

THE ROLEN

NORTH CAROLINA

SQUARE FOOTAGES

FIRST FLOOR (HTD.) = 1200 sf SECOND FLOOR (HTD.) = 1094 sf 2294 sf GARAGE = 408 sf FRONT PORCH = 106 sf REAR PORCH = 132 sf TOTAL = 2864 sf

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D- 1-2f DETAILS - FRAMING

GENERAL CONTRACTOR

LGI HOMES

SCOTT STERLING
V.P. OF CONSTRUCTION FOR NC / SC
704-953-3824

ARCHITECT

SECOND FLOOR ELECTRICAL PLAN

COX ARCHITECTURE & DESIGN, PLLC

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ENGINEER

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COVER SHEET

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DATE CREATED: 1/19/2025



GENERAL NOTES

-DO NOT SCALE DRAWINGS; DESIGNATED DIMENSIONS SHALL BE USED IN PREFERENCE TO MEASUREMENTS BY SCALE.

-GENERAL CONTRACTOR SHALL VERIFY AND COMPLY TO ALL LOCAL & NATIONAL BUILDING CODES. CONTACT ARCHITECT IF INSPECTORS REQUIRE REVISIONS OR ALTERATIONS TO DRAWINGS.

-ALL SUB-CONTRACTORS SHALL BE RESPONSIBLE FOR DAMAGE TO OTHER TRADES.

DESIGN SPECIFICATIONS

<u>USE GROUP:</u> (2018 NCBC:R)

"RESIDENTIAL" ONE & TWO FAMILY DWELLING

CONSTRUCTION CLASS: (2018 NCBC:R)

"RESIDENTIAL"

HEIGHT & AREA LIMIT: (LOCAL ZONING)

35' MAXIMUM 2-STORY HEIGHT

EMERGENCY ESCAPE: (2018 NCBC:R)

EGRESS OR RESCUE WINDOWS FROM SLEEPING ROOMS

SHALL HAVE MINIMUM OF 5.7 SQ. FT. NET CLEAR OPENING

(5.0 SQ. FT. NET OPENING @ GRADE FLOOR)

MINIMUM 20" WIDTH.

MINIMUM 24" HEIGHT.

MAXIMUM 44" SILL HEIGHT

 $\frac{\text{GARAGE / HOUSE CEILING / ASSEMBLY:}}{\frac{1}{2}\text{" GYPSUM WALL BOARD}}$

%" TYPE "X" GYPSUM BOARD CEILING WHERE LIVING IS ABOVE

20 MINUTE RATED GARAGE / HOUSE DOOR

ATTIC VENTILATION:

[TOTAL ATTIC SQ. FT.] / [300] = SQ. FT. AREA REQUIRED

RIDGE VENT:

[LINEAR FEET OF VENT] X [18 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED SOFFIT VENT:

[LINEAR FEET OF VENT] X [7 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED EDGE SHINGLE OVER VENT:

[LINEAR FEET OF VENT] X [9 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED ROOF LOUVER VENTS:

[NUMBER OF VENTS] X [70 SQUARE INCHES IN FREE AREA] / 12 = SQ. FT. PROVIDED CRAWL SPACE VENTILATION:

[TOTAL CRAWL SPACE SQ. FT.] / [300] = SQ. FT. AREA REQUIRED FOUNDATION VENT:

FREE SPACE PROVIDED BY VENT = F

[FREE AREA REQUIRED] / F = NUMBER OF VENTS REQUIRED

THE ROLEN

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PERMIT SET

● 19 JANUARY 2025

GENERAL NOTES

A1.1

FLOOR PLAN NOTES

-CONTRACTORS TO FIELD VERIFY ALL DIMENSIONS & NOTIFY ARCHITECT OF ANY DISCREPANCIES, ERRORS OR OMISSIONS PRIOR TO EXECUTION OF WORK.

-CLEANUP TO OCCUR DAILY.

-G.C. TO VERIFY FINISH GRADE a HOUSE TO DETERMINE NUMBER OF STEPS.

-MECHANICAL CONTRACTOR TO COORDINATE W/ ARCHITECT LOCATION OF MAIN TRUNK & DISTRIBUTION LINES, REGISTERS (CENTER ALL REGISTERS ON WINDOWS), THERMOSTATS, AIR HANDLER & CONDENSERS.

-CEILING HEIGHTS LISTED ARE DIMENSIONED TO FRAMING (TOP OF SUBFLOOR TO UNDERSIDE OF FRAMING ABOVE)

-CONCRETE SLABS & SETTING BEDS TO ACCOMMODATE FOR ADEQUATE WATER DRAINAGE AT GARAGES AND PORCHES

-ATTIC ACCESS DROP-DOWN STAIRS TO CONFORM WITH LOCAL AUTHORITIES BASED ON R807.1. MINIMUM NET CLEAR OPENING OF 20" x 30". ALL ATTIC ACCESS STAIRS TO BE WEATHER STRIPPED & SEALED WITH R-VALUES THAT CONFORM WITH LOCAL AUTHORITIES BASED ON N1102.2.4. GC TO PROVIDE & INSTALL INSULATION DAMS TO RESTRICT TYPICAL ATTIC INSULATION FROM FALLING THROUGH ATTIC ACCESS OPENING. RIGID FOAM BOX COVER TO BE INSTALLED & SEALED AROUND FRAMING OF OPENING, NOT TO IMPEDE OR OBSTRUCT PERFORMANCE OF ADJACENT TYPICAL ATTIC INSULATION.

-HOSE BIBB(S) TO BE LOCATED 24" ABOVE FIRST FLOOR FINISHED FLOOR

WINDOW NOTES

-ALL WINDOW DIMENSIONS ARE BASED ON M.I. WINDOW ROUGH OPENING CALL OUTS, UNO. FINAL SELECTION OF WINDOW SIZES ARE TO BE VERIFIED IN THE FIELD.

-WINDOWS TO BE INSTALLED BY CERTIFIED WINDOW INSTALLER PER MANUFACTURER'S INSTALLATION

-WINDOW SUPPLIER TO SPECIFY & ORDER TEMPERED GLASS IN WINDOWS AS REQUIRED BY LOCAL CODE.

-G.C. AND WINDOW SUPPLIER TO VERIFY THAT EACH BEDROOM TO HAVE A MINIMUM OF ONE WINDOW WHICH MEETS EMERGENCY EGRESS AS REQUIRED BY LOCAL AUTHORITIES. WINDOW SUPPLIER TO ADD EGRESS HARDWARE TO CASEMENT WINDOWS IF NECESSARY.

-TOP OF INTERIOR CASING @ ADJACENT DOORS & WINDOWS TO ALIGN WHEN HEADER CALL OUTS ARE EQUAL

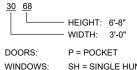
DOOR NOTES

-ATTIC ACCESS DOORS TO INCLUDE WEATHER STRIPPING & INSULATION

-TOP OF INTERIOR CASING $\ensuremath{\textcircled{@}}$ ADJACENT DOORS & WINDOWS TO ALIGN WHEN HEADER CALL OUTS ARE EQUAL

-DOOR SUPPLIER TO SPECIFY & ORDER TEMPERED GLASS IN DOORS AS REQUIRED BY LOCAL CODE.

DOOR & WINDOW LEGEND



VINDOWS: SH = SINGLE HUNG T = TRANSOM F = FIXED

INSULATION NOTES

INSULATION VALUES PER NCRC ECC 2018 CH. 11 ENERGY CONSERVATION CODE (2024 ECC STILL UNDER LEGISLATIVE REVIEW)

CLIMATE	ZONE 3A	CLIMATE	ZONE 4A
TABLE N1102.1.2		TABLE N1	102.1.2
CEILING: FLOOR: WALL: SLAB:	R-38 R-19 R-15 R-0	CEILING: FLOOR: WALL: SLAB:	R-38 R-19 R-15 R-10

SQUARE FOOTAGES

= 1200 sf

SECOND FLOOR (HTD.) $= \frac{1094}{2294} \text{ sf}$ GARAGE = 408 sfFRONT PORCH = 106 sfREAR PORCH = 132 sf

FIRST FLOOR (HTD.)

TOTAL = 2864 sf

FLOOR PLAN LEGEND

5 SHELVES 1R 2S 1 ROD, 2 SHELVES 2R 2S 2 ROD, 2 SHELVES HANGING ROD CO CASED OPENING W D WASHER, DRYER D/W DISH WASHER FRIG REFRIGERATOR LS LAZY SUSAN MIRROR M SHOWER HEAD

WALL SCHEDULE

FRAMED WALLS
---- OVERHEAD/BELOW

ALL WALLS ARE 2x4 WOOD STUD WALLS, UNO 5 1/2" DIMENSION INDICATES 2x6 WOOD STUD WALL

STAIR NOTES

-STAIR FABRICATOR / INSTALLER TO VERIFY THAT STAIRS MEET ALL REQUIRED CODES

-ADJUSTMENTS TO STAIR TO BE CONFIRMED W! ARCHITECT & CONTRACTOR PRIOR TO STAIR CONSTRUCTION

CEILING HEIGHT NOTES

** UNLESS NOTED OTHERWISE **

9' - 1 ½" CEILING HEIGHTS ON FIRST FLOOR 8' - 1 ½" CEILING HEIGHTS ON SECOND FLOOR

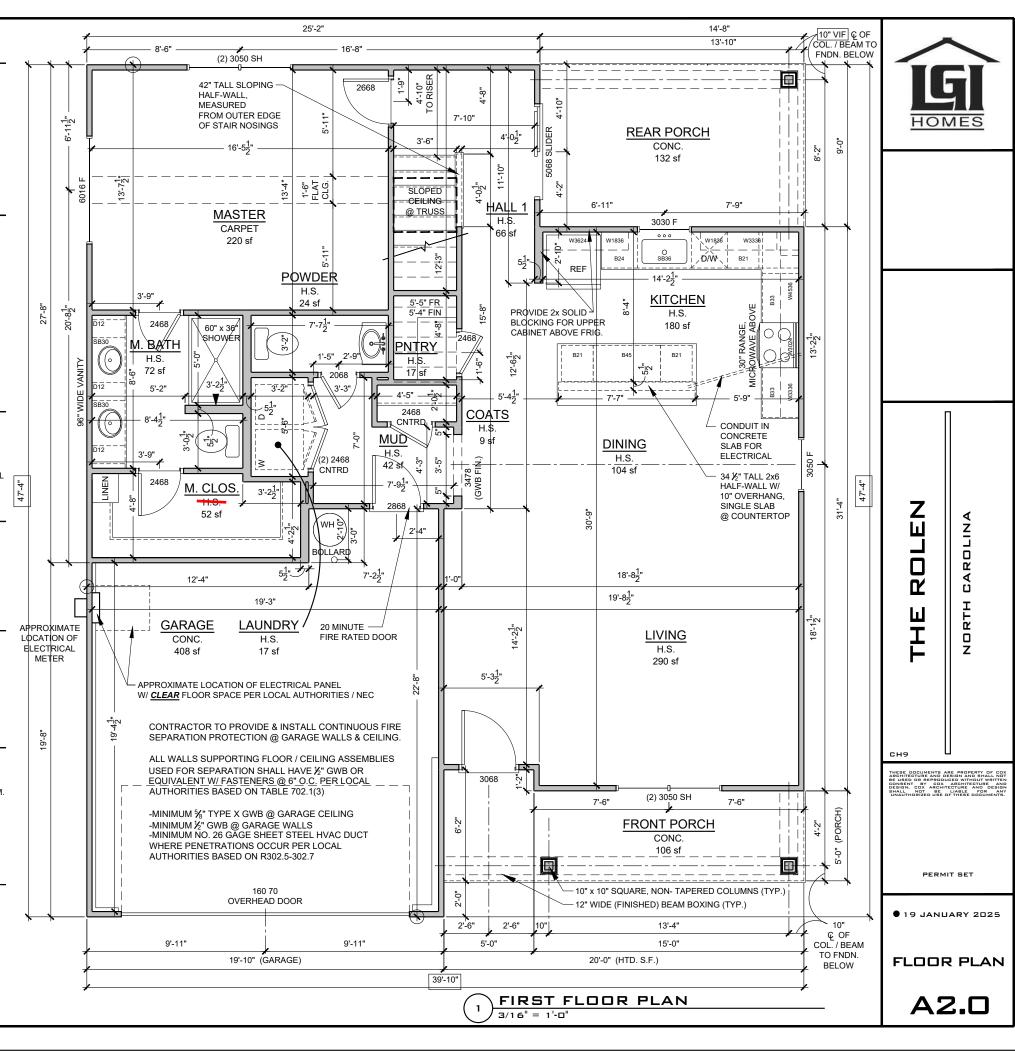
> MEASURED FROM TOP OF SUBFLOOR / CONCRETE SLAB TO BOTTOM OF FLOOR JOISTS / ROOF TRUSSES

COLUMN NOTES

COLUMNS TO BE: AFCO OR COLUMN OF EQUAL BEARING CAPACITY. (6000 # MINIMUM)
TOP CONNECTION: (2) #8 - ½" x 3" STAINLESS STEEL SCREWS PER SIDE INSERTED INTO BEAM.
BOTTOM CONNECTION: (3) UBS - #18043
BRACKETS FASTENED WITH (2) ½" x 1 ½"
SCREWS INTO COLUMN & (2) ½" x 3 ¾"
CONCRETE SCREWS THROUGH FASTENER
INTO CONCRETE

ELECTRICAL PANEL/METER

MAXIMUM DISTANCE BETWEEN ELECTRICAL PANEL & ELECTRICAL METER (NEC 230.70) TO BE DETERMINED BY LOCAL AUTHORITY.



FLOOR PLAN NOTES

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-CLEANUP TO OCCUR DAILY.

