

Trenco 818 Soundside Rd Edenton, NC 27932

Re: 25-0502-A FFF-LOT #9 Roof

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Riverside Roof Truss.

Pages or sheets covered by this seal: I72045671 thru I72045697

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



March 17,2025

# Gilbert, Eric

**IMPORTANT NOTE:** The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



TOP CHORD

BOT CHORD

BOT CHORD

REACTIONS.

WEBS

OTHERS

TOP CHORD 2x4 SP No.2

2x4 SP No.2

2x4 SP No.3

2x4 SP No.3

Max Grav 8=415(LC 2), 6=415(LC 2)

Max Uplift 8=-55(LC 16), 6=-55(LC 16)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-346/109, 3-4=-346/109, 2-8=-375/173, 4-6=-375/173

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

(size) 8=0-3-0, 6=0-3-0 Max Horz 8=-93(LC 14)

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 2-1-0, Interior(1) 2-1-0 to 4-6-8, Exterior(2R) 4-6-8 to 7-6-8 , Interior(1) 7-6-8 to 10-0-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 55 lb uplift at joint 8 and 55 lb uplift at ioint 6
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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818 Soundside Road



<u>9-8-15</u> 9-8-15	<u>19-5-13</u> 9-8-15	+ 29-2-12 9-8-15	39-7-6 10-4-10	50-0-0 10-4-10
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2018/TPI2014	CSI.         DEFL.           TC         0.47         Vert(LL)           BC         0.49         Vert(CT           WB         0.92         Horz(CT           Matrix-MS         Horz(CT)	in (loc) l/defl L/d -0.12 18-20 >999 240 ) -0.21 18-20 >999 180 ) 0.02 16 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 354 lb         FT = 20%
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2		<b>BRACING-</b> TOP CHORD BOT CHORD	Structural wood sheathing directly a Rigid ceiling directly applied or 6-0-0	pplied or 5-4-13 oc purlins. ) oc bracing.

WEBS

1 Row at midpt

2 Rows at 1/3 pts

6-18, 8-16

7-16

REACTIONS. (size) 2=0-3-8, 16=0-3-8, 12=0-3-8 Max Horz 2=190(LC 15) Max Uplift 2=-90(LC 16), 16=-144(LC 16), 12=-66(LC 16)

2x4 SP No.3

Max Grav 2=1167(LC 28), 16=2859(LC 30), 12=691(LC 29)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- TOP CHORD 2-4=-1806/182, 4-6=-1698/226, 6-7=-713/220, 7-8=0/853, 8-10=-622/140, 10-12=-766/119
- BOT CHORD 2-20=-74/1747. 18-20=0/952. 14-16=-340/92. 12-14=-11/707

Left 2x4 SP No.3 2-6-0, Right 2x4 SP No.3 2-6-0

- WEBS 4-20=-411/164, 6-20=-56/969, 6-18=-887/222, 7-18=-101/1408, 7-16=-1818/149,
  - 8-16=-963/215, 8-14=-34/1050, 10-14=-467/167

