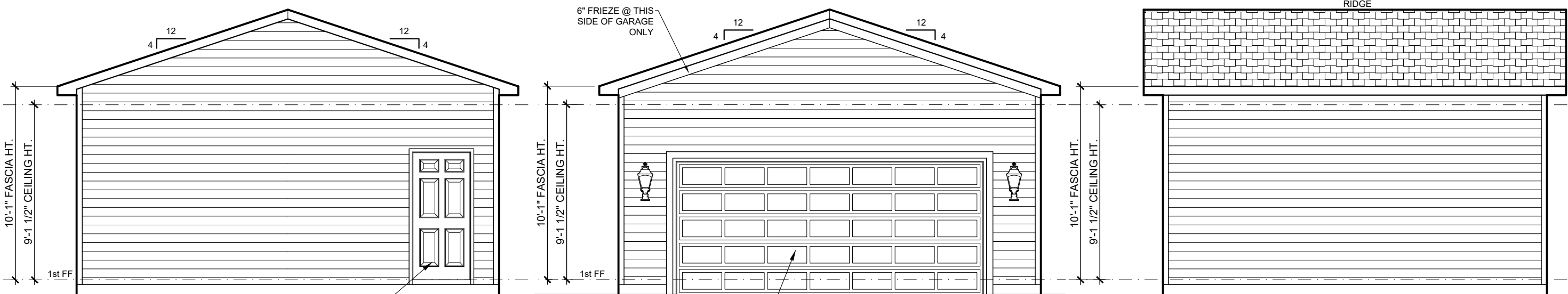


2-CAR GARAGE

ALL OVERHANGS 12" UNLESS OTHERWISE NOTED.



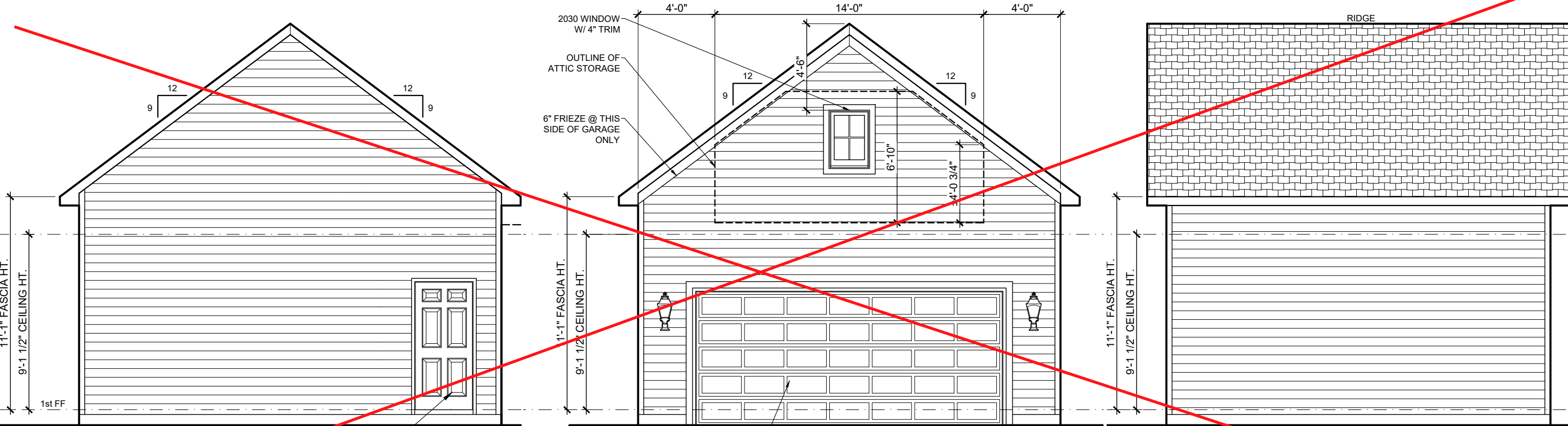
Square Footages:
 1-Car Garage 270 sf
 2-Car Garage 440 sf



REAR ELEVATION
 3/16" = 1'-0"

FRONT ELEVATION
 3/16" = 1'-0"

LEFT/RIGHT ELEVATION
 3/16" = 1'-0"



REAR ELEVATION W/ ATTIC OPTION
 3/16" = 1'-0"

FRONT ELEVATION W/ ATTIC OPTION
 3/16" = 1'-0"