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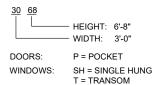
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DOOR & WINDOW LEGEND



INSULATION NOTES

INSULATION VALUES PER NCRC ECC 2018 CH. 11 ENERGY CONSERVATION CODE (2024 ECC STILL UNDER LEGISLATIVE REVIEW)

CLIMATE ZONE 3A	CLIMATE ZONE 4A
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WALL SCHEDULE

OVERHEAD/BELOW

FRAMED WALLS

ALL WALLS ARE 2x4 WOOD STUD WALLS, UNO 5 1/2" DIMENSION INDICATES 2x6 WOOD STUD WALL

STAIR NOTES

-STAIR FABRICATOR / INSTALLER TO VERIFY THAT STAIRS MEET ALL REQUIRED CODES

-ADJUSTMENTS TO STAIR TO BE CONFIRMED W/ ARCHITECT & CONTRACTOR PRIOR TO STAIR CONSTRUCTION

CEILING HEIGHT NOTES

** UNLESS NOTED OTHERWISE ** 9' - 11/2" CEILING HEIGHTS ON FIRST FLOOR 8' - 1 1/2" CEILING HEIGHTS ON SECOND FLOOR

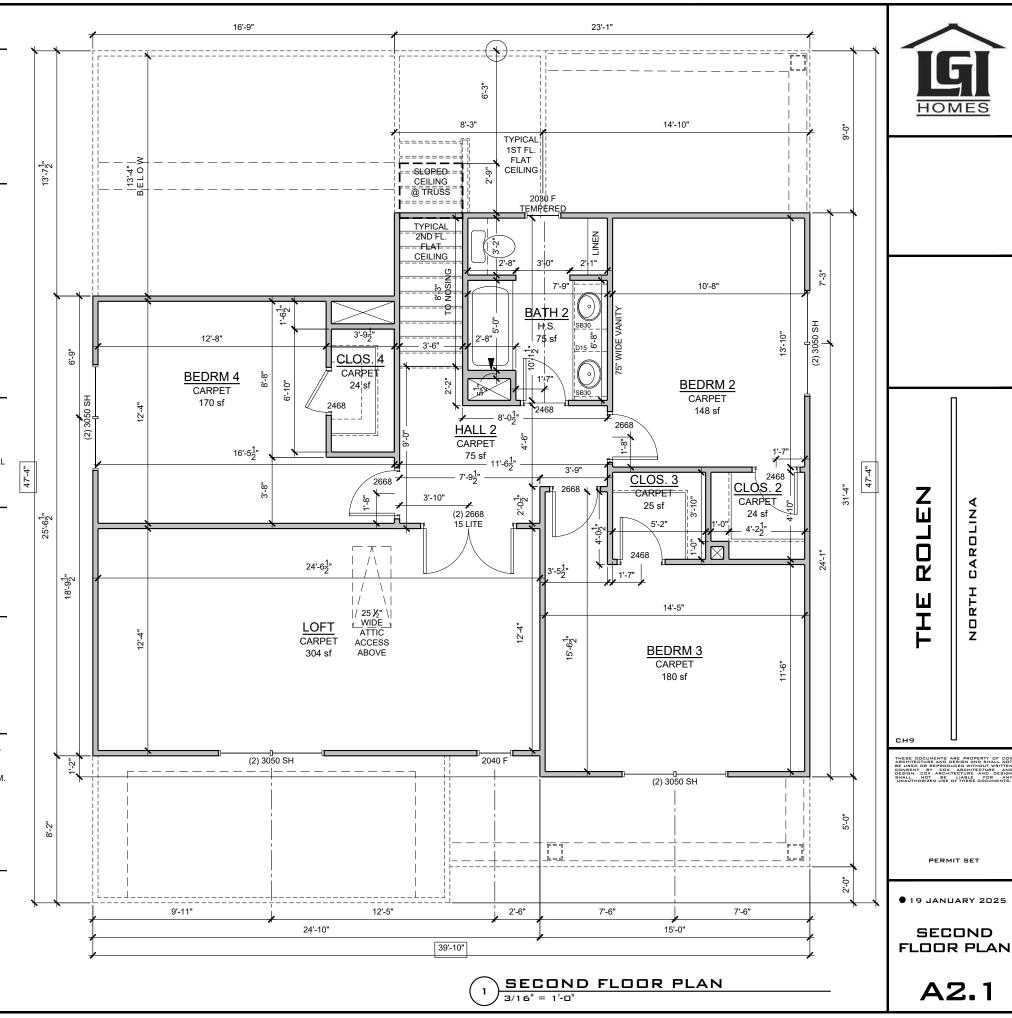
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COLUMN NOTES

COLUMNS TO BE: AFCO OR COLUMN OF EQUAL BEARING CAPACITY. (6000 # MINIMUM) TOP CONNECTION: (2) #8 - ½" x 3" STAINLESS STEEL SCREWS PER SIDE INSERTED INTO BEAM. BOTTOM CONNECTION: (3) UBS - #18043 BRACKETS FASTENED WITH (2) 1/4" x 1 1/4" SCREWS INTO COLUMN & (2) ½" x 3 ¾" CONCRETE SCREWS THROUGH FASTENER

ELECTRICAL PANEL/METER

MAXIMUM DISTANCE BETWEEN ELECTRICAL PANEL & ELECTRICAL METER (NEC 230.70) TO BE DETERMINED BY LOCAL AUTHORITY.



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ROOF NOTES

-CONTRACTORS TO FIELD VERIFY ALL DIMENSIONS & NOTIFY ARCHITECT OF ANY DISCREPANCIES, ERRORS OR OMISSIONS PRIOR TO EXECUTION OF WORK.

-ALL ROOF PENETRATIONS TO BE PLACED ON REAR SIDE OF MAIN RIDGE OR AS SPECIFIED BY ARCHITECT. PAINT TO MATCH SHINGLE COLOR.

-ATTIC INSULATION TO BE BATT. INSUL. PER LOCAL CODE, PROVIDE BAFFLES @ PERIMETER TO ALLOW 2" FOR AIRFLOW FROM EAVE VENTS TO RIDGE VENTS.

-ROOF SHEATHING TO BE ½" T&G PLYWOOD W/ METAL CLIPS @ ENDS.

-ALL BATHROOM & DRYER VENT PENETRATIONS TO RUN TOWARD REAR OF HOUSE & VENT IN REAR OUTSIDE WALL OR ROOF BEHIND MAIN RIDGE.

-GUTTER & DOWNSPOUT INSTALLER TO PROVIDE ADEQUATE UNITS PER MANUFACTURER SPECIFICATIONS BASED ON ROOF COVERAGE. SUB-CONTACTOR TO VERIFY NUMBER & LOCATION OF DOWNSPOUTS

-ALL SHINGLED ROOFS WITH A PITCH OF 4:12 OR LESS REQUIRE (2) LAYERS OF 30# FELT PAPER PER LOCAL CODES

ELEVATION NOTES

-ALL REPRESENTATIONS OF GRADE LEVELS ARE FOR DRAWING PURPOSES ONLY, AND TO BE VERIFIED IN FIELD.

-ALL EXTERIOR ELEVATION DIMENSIONS ARE FRAMING DIMENSIONS, UNO. G.C. TO FIELD VERIFY DIMENSIONS LOCATED AT SLOPED FRAMING AND / OR CONCRETE SLABS & PADS

-ALL TRUSS PROFILE DIMENSIONS TO BE VERIFIED BY TRUSS MANUFACTURER. TRUSS MANUFACTURER TO NOTIFY ARCHITECT IF TRUSS PROFILES / DIMENSIONS CHANGE.

-ALL BUILDINGS CONSTRUCTED WITH LESS THAN A 10' FIRE SEPARATION DISTANCE BETWEEN SHALL COMPLY WITH LOCAL AUTHORITIES BASED ON R302.1.1:

IN CONSTRUCTION USING VINYL OR ALUMINUM SOFFIT MATERIAL, THE FOLLOWING APPLICATION SHALL APPLY. SOFFIT ASSEMBLIES MUST BE SECURELY ATTACHED TO FRAMING MEMBERS AND APPLIED OVER FIRE-RETARDANT-TREATED WOOD, 23/32-INCH WOOD SHEATHING OR 5/8-INCH EXTERIOR GRADE OR MOISTURE RESISTANT GYPSUM BOARD. VENTING REQUIREMENTS SHALL BE PROVIDED IN BOTH SOFFIT AND UNDERLAYMENT. VENTS SHALL BE EITHER NOMINAL 2-INCH CONTINUOUS OR EQUIVALENT INTERMITTENT AND SHALL NOT EXCEED THE MINIMUM NET FREE AIR REQUIREMENTS ESTABLISHED IN SECTION R806.2 BY MORE THAN 50 PERCENT.

CEILING HEIGHT NOTES

** UNLESS NOTED OTHERWISE **

9' - 1 1/2" CEILING HEIGHTS ON FIRST FLOOR

8' - 1 1/2" CEILING HEIGHTS ON SECOND FLOOR

MEASURED FROM TOP OF SUBFLOOR / CONCRETE SLAB TO BOTTOM OF FLOOR JOISTS / ROOF TRUSSES

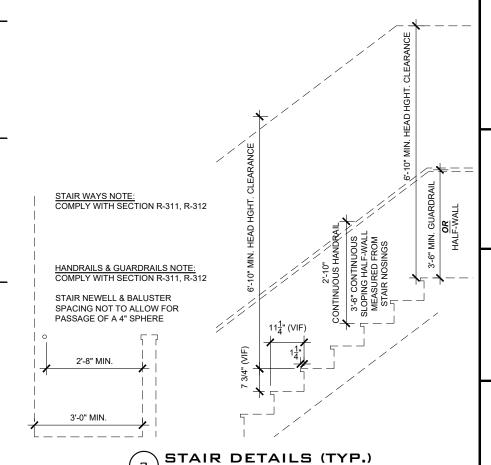
COLUMN NOTES

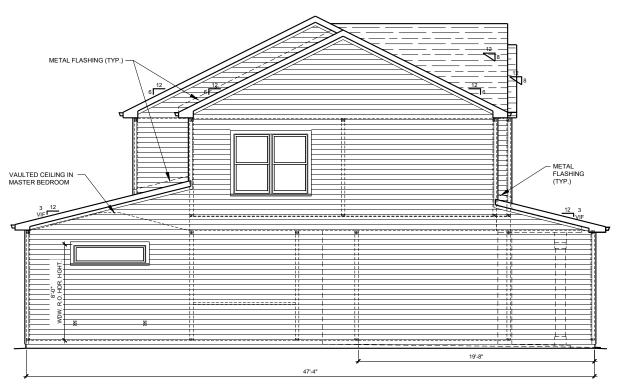
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INSULATION NOTES

INSULATION VALUES PER NCRC ECC 2018 CH. 11 ENERGY CONSERVATION CODE (2024 ECC STILL UNDER LEGISLATIVE REVIEW)

CLIMATE ZONE 3A		CLIMATE ZONE 4A		
TABLE N1102.1.2		TABLE N1102.1.2	TABLE N1102.1.2	
CEILING: FLOOR: WALL: SLAB:	R-38 R-19 R-15 R-0	CEILING: R-38 FLOOR: R-19 WALL: R-15 SLAB: R-10		