#### NOTES-

WEBS

SLIDER

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=50ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-1-0, Interior(1) 4-1-0 to 25-0-0, Exterior(2R) 25-0-0 to 30-0-0, Interior(1) 30-0-0 to 50-11-0 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 4x4 MT20 unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 90 lb uplift at joint 2, 144 lb uplift at joint 16 and 66 lb uplift at joint 12.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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9-8-15	19-5-13	29-2-12		39-7-6		49-9-0		
9-8-15	9-8-15	9-8-15		10-4-10		10-1-10		
LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 11.6/15.0 TCDI 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15	<b>CSI.</b> TC 0.48 BC 0.50	<b>DEFL.</b> Vert(LL) Vert(CT)	in (loc) l/defl -0.12 17-19 >999 -0.21 17-19 >999	L/d 240 180	PLATES MT20	<b>GRIP</b> 244/190	
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2018/TPI2014	WB 0.92 Matrix-MS	Horz(CT)	0.02 15 n/a	n/a	Weight: 351 lb	FT = 20%	
LUMBER- TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 SLIDER Left 2x4 SP No.3	3 2-6-0, Right 2x4 SP No.3 2-6-0	BF TC BC WI	ACING- P CHORD T CHORD EBS	Structural wood sheathir Rigid ceiling directly app 1 Row at midpt 2 Rows at 1/3 pts	ng directly app plied or 6-0-0 6-17, 8-1 7-15	plied or 5-4-11 oc purlir oc bracing. 5	IS.	
REACTIONS. (size) 2=0- Max Horz 2=1 Max Uplift 2=-8 Max Grav 2=1	REACTIONS.       (size)       2=0-3-8, 15=0-3-8, 12=Mechanical         Max Horz       2=188(LC 15)         Max Uplift       2=-89(LC 16), 15=-147(LC 16), 12=-34(LC 16)         Max Grav       2=1168(LC 28), 15=2846(LC 30), 12=629(LC 29)							
FORCES.       (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-4=-1809/180, 4-6=-1702/224, 6-7=-717/218, 7-8=0/843, 8-10=-598/141, 10-12=-737/120         BOT CHORD       2-19=-95/1745, 17-19=-14/949, 13-15=-334/77, 12-13=-37/680         WEBS       4-19=-411/164, 6-19=-56/969, 6-17=-887/221, 7-15=-1808/152, 7-17=-101/1404, 8-15=-954/1215, 8-13=-39/1024, 10-13=-456/172								
<ul> <li>NOTES-</li> <li>1) Unbalanced roof ive loads have been considered for this design.</li> <li>2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=50ft; eave=6ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -0-11-0 to 4-0-11, Interior(1) 4-0-11 to 25-0-0, Exterior(2R) 25-0-0 to 29-11-11, Interior(1) 29-11-11 to 49-9-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces &amp; MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10</li> <li>4) Unbalanced snow loads have been considered for this design.</li> <li>5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.</li> <li>6) All plates are 4x4 MT20 unless otherwise indicated.</li> <li>7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>8) * This truss has been designed for a 10.0 psf outom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.</li> <li>9) Refer to girder(s) for truss to truss connections.</li> <li>10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2, 147 lb uplift at joint 12.</li> <li>11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.</li> </ul>								

- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 89 lb uplift at joint 2, 147 lb uplift at joint 15 and 34 lb uplift at joint 12.
- 11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.





building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS//TPI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



	9-11-4	19-7-0	29-2-12	40-6-4	49-9-0	
I	9-11-4	9-7-12	9-7-12	11-3-8	9-2-12	
LOADIN TCLL (rc Snow (P TCDL BCLL BCLL BCDL	G (psf) of) 20.0 f/Pg) 11.6/15.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2018/TPI2014	CSI. DEI TC 0.40 Ver BC 0.52 Ver WB 0.83 Hor Matrix-MS	FL. in (loc) l/defl (LL) -0.12 17-19 >999 (CT) -0.22 17-19 >999 z(CT) 0.03 15 n/a	L/d PLATES GRIP 240 MT20 244/19 180 n/a Weight: 352 lb FT =	90 = 20%
LUMBEI TOP CH BOT CH WEBS SLIDER REACTI	R- ORD 2x6 SP No.2 ORD 2x6 SP No.2 2x4 SP No.3 Left 2x4 SP No. ONS. All bearings 0	3 2-6-0, Right 2x4 SP No.3 2-6-0 -3-8 except (jt=length) 12=Mechanical.	BRACING- TOP CHORI BOT CHORI WEBS	<ul> <li>Structural wood sheathin</li> <li>Rigid ceiling directly appl</li> <li>6-0-0 oc bracing: 13-15.</li> <li>1 Row at midpt</li> <li>2 Rows at 1/3 pts</li> </ul>	g directly applied or 5-2-3 oc purlins. ied or 10-0-0 oc bracing, Except: 6-17, 8-15 7-15	
	(ID) - Max Horz 2=18 Max Uplift All Max Grav All 12=4	uplift 100 lb or less at joint(s) 13, 12 exc reactions 250 lb or less at joint(s) excep 466(LC 35)	ept 2=-104(LC 16), 15=-106(LC 16) 2=1235(LC 28), 15=2467(LC 28), 1	3=571(LC 37),		
FORCES TOP CH	<ol> <li>(lb) - Max. Comp./N ORD 2-4=-1952/213 10-12=-370/13</li> </ol>	1ax. Ten All forces 250 (lb) or less exc 3, 4-6=-1828/250, 6-7=-847/252, 7-8=0/6 35	ept when shown. 04, 8-10=-462/246,			
BOT CH WEBS	ORD 2-19=-127/187 4-19=-403/162 8-15=-641/216	75, 17-19=-48/1097, 13-15=-315/96, 12- 2, 6-19=-45/940, 6-17=-890/218, 7-17=- 6, 8-13=-108/588, 10-13=-530/197	13=-40/341 02/1385, 7-15=-1628/114,			
NOTES- 1) Unbai 2) Wind: II; Exp 29-11 and fc 3) TCLL DOL= 4) Unbai 5) This t non-c 6) All pla 7) This t	anced roof live loads ha ASCE 7-16; Vult=130n D B; Enclosed; MWFRS -11, Interior(1) 29-11-1 <sup>-1</sup> roces & MWFRS for rea ASCE 7-16; Pr=20.0 p 1.15); Is=1.0; Rough C anced snow loads have russ has been designed oncurrent with other live ttes are 4x4 MT20 unlest russ has been designed	ave been considered for this design. hph (3-second gust) Vasd=103mph; TCI (directional) and C-C Exterior(2E) -0-11 1 to 49-9-0 zone; cantilever left and right ctions shown; Lumber DOL=1.60 plate g sf (roof LL: Lum DOL=1.15 Plate DOL= at B; Partially Exp.; Ce=1.0; Cs=1.00; Ct b been considered for this design. d for greater of min roof live load of 12.0 a loads. ss otherwise indicated. f or a 10.0 psf bottom chord live load no	DL=6.0psf; BCDL=6.0psf; h=25ft; B= 0 to 4-0-11, Interior(1) 4-0-11 to 25 exposed ; end vertical left and right rip DOL=1.60 .15); Pg=15.0 psf; Pf=11.6 psf (Lur =1.10 psf or 1.00 times flat roof load of 11	45ft; L=50ft; eave=6ft; Cat. -0-0, Exterior(2R) 25-0-0 to exposed;C-C for members n DOL=1.15 Plate 6 psf on overhangs s.	SEAL	. Minn

