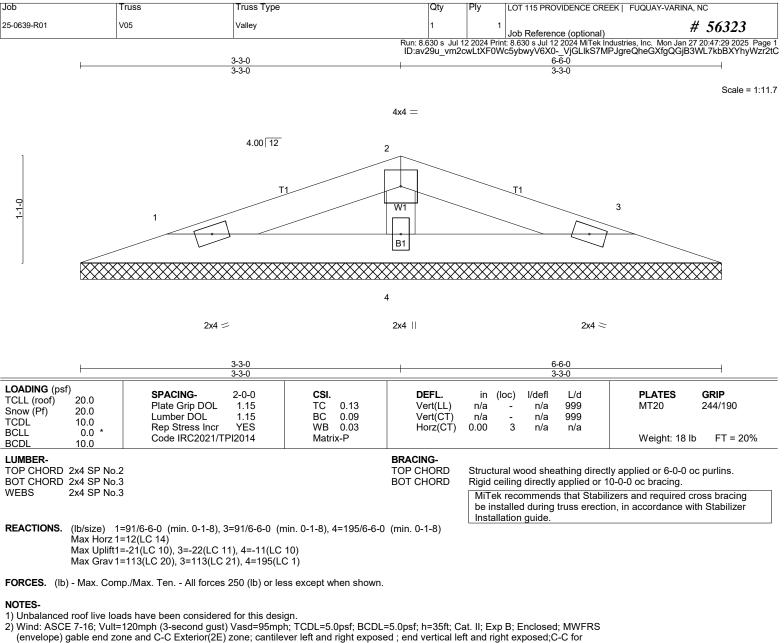
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

#### LOAD CASE(S) Standard





members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

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LOAD CASE(S) Standard