FRONT ELEVATION





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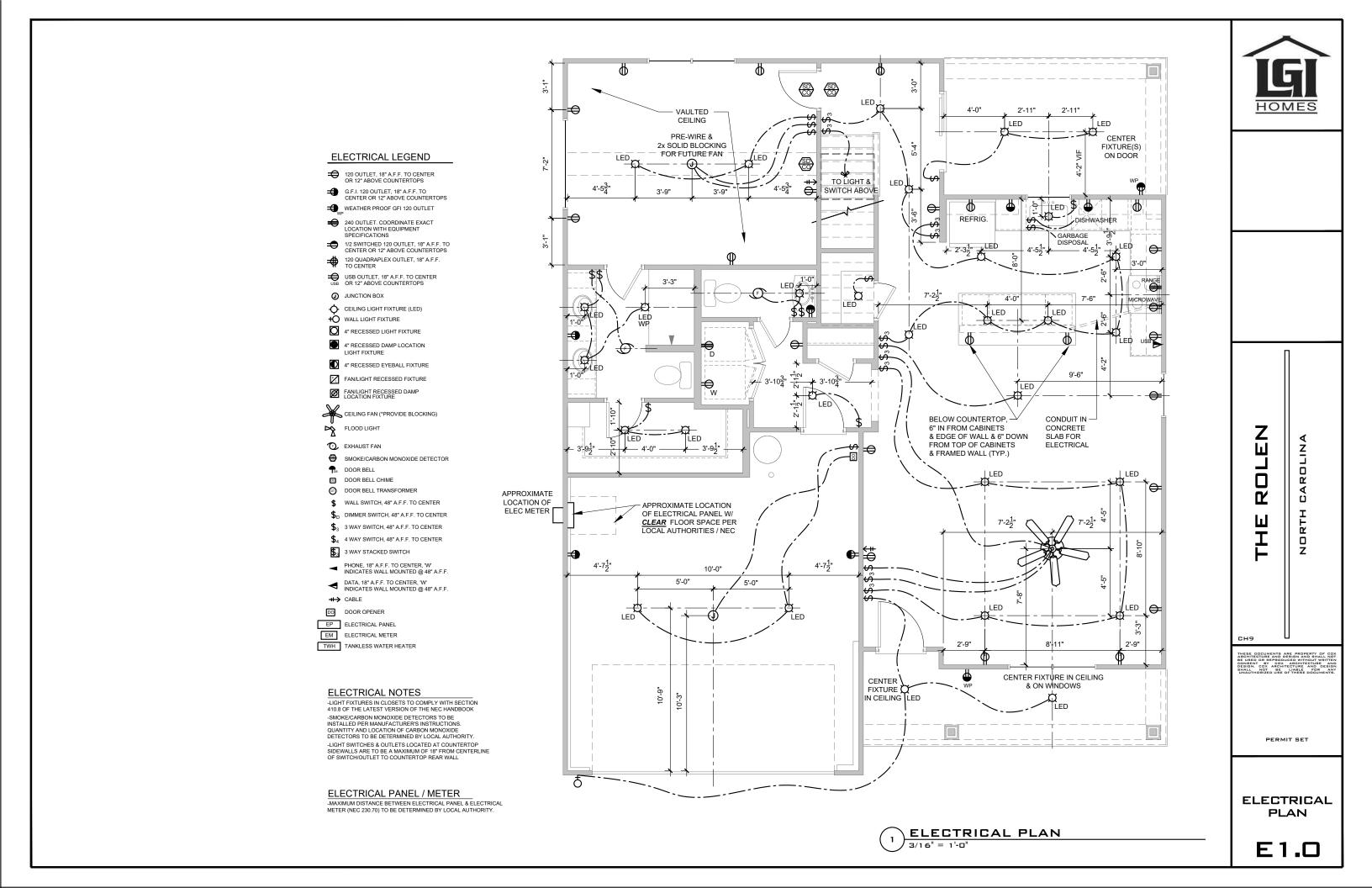
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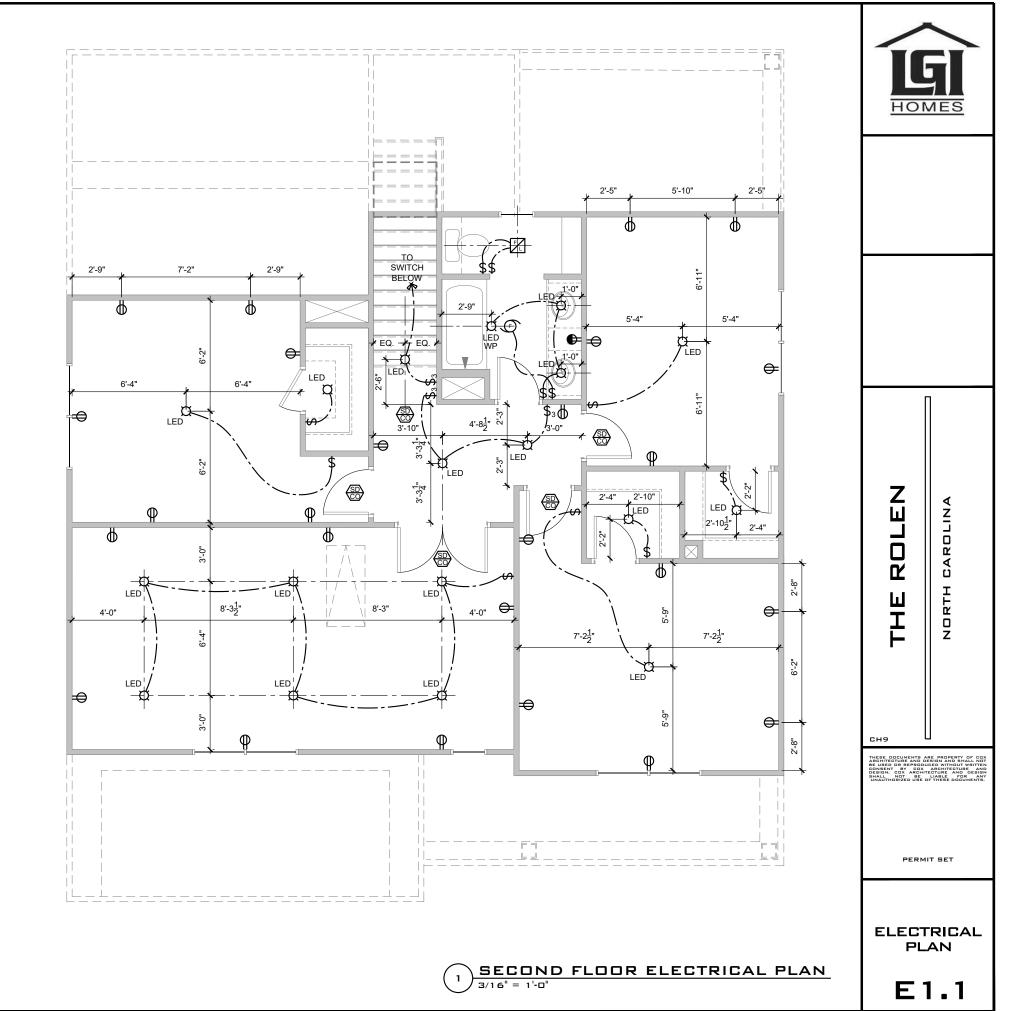
● 19 JANUARY 2025

FRONT **ELEVATION**

A3.0







ELECTRICAL LEGEND

120 OUTLET, 18" A.F.F. TO CENTER OR 12" ABOVE COUNTERTOPS

G.F.I. 120 OUTLET, 18" A.F.F. TO CENTER OR 12" ABOVE COUNTERTOPS

WEATHER PROOF GFI 120 OUTLET

240 OUTLET. COORDINATE EXACT LOCATION WITH EQUIPMENT SPECIFICATIONS

1/2 SWITCHED 120 OUTLET, 18" A.F.F. TO CENTER OR 12" ABOVE COUNTERTOPS

120 QUADRAPLEX OUTLET, 18" A.F.F. TO CENTER

USB OUTLET, 18" A.F.F. TO CENTER OR 12" ABOVE COUNTERTOPS

JUNCTION BOX

CEILING LIGHT FIXTURE (LED)

WALL LIGHT FIXTURE 4" RECESSED LIGHT FIXTURE

4" RECESSED DAMP LOCATION LIGHT FIXTURE

4" RECESSED EYEBALL FIXTURE

FAN/LIGHT RECESSED FIXTURE

FAN/LIGHT RECESSED DAMP LOCATION FIXTURE

CEILING FAN (*PROVIDE BLOCKING) FLOOD LIGHT

EXHAUST FAN

SMOKE/CARBON MONOXIDE DETECTOR **↑** DOOR BELL

DOOR BELL CHIME ⅎ DOOR BELL TRANSFORMER

\$ WALL SWITCH, 48" A.F.F. TO CENTER

\$D DIMMER SWITCH, 48" A.F.F. TO CENTER \$3 3 WAY SWITCH, 48" A.F.F. TO CENTER

\$4 4 WAY SWITCH, 48" A.F.F. TO CENTER

3 WAY STACKED SWITCH

PHONE, 18" A.F.F. TO CENTER, "W' INDICATES WALL MOUNTED @ 48" A.F.F.

DATA, 18" A.F.F. TO CENTER, "W" INDICATES WALL MOUNTED @ 48" A.F.F.

++> CABLE

DO DOOR OPENER

EP ELECTRICAL PANEL TWH TANKLESS WATER HEATER

EM ELECTRICAL METER

ELECTRICAL NOTES

-LIGHT FIXTURES IN CLOSETS TO COMPLY WITH SECTION 410.8 OF THE LATEST VERSION OF THE NEC HANDBOOK -SMOKE/CARBON MONOXIDE DETECTORS TO BE INSTALLED PER MANUFACTURER'S INSTRUCTIONS. QUANTITY AND LOCATION OF CARBON MONOXIDE
DETECTORS TO BE DETERMINED BY LOCAL AUTHORITY. -LIGHT SWITCHES & OUTLETS LOCATED AT COUNTERTOP SIDEWALLS ARE TO BE A MAXIMUM OF 18" FROM CENTERLINE OF SWITCH/OUTLET TO COUNTERTOP REAR WALL

ELECTRICAL PANEL / METER

-MAXIMUM DISTANCE BETWEEN ELECTRICAL PANEL & ELECTRICAL METER (NEC 230.70) TO BE DETERMINED BY LOCAL AUTHORITY.

- THESE DRAWINGS AND ITS CONTENTS ARE THE PROPERTY OF QUEEN CITY CONSULTING AND DESIGN, PLLC. (QC) AND THE CLIENT AS NOTED ON THIS PAGE, DISTRIBUTION TO ANY OTHER PARTIES FOR PURPOSES OTHER THAN THOSE DIRECTLY CONCERNED WITH THE TITLED PROJECT WITHOUT PRIOR WRITTEN CONSENT FROM QC IS STRICTLY PROHIBITED.
- THE ENGINEER'S NAME PRESENT ON THE SEAL OF THESE DRAWINGS IS THE ENGINEER OF RECORD (EOR).
- DETAILS NOTED AS "TYPICAL" SHALL BE USED WHENEVER APPLICABLE. REFER TO SPECIFICATIONS FOR INFORMATION NOT COVERED BY THESE NOTES OR
- DRAWINGS.

 4. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION. FURTHERMORE, QC WILL NOT BE HELD RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CONFORM TO THE CONSTRUCTION DOCUMENTS, INCLUDING THIS STRUCTURAL SET, SHOULD ANY NON-CONFORMITIES
- 5. THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS
- PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY.

 6. ANY OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE STRUCTURAL DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF, AND RESOLVED WITH, THE ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
- ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE (NCRC), 2018 EDITION, PLUS ALL LOCAL
- CODES AND REGULATIONS.

 8. SEISMIC DESIGN SHALL BE PER SECTION R301.2.2 OF THE 2018 NCRC AND IS BASED OFF OF LOCAL SEISMIC DESIGN CATEGORIES.

- FOOTING AND FOUNDATION NOTES:

 1. FOUNDATION DESIGN IS BASED ON A MINIMUM ALLOWING BEARING CAPACITY OF 2.000 PSF, CONTACT THE EOR IF BEARING CAPACITY IS NOT ACHIEVED. NO EXCAVATION SHALL OCCUR WITHIN A 45 DEGREE LINE PROJECTED FROM THE BOTTOM OF THE BUILDING FOUNDATION IS PERMITTED, UNLESS IT IS SPECIFICALLY APPROVED BY THE EOR.
- THE BOTTOM OF ALL FOOTINGS SHALL EXTEND BELOW THE FROST LINE FOR THE REGION, AS SPECIFIED BY THE LOCAL MUNICIPALITY. HOWEVER, THE BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 12" BELOW GRADE.
- CONTRACTOR TO ENSURE THAT ALL DRAINAGE IS DIRECTED AWAY FROM THE EXTERIOR FOOTINGS (MIN. 2% SLOPE)
- EXCAVATIONS OF FOOTINGS SHALL BE TEMPORARILY PROTECTED WITH A 10 MIL POLYETHYLENE MEMBRANE IF CONCRETE IS NOT PLACED WITHIN 24 HOURS OF EXCAVATION
- DO NOT PLACE CONCRETE OR OTHER CEMENTITIOUS MATERIALS AGAINST SUBGRADE WITH ANY DELETERIOUS MATERIALS PRESENT, INCLUDING BUT NOT LIMITED TO: WATER, ICE, FROST, OR LOOSE MATERIAL,
- ALL FOOTINGS ARE TO HAVE MINIMUM 2" PROJECTION ON EACH SIDE OF FOUNDATION WALLS (EXCEPT FOR MONOLITHIC SLAB FOUNDATIONS).

- POURED CONCRETE IS TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS.
- AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C.3.3.
- ALL MATERIALS USED FOR CONCRETE SHALL CONFORM TO ACI 318, ACI 301, OR ASTM C1157.
 THE PLACING OF ALL CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318 AND ASTM C94 REQUIREMENTS.

- ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE EOR. ADMIXTURES SHALL COMPLY WITH ASTM C494 AND C1017.
 CONCRETE SLABS-ON-GRADE SHALL BE CONSTRUCTED IN A MANNER THAT COMPLIES WITH ACI 302.1R-96.
 CONTROL OR SAW CUT JOINTS SHALL BE CUT TO A MINIMUM OF 1/4 OF THE THICKNESS OF THE RESPECTIVE CONCRETE ELEMENT. CONTROL JOINTS
- LOCATED WITHIN INTERIOR AND EXTERIOR SLABS-ON-GRADE SHALL BE SPACED AT A MAXIMUM OF 12' O.C. CONTROL JOINTS SHALL COMPLY WITH ACI 301

- CONCRETE REINFORCEMENT:

 1. BAR REINFORCEMENT SHALL BE CONFORM TO ASTM A615, GRADE 60 STEEL.
- 2. THE FOLLOWING MINIMUM CLEAR COVER SHALL BE PROVIDED OVER REINFORCING BARS:
- CONCRETE EXPOSED TO EARTH = 3"
- CONCRETE EXPOSED TO WEATHER = 1-1/2"
- SLABS NOT EXPOSED TO WEATHER = 3/4"
- CONCRETE BEAMS & COLUMNS = 1-1/2
- BRICK AND/OR POROUS MATERIAL SHALL NOT BE USED TO SUPPORT FOOTING STEEL OFF THE GROUND. PLASTIC REBAR CHAIRS OR PRECAST CONCRETE DOBIES MAY BE USED.
- 4. SPLICES IN REINFORCING STEEL SHALL BE A MINIMUM OF 45X THE DIAMETER, UP TO A #6 REBAR LARGER THAN #6 REQUIRES A MINIMUM LAP
- 5. ALL CONCRETE WALLS SHALL BE DOWELED TO THEIR SUPPORTING FOOTINGS, BEAMS, PADS, ETC. WITH BARS OF THE SAME SIZE AND SPACING AS THE VERTICAL BARS LOCATED WITHIN THE WALL, UNLESS OTHERWISE NOTED. ANCHORAGE OF DOWELS SHALL BE THE EQUIVALENT OF A BAR SPLICE.