8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

Refer to girder(s) for truss to truss connections.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 12 except (jt=lb) 2=104, 15=106.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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1	11-1-12	1	22-0-0	27-6-0	33-6-	4	41-5-14	1	49-9-0	I.
Г	11-1-12		10-10-4	5-6-0	6-0-	4	7-11-10	1	8-3-2	
LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL BCDL	sf) 20.0 11.6/15.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/TPI:	2-0-0         CSI.           1.15         TC           1.15         BC           YES         WB           2014         Matr	0.47 0.95 0.82 ix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (l -0.29 18- -0.46 18- 0.05	loc) l/defl -20 >999 -20 >881 15 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 367 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER-       BRACING-         TOP CHORD       2x6 SP No.2         3OT CHORD       2x6 SP 2400F 2.0E, 17-20: 2x4 SP No.2         WEBS       2x4 SP No.3 *Except*         7-15: 2x4 SP DSS         SLIDER       Left 2x4 SP No.3 2-6-0, Right 2x4 SP No.3 2-6-0         REACTIONS.       (size)       2=0-3-8, 15=0-3-8, 12=Mechanical         Max Horz       2=188(LC 15)         Max Uplift 2=-80(LC 16), 15=-33(LC 16), 12=-26(LC 16)         Max Grav       2=1536(LC 28), 15=-3080(LC 30), 12=478(LC 35)										
FORCES. (IL TOP CHORD BOT CHORD WEBS	b) - Max. Comp./M 2-4=-2591/182 2-23=-96/2456 12-13=-61/400 4-23=-398/165 7-17=-2116/31	ax. Ten All forces 250 (lb , 4-6=-2393/187, 6-7=-133 , 21-23=-6/1755, 16-21=0/ , 6-23=-20/824, 6-21=-924 , 15-17=-2202/0, 8-15=-53	o) or less except when sł 11/169, 7-8=0/688, 8-10= /811, 15-16=0/811, 13-15 //211, 20-21=-0/1488, 7-2 /9/216, 10-15=-854/150,	nown. 0/662, 10-12= 5=-61/400, 20=0/1600, 16-18=-315/0	433/107					
NOTES- 1) Unbalanced 2) Wind: ASCI II; Exp B; El 29-11-11, Ir and forces a 3) TCLL: ASC DOL=1.15); 4) Unbalanced 5) This truss h	d roof live loads ha E 7-16; Vult=130m nclosed; MWFRS nterior(1) 29-11-11 & MWFRS for reac E 7-16; Pr=20.0 ps ; Is=1.0; Rough Ca d snow loads have ias been designed	we been considered for this ph (3-second gust) Vasd= (directional) and C-C Exter to 49-9-0 zone; cantilever ztions shown; Lumber DOL sf (roof LL: Lum DOL=1.15 at B; Partially Exp.; Ce=1.0 been considered for this d for greater of min roof live	s design. 103mph; TCDL=6.0psf; I rior(2E) -0-11-0 to 4-0-11 left and right exposed; ; =1.60 plate grip DOL=1. Plate DOL=1.15); Pg=1: ; Cs=1.00; Ct=1.10 lesign. load of 12.0 psf or 1.00	BCDL=6.0psf; , Interior(1) 4- end vertical le 60 5.0 psf; Pf=11 times flat roof	h=25ft; B=45ft; I -0-11 to 25-0-0, F ft and right expo .6 psf (Lum DOL load of 11.6 psf	_=50ft; eave Exterior(2R) sed;C-C for =1.15 Plate on overhance	e=6ft; Cat. 25-0-0 to members	A LINE	TH CARO	N.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