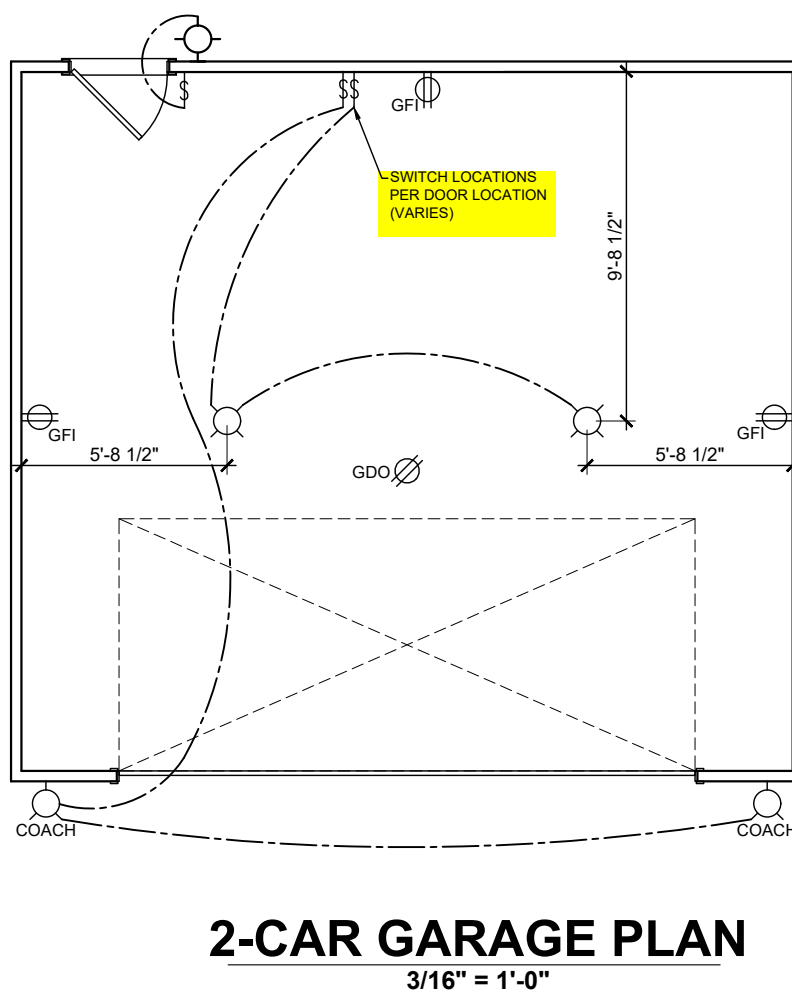
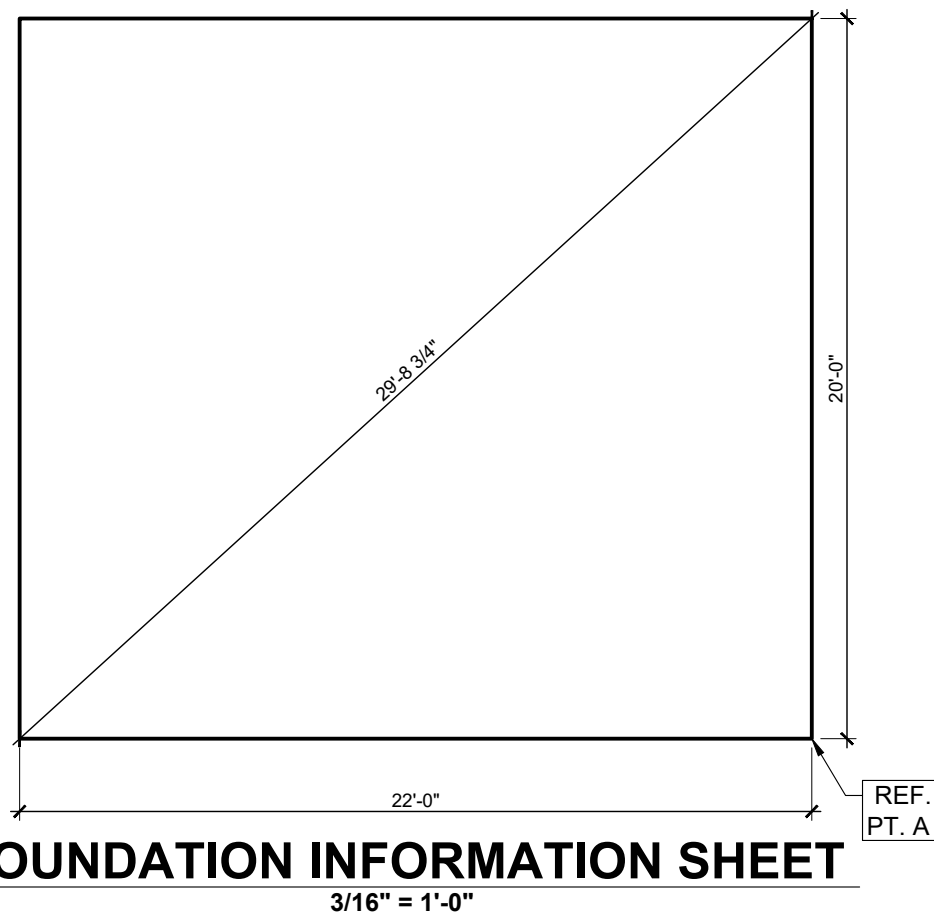
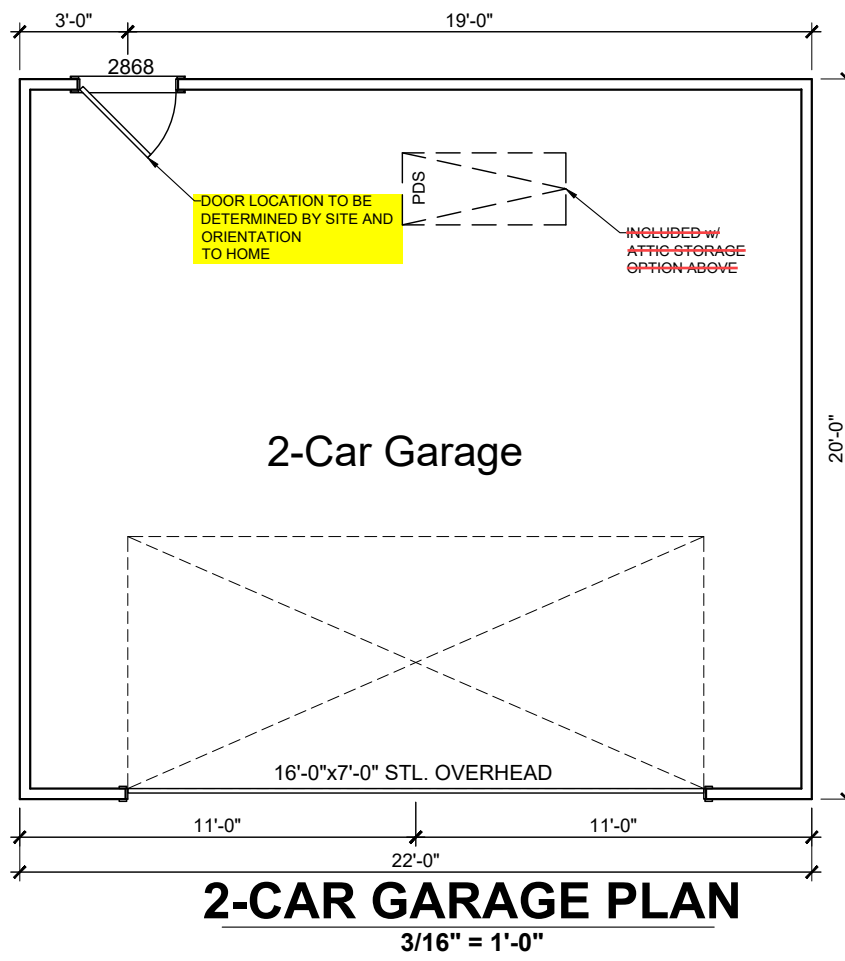
SEE SHEET A3 FOR ENHANCED OPTIONS.

LEFT/RIGHT ELEVATION W/ ATTIC OPTION
 3/16" = 1'-0"

Detached Garage 2-Car
 V.02.00.00.00
 REVERSAL

Features:
 Drawn By: ATW
 Rev By: am
 Date: 7/11/2022

A1



B2

Features:

Drawn By: ATW Rev By: am ar

Date: 7/11/2022

Detached Garage 2-Car
v.02.00.00.00

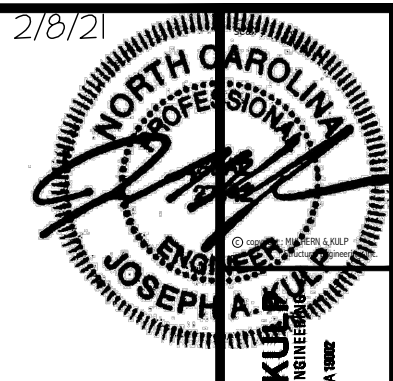
REVERSAL

Square Footages:

1-Car Garage273 sf

2-Car Garage.....440 sf





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CONNECTION SPECIFICATIONS (TYP. U.N.O.)

Table with 3 columns: DESCRIPTION OF BLDG. ELEMENT, 3"x0.131" NAILS, 3"x0.120" NAILS. Rows include JOIST TO SOLE PLATE, STUD TO SOLE PLATE, RIM TO TOP PLATE, RAFTER/TRUSS TO TOP PLATE, etc.

* 2 1/2"x0.113 IS AN ACCEPTABLE ALTERNATIVE TO A 3"x0.120", SAME SPACING OR NUMBER OF NAILS. (ONLY ACCEPTABLE WHERE * ARE SHOWN)

GENERAL STRUCTURAL NOTES

FOUNDATION

- DESIGN IS BASED ON 2018 NORTH CAROLINA STATE RESIDENTIAL CODE.
FOOTING DESIGN - 2,000 PSF ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED.
FASTEN 2x4/6 SILL PLATES TO CONC. FND WITH A MINIMUM OF 2 ANCHORS PER PLATE, 12" MAX. FROM PLATE ENDS - UTILIZING:
- 1/2" DIA. ANCHOR BOLTS @ 6'-0" O.C. W/ 1" MIN. EMBEDMENT OR SIMPSON MASA ANCHORS @ 6'-0" O.C.
- SIMPSON STRONG-BOLT 2 WEDGE ANCHOR (5TB2-50700 - 1/2" x 1") MAY BE USED IN PLACE OF ANCHOR BOLTS

EXTERIOR & SHEAR WALL SHEATHING SPECIFICATIONS

THIS MODEL HAS BEEN DESIGNED TO RESIST LATERAL FORCES RESULTING FROM: 115 MPH WIND IN 2018 NC5BC:RC (115 MPH WIND SPEED IN ASCE 7-10 WIND MAP, PER NC5BC R301.2.1.1) EXP. B & SEISMIC CAT. C.