- ALL WOOD FRAMING MEMBERS ARE DESIGNED TO BE SPRUCE-PINE-FIR (SPF) #2, UNLESS OTHERWISE NOTED ON THE PLAN. GRADE MARKS SHALL BE MADE BY A RECOGNIZED GRADING AGENCY.
- FRAMING MEMBERS EXPOSED TO WEATHER OR IN DIRECT CONTACT WITH SOIL, CONCRETE, OR MASONRY SHALL BE PRESSURE TREATED SPRUCE-PINE-FIR #2 AND SHALL COMPLY WITH THE AWPA STANDARD C-15
- ALL FASTENERS SUCH AS NAILS, BOLTS, SCREWS, ANCHOR BOLTS, ETC. ATTACHING PRESSURE TREATED OR FIRE—RETARDANT TREATED WOOD SHALL BE HOT-DIPPED ZINC COATED GALVANIZED OR STAINLESS STEEL (ASTM A153)
- LVL ENGINEERED WOOD SHALL HAVE THE FOLLOWING MINIMUM DESIGN VALUES:

 - E = 1,900,000 PSI FB = 2600 PSI FV = 285 PSI FT = 1555 PSI
- 5. PSL ENGINEERED WOOD SHALL HAVE THE FOLLOWING MINIMUM DESIGN VALUES:
 5.1. E = 2,000,000 PSI
 5.2. FB = 2900 PSI
- = 1755 PSI
- 6. LSL ENGINEERED WOOD SHALL HAVE THE FOLLOWING MINIMUM DESIGN VALUES:
 6.1. E = 1,550,000 PSI
 6.2. FB = 2250 PSI
- ALL BEARING HEADERS TO BE 2-2X6 SUPPORTED WITH MINIMUM (1) 2X4 JACK STUD AND (1) 2X4 KING STUD AT EACH END, UNLESS NOTED OTHERWISE ON THE PLANS. NON-LOAD BEARING HEADERS SHALL BE MINIMUM 2-2X4.

 SOLID BLOCKING IS TO BE INSTALLED AT ALL POINT LOAD THROUGH FLOOR LEVELS TO THE FOUNDATION OR TO THE NEAREST STRUCTURAL ELEMENT.

 ALL WOOD STRUCTURAL MEMBERS THAT ARE SPECIFIED ARE MINIMUM SIZES. CONTRACTOR MAY INSTALL LARGER SIZES FOR EASE OF CONSTRUCTION, IF

- 10. ALL NAILS SHALL BE COMMON NAILS, UNLESS NOTED OTHERWISE ON PLANS AND DETAILS.
- 11. ALL LAG SCREWS ARE TO BE PREDRILLED. DRILL DIAMETER IS TO BE 60 PERCENT OF THE SHANK DIAMETER. IN ADDITION, LAG SCREWS SHALL COMPLY WITH ANSI/ASMF STANDARD B18.2.1-1981.
- 12. ALL BOLT HEADS AND NUTS BEARING ON WOOD SHALL HAVE STANDARD CUT WASHERS. HOLES FOR BOLTS SHALL BE BORED 1/16" LARGER THAN THE NOMINAL BOLT DIAMFTER.
- 13. PROVIDE FULL BEARING WHERE ALL BEAMS MEET SUPPORTING FRAMING MEMBERS.
- 14. UNLESS OTHERWISE NOTED ON PLANS, SIZE, HEIGHT, AND SPACING OF WOOD STUDS SHALL BE IN ACCORDANCE WITH SECTION R602.3.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL CODE. WOOD FRAMED WALLS SHALL CONSIST OF SPRUCE-PINE-FIR NO.2 GRADED MATERIAL.

 15. UNLESS OTHERWISE NOTED, FOUR-PLY LYL BEAMS SHALL HAVE PLIES FASTENED TOGETHER WITH TWO ROWS OF 1/2" DIAMETER BOLTS SPACED AT 16"
- O.C. THE BOLTS SHALL BE LOCATED A MINIMUM OF 2-1/2" AND A MAXIMUM OF 3-1/2" FROM THE TOP OF BOTTOM OF THE BEAM.

- 1.1 ALL ROOF TRUSSES MUST BE BUILT IN ACCORDANCE WITH THE TRUSS MANUFACTURER'S REQUIREMENTS. TIF-DOWN CONNECTIONS TO 1.1. ALL NOVE TROSSES MUST BE SOLID IN ACCORDANCE WITH THE TROSS MANUFACTURER'S REQUIREMENTS. IT IN-DUMN CONNECTIONS OF RESTS TUPICT SHALL BE INSTALLED WHERE REQUIRED. WHEN ROOF TRUSS MANUFACTURERS DO NOT PROVIDE THE REQUIRED CONNECTORS, IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE ROOF TRUSS ENGINEER OR THE EOR TO PROVIDE AN ADEQUATE CONNECTION. 1.2. ROOF TRUSS LAYOUTS ARE TO BE IN COMPLANCE WITH THE OVERALL DESIGN SPECIFIED ON THE PLANS. ALL DEVAITONS ARE TO BE BROUGHT TO THE ATTENTION OF THE EOR PRIOR TO INSTALLATION.
- 1.3. ROOF TRUSSES SHALL BE BRACED PER THE MANUFACTURER'S INSTRUCTIONS AND PER THE SBCA BUILDING COMPONENT SAFETY INFORMATION (BCSI) GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED ROOF TRUSSES.
- 1.4. PROVIDE 2X4 LADDER FRAMING SPACED AT 24" O.C. BETWEEN ADJACENT ROOF TRUSSES WHERE FALSE DORMERS ARE LOCATED.
- 1.5. INSTALL MINIMUM 7/16" OSB ROOF SHEATHING.
 1.6. INSTALL ROOF TRUSSES PER SECTION R802.10 IN THE 2018 NCRC. WHERE TRUSS HEELS EXCEED 9-1/4" AND ARE LOCATED OVER BRACED WALL PANELS, BLOCKING IS TO BE INSTALLED PER SECTION R602.10.5 OF THE 2018 NCRC.
- 2. STICK FRAMED ROOFS
 - COLLAR TIES SHALL BE 2X6 SPACED AT 48" O.C. AT ALL RIDGES UNLESS NOTED OTHERWISE AND CONNECTED IN THE UPPER THIRD OF THE ATTIC SPACE USING (3) 10D COMMON NAILS.

 - FUR DOWN ALL RIDGES AS NEEDED SO THAT RAFTERS HAVE FULL CONTACT.
 CEILING JOISTS WHEN ERECTED PARALLEL TO RAFTERS MUST BE SISTERED TO RAFTERS AND SECURED AS PER TABLE R802.5.1(A) OF THE 2018 NORTH CAROLINA RESIDENTIAL CODE.

 4. IN ADDITION TO THE NCRC FASTENER SCHEDULE, UNLESS NOTED OTHERWISE ON THE PLAN, ROOF MEMBERS SHALL BE TIED DOWN WITH
 - ADDITIONAL METAL CONNECTORS. INSTALL A SIMPSON H2.5A CONNECTOR AT EVERY RAFTER TO FASTEN THE LOWER END OF THE RAFTER TO THE
 - TOP PLATE OR BEAM BELOW.
 2.5. INSTALL MINIMUM 7/16" OSB ROOF SHEATHING.



QUEEN CITY CONSULTING AND DESIGN, PLLC

STRUCTURAL PLANS PREPARED FOR:

ROLEN PLAN - LH VERSION

PROJECT ADDRESS

LGL HOMES 5511 CAPITAL CENTER DRIVE, SUITE 560 RALEIGH, NC 27603

DESIGNER: QUEEN CITY CONSULTING AND DESIGN, PLLC. 2459 WILKINSON BLVD, SUITE 300 CHARLOTTE, NC 28208

Revision No.	Date	Description
0	01.17.25	ORIGINAL ENGINEERING

DESIGN SPECIFICATIONS:

CONSTRUCTION TYPE: RESIDENTIAL

APPLICABLE BUILDING CODES:

- 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AMENDMENTS
- ASCE 7-10: MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES

ULTIMATE DESIGN WIND SPEED: 130MPH, EXPOSURE B

ASSUMED SOIL BEARING CAPACITY: 2000PSF

COMPONENT AND CLADDING LOADS SHALL BE DERIVED PER TABLES R301.2(2) AND R301.2(3)

ENGINEERING SEAL APPLIES TO STRUCTURAL COMPONENTS ONLY

QC ASSUMES NO LIABILITY FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, SAFETY PRECAUTIONS, OR DEVIATIONS/DISCREPANCIES THAT MAY OCCUR IN THE PLAN ANY DEVIATIONS OR DISCREPANCIES ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF QUEEN CITY CONSULTING AND DESIGN PLLC

THE ARCHITECTURAL PLANS USED FOR STRUCTURAL DRAWINGS AND ANALYSIS HAVE BEEN PROVIDED BY COX ARCHITECTURE AND DESIGN, PLLC AND HAVE BEEN COMPLETED/REVISED ON 01/15/25 NOTIFY OC OF ANY ALTERATIONS MADE TO THE PLANS AFTER THE DATE SHOWN HEREIN.

		LIVE	LOADS
CONVENTIONAL 2	K ROOF	20	PSF
ROOF TRUS	S	20	PSF
ATTIC ROOF TE	RUSS	60	PSF
FLOOR LIVE TYP. [OWELLING	40	PSF
SLEEPING ARI	EAS	30	PSF
DECKS		40	PSF
PASSENGER VEHICLE	E GARAGE	50	PSF
BALCONIES		40	PSF
ATTICS WITH ST	ORAGE	20	PSF
ATTICS WITHOUT S	STORAGE	10	PSF
GROUND SNOW	LOAD	15	PSF

	DEAD LOADS
CONVENTIONAL 2X ROOF	15 PSF
ROOF TRUSS	20 PSF
CONVENTIONAL 2X FLOOR	10 PSF
I-J0IST	15 PSF
FLOOR TRUSS	15 PSF

PAGE LIST:	
Page Symbol	DESCRIPTION
CS	COVER SHEET, SPECIFICATIONS, REVISIONS
F-1m	MONOLITHIC SLAB FOUNDATION
F-1s	STEMWALL FOUNDATION
F-1c	CRAWLSPACE FOUNDATION
S-1	FIRST FLOOR FRAMING PLAN
S-2	SECOND FLOOR FRAMING PLAN
D-1m	MONOLITHIC SLAB DETAILS
D-1s	STEMWALL DETAILS
D-1c	CRAWLSPACE DETAILS
D-1f	FRAMING DETAILS







N. I. SHEET NAME: COVER

CLIENT: LCL HOMES

PLAN NAME: ROLEN-LH VERSION

NEIGHBORHOOD: TRN

LOT AND ADDRESS:

LOT # TRD PROJECT NUMBER:

I GIXXXX DRAWN BY:

EO & MSB DATE:

01.17.2025 SCALE: 1/4"=1'-0" ON 22"x34" 1/8"=1'-0" ON 11"x17"

PAGE:

CS

MONOSLAB FOUNDATION NOTES:

- DISCLAIMER: ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION, PLUS ALL LOCAL
- CODES AND REGULATIONS.

 THE FOUNDATION HAS BEEN DESIGNED WITH AN ASSUMED 2000 PSF MINIMUM ALLOWABLE SOIL BEARING CAPACITY. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE SOIL BEARING CAPACITY PRIOR TO CONCRETE PLACEMENT. CONTACT QC IF DESIRED BEARING CAPACITY IS NOT ACHIEVED.
- PLACEMENT. COUNCIL OF THE SEARCH SEARCH CAPACITY IS NOT ACHIEVED.
 ALL POURED CONCRETE IS TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000
 PSI AT 28 DAYS. PLACE CONCRETE IN ACCORDANCE WITH ACI STANDARD 318.
 THE BOTTOM OF ALL FOOTINGS SHALL EXTEND BELOW THE FROST LINE FOR THE
 REGION, AS SPECIFIED BY THE LOCAL WONICIPALITY, HOWEVER, THE BOTTOM OF
 ALL FOOTINGS SHALL BE A MINIMUM OF 12" BELOW GRADE.
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST MASONRY WALLS IS 4'. FOR GREATER THAN 4', REFER TO SECTION R404.1 OF THE 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE, OR CONTACT QC FOR ADDITIONAL ENGINEERING. PERIMETER INSULATION IS TO BE INSTALLED PER THE 2018 NCRC AND PER
- PERIMIER INSURATION IS TO BE INSTALLED PER THE 2018 NURK AND PER LOCAL MONICIPALITY.
 WOOD SILL PLATES AT LOAD BEARING AND BRACED WALLS SHALL BE ANCHORED TO THE FOUNDATION WITH 1/2" DIAMETER BOLTS SPACED AT A MAXIMUM OF 6'
 O.C. THERE SHALL BE A MINIMUM OF TWO BOLTS PER PLATE SECTION. BOLTS THERE SPALL BY A MINIMUM OF 7" INTO CONCRETE AND SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE PLATE. BOLTS TO BE LOCATED NOT MORE THAN 12"
- FROM ANY CORNERS OR BREAKS WITHIN THE SILL PLATE.

 ALL FOOTINGS & SLABS ARE TO BEAR ON UNDISTURBED SOIL OR 95%
 COMPACTED FILL, VERPIED BY ENGINEER OR CODE OFFICIAL

 DIMENSIONS SHOWN ON FOUNDATION DRAWINGS ARE TO EDGE OF FRAMING AND NOT TO EDGE OF BRICK VENEER.
- WITH CLASS 1 SOILS (TABLE R405.1), A 4" CRUSHED STONE BASE COURSE IS NOT REQUIRED.
- NOT REQUIRED.
 ALL GRADING AND FOUNDATION WORK MUST BE OBSERVED AND APPROVED PRIOR
 TO PLACEMENT OF CONCRETE.
 CONCRETE SLABS SHALL BE 4" THICK AND CONSTRUCTED OUT OF 3000 PSI MIN.
 COMPRESSIVE STRENGTH WITH 6"x6" W1.4-WH.1.4 WELDED WIRE FABRIC OR
 FIBERMEST CONCRETE OVER 10 MIL. THICK VAPOR BARRIER ON 95% COMPACTED
 FILL, VERIFIED BY EITHER ENGINEER OR CODE OFFICIAL.
 CONCRETE CURBS THAT ARE USED TO SUPPORT PORTAL FRAME WALLS SHALL BE
 A MINIMUM OF 8" WIRE
- A MINIMUM OF 8" WIDE
- ABBREVIATIONS:
 - SJ = SINGLE JOIST FT = FLOOR TRUSS DR = DOUBLE RAFTER TR = TRIPLE RAFTER
- ABBREVIATIONS:

 DJ = DOUBLE JOIST

 GT = GIRDER TRUSS

 SC = STUD COLUMN

 EE = EACH END

 TJ = TRIPLE JOIST
- OC = ON CENTER
- CL = CENTERLINE
 COL = COLUMN
 PT = PRESSURE TREATED PLFA = POINT LOAD FROM ABOVE NTS = NOT TO SCALE
 UNO = UNLESS NOTED OTHERWISE
 K = KING STUD
 MANUF = MANUFACTURER
- J = JACK STUD CONT = CONTINUOUS
- TERMITE TREATMENT NOTE: CONTRACTOR IS TO INSTALL PROTECTION AGAINST SUBTERRANEAN TERMITES PER SECTION R318 OF THE 2018 NCRC.