8) Refer to girder(s) for truss to truss connections.

non-concurrent with other live loads.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 15, 12.
 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and

referenced standard ANSI/TPI 1.



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#### REACTIONS. All bearings 49-7-8.

(lb) - Max Horz 2=203(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 31, 48, 49, 50, 51, 52, 54, 55, 56, 57, 58, 59, 43, 42, 41, 40, 39, 37, 36, 35, 34, 33, 2 except 32=-128(LC 16)

Max Grav All reactions 250 lb or less at joint(s) 31, 45, 46, 48, 49, 50, 51, 52, 54, 55, 56, 57, 58, 59, 44, 43, 42, 41, 40, 39, 37, 36, 35, 34, 33, 32, 2

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 13-14=-109/263, 14-15=-123/302, 15-16=-131/326, 16-17=-131/326, 17-18=-123/302, 18-19=-109/263

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=50ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -1-0-0 to 3-11-11, Exterior(2N) 3-11-11 to 25-0-0, Corner(3R) 25-0-0 to 29-11-11, Exterior(2N) 29-11-11 to 49-5-12 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

11) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 31, 48, 49, 50, 51, 52, 54, 55, 56, 57, 58, 59, 43, 42, 41, 40, 39, 37, 36, 35, 34, 33, 2 except (jt=lb) 32=128.

13) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





818 Soundside Road



TRENCS

818 Soundside Road

Edenton, NC 27932

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	FFF-LOT #9 Roof	
						172045678
25-0502-A	T04G	COMMON GIRDER	1	2		
				<b>_</b>	Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Fe	b 19 2025 MiTek Industries, Inc. Fri Mar 14 11:22:34 2025	Page 2

ID:tdHS5lWyLng?jaR9E1eBtqyly9\_-0CTX5zsXjGI2SUEddm02wJ\_wAnHzo8nQsG365lzb1Yp

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-43, 3-5=-43, 6-10=-20

Concentrated Loads (lb)

Vert: 13=-451(B) 14=-445(B) 15=-445(B) 16=-445(B) 17=-445(B) 18=-445(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent outlapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 12) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12, 11.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

SEAL 036322 MGINEER March 17,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANS/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcaccomponents.com)

A MiTek A 818 Soundside Road



A MiTek A 818 Soundside Road

Edenton, NC 27932

March 17,2025





Job	Truss	Truss Type	Qty	Ply	FFF-LOT #9 Roof	
						172045681
25-0502-A	T05A	Common	1	1		
					Job Reference (optional)	
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Fe	b 19 2025 MiTek Industries, Inc. Fri Mar 14 11:22:36 2025	Page 2

ID:tdHS5IWyLng?jaR9E1eBtqyly9\_-zbbHWeuoFuYminO?IB3W?k46SbrlG?ejKZYCAezb1Yn

LOAD CASE(S) Standard Concentrated Loads (lb) Vert: 14=-500

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LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins,
BOT CHORD	2x4 SP No.2		except end verticals.
WEBS	2x4 SP No.3	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
OTHERS	2x4 SP No.3		6-0-0 oc bracing: 23-24,14-15.