THE ENGINEERED DESIGN WAS COMPLETED PER 2015 IBC (SECTION 1609) & ASCE 7-10, AS PERMITTED BY R301.3 OF THE 2018 NC5BC. DESIGN WIND UPLIFT LOADS HAVE BEEN CALCULATED UTILIZING ASCE 7-10 (ACCEPTED ENGINEERING PRACTICE) AS ALLOWED PER 2018 NC5BC:RC SECTION R602.11.1.1. THIS MODEL HAS BEEN DETAILED WHERE REQUIRED & ENGINEERED TO RESIST THE WIND UPLIFT LOAD PATH PER SECTIONS R602.3.5 & R602.11.

EXT. WALL SHEATHING SPECIFICATION

- 7/16" OSB OR 1/2" PLYWOOD: FASTEN SHEATHING W/ 2 3/8"x0.113 NAILS @ 6" O.C. AT EDGES & @ 12" O.C. IN THE PANEL FIELD.
ALL SHEATHING PANELS SHALL BE ORIENTED VERTICALLY (LONG DIRECTION PARALLEL TO STUDS) AND INSTALLED FULL HEIGHT OF SHEAR WALL - OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT ALL UNSUPPORTED PANEL EDGES & EDGE FASTENING.

3" O.C. EDGE NAILING

- AT DESIGNATED AREAS - FASTEN PANEL EDGES OF WOOD STRUCTURAL WALL SHEATHING TO FRAMING W/ 2 3/8" x 0.113" NAILS @ 3" O.C. NO STAPLE ALTERNATIVE AVAILABLE AT THIS SPEC. ALL SHEATHING PANELS SHALL BE ORIENTED VERTICALLY (LONG DIRECTION PARALLEL TO STUDS) AND INSTALLED FULL HEIGHT OF SHEAR WALL - OR 2x HORIZONTAL BLOCKING SHALL BE PROVIDED TO SUPPORT UNSUPPORTED PANEL EDGES AND 3" O.C. EDGE FASTENING.

NOTES

- SEE CONNECTION SPECIFICATIONS CHART FOR STANDARD SHEAR TRANSFER DETAILING. IF ADDITIONAL CAPACITY IS REQUIRED BY DESIGN, IT WILL BE SPECIFICALLY NOTED ON PLAN.
REFER TO DETAIL SHEET.
"NUMBERED" DETAILS ARE REFERENCED ON PLAN.
DESIGN ASSUMES 16" O.C. MAX. STUD SPACING, U.N.O.
LAP SHEATHING @ 1ST & 2ND FLOOR DECKS PER TYPICAL DETAILS PROVIDED.
DESIGN ASSUMES ALL INTERIOR SHEAR WALLS AND EXTERIOR WALLS ARE CONTINUOUS SHEATHED ABOVE AND BELOW OPENINGS.
WHERE PANELS ARE APPLIED TO BOTH FACES OF WALL, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS.
ALL STRUCTURAL PANELS ARE TO BE DIRECTLY APPLIED TO STUD FRAMING.
PRE-MANUFACTURED PANELIZED WALLS: FASTEN TOGETHER END STUDS OF WALL PANELS SHEATHED W/ OSB OR PLYWOOD AS FOLLOWS: P1 SHEAR PANELS - 3" x 0.120" NAILS @ 6" O.C. P3 SHEAR PANELS - 3" x 0.120" NAILS @ 4" O.C.
INDICATES LOCATION AND EXTENT OF SHEARWALL WHICH REQUIRES SHEATHING AND/OR FASTENING SPECIFICATIONS BEYOND THAT OF STANDARD CONSTRUCTION

GENERAL STRUCTURAL NOTES

FLOOR FRAMING

- TRUSSES SHALL BE DESIGNED BY MANUF. TO MEET OR EXCEED L/480 LIVE LOAD DEFLECTION CRITERIA.
PER THE GUIDELINES OF THE TILE COUNCIL OF NORTH AMERICA (TCNA HANDBOOK), IT SHALL BE THE FLOOR FINISH INSTALLER'S RESPONSIBILITY TO VERIFY THAT THE FINISHES TO BE INSTALLED MATCH THE DESIGN CRITERIA NOTED ABOVE (UNDER "DESIGN LOADS").
METAL HANGERS SHALL BE SPECIFIED BY MANUFACTURER, U.N.O.
TRUSS SHOP DWGS. SHALL BE SUBMITTED TO ARCH. & ENG. FOR REVIEW AND APPROVAL PRIOR TO FABRICATION OR DELIVERY.
FLOOR SHEATHING SHALL BE 23/32" A.P.A. RATED STURD-I-FLOOR® 24" O.C., EXPOSURE 1 (OR APPROVED EQUAL) WITH TONGUE AND GROOVE EDGES. FASTEN TO FRAMING MEMBERS W/ GLUE AND:
- 2 1/2" x 0.131" NAILS @ 6" O.C. @ PANEL EDGES @ 12" O.C. FIELD.
- 2 3/8" x 0.120" NAILS @ 4" O.C. @ PANEL EDGES @ 8" O.C. FIELD.
- 2 3/8" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES @ 6" O.C. IN FIELD.

ROOF FRAMING

- ROOF SHEATHING SHALL BE 7/16" A.P.A. RATED SHEATHING 24/16 EXPOSURE 1 (OR APPROVED EQUAL). FASTEN TO FRAMING MEMBERS:
- W/ 2 1/2" x 0.131" NAILS @ 6" O.C. @ PANEL EDGES @ 12" O.C. FIELD.
- W/ 2 3/8" x 0.120" NAILS @ 4" O.C. @ PANEL EDGES @ 8" O.C. FIELD.
- W/ 2 3/8" x 0.113" NAILS @ 3" O.C. @ PANEL EDGES @ 6" O.C. FIELD.
WITHIN 48" OF ALL ROOF EDGES, RIDGES, & HIPs FASTEN ROOF SHEATHING FIELDS PER EDGE NAILING SPEC.
FASTEN EACH ROOF TRUSS TO TOP PLATE W/ SIMPSON H2.5A HURRICANE CLIP (OR APPROVED EQUAL) @ ALL BEARING POINTS. CLIPS MUST BE INSTALLED ON EXTERIOR FACE OF EXTERIOR WALLS OR THE SAME SIDE AS THE SHEAR WALL SHEATHING.
ALL ROOF GIRDER TRUSSES AND FLUSH BEAMS WITHIN THE ROOF SYSTEM SHALL BE FASTENED TO THE DOUBLE TOP PLATES AND POST SUPPORT W/ (2) SIMPSON H2.5A CLIPS OR APPR. EQUAL (TYP. U.N.O. ON PLANS)
TRUSS ALL VALLEY SETS. FASTEN TO TRUSS BELOW W/ SIMPSON H2.5A TIES AT EACH END AND AT EACH TRUSS BETWEEN.
METAL HANGERS SHALL BE SPECIFIED BY THE MANUFACTURER, U.N.O.
ROOF TRUSS SHOP DWGS. SHALL BE SUBMITTED TO ARCH. & ENG. FOR REVIEW AND APPROVAL PRIOR TO FABRICATION OR DELIVERY.
ERECT AND INSTALL ROOF TRUSSES PER MTC & TPI'S BCSI 1 'GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.'
SUPPORT SHORT SPAN ROOF TRUSSES W/ 2x4 LEDGER FASTENED TO FRAMING W/ (2) 3" x 0.120" NAILS @ 16" O.C. (UP TO 1" SPAN).