ANCHORAGE	SCHEDULE	
ANCHOR	MIN. SPACING	MIN. CONC. EMBEDMENT
1/2" DIA. A307 BOLTS W/ 90 DEGREE BEND	6'-0"	7"
SIMPSON MASA MUDSILL ANCHOR	6'-0"	4"
1/2" DIAMETER THREADED ROD W/ SET-3G EPOXY	6'-0"	7"
1/2" DIAMETER SIMPSON TITEN CONCRETE SCREWS	6'-0"	4-1/4"

		39'-10"	
		25'-2"	
\	<u> </u>		\ \
13'-9["	I	10'-10\frac{3}{4}" CONC SLAB	
47'-4" 13'-10 3 "		THICKENED SLAB (MP) 16" X20"D CONCRETE TURNDOWN, TYP. AT EXTERIOR W/ SIDING 10" THICKENED FTG. AT GARAGE WALL THICKENED FTG. AT GARAGE WALL TO THICKENED THICKENE	47'-4"
11-4	I,	7'-21" 20'-31"	
		16'-3" 1'-9½" 1'-9½"	
		19-10° - J 20°-0°	
		مل	

FOOTING SCHEDULE		
LABEL	SIZE	REBAR
A	24"x24"x10"	N/A
В	30"X30"X10"	N/A
С	36"X36"X12"	#4 @ 8" O.C. EA WAY
D	42"X42"X12"	#4 @ 8" O.C. EA WAY
E	48"X48"X12"	#5 @ 8" O.C. EA WAY







SLAB SHEET NAME:
MONOLITHIC
FOUNDATION

CLIENT: LGI HOMES

PLAN NAME: ROLEN-LH VERSION

NEIGHBORHOOD: TRD

LOT AND ADDRESS: LOT # TBD

PROJECT NUMBER: LGIXXXX

DRAWN BY: EO & MSB

DATE: 01.17.2025

SCALE: 1/4"=1'-0" ON 22"x34" 1/8"=1'-0" ON 11"x17"

PAGE:

F-1.1m

FRAMING NOTES:

- REFER TO COVER PAGE FOR ADDITIONAL NOTES
 ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH
- ALL CONSINCTION SHALL CONFORM 10 THE LATEST REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE (NCRC), 2018 EDITION, PLUS ALL LOCAL CODES AND REGULATIONS.

 THE EOR SHALL REVIEW EWP AND TRUSS LAYOUTS FOR ACCURACY PRIOR TO CONSTRUCTION. SOLID BLOCKING IS TO BE INSTALLED AT ALL POINT LOADS THROUGH FLOOR LEVELS TO THE FOUNDATION OR TO THE NEAREST STRUCTURAL ELEMENT. BLOCKING SHALL BE EQUAL TO OR GREATER THAN THE SUPPORT ABOVE.
- SELICITED HAVE THE SUPPORT ABOVE.

 BUILT-UP WOOD COLUMNS CONSISTING OF MULTIPLE STUDS SHALL HAVE EACH LAMINATION NAILED WITH 16D NAILS SPACED AT 9" O.C. FOR BUILT-UP COLUMNS CONSISTING OF (4) PUES OR MORE, SECURE PLIES TOGETHER WITH HORIZONTAL SIMPSON CS-16 COIL STRAPS COLUMNS CONSISTING OF (4) PUES TO MORE, SECURE PLIES TOGETHER WITH HORIZONTAL SIMPSON CS-16 COIL STRAPS LOCATED AT QUARTER POINTS.
- LOCATED AT QUARTER POINTS.

 CONTRACTOR SHALL ENSURE THAT ALL BEAMS, HEADERS, AND STRUCTURAL COMPONENTS
 ARE FULLY BEARING ON THE SUPPORTING MEMBERS. ANY CAPS IN THE FRAMING SHALL BE
 SHIMMED APPROPRIATELY WITH EITHER METAL SHIMS OF WOOD SHIMS AS NECESSARY.
 HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. ANY HEADERS INSTALLED THAT ARE
 GREATER IN SIZE ARE AN ADEQUATE REPLACEMENTS PROVIDED THE MATERIAL IS OF THE
 SAME OR GREATER STRUCTURAL PROPRETIES.
- WHERE TOP PLATE HAS BEEN CUT TO ACCOMMODATE FLUSH HEADERS/BEAMS, INSTALL A
- MINIMUM 16" LONG HORIZONTAL CS-16 STRAP EXTENDING 12" PAST THE BREAK ON EACH UNLESS OTHERWISE NOTED, FOUR-PLY LVL BEAMS SHALL HAVE PLIES FASTENED TOGETHER WITH TWO (2) ROWS OF 1/2" DIAMETER BOLTS SPACED AT 16" O.C. THE BOLTS SHALL BE LOCATED A MINIMUM OF 2-1/2" AND A MAXIMUM OF 3-1/2" FROM THE TOP AND BOTTOM
- OF THE BEAM.

 ALL LOAD BEARING WALLS TO BE 2X4 U.N.O.

 ABBREVIATIONS:

DJ = DOUBLE JOIST GT = GIRDER TRUSS SJ = SINGLE JOIST FT = FLOOR TRUSS FI = FLOOR INUSS
DR = DOUBLE RAFTER
TR = TRIPLE RAFTER
OC = ON CENTER
PLFA = POINT LOAD FROM ABOVE
NTS = NOT TO SCALE
UNO = UNLESS NOTED OTHERWISE
UN EXECUTED. SC = STUD COLUMN
EE = EACH END
TJ = TRIPLE JOIST
CL = CENTERLINE COL = COLUMN PT = PRESSURE TREATED = JACK STUD CONT = CONTINUOUS MANUE = MANUEACTURER

#J # OF JACK STUDS STUD COLUMN ■ POINT LOAD FROM ABOVE LOAD BEARING WALL

NON LOAD BEARING WALL

IEADER	SCHEDULE:

LABEL	SIZE
A	2x6 W/ (1) JACK STUD E.E.*
В	2x8 W/ (2) JACK STUDS E.E.*
С	2x10 W/ (2) JACK STUDS E.E.*
D	2x12 W/ (2) JACK STUDS E.E.*
E	9-1/4" LVL W/ (3) JACK STUDS E.E.*
F	11-7/8" LVL W/ (3) JACK STUDS E.E. *

*THE AMOUNT OF PLYS FOR THE HEADER IS DETERMINED BY THE WIDTH OF THE WALL (2X4 WALL=2 PLYS, 2X6 WALL=3 PLYS, ETC.). AMOUNT OF JACK STUDS SHOWN ON PLAN TAKE PRECEDENCE OVER TABLE

HEADER SPAN	MINIMUM KING STUDS E.E.
3'-0" OR LESS	(1)
3'-0" TO 6'-0"	(2)
6'-0" TO 9'-0"	(3)
9'-0" TO 12'-0"	(4)
12'-0" TO 16'-0"	(6)

- ALL STRUCTURAL LOAD BEARING WALLS SHALL BE CONSTRUCTED OUT OF 2X4 OR 2X6 STUDS AT 16" O.C. U.N.O. FOR UP TO 10' WALLS
- ALL NON LOAD BEARING WALLS SHALL BE CONSTRUCTED OUT OF 2X4 OR 2X6 STUDS AT 24" O.C. U.N.O. FOR UP TO 10' WALLS
- BALLOON FRAMED WALLS SHALL BE CONSTRUCTED WITH 2X4 STUDS AT 12" O.C. OR 2X6 STUDS AT 16" O.C. WITH CROSS BRACING AT 6'-0" O.C. VERTICALLY OR ACCORDING TO THE CHART BELOW:

STUD SIZE	SPACING
2X4	12" O.C.
2X6	16" O.C.
(2) 2X4/2X6	12" O.C./12" O.C.
(2) 2X6/2X8	16" O.C./12" O.C.
(2) 2X6	12" O.C.
	2X4 2X6 (2) 2X4/2X6 (2) 2X6/2X8

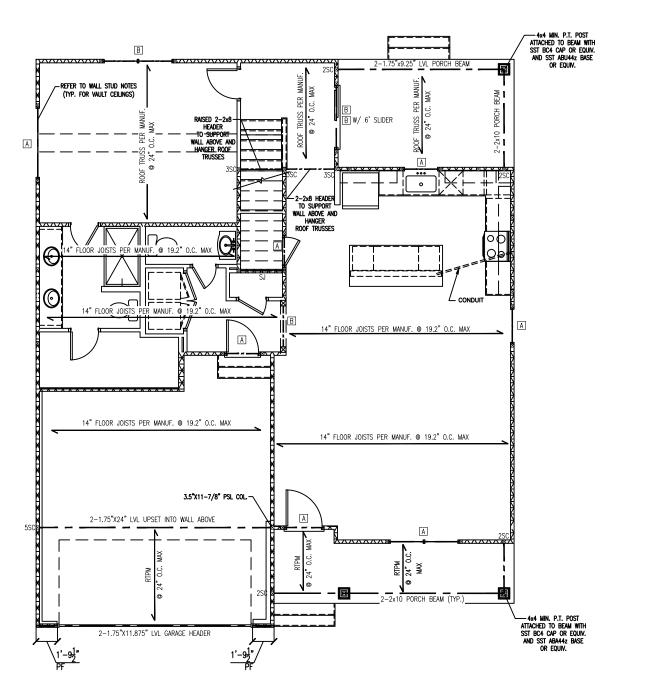
BRICK	LINTEL	SCHEDUL

SPAN	HEIGHT OF BRICK	LINTEL
3'-0" OR LESS	20' MAX	L3"x3"x1/4"
3'-0" TO 6'-0"	6' MAX 12' MAX 20' MAX	L3"x3"x1/4" L4"x3"x1/4" L5"x3-1/2"x5/16"
6'-0" TO 12'-0"	6' MAX 12' MAX	L5"x3-1/2"x5/16" L6x3-1/2"x5/16"
12'-0" TO 16'-0"	12' MAX	L8"x4"x1/2"

ATTACH ALL LINTELS TO THE SUPPORTING HEADER WITH (2) ROWS OF MINIMUM 3.5" LONG 1/2" DIAMETER LAG SCREWS AT 16" O.C. ENDS OF LINTEL SHALL BEAR AT LEAST 3.5" IN THE ADJACENT BRICI

NOTE: WALL BRACING HAS BEEN ANALYZED USING CS-WSP PER SECTION R602.10 OF THE 2018 NCRC. MIXED METHODS PER TABLE R602.10.1 ARE DESIGNATED ON THE PLAN.

NOTE: FLOOR JOISTS MAY INCLUDE FLOOR TRUSSES OR I-JOISTS, AS CONTRACTOR DESIRES.



TYPICAL	HANGERS FOR JOIS	ST & BEAMS
	-SIMPSON-	-USP-
MEMBERS	HANGER	HANGER
2x8	LUS28	JUS28
2x10	LUS210	JUS210
2x12	LUS210	JUS210
2-2x8	HUS28-2	JUS28-2
2-2x10	HUS210-2	JUS210-2 JUS212-2
2-2x12 3-2x8	HUS212-2 LUS28-3	JUS212-2 JUS28-3
3-2x0 3-2x10	LUS210-3	JUS210-3
3-2x10 3-2x12	HU212-3 MIN.	JUS212-3 MIN.
2-134"x914" LVL	HGUS410	THDH410
2-134"x95" LVL	HGUS410	THDH410
2-134"x1114" LVL	HGUS412	THDH412
2-134"x1134" LVL	HGUS412	THDH412
2-134"x14" LVL	HGUS414	THDH414
2-134"x16" LVL	HGUS414	THDH414
2-134"x18" LVL	HGUS414	THDH414
2-134"x24" LVL	HGUS414	THDH414
3-134"x914" LVL	HGUS5,50/10	THDH610
3-134"x9½" LVL	HGUS5.50/10	THDH610
3-134"x1114" LVL	HGUS5.50/12	THDH612
3-134"x1174" LVL	HGUS5.50/12	THDH612
3-134"x14" LVL	HGUS5.50/14	THDH614
3-134"x16" LVL	HGUS5.50/14	THDH614
3-134"x18" LVL	HGUS5.50/14	THDH614
3-134"x24" LVL	HGUS5.50/14	THDH614
4-134"x914" LVL	HGUS7.25/10	THDH7210
4-134"x91/4" LVL	HGUS7.25/10	THDH7210
4-134"x1114" LVL	HGUS7 25/12	THDH7212
4-134"x1134" LVL	HGUS7.25/12 HGUS7.25/12	THDH7212
4-134"x14" LVL	HGUS7.25/14	THDH7214
4-134"x16" LVL	HGUS7.25/14	THDH7214
4-134"x18" LVL	HGUS7.25/14	THDH7214
4-1/4 X10 LVL	110037.23/ 14	1110117214

NOTE: ALL HANGERS BY SIMPSON STRONG TIE CO., INC. (BRAND - NAME EQUIVALENTS ACCEPTABLE)

TRUSS UPLIFT CONNECTOR SCHEDULE

MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND
600 LBS	H2.5A	PER WALL SHEATHING	& FASTENERS
1200 LBS	(2) H2.5A	CS16 (END = 11")	DTT2Z
1450 LBS	HTS20	CS16 (END = 11")	DTT2Z
2000 LBS	(2) MTS20	(2) CS16 (END = 11")	DTT2Z
2900 LBS	(2) HTS20	(2) CS16 (END = 11")	HTT4
3685 LBS	LGT3-SDS2.5	MSTC52	HTT4
A ALL DOODLOTS LISTED ADE SINDSON STRONG TIE FOLIMALENT DOODLOTS			

1. ALL PRODUCTS LISTED ARE SIMPSON STRONG—TIE. EQUIVALENT PRODUCTS MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.

