



Carter Sanford Component Plant
298 Harvey Faulk Rd
Sanford, NC 27332

Phone #:919-775-1450



Builder: A&G Residential

Model: 60 Harnett Lakes/ Havilland

THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.
9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

Date: _____

General Notes:

** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER FIRST. CUSTOMER TAKES FULL RESPONSIBILITY FOR COMPONENTS IF CUT BEFORE AUTHORIZATION.

** ALL BEARING POINTS MUST BE INSTALLED PRIOR TO SETTING ANY COMPONENTS.

Revisions	
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name
00/00/00	Name

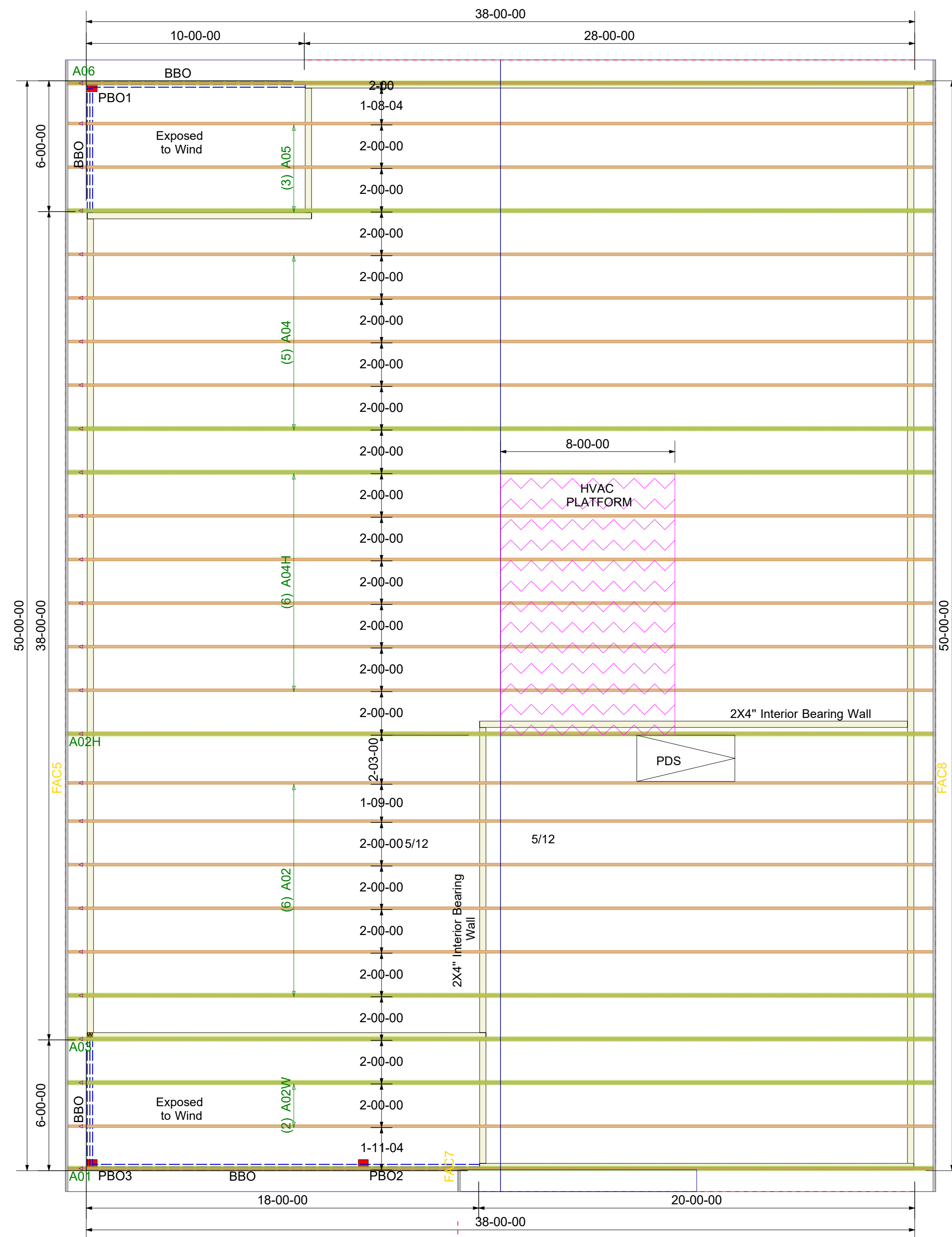
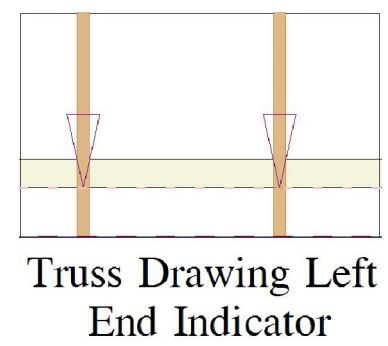
THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor systems and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing of Wood Trusses" available from the Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53179.