ADDITIONAL NOTES FOR TRUSS & I-JOIST MANUFACTURER

- ROOF TRUSS, FLOOR TRUSS AND ENGINEERED JOISTS SHALL BE DESIGNED TO MEET THE DIFFERENTIAL DEFLECTION CRITERIA BELOW, UNLESS NOTED OTHERWISE ON PLAN. MULHERN & KULP CANNOT BE HELD RESPONSIBLE FOR ANY STRUCTURAL ISSUES RELATED TO ANY BUILDING COMPONENT IF COMPONENT SHOP DRAWINGS ARE NOT SUBMITTED TO MKK FOR REVIEW PRIOR TO FABRICATION, DELIVERY, OR INSTALLATION.
TRUSSES/JOISTS SHALL BE DESIGNED SO THAT DIFFERENTIAL DEFLECTION BETWEEN ADJACENT PARALLEL TRUSSES/JOISTS OR GIRDER TRUSSES/FLUSH BEAMS DO NOT EXCEED THE FOLLOWING:
A. ROOF TRUSSES: 1/4" DEAD LOAD
B. FLOOR TRUSSES, ATTIC TRUSSES, & I-JOISTS: 1/8" DEAD LOAD
C. FLOOR TRUSSES & ATTIC TRUSSES ADJACENT TO FLOOR FRAMING BY OTHERS: LIMIT ABSOLUTE TRUSS DEFLECTION TO 3/16" DEAD LOAD. (NOT DIFFERENTIAL DEFLECTION)

LIST OF ABBREVIATIONS

Table with 2 columns: Abbreviation and Full Name. Includes B.F., B.M., BOT., BRG., B.J.N.A., CANT'D, CONC., CONT., DBL., DIM., EA., EQ., EXT., E.J.N., F.T., FND., FTG., G.T., HD., HDR., INT., INTERIOR, JACK STUD, JACK TRUSS, KING STUD, MANUF., MANUFACTURER, MAX., MAXIMUM, MIN., MINIMUM, N.T.S., NOT TO SCALE, OPT., OPTIONAL, P.A., POST ABOVE, P.T., PRESSURE TREATED, PKT., POCKET, REQ'D, REQUIRED, R.T., ROOF TRUSS, SCHED., SCHEDULE, SIM., SIMILAR, STRUCT., STRUCTURAL, T.O.F., TOP OF FOOTING, TYP., TYPICAL.

VENEER LINTEL SCHEDULE

Table with 3 columns: SPAN (MAX), HEIGHT OF VENEER ABOVE LINTEL, STEEL ANGLE SIZE. Rows include 3'-0", 6'-0", 8'-0", 9'-6", 16'-0".

- ALL LINTELS:
- SHALL SUPPORT 2 3/8" - 3 1/2" VENEER W/ 40 psf MAXIMUM WEIGHT.
< 16" SHALL HAVE 4" MIN. BEARING
> 16" SHALL HAVE 8" MIN. BEARING
< 16" SHALL NOT BE FASTENED BACK TO HEADER.
> 16" SHALL BE FASTENED BACK TO WOOD HEADER IN WALL @ 48" O.C. W/ 1/2" DIA. x 3 1/2" LONG LAG SCREWS IN 2" LONG VERTICALLY SLOTTED HOLES.
- MAX. VENEER HT. APPLIES TO ANY PORTION OF BRICK OVER THE OPENING.
- ALL LINTELS SHALL BE LONG LEG VERTICAL.
- WHEN SUPPORTING VENEER < 3" WIDE THE EXTERIOR TOE OF THE HORIZONTAL LEG MAY BE CUT IN THE FIELD TO BE 3 1/2" WIDE OVER THE BEARING LENGTH ONLY. THIS IS TO ALLOW FOR MORTAR JOINT FINISHING.
- SEE STRUCTURAL PLANS FOR ANY LINTEL CONDITION NOT ENCOMPASSED BY THE ABOVE PARAMETERS.
* FOR QUEEN VENEER USE L4x3x1/4".
** FOR 3 1/2" VENEER ONLY. SEE PLAN FOR VENEER SUPPORT IF VENEER < 3 1/2" THICK.

NON-BEARING HEADER SCHEDULE

Table with 3 columns: SPAN, 2x4 NON-BEARING PARTITION WALL, 2x6 NON-BEARING PARTITION WALL. Rows include UP TO 3'-0", UP TO 6'-0", UP TO 8'-0", UP TO 12'-0".

LEGEND

- INTERIOR BEARING WALL
BEARING WALL ABOVE (B.N.A.) -AND/OR- SHEAR WALL ABOVE (S.N.A.)
BEAM / HEADER
EXTENT OF OVERFRAMING (O.F.)
EXTENT OF TILE OVER FLOOR
EXTENT OF BRACING PANELS/SHEARWALL W/ ADDL. CONNECTION REQUIREMENTS
INDICATES HOLD-DOWN OR STRAP. REFER TO SCHEDULE.
METAL HANGER
INDICATES POST ABOVE (P.A.) PROVIDE SOLID BLOCKING UNDER POST OR JAMB ABOVE.

MEANS & METHODS NOTES

THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE AFTER THE BUILDING IS FINISHED AND ALL PLAN, DETAIL, AND NOTE SPECIFICATIONS HAVE BEEN COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE THE ERECTION PROCEDURES AND SEQUENCE TO INSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING CONSTRUCTION. THIS INCLUDES, BUT IS NOT LIMITED TO, THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS, AND TIE-DOWNS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING AND BRACING REQUIRED TO STABILIZE AND PROTECT EXISTING AND ADJACENT STRUCTURES AND SYSTEMS DURING COURSE OF DEMOLITION AND CONSTRUCTION OF THE PROJECT.

STRUCTURAL DESIGN AND SPECIFICATIONS ASSUME THAT ALL SUPPORTING AND NON-SUPPORTING ELEMENTS IN CONTACT WITH FLOOR FRAMING ARE LEVEL, INCLUDING, BUT NOT LIMITED TO: FOUNDATIONS, SLABS ON GRADE, BEAMS, WALLS, AND NON-BEARING ELEMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY LEVELNESS AND MAKE ADJUSTMENTS AS NECESSARY, INCLUDING CONSIDERATION OF THOSE AREAS THAT MAY BE WITHIN CONTRACTUAL, INDUSTRY, OR WARRANTY TOLERANCES.

HOLD-DOWN SCHEDULE

Table with 2 columns: SYMBOL, SPECIFICATION. Rows include HD-1 SIMPSON HTT4 HOLD-DOWN, HD-2 SIMPSON CS16 STRAP (14" MIN. END LENGTH).

* UTILIZE 5/8"x24 ANCHOR BOLT W/ 10" MIN. EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. RECOMMENDATIONS. ALTERNATE TO 5/8"x24 ANCHOR BOLT SPECIFICATION INTO CONCRETE: UTILIZE SIMPSON "SET" EPOXY SYSTEM TO FASTEN 3/8" THREADED ROD INTO CONCRETE FOUNDATION. PROVIDE 8" MIN. EMBEDMENT INTO CONCRETE. INSTALL PER MANUF. RECOMMENDATIONS. DO NOT LOCATE ANCHORS WITHIN 1 3/4" OF EDGE OF FOUNDATION.