AN 16 O SEE PER MANORACIONES SPECIFICATIONS.

2. UPUIFT VALUES LISTED ARE FOR SPF #2 GRADE MEMBERS.

3. REFER TO TRUSS LAYOUT PER MANUF. FOR UPUIFT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTORS SPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE.
4. CONTACT QC FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE

LISTED ABOVE.

WALL BRACING LEGEND:

◆ ATTACH (2) 2X4 STUD COLUMN TO FOUNDATION WITH SST LTTP2 HOLD DOWN, OR EQUIVALENT HARDWARE.

BRACED WALL

WALL BRACING NOTES

- REFER TO COVER PAGE FOR ADDITIONAL NOTES.
 BRACING DESIGN CONFORMS TO THE 2018 NCRC AND ALL LOCAL AMENDMENTS
 FOR A MAXIMUM WIND SPEED OF 130 MPH AND SEISMIC ZONES A-C
- WALL BRACING HAS BEEN ANALYZED PER SECTION REQ2.10 OF THE 2018 NCRC.
 CS-WSP IS THE COMMON BRACING METHOD USED, WHERE APPLICABLE. MIXED
 METHODS, OTHER THAN CS-WSP, SHOWN WITHIN TABLE RE02.10.1 ARE
- DESIGNATED ON THE PLAN.
 ALL BRACING COMPONENTS SHALL COMPLY TO SECTION R602.10.1 OF THE 2018
- MINIMUM PANEL LENGTH SHALL BE 24" OR THE MINIMUM AS STATED IN R602.10.1 OF THE 2018 NCRC.
- BRACED WALL PANELS SHALL BE WITHIN 12'-0" FROM THE ENDS OF A BRACED WALL LINE AND SPACED NO GREATER THAN 21'. INTERIOR OF EXTERIOR BRACED WALLS SHALL BE SHEATHED CONTINUOUSLY WITH
- 1/2" THICK GYPSUM, U.N.O. HOLD DOWNS SHALL BE INSTALLED FOR BRACED WALL END CONDITIONS PER
- SECTION R602.10.4 AND FIGURE R602.10.3(3) OF THE 2018 NCRC.
 REFER TO THE CHART BELOW FOR BRACED WALL METHODS AND CONNECTIONS.

	METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION
	CONTINUOUS SHEATHING WOOD STRUCTURAL PANEL (CS-WSP)	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS AT 6" O.C. ON EDGE AND 12" O.C. ON FIELD
	GYPSUM BOARD (GB)	GYPSUM BOARD	1/2"	5d COOLER NAILS AT 7" O.C. ON EDGE AND FIELD
	WOOD STRUCTURAL PANEL (WSP)	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS AT 6" O.C. ON EDGE AND 12" O.C. ON FIELD
	PORTAL FRAME (PF)	WOOD STRUCTURAL	7/16"	SEE DETAIL 1/D-1f

- REFER TO COVER PAGE FOR ADDITIONAL WOOD FRAMING NOTES
- ALL ROOF TRUSSES SHALL BE ATTACHED TO WALL PLATES WITH MINIMUM (1) SIMPSON H2.5A OR PER MANUFACTURER'S INSTRUCTIONS OR PER SECTION R802.11 OF THE 2018 NCRC, WHICHEVER IS GREATER.
- ROOF TRUSSES ARE TO BE INSTALLED PER SECTION R802.10 IN THE 2018 NCRC

PANEL

- ROOF TRUSSES SHALL BE BRACED PER THE MANUFACTURER'S INSTRUCTIONS AND PER THE SBCA BUILDING COMPONENT SAFETY INFORMATION (BCSI) GUIDE TO GOOD PRACTICE FOR HANDING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.
- DO NOT CUT OR ALTER ROOF TRUSSES
- ROOF TRUSS MANUFACTURER SHALL VERIFY AND DESIGN FOR POSITION OF PULL DOWN STAIRS AND ATTIC PLATFORM.
- WHERE TRUSS HEELS EXCEED 9-1/4" AND ARE LOCATED OVER BRACED WALL PANELS AS SHOWN ON THE PLANS, BLOCKING SHALL BE INSTALLED PER SECTION R602.10.5 OF THE 2018 NCRC.







PLAN .00R SHEET NAME: FIRST FLC FRAMING

CLIENT: LGI HOMES

PLAN NAME: ROLEN-LH VERSION

NEIGHBORHOOD:

LOT AND ADDRESS: LOT # TBD

PROJECT NUMBER: LGIXXXX

EO & MSB

1.17.2025

SCALE: 1/4"=1'-0" ON 22"x34" 1/8"=1'-0" ON 11"x17"

PAGE:

S-1.1

- REFER TO COVER PAGE FOR ADDITIONAL NOTES
 ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH
- CAROLINA RESIDENTIAL CODE (NCRC), 2018 EDITION, PLUS ALL LOCAL CODES AND
- CAROUNA RESIDES.

 THE EOR SHALL REVIEW EWP AND TRUSS LAYOUTS FOR ACCURACY PRIOR TO CONSTRUCTION.

 SOLD BLOCKING IS TO BE INSTALLD AT ALL POINT LOADS THROUGH FLOOR LEVELS TO THE FOUNDATION OR TO THE REAREST STRUCTURAL ELEMENT. BLOCKING SHALL BE EQUAL TO OR GREATER THAN THE SUPPORT ABOVE.
- GREATER HAN THE SUPPORT ABOVE.

 BUILT-UP WOOD COLUMNS CONSISTING OF MULTIPLE STUDS SHALL HAVE EACH LAMINATION
 NAILED WITH 16D NAILS SPACED AT 9" O.C. FOR BUILT-UP COLUMNS CONSISTING OF (4)
 PLIES OR MORE, SECURE PLIES TOGETHER WITH HORIZONTAL SIMPSON CS-16 COIL STRAPS LOCATED AT QUARTER POINTS.
 CONTRACTOR SHALL ENSURE THAT ALL BEAMS, HEADERS, AND STRUCTURAL COMPONENTS
- CONINACION SHALL ENSORE IHAI ALL BEAMS, HEADERS, AND STRUCTURAL COMPONENTS ARE FULLY BEAMING ON THE SUPPORTING MEMBERS. ANY GAPS IN THE FRAMING SHALL BE SHIMMED APPROPRIATELY WITH EITHER METAL SHIMS OF WOOD SHIMS AS NECESSARY. HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. ANY HEADERS INSTALLED THAT ARE CREATER IN SIZE ARE AN ADEQUATE REPLACEMENTS PROVIDED THE MATERIAL IS OF THE SAME OR GREATER STRUCTURAL PROPERTIES.
- WHERE TOP PLATE HAS BEEN CUT TO ACCOMMODATE FLUSH HEADERS/BEAMS, INSTALL A MINIMUM 16" LONG HORIZONTAL CS-16 STRAP EXTENDING 12" PAST THE BREAK ON EACH
- SIDE.
 UNILESS OTHERWISE NOTED, FOUR-PLY LVL BEAMS SHALL HAVE PLIES FASTENED TOGETHER
 WITH TWO (2) ROWS OF 1/2" DIAMETER BOLTS SPACED AT 16" O.C. THE BOLTS SHALL BE LOCATED A MINIMUM OF 2-1/2" AND A MAXIMUM OF 3-1/2" FROM THE TOP AND BOTTOM
- OF THE BEAM.
 ALL LOAD BEARING WALLS TO BE 2X4 U.N.O.

ABBREVIATIONS: DJ = DOUBLE JOIST GT = GIRDER TRUSS SJ = SINGLE JOIST FT = FLOOR TRUSS FI = FLOOR INUSS
DR = DOUBLE RAFTER
TR = TRIPLE RAFTER
OC = ON CENTER
PLFA = POINT LOAD FROM ABOVE
NTS = NOT TO SCALE
UNO = UNLESS NOTED OTHERWISE
UN EXECUTED. SC = STUD COLUMN EE = EACH END COL = COLUMN PT = PRESSURE TREATED = JACK STUD MANUE = MANUEACTURER CONT = CONTINUOUS

LEGEND:

#J # OF JACK STUDS STUD COLUMN ■ POINT LOAD FROM ABOVE

LOAD BEARING WALL

NON LOAD BEARING WALL

HEADER SCHEDULE:	
LABEL	SIZE
A	2x6 W/ (1) JACK STUD E.E.*
В	2x8 W/ (2) JACK STUDS E.E.*
C	2x10 W/ (2) JACK STUDS E.E.*
D	2x12 W/ (2) JACK STUDS E.E.*
E	9-1/4" LVL W/ (3) JACK STUDS E.E.*
F	11-7/8" LVL W/ (3) JACK STUDS E.E. *

*THE AMOUNT OF PLYS FOR THE HEADER IS DETERMINED BY THE WIDTH OF THE WALL (2X4 WALL=2 PLYS, 2X6 WALL=3 PLYS, ETC.). AMOUNT OF JACK STUDS SHOWN ON PLAN TAKE PRECEDENCE OVER TABLE

HEADER SPAN	MINIMUM KING STUDS E.E.
3'-0" OR LESS	(1)
3'-0" TO 6'-0"	(2)
6'-0" TO 9'-0"	(3)
9'-0" TO 12'-0"	(4)
12'-0" TO 16'-0"	(6)

WALL STUD NOTES:

- ALL STRUCTURAL LOAD BEARING WALLS SHALL BE CONSTRUCTED OUT OF 2X4 OR 2X6 STUDS AT 16" O.C. U.N.O. FOR UP TO 10' WALLS ALL NON LOAD BEARING WALLS SHALL BE CONSTRUCTED OUT OF 2X4 OR 2X6
- STUDS AT 24" O.C. U.N.O. FOR UP TO 10' WALLS
- BALLOON FRAMED WALLS SHALL BE CONSTRUCTED WITH 2X4 STUDS AT 12" O.C. OR 2X6 STUDS AT 16" O.C. WITH CROSS BRACING AT 6'-0" O.C. VERTICALLY OR ACCORDING TO THE CHART BELOW:

HEIGHT (PLATE TO PLATE)	STUD SIZE	SPACING
12'-0"	2X4	12" O.C.
15'-0"	2X6	16" O.C.
17'-0"	(2) 2X4/2X6	12" 0.C./12" 0.C.
21'-0"	(2) 2X6/2X8	16" 0.C./12" 0.C.
25'-0"	(2) 2X6	12" O.C.

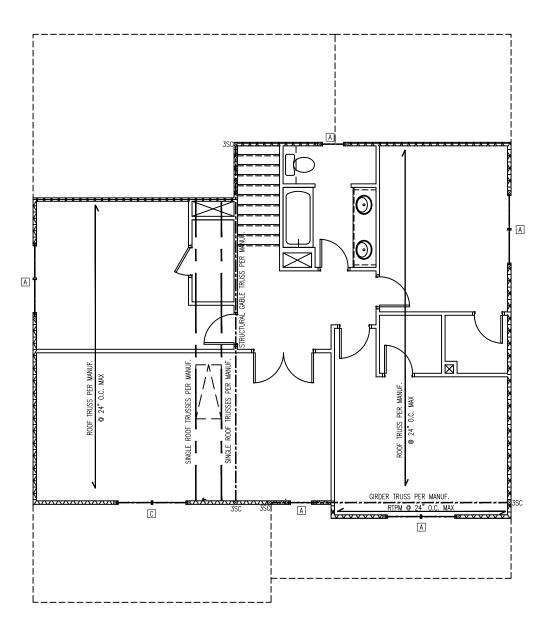
BRICK	LINTEL	SCHEDUL

SPAN	HEIGHT OF BRICK	LINTEL
3'-0" OR LESS	20' MAX	L3"x3"x1/4"
3'-0" TO 6'-0"	6' MAX 12' MAX 20' MAX	L3"x3"x1/4" L4"x3"x1/4" L5"x3-1/2"x5/16"
6'-0" TO 12'-0"	6' MAX 12' MAX	L5"x3-1/2"x5/16" L6x3-1/2"x5/16"
12'-0" TO 16'-0"	12' MAX	L8"x4"x1/2"

ATTACH ALL LINTELS TO THE SUPPORTING HEADER WITH (2) ROWS O MINIMUM 3.5" LONG 1/2" DIAMETER LAG SCREWS AT 16" O.C. ENDS OF LINTEL SHALL BEAR AT LEAST 3.5" IN THE ADJACENT BRICK

NOTE: WALL BRACING HAS BEEN ANALYZED USING CS-WSP PER SECTION R602.10 OF THE 2018 NCRC. MIXED METHODS PER TABLE R602.10.1 ARE DESIGNATED ON THE PLAN.

NOTE: FLOOR JOISTS MAY INCLUDE FLOOR TRUSSES OR I-JOISTS, AS CONTRACTOR DESIRES.