#### REACTIONS. All bearings 20-8-0.

Max Horz 24=161(LC 15) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 24, 20, 21, 22, 23, 18, 17, 16, 15 All reactions 250 lb or less at joint(s) 24, 14, 19, 20, 21, 22, 23, 18, 17, 16, 15 Max Grav

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) -0-11-0 to 2-4-0, Exterior(2N) 2-4-0 to 10-4-0, Corner(3R) 10-4-0 to 13-4-0, Exterior(2N) 13-4-0 to 21-7-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate 4) DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 11.6 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 10) Gable studs spaced at 2-0-0 oc.
- 11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 12) will fit between the bottom chord and any other members.
- 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 24, 20, 21, 22, 23, 18, 17, 16, 15.
- 14) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road



building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS//TPI Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

Job	Truss	Truss Type	Qty	Ply	FFF-LOT #9 Roof
					172045683
25-0502-A	T06G	GABLE	1	2	
				<b>Z</b>	Job Reference (optional)
Riverside Roof Truss, LLC,	Danville, Va - 24541,			8.730 s Fe	b 19 2025 MiTek Industries, Inc. Fri Mar 14 11:22:38 2025 Page 2

### NOTES-

ID:tdHS5IWyLng?jaR9E1eBtqyly9\_-vzj2xKw2nVoTx5YOsc5\_499WGOZ\_kqu0nt1JEWzb1YI

14) Use Simpson Strong-Tie LUS26 (4-10d Girder, 3-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 1-5-4 from the left end to 27-5-4 to connect truss(es) to back face of bottom chord.

15) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-5=-43, 5-7=-43, 8-14=-20

Concentrated Loads (lb)

Vert: 12=-319(B) 10=-319(B) 29=-319(B) 30=-319(B) 31=-319(B) 32=-319(B) 33=-319(B) 34=-319(B) 36=-319(B) 37=-319(B) 38=-334(B) 39=-334(B) 40=-334(B) 40=-3 41=-340(B)

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BC2E Building Component Schut beformation, available from the Structure Building Component Advanciation (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 5-0-0, Exterior(2R) 5-0-0 to 8-0-0, Interior(1) 8-0-0 to 9-5-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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818 Soundside Road



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

### NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

> mmm March 17,2025

G

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall bilding design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





818 Soundside Road





818 Soundside Road





A. GIL



**RENCO** 

March 17,2025





A MiTek A 818 Soundside Road



NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 0-6-8 to 3-6-8, Interior(1) 3-6-8 to 6-4-5, Exterior(2R) 6-4-5 to 9-4-5, Interior(1) 9-4-5 to 12-2-2 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 8, 6.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



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BOT CHORD 2x4 SP No.2 OTHERS 2x4 SP No.3

1=6-11-3, 3=6-11-3, 4=6-11-3 REACTIONS. (size) Max Horz 1=-34(LC 14) Max Uplift 1=-20(LC 16), 3=-20(LC 16) Max Grav 1=126(LC 20), 3=126(LC 21), 4=226(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

## NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=15.0 psf; Pf=11.6 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Unbalanced snow loads have been considered for this design.

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

9) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



2x4 💋

2x4 🔌

Structural wood sheathing directly applied or 4-1-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

0-0 <u>-7</u> 0-0-7	-deel	<u>4</u> -1- 4-1-	1 <u>2</u> 5					
LOADING         (psf)           TCLL (roof)         20.0           Snow (Pf/Pg)         11.6/15.0           TCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.04 BC 0.11 WB 0.00	DEFL. Vert(LL) r Vert(CT) r Horz(CT) 0.	in (loc) n/a - n/a - 00 3	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20	<b>GRIP</b> 244/190
BCDL 0.0 *	Code IRC2018/TPI2014	Matrix-P					Weight: 11 lb	FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2		BI TC	RACING- OP CHORD Struc	tural wood s	heathing	directly app	blied or 4-1-12 oc purl	ins.

BOT CHORD

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

REACTIONS. (size) 1=4-0-14, 3=4-0-14 Max Horz 1=-18(LC 14) Max Uplift 1=-7(LC 16), 3=-7(LC 16) Max Grav 1=123(LC 2), 3=123(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

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818 Soundside Road