GENERAL STRUCTURAL NOTES

- ALL FOOTINGS SHALL BEAR BELOW FROST LINE (TYP.) OR 12" MIN IN REGIONS WHERE CODE FROST DEPTH IS NOT APPLICABLE. CONSULT SOILS REPORT OR BUILDING DEPT. FOR MINIMUM DEPTH BELOW GRADE.
FOOTINGS AND SLABS ON GRADE SHALL BEAR ON VIRGIN SOIL OR 15% COMPACTED FILL.
PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP.
JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" O.C. (MAXIMUM)
JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (1:1 RATIO), WITH A MAXIMUM OF 1:1.5 RATIO
CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL SLABS
DIMENSIONS BY OTHERS, BUILDER TO VERIFY.
BUILDER TO PROVIDE SUBTERRANEAN TERMITES PROTECTION MEETING THE REQUIREMENTS OF R318 OR LOCAL CODE REQUIREMENTS
CRANLSPACE WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY EITHER ADEQUATE TEMPORARY BRACING OR INSTALLATION OF FIRST FLOOR DECK.
CRANLSPACE WALL DESIGN IS BASED ON BACKFILL SOIL CLASSIFICATIONS OF GM, GP, SM, SP (30 pcf) OR GM, GC, SM, SM-SC, ML (45 pcf), IF SC, ML-CL, OR CL (60 pcf) SOIL IS ENCOUNTERED ON SITE, CONTACT MULHERN & KULP FOR FURTHER EVALUATION OF FOUNDATION DESIGN.
CONCRETE MASONRY UNITS (CMU) SHALL BE ASTM C40 WITH A MIN. COMPRESSIVE STRENGTH OF 1900 psi (Fm=1500 psi). MORTAR SHALL BE ASTM C270, TYPE S. CMU DESIGN PER ACI 530 & 530.1.
CMU FOUNDATION WALLS SHALL HAVE 'DUR-O-WALL' HORIZONTAL JOINT REINFORCEMENT (OR EQUAL) - 9 GA. MINIMUM @ 16" O.C.
PROVIDE 2x8 x 16" LONG P.T. PLATE ON TOP OF ALL CRANL SPACE PIERS. TOP COURSE OF PIERS SHALL BE SOLID MASONRY OR FILLED SOLID.
PROVIDE 2x6 P.T. PLATE ON INTERIOR CRANL SPACE WALLS, FASTENED PER ANCHORAGE SPECIFICATION NOTED ABOVE.
BASEMENT FOUNDATION WALL DESIGN BASED ON:
- 10' HEIGHT - TALLER WALLS MUST BE ENGINEERED. NOMINAL WIDTH (8" FOR 8" THICK WALL).
BASEMENT WALL DESIGN IS BASED ON 45 PCF.
BASEMENT WALLS SHALL BE BRACED, PRIOR TO BACKFILLING, BY ADEQUATE TEMPORARY BRACING OR INSTALL 1st FLOOR DECK.
PROVIDE (2) #5 BARS AROUND ALL SIDES OF OPENINGS IN CONCRETE BSMT. FND. WALL WITH 2" CLEAR. REINFORCEMENT SHALL EXTEND 12" PAST CORNER OF OPENING IN ALL DIRECTIONS.
FOR OPENINGS UP TO 36", PROVIDE MINIMUM 10" CONCRETE DEPTH OVER OPENING OR (3)2x10 W/ (2)2x6 JACK STUDS, U.N.O.
LARGER OPENINGS SHALL BE PER PLAN.
PROVIDE CONTROL JOINTS AT ALL INSIDE CORNERS OF SLAB EDGES, AND OTHER LOCATIONS WHERE SLAB CRACKS ARE LIKELY TO DEVELOP.
JOINTS SHALL BE LOCATED @ 10'-0" O.C. (RECOMMENDED) OR 15'-0" O.C. (MAXIMUM)
JOINT GRID PATTERN SHALL BE AS CLOSE TO SQUARES AS POSSIBLE (1:1 RATIO), WITH A MAXIMUM OF 1:1.5 RATIO
CONTROL JOINTS SHALL NOT BE INSTALLED IN STRUCTURAL SLABS

M&K project number: 192-17022
project mgr: JAK
drawn by: AMG
issue date: 05-03-19

REVISIONS:
date: initial:
02/12/2018 RAP CHANGE FLOOR TRUSS LAYOUT & BEAR
04/11/2018 RAP UPDATE PLAN 1 & 2 FOR PLAN 2

ARCH: v.01.00.00.00



GENERAL STRUCTURAL NOTES
ALEXANDRIA II
WIND SPEED < 115 MPH NORTH CAROLINA

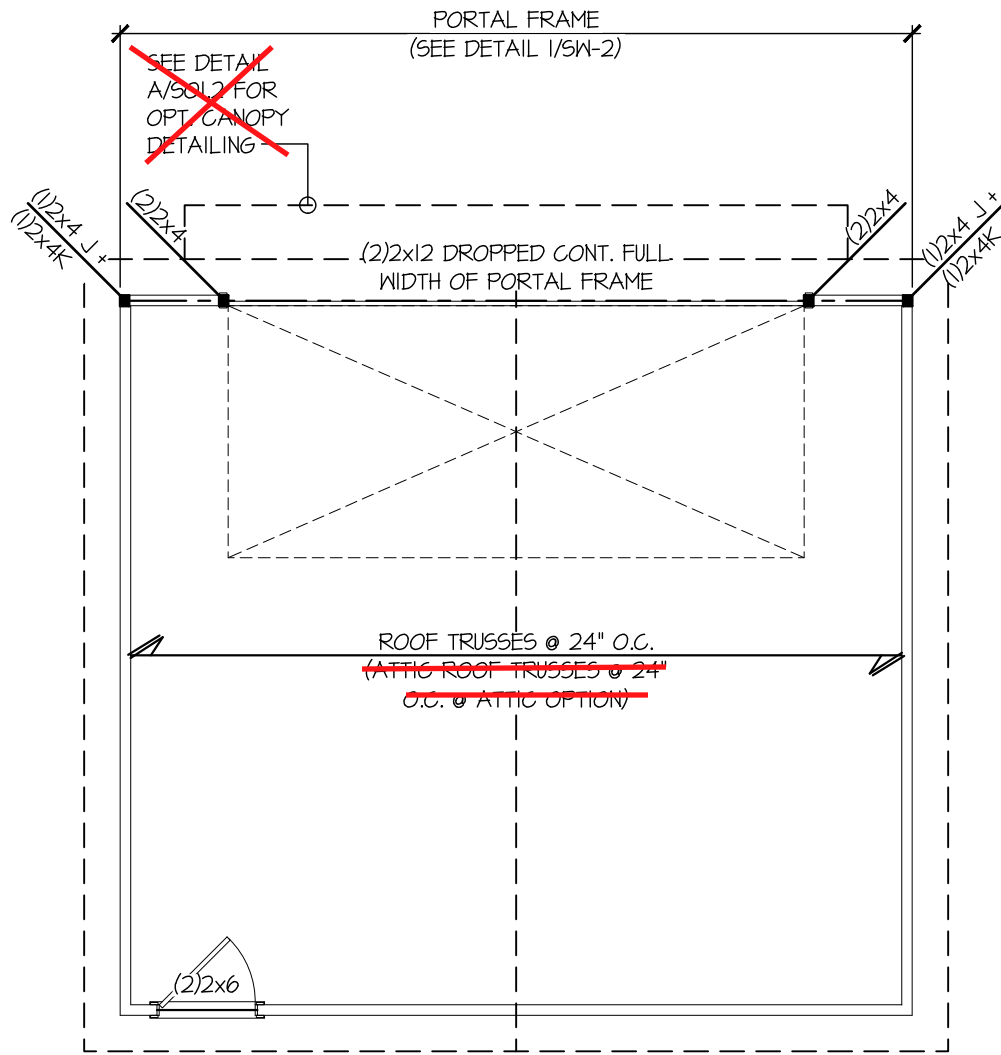
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ADD'L HARDWARE SCHEDULE FOR
OPT. DETACHED 2-CAR GARAGE

QTY	PRODUCT
22	SIMPSON H2.5A CLIP
4	SIMPSON CS16 (26" STRAP)

HARDWARE NOTES:

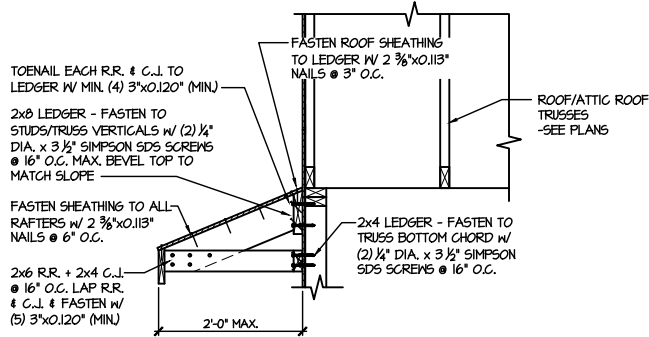
- ALL NAIL HOLES FILLED
- ALL COUNTS ESTIMATED
- HARDWARE COUNT DOES NOT INCLUDE TRUSS TO TRUSS CONNECTIONS