TYPICAL	HANGERS FOR JOIS	ST & BEAMS
	-SIMPSON-	-USP-
MEMBERS	HANGER	HANGER
2x8	LUS28	JUS28
2x10	LUS210	JUS210
2x12	LUS210	JUS210
2-2x8	HUS28-2	JUS28-2
2-2x10	HUS210-2 HUS212-2 LUS28-3 LUS210-3	JUS210-2
2-2x12	HUS212-2	JUS212-2
3-2x8 3-2x10	LUS28-3 LUS210-3	JUS28-3 JUS210-3
3-2x10 3-2x12	HU212-3 MIN.	JUS210-3 MIN.
2-134"x914" LVL	HGUS410	THDH410
2-134"x9%" LVL	HGUS410	THDH410
2-134"x1114" LVL	HGUS412	THDH410
2-134"x1134" LVL	HGUS412	THDH412
2-134 x1128 LVL 2-134 x14" LVL	HGUS414	THDH414
2-134 x14 LVL 2-134 x16" LVL	HGUS414	THDH414
2-134"x18" LVL 2-134"x18" LVL	HGUS414 HGUS414	THDH414
2-134"x24" LVL	HGUS414 HGUS414	THDH414
		THDH414
3-1¾"x9¼" LVL	HGUS5.50/10	
3-1¾"x9½" LVL	HGUS5.50/10	
3-1% X1 1/4 LVL	HGUS5.50/12	THDH612
3-1% x11% LVL	HGUS5.50/12	THDH612
3-1% x14" LVL	HGUS5.50/14	THDH614
3-1% x16" LVL	HGUS5.50/14	THDH614
3-1% x18" LVL	HGUS5.50/14	THDH614
3-1% x24" LVL	HGUS5.50/14	THDH614
4-1% x9% LVL	HGUS7.25/10	THDH7210
4-1% x9% LVL	HGUS7.25/10	THDH7210
4-1½"x11½" LVL	HGUS7.25/12	THDH7212
3-1½ 11½ 11. 3-1½ 11½ 11. 3-1½ 11½ 11. 3-1½ 116 1. 3-1½ 116 1. 3-1½ 116 1. 4-1½ 11½ 1.	HGUS7.25/12	THDH7212
4-1¾"x14" LVL	HGUS7.25/14	THDH7214
4-1¾"x16" LVL	HGUS7.25/14	THDH7214
4-1¾"x18" LVL	HGUS7.25/14	THDH7214

NOTE: ALL HANGERS BY SIMPSON STRONG TIE CO., INC. (BRAND - NAME EQUIVALENTS ACCEPTABLE)

TRUSS UPLIFT CONNECTOR SCHEDULE

MAX. UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND
600 LBS	H2.5A	PER WALL SHEATHING	& FASTENERS
1200 LBS	(2) H2.5A	CS16 (END = 11")	DTT2Z
1450 LBS	HTS20	CS16 (END = 11")	DTT2Z
2000 LBS	(2) MTS20	(2) CS16 (END = 11")	DTT2Z
2900 LBS	(2) HTS20	(2) CS16 (END = 11")	HTT4
3685 LBS	LGT3-SDS2.5	MSTC52	HTT4
1 ALL DOOD	ICTC LICTED ADE CIN	DOON CTDONG TIE FOLINA	ENT DRODUCTO

MAY BE USED PER MANUFACTURER'S SPECIFICATIONS.

VANT DE OSCUP FIX MANORACIONAS SPECIFICATIONS.

2. UPULIT VALUES LISTED ARE FOR SPF #2 GRADE MEMBERS.

3. REFER TO TRUSS LAYOUT PER MANUF. FOR UPULIT VALUES AND TRUSS TO TRUSS CONNECTIONS. CONNECTORS SPECIFIED BY TRUSS MANUFACTURER

OVERRIDE THOSE LISTED ABOVE.
4. CONTACT QC FOR REQUIRED CONNECTORS WHEN LOADS EXCEED THOSE LISTED ABOVE.

WALL BRACING LEGEND:

↑ ATTACH (2) 2X4 STUD COLUMN TO FRAMING BELOW WITH (2) 30" SST CS16 STRAPS, OR EQUIV. HARDWARE. CENTERED ON THE FLOOR SYSTEM BELOW

BRACED WALL

- REFER TO COVER PAGE FOR ADDITIONAL NOTES.
 BRACING DESIGN CONFORMS TO THE 2018 NCRC AND ALL LOCAL AMENDMENTS
 FOR A MAXIMUM WIND SPEED OF 130 MPH AND SEISMIC ZONES A-C
- WALL BRACING HAS BEEN ANALYZED PER SECTION R602.10 OF THE 2018 NCRC.
 CS-WSP IS THE COMMON BRACING METHOD USED, WHERE APPLICABLE. MIXED
 METHODS, OTHER THAN CS-WSP, SHOWN WITHIN TABLE R602.10.1 ARE
- ALL BRACING COMPONENTS SHALL COMPLY TO SECTION R602.10.1 OF THE 2018
- MINIMUM PANEL LENGTH SHALL BE 24" OR THE MINIMUM AS STATED IN R602.10.1 OF THE 2018 NCRC.
- BRACED WALL PANELS SHALL BE WITHIN 12'-0" FROM THE ENDS OF A BRACED WALL LINE AND SPACED NO GREATER THAN 21'. INTERIOR OF EXTERIOR BRACED WALLS SHALL BE SHEATHED CONTINUOUSLY WITH
- 1/2" THICK GYPSUM, U.N.O. HOLD DOWNS SHALL BE INSTALLED FOR BRACED WALL END CONDITIONS PER
- SECTION R602.10.4 AND FIGURE R602.10.3(3) OF THE 2018 NCRC.
 REFER TO THE CHART BELOW FOR BRACED WALL METHODS AND CONNECTIONS.

METHOD	MATERIAL	MIN. THICKNESS	REQUIRED CONNECTION
CONTINUOUS SHEATHING WOOD STRUCTURAL PANEL (CS-WSP)	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS AT 6" O.C. ON EDGE AND 12" O.C. ON FIELD
GYPSUM BOARD (GB)	GYPSUM BOARD	1/2*	5d COOLER NAILS AT 7" O.C. ON EDGE AND FIELD
WOOD STRUCTURAL PANEL (WSP)	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS AT 6" O.C. ON EDGE AND 12" O.C. ON FIELD
PORTAL FRAME (PF)	WOOD STRUCTURAL PANEL	7/16"	SEE DETAIL 1/D-1f

- REFER TO COVER PAGE FOR ADDITIONAL WOOD FRAMING NOTES.
- ALL ROOF TRUSSES SHALL BE ATTACHED TO WALL PLATES WITH MINIMUM (1) SIMPSON H2.5A OR PER MANUFACTURER'S INSTRUCTIONS OR PER SECTION R802.11 OF THE 2018 NCRC, WHICHEVER IS GREATER.
- ROOF TRUSSES ARE TO BE INSTALLED PER SECTION R802.10 IN THE 2018 NCRC ROOF TRUSSES SHALL BE BRACED PER THE MANUFACTURER'S INSTRUCTIONS AND PER THE SBCA BUILDING COMPONENT SAFETY INFORMATION (BCSI) GUIDE TO GOOD PRACTICE FOR HANDING,
 - INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES. DO NOT CUT OR ALTER ROOF TRUSSES
 - ROOF TRUSS MANUFACTURER SHALL VERIFY AND DESIGN FOR POSITION OF PULL DOWN STAIRS
- WHERE TRUSS HEELS EXCEED 9-1/4" AND ARE LOCATED OVER BRACED WALL PANELS AS SHOWN ON THE PLANS, BLOCKING SHALL BE INSTALLED PER SECTION R602.10.5 OF THE 2018 NCRC.







FLOOR PLAN SECOND SEC FRA

CLIENT: LGL HOMES

PLAN NAME: ROLEN-LH VERSION

NEIGHBORHOOD: TRD

LOT AND ADDRESS: LOT # TBD

PROJECT NUMBER: LGIXXXX

DRAWN BY: EO & MSB

DATE: 01.17.2025

SCALE: 1/4"=1'-0" ON 22"x34" 1/8"=1'-0" ON 11"x17"

PAGE:

S-2.1

GENERAL STRUCTURAL NOTES:

- These drawings and its contents are the property of Queen City Consulting and Design, PLLC, (QC) and the client as noted on this page. Distribution to any other parties for purposes other than those directly concerned with the titled project without prior written consent from QC is strictly prohibited.
- The engineer's name present on the seal of these drawings is the engineer of record (EOR).
- Details noted as "Typical" shall be used whenever applicable. Refer to specifications for information not covered by these notes or drawings.
- 4. It is the responsibility of the contractor to verify all dimensions prior to construction. Furthermore, QC will not be held responsible for the
- contractor's failure to conform to the construction documents, including this structural set, should any non-conformities occur. The contractor shall assume sole and complete responsibility for job site conditions during the course of construction of this project, including
- 6. Any omissions and conflicts between the various elements of the structural drawings and/or specifications shall be brought to the attention of, and
- resolved with, the engineer before proceeding with any work so involved. 7. All construction shall conform to the latest requirements of the North Carolina Residential Code (NCRC), 2018 Edition, plus all local codes and regulations.
- 8. Seismic design shall be per section R301.2.2 of the 2018 NCRC and is based off of local seismic design categories.

FOOTING AND FOUNDATION NOTES:

- Foundation Design is based on a minimum allowing bearing capacity of 2,000 PSF. Contact the EOR if bearing capacity is not achieved.

 No excavation shall occur within a 45 degree line projected from the bottom of the building foundation is permitted, unless it is specifically
- 3. The bottom of all footings shall extend below the frost line for the region, as specified by the local municipality. However, the bottom of all footings shall be a minimum of 12" below grade.
- 4. Contractor to ensure that all drainage is directed away from the exterior footings (Min. 2% slope).
- 5. Excavations of footings shall be temporarily protected with a 10 mil polyethylene membrane if concrete is not placed within 24 hours of
- 6. Do not place concrete or other cementitious materials against subgrade with any deleterious materials present, including but not limited to: water, ice, frost, or loose material.
- 7. All footings are to have minimum 2" projection on each side of foundation walls (except for monolithic slab foundations).

- Poured concrete is to have a minimum compressive strength of 3000 psi at 28 days. Aggregates for normal weight concrete shall conform to ASTM C33.
- All materials used for concrete shall conform to ACI 318, ACI 301, or ASTM C1157.
- The placing of all concrete shall be in accordance with ACI 318 and ASTM C94 requirements
- Admixtures may be used with prior approval of the EOR. Admixtures shall comply with ASTM C494 and C1017. Concrete slabs-on-grade shall be constructed in a manner that complies with ACI 302.1R-96.
- Control or saw cut joints shall be cut to a minimum of 1/4 of the thickness of the respective concrete element. Control joints located within interior and exterior slabs-on-grade shall be spaced at a maximum of 12' O.C. Control joints shall comply with ACI 301.

- Bar reinforcement shall be conform to ASTM A615, grade 60 steel.
- The following minimum clear cover shall be provided over reinforcing bars:
 - Concrete exposed to earth
 - Concrete exposed to weather = 1-1/2
 - Slabs not exposed to weather = 3/4"
 - Concrete Beams & Columns = 1-1/2"
- 3. Brick and/or porous material shall not be used to support footing steel off the ground. Plastic rebar chairs or precast concrete dobies may be
- 4. Splices in reinforcing steel shall be a minimum of 45x the diameter, up to a #6 rebar. Rebar larger than #6 requires a minimum lap splice of 56x the diameter.
- 5. All concrete walls shall be doweled to their supporting footings, beams, pads, etc. with bars of the same size and spacing as the vertical bars located within the wall, unless otherwise noted. Anchorage of dowels shall be the equivalent of a bar splice.

GENERAL WOOD FRAMING:

- All wood framing members are designed to be Spruce-Pine-Fir (SPF) #2, unless otherwise noted on the plan. Grade marks shall be made by a recognized grading agency.
- 2. Framing members exposed to weather or in direct contact with soil, concrete, or masonry shall be pressure treated Spruce-Pine-Fir #2 and shall comply with the AWPA standard C-15.
- 3. All fasteners such as nails, bolts, screws, anchor bolts, etc. attaching pressure treated or fire-retardant treated wood shall be hot-dipped zinc coated galvanized or stainless steel (ASTM A153).
- 4. LVL engineered wood shall have the following minimum design values:
 - E = 1,900,000 psi Fb = 2600 psi
 - Fv = 285 psi Ft = 1555 psi = 1555 psi
- 5. PSL engineered wood shall have the following minimum design values:
- = 2,000,000 psi
 - Fb = 2900 psi
- Fv = 290 psi = 1755 psi
- 6. LSL engineered wood shall have the following minimum design values:
 - E = 1,550,000 psi Fb = 2250 psi 6.2.
 - Fv = 400 psi Ft = 1075 psi
- All bearing headers to be 2-2x6 supported with minimum (1) 2x4 jack stud and (1) 2x4 king stud at each end, unless noted otherwise on the plans. Non-load bearing headers shall be minimum 2-2x4.
- Solid blocking is to be installed at all point load through floor levels to the foundation or to the nearest structural element.
- All wood structural members that are specified are minimum sizes. Contractor may install larger sizes for ease of construction, if desired.
- 10. All nails shall be common nails, unless noted otherwise on plans and details.
- 11. All lag screws are to be predrilled. Drill diameter is to be 60 percent of the shank diameter. In addition, lag screws shall comply with ANSI/ASME standard B18.2.1-1981.
- 12. All bolt heads and nuts bearing on wood shall have standard cut washers. Holes for bolts shall be bored 1/16" larger than the nominal bolt
- Provide full bearing where all beams meet supporting framing members.
- 14. Unless otherwise noted on plans, size, height, and spacing of wood studs shall be in accordance with section R602.3.1 of the 2018 North Carolina Residential Code. Wood framed walls shall consist of Spruce—Pine—Fir No.2 graded material.
- 15. Unless otherwise noted, four-ply LVL beams shall have plies fastened together with two rows of 1/2" diameter bolts spaced at 16" o.c. The bolts shall be located a minimum of 2-1/2" and a maximum of 3-1/2" from the top of bottom of the beam.