A&G Residential
 60 Harnett
 Lakes-Roof-Haviland A
 ROOF PLACEMENT PLAN

Scale: NTS
 Date: 2/4/2025
 Designer: Gladys Rivas
 Project Number: 25010296-A
 Sheet Number: 1/1

** DAMAGED COMPONENTS SHOULD NOT BE INSTALLED UNLESS TOLD TO BY THE COMPONENT PLANT. ** TRUSS TO TRUSS CONNECTIONS ARE TOE-NAILED, UNLESS NOTED OTHERWISE.
 ** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.
 ** GIRDERS MUST BE FULLY CONNECTED TOGETHER PRIOR TO ADDING ANY LOADS. ** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.



Truss Connector Total List			
Manual	Product	Qty	
Simpson	One H2.5A	67	

** ALL UPLIFT CONNECTORS SHOWN WITHIN THESE DOCUMENTS ARE RECOMMENDATIONS ONLY. PER ANS/ITPI 1, ALL UPLIFT CONNECTORS ARE THE RESPONSIBILITY OF THE BLDG DESIGNER AND/OR CONTRACTOR.

** TRIANGULAR SYMBOL NEAR END OF TRUSS INDICATES LEFT END OF TRUSS AS SHOWN ON INDIVIDUAL TRUSS DRAWINGS.

** PLUMBING DROPS NOTED ARE IN THE APPROXIMATE LOCATIONS PER PLAN. BUILDER TO VERIFY LOCATIONS BEFORE SETTING TRUSSES.

** REFER TO FINAL TRUSS ENGINEERING SHEETS FOR PLY TO PLY CONNECTIONS.

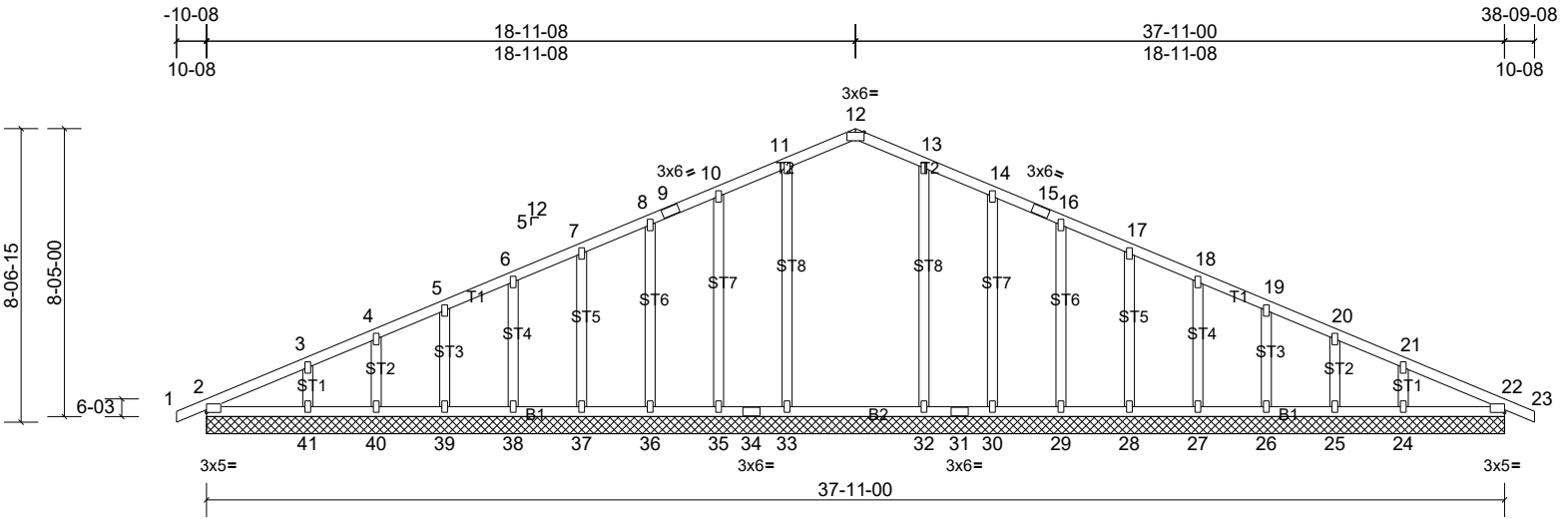
Job 25010296-A	Truss A01	Truss Type Common Supported Gable	Qty 1	Ply 1	60 Harnett Lakes-Roof-Havilland A Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:67.5

Plate Offsets (X, Y): [12:3-00,Edge]

Loading	(psf)	Spacing	1-11-04	CSI	0.37	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(CT)	0.01	22	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 219 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS All bearings 37-11-00.
(lb) - Max Horiz 2=127 (LC 14), 42=127 (LC 14)
Max Uplift All uplift 100 (lb) or less at joint(s) 2, 24, 25, 26, 27, 28, 29, 30, 35, 36, 37, 38, 39, 40, 41, 42
Max Grav All reactions 250 (lb) or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 35, 36, 37, 38, 39, 40, 42 except 24=480 (LC 6), 32=475 (LC 6), 33=441 (LC 5), 41=259 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 10-11=-42/251
WEBS 11-33=-325/32, 13-32=-370/32, 21-24=-277/144

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-11-8, Exterior(2N) 2-11-8 to 14-11-8, Corner(3R) 14-11-8 to 22-11-8, Exterior(2N) 22-11-8 to 34-11-8, Corner(3E) 34-11-8 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 35, 36, 37, 38, 39, 40, 41, 30, 29, 28, 27, 26, 25, 24, 2.

LOAD CASE(S) Standard