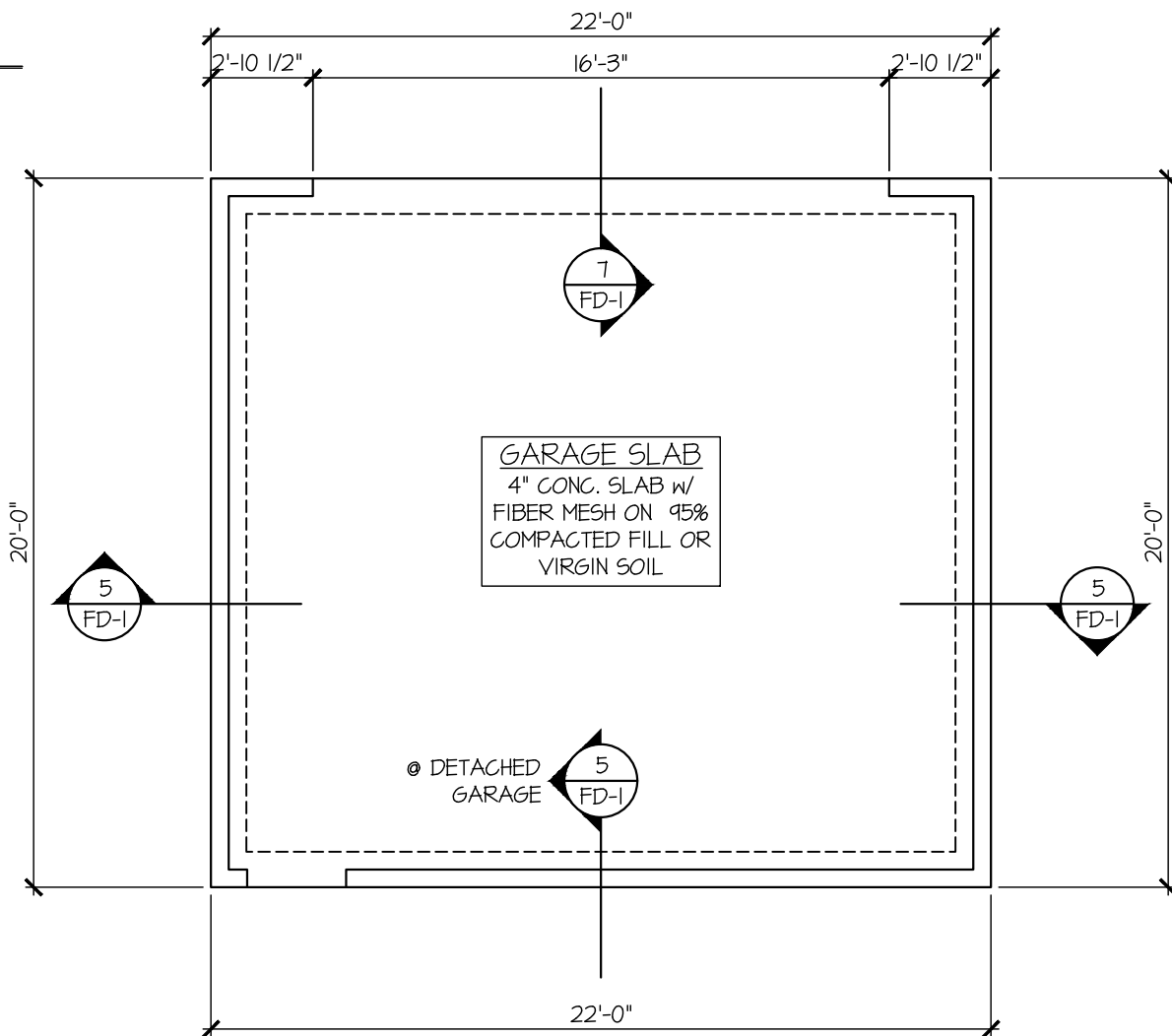
ROOF FRAMING PLAN
OPT. DETACHED 2-CAR GARAGE

SCALE: 3/16"=1'-0"
(1ST FLOOR WALLS SHOWN)

ALL ELEVS. SIM.
ATTIC OPTION SIM.



A DETAIL @ CANOPY
SCALE: 22/64 = 3/4"=1'-0"
1/8" = 5/8"=1'-0"



FOUNDATION PLAN

OPT. DETACHED 2-CAR GARAGE

SCALE: 3/16"=1'-0"

ALL ELEVS. SIM.
ATTIC OPTION SIM.

REFER TO S-0.0 FOR
TYPICAL STRUCTURAL NOTES
& SCHEDULES

SO1.3M

OPTION FRAMING PLANS
ALEXANDRIA II
DETACHED 2-CAR GARAGE
WIND SPEED < 115 MPH NORTH CAROLINA



ARCH: 01.00.00.000

date: 02/12/2019

drawn by: JAK

issue date: 05-03-19

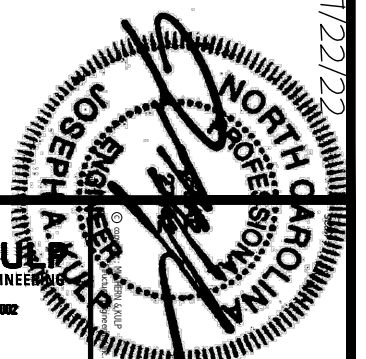
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192-17022

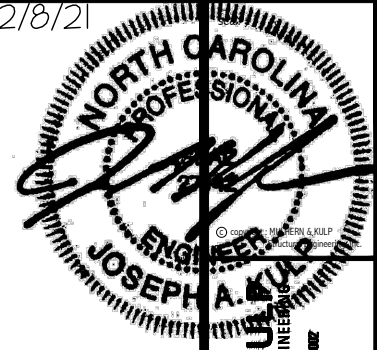


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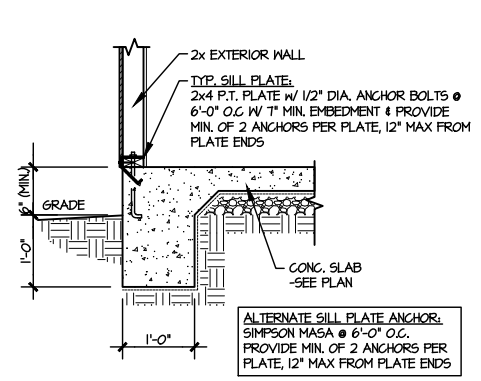
300 Brookside Ave, Building 4 - Amber, PA 19002
p 215-646-8001 - mulhern+kurtz.com
NC License # C-3825