ROOF FRAMING NOTES: 1. Truss Built Roofs

- 1.1. All roof trusses must be built in accordance with the truss manufacturer's requirements. Tie-down connections to resist uplift shall be installed where required. When roof truss manufacturers do not provide the required connectors, it is the responsibility of the contracto o notify the roof truss engineer or the EOR to provide an adequate connection.
- 1.2. Roof truss layouts are to be in compliance with the overall design specified on the plans. All deviations are to be brought to the attention of the EOR prior to installation.
- 1.3. Roof trusses shall be braced per the manufacturer's instructions and per the SBCA Building Component Safety Information (BCSI) Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Roof Trusses.
- 1.4. Provide 2x4 ladder framing spaced at 24" o.c. between adjacent roof trusses where false dormers are located
- 1.5. Install minimum 7/16" OSB roof sheathing.

using (3) 10d common nails.

- 1.6. Install roof trusses per section R802.10 in the 2018 NCRC. Where truss heels exceed 9-1/4" and are located over braced wall panels, blocking is to be installed per section R602.10.5 of the 2018 NCRC.
- 2. Stick Framed Roofs Collar ties shall be 2x6 spaced at 48" o.c. at all ridges unless noted otherwise and connected in the upper third of the attic space
 - Fur down all ridges as needed so that rafters have full contact. Ceiling joists when erected parallel to rafters must be sistered to rafters and secured as per table R802.5.1(a) of the 2018 North Carolina Residential Code
 - In addition to the NCRC fastener schedule, unless noted otherwise on the plan, roof members shall be tied down with additional metal connectors. Install a Simpson H2.5A connector at every rafter to fasten the lower end of the rafter to the top plate or beam below 2.5. Install minimum 7/16" OSB roof sheathing.



UEEN CITY CONSULTING AND DESIGN, PLLC

STRUCTURAL PLANS PREPARED FOR:

STANDARD DETAILS

PROJECT ADDRESS:

OWNER:

QUEEN CITY CONSULTING AND DESIGN, PLLC. 2459 WILKINSON BLVD SUITE 300 CHARLOTTE NC 28208

DESIGN SPECIFICATIONS

Construction Type: Residential

Applicable Building Codes:

 2018 North Carolina Residential Building Code with All Local Amendments ASCE 7-10: Minimum Design Loads for Buildings and Other Structures

Ultimate Design Wind Speed: 130MPH, EXPOSURE B

Assumed Soil Bearing Capacity: 2000psf

Component and Cladding loads shall be derived per Tables R301.2(2) and R301.2(3)

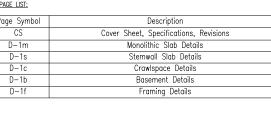
SEAL APPLIES TO STRUCTURAL ONLY

ENGINEERING SEAL APPLIES TO STRUCTURAL COMPONENTS ONLY. QC ASSUMES NO LIABILITY FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, SAFETY PRECAUTIONS, OR DEVIATIONS/DISCREPANCIES THAT MAY OCCUR IN THE PLAN. ANY DEVIATIONS OR DISCREPANCIES ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF QUEEN CITY CONSULTING AND DESIGN PLLC.

	LIVE LOADS
Roof 2x Conventional	20 PSF
Roof Truss	20 PSF
Attic Roof Truss	60 PSF
Floor Live Typ. Dwelling	40 PSF
Sleeping Areas	30 PSF
Decks	40 PSF
Passenger Vehicle Garage	50 PSF
Balconies	40 PSF
Attics with Storage	20 PSF
Attics without Storage	10 PSF
Ground Snow Load	15 PSF

	DEAD LOADS
Roof 2x Conventional	15 PSF
Roof Truss	20 PSF
Conventional 2x Floor	10 PSF
I-Joist	15 PSF
Floor Truss	15 PSF

PAGE LIST:	
Page Symbol	Description
CS	Cover Sheet, Specifications, Revisions
D-1m	Monolithic Slab Details
D-1s	Stemwall Slab Details
D-1c	Crawlspace Details
D-1b	Basement Details
D-1f	Framing Details



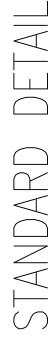
Revision No.	Date	Description
0	12.26.23	ORIGINAL ENGINEERING
1	05.05.24	Added Stem Wall Details
2	05.16.24	ADDED BRICK CRAWL SPACE DETAILS
3	08.30.24	ADDED STAIR DETAIL AND TURNDOWN DETAIL
4	12.11.24	ADDED BASEMENT DETAILS
'		



1.1.7.25

(T)

QUEEN CITY



CLIENT: IGL HOMES

DRAWN BY

SCALE: DETAILS ARE N.T.S

 $\sqrt{D-1}$ m $/_{N.T.S.}$

 $\sqrt{D-1}$ m/n.t.s. – Brick veneer and siding options shown above

QUEEN CITY





D-1m











CLIENT: LGI HOMES

LGI HOMES

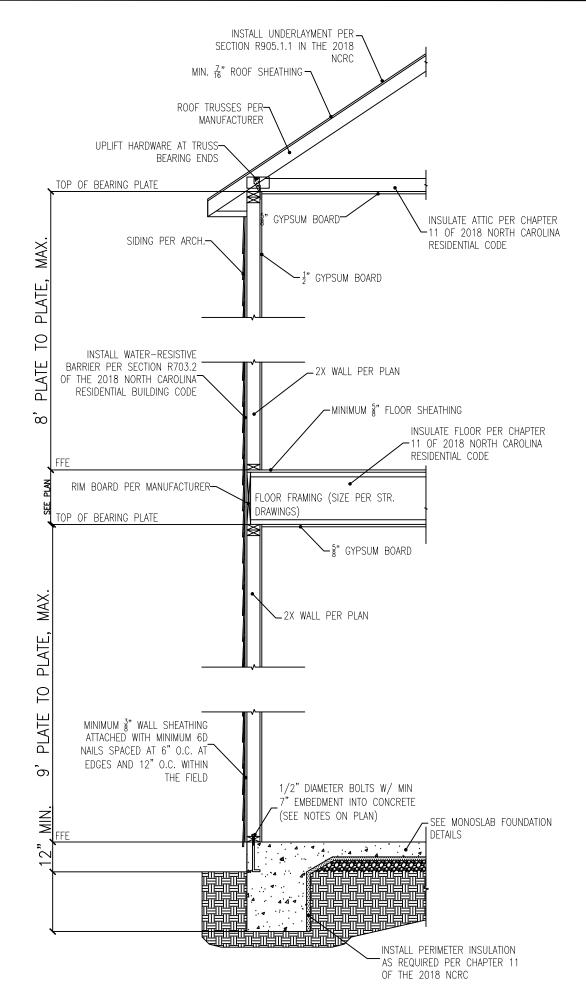
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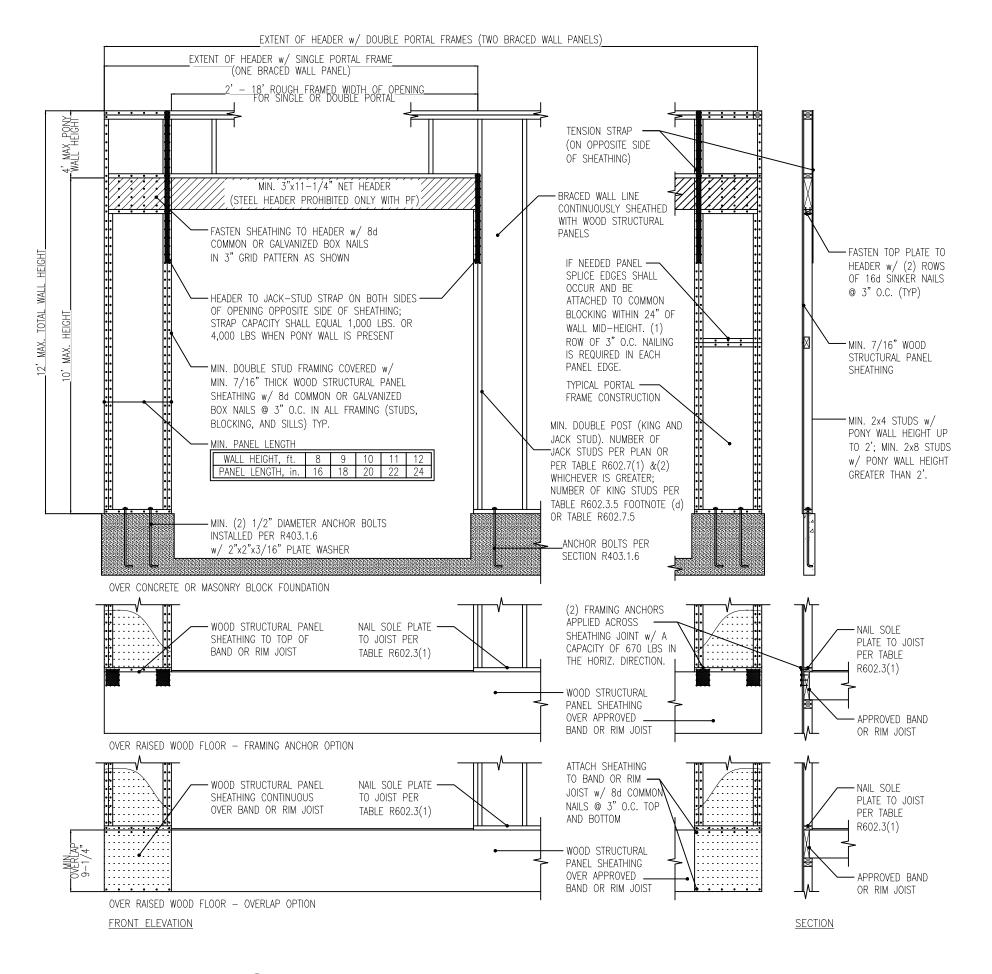
1/17/2025 SCALE: DETAILS ARE N.T.S.

DEIALS ALL

D-2m

1 TYP. EXTERIOR LOAD BEARING WALL PORTION OF STREET STREET













CLIENT: LGI HOMES

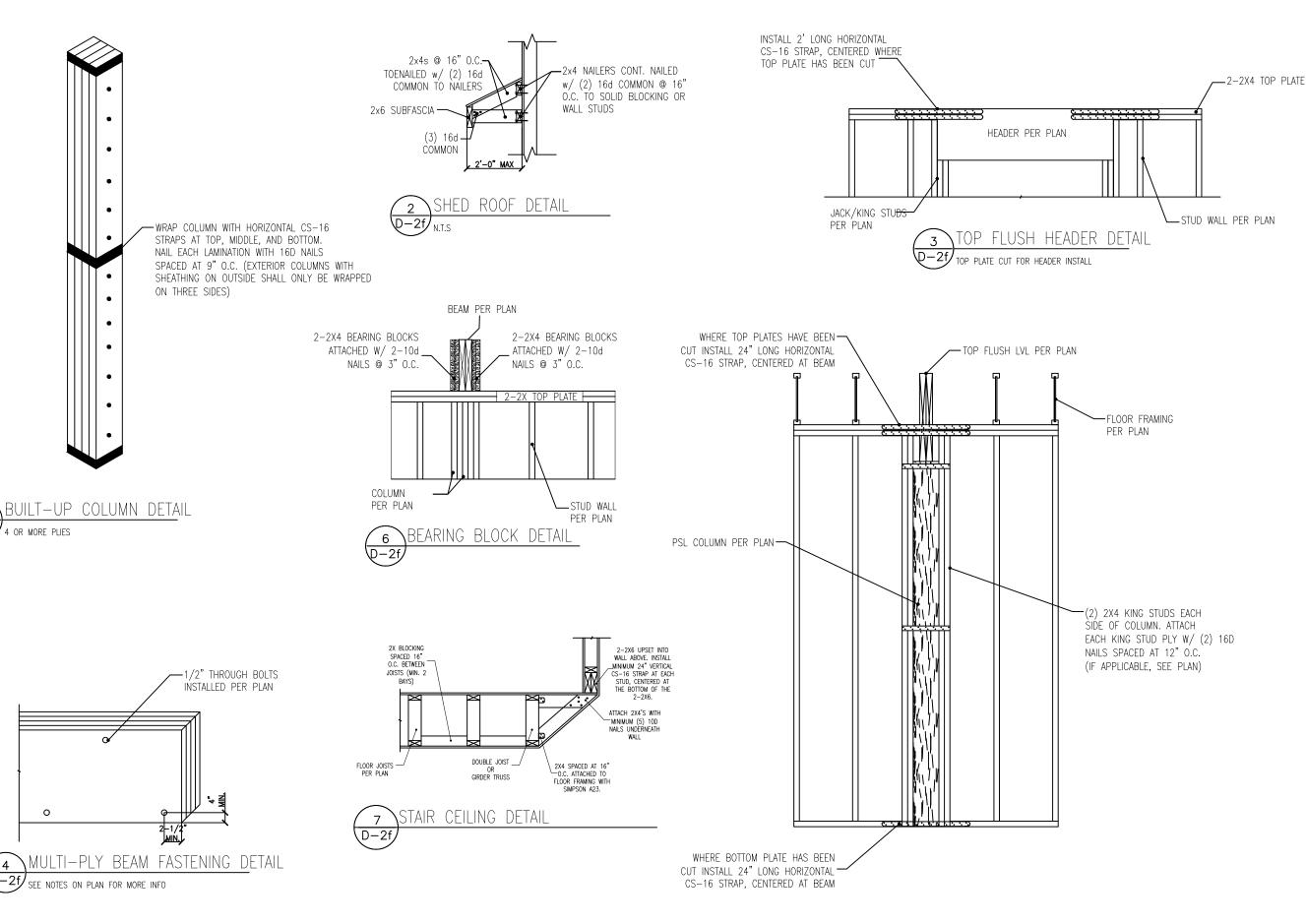
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DATE:

SCALE: DETAILS ARE N.T.S.

DACE:

D-1f











CLIENT: LGI HOMES

LGI HOMES

DRAWN BY:

CTB

DATE: 1/17/2025

SCALE: DETAILS ARE N.T.S.

PAGE:

D-2f