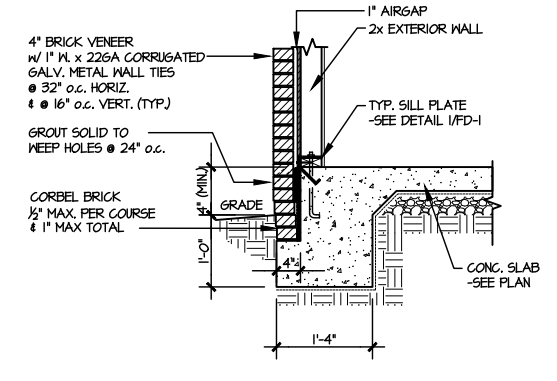
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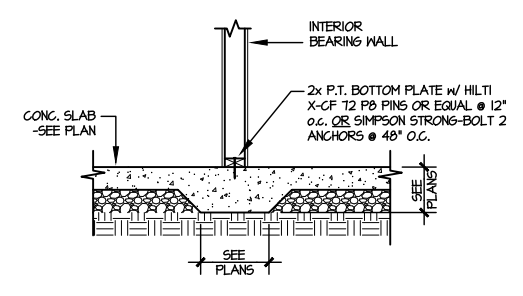
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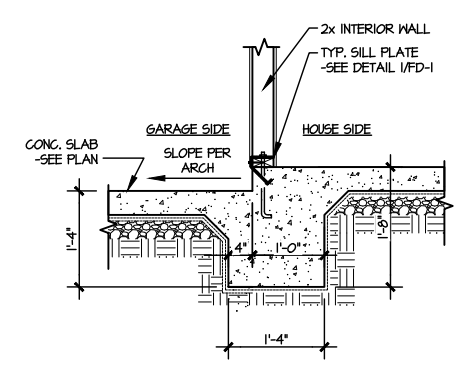
1 TYPICAL TURNDOWN @ EXT. WALL
SCALE 3/8"=1'-0"



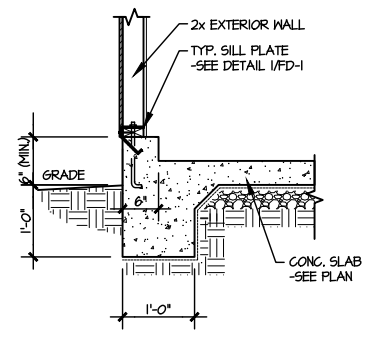
2 TYPICAL TURNDOWN @ EXT. WALL (BRICK)
SCALE 3/8"=1'-0"



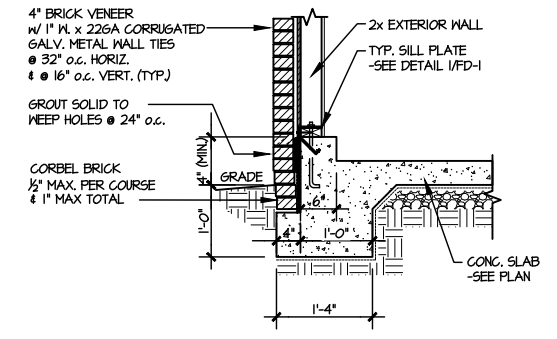
3 TYPICAL THICKENED SLAB @ INTERIOR BEARING WALL
SCALE 3/8"=1'-0"



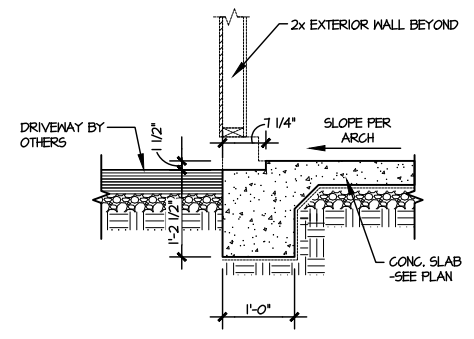
4 TYPICAL INT. FOOTING BETWEEN HOUSE & GARAGE
SCALE 3/8"=1'-0"



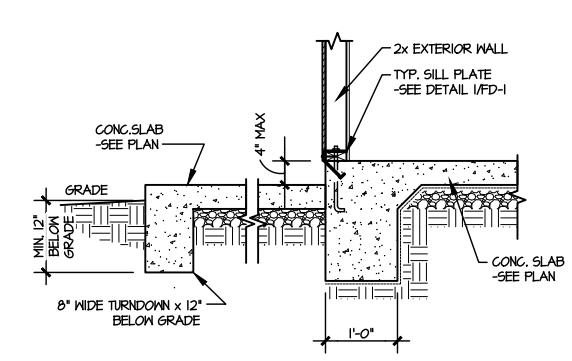
5 TYPICAL TURNDOWN @ EXT. GARAGE WALL
SCALE 3/8"=1'-0"



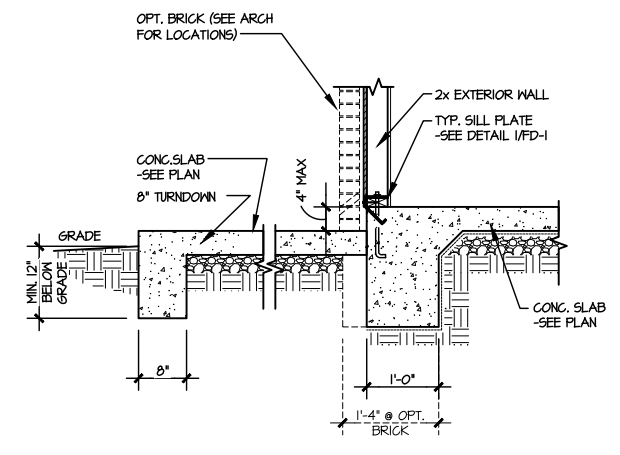
6 TYPICAL TURNDOWN @ EXT. GARAGE WALL (BRICK)
SCALE 3/8"=1'-0"



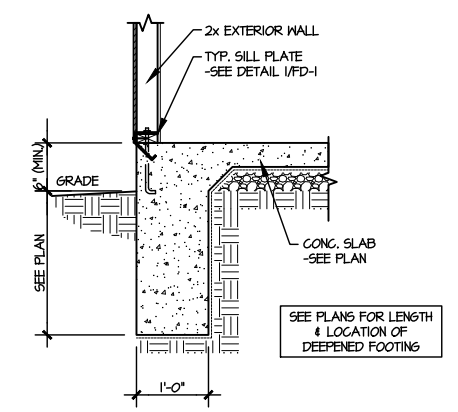
7 GARAGE OPENING
SCALE 3/8"=1'-0"



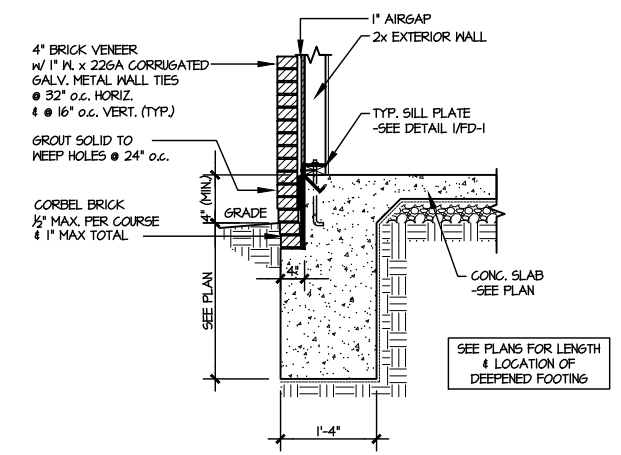
8 TYPICAL TURNDOWN @ PATIO/PORCH
SCALE 3/8"=1'-0"



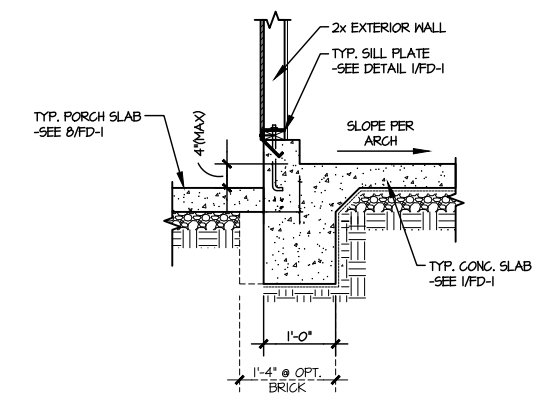
9 TYPICAL TURNDOWN @ PATIO/PORCH (BRICK)
SCALE 3/8"=1'-0"



10 TYPICAL TURNDOWN W/ DEEPEMED FTGS. @ EXT. WALL
SCALE 3/8"=1'-0"



11 TYPICAL TURNDOWN @ EXT. WALL (BRICK)
SCALE 3/8"=1'-0"



12 TYPICAL TURNDOWN @ COVERED PORCH/ATTACHED GARAGE
SCALE 3/8"=1'-0"

M&K project number: 192-17022
project mgr: JAK
drawn by: AMG
issue date: 05-03-19

REVISIONS:	date:	initial:
02/12/2018	RAP	CHANGE FLOOR TRUSS LAYOUT & BEAR
04/11/2018	RAP	UPDATE PLAN 1 & SCORP PLAN

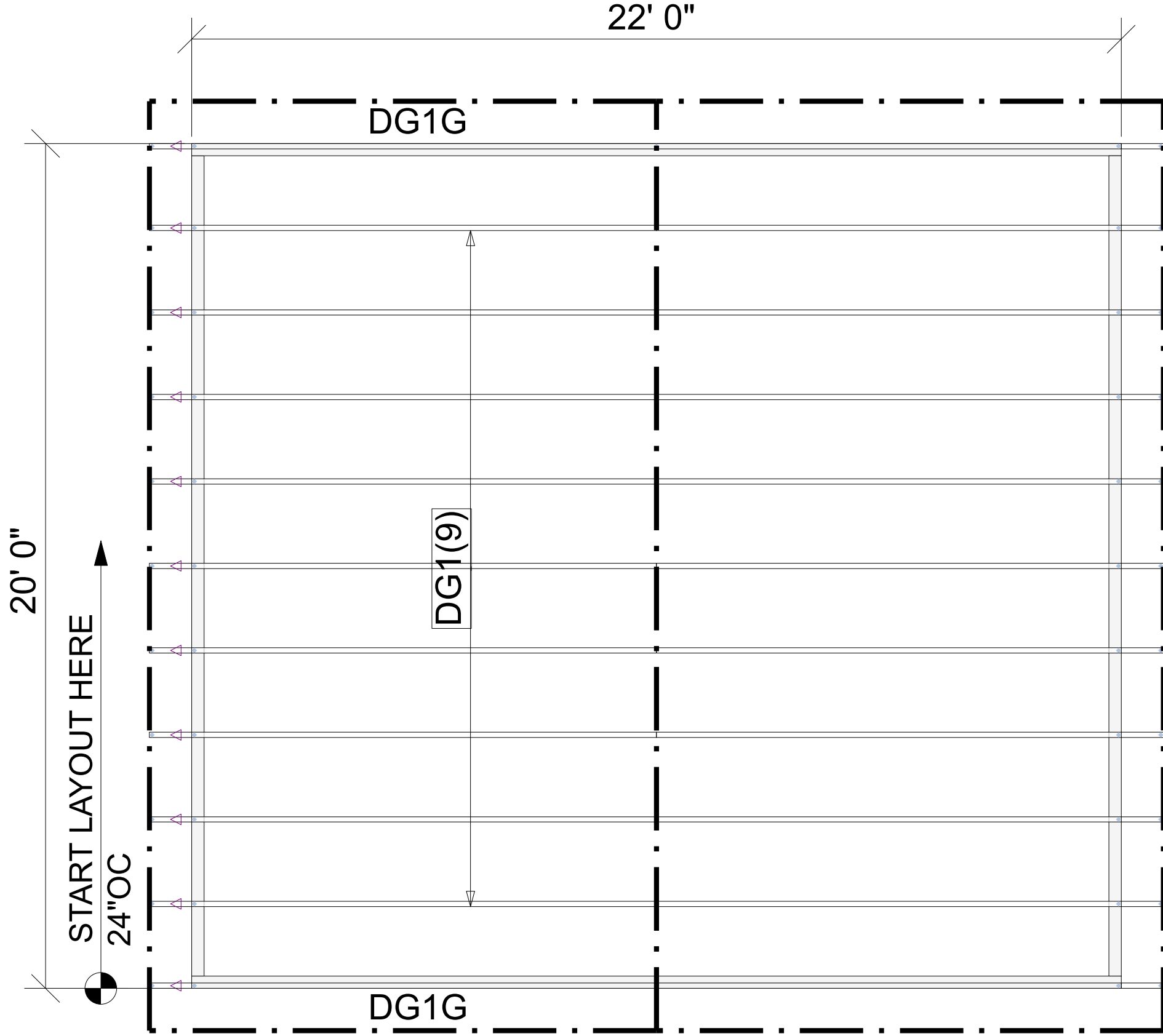
ARCH: v.01.00.00.00



FOUNDATION DETAILS
ALEXANDRIA II
WIND SPEED < 115 MPH NORTH CAROLINA

THIS IS A TRUSS PLACEMENT DIAGRAM (TPD) ONLY; NOT AN ENGINEERED DOCUMENT. Trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual truss design drawings (TDD's) for each truss design identified on the TPD. The Contractor is responsible for the temporary bracing of the roof and floor system, and requirements for the permanent restraint/bracing of truss systems may be met by following the methods outlined in ANSI-TPI 1-2014 - 2.3.3. The design of the support structure including but not limited to headers, beams, walls, and columns is also the responsibility of the building designer. For general guidance regarding installation and bracing, consult "Building Component Safety Information" (BCSI) available from the SBC Association (www.sbcassociation.com). It is the responsibility of the General Contractor to verify that the provided component layout matches the final intended construction plans, loading conditions, and use. If they do not, it is the responsibility of the General Contractor to notify UFP and provide plans containing the latest specifications and designs. UFP will not be responsible for plan changes by others after final approval of shop drawings, or for errors or modifications made on-site during construction. DO NOT CUT, NOTCH, DRILL, OR OTHERWISE "REPAIR" MANUFACTURED TRUSSES IN ANY WAY WITHOUT PRIOR WRITTEN AUTHORIZATION BY A LICENSED PROFESSIONAL DESIGNATED BY UFP. The Framing is responsible to verify all dimensions, including adjusting member spacing within tolerances to allow for the drop and rise of plumbing/HVAC, unless noted otherwise. Truss-to-wall connections, if shown, are for uplift only and do not consider lateral loads. All connectors on this project are to be installed per the connector manufacturer's specifications. All connectors shown that are not truss-to-truss are suggestions only and are to be verified by the Building Designer or Engineer of Record for suitability to this particular project. UFP accepts no responsibility for the specific application or suitability of any connector that is not truss-to-truss as they apply to this specific structure.

ROOF PLACEMENT PLAN



△ INDICATES LEFT END OF TRUSS SCALE: N.T.S

ROOF AREA: 556.56 ft ² sqft	RIDGE LINE: 22 ft	VALLEY LINES: 0 ft	HIP LINES: 0 ft	THESE VALUES ARE APPROXIMATE ONLY																								
2 CAR DETACHED GRGE		MUNGO HOMES																										
<table border="1" style="width: 100%; border-collapse: collapse;"> <thead> <tr> <th colspan="2">REVISIONS</th> <th>DSN</th> </tr> <tr> <th>DATE</th> <th>DESCRIPTION</th> <th></th> </tr> </thead> <tbody> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> <tr><td> </td><td> </td><td> </td></tr> </tbody> </table>					REVISIONS		DSN	DATE	DESCRIPTION																			
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TRUSS TRAX UFP CONSTRUCTION TrussTraxUFP.com		This drawing is property of UFP Site Built, LLC. Any unauthorized use of this document without written permission is prohibited. UFP relinquishes ownership of delivered product upon delivery. Owner of product must obtain UFP's authorization prior to any alteration or modification of product. UFP will not be held responsible for any unauthorized modifications done or costs incurred without prior written authorization from UFP.																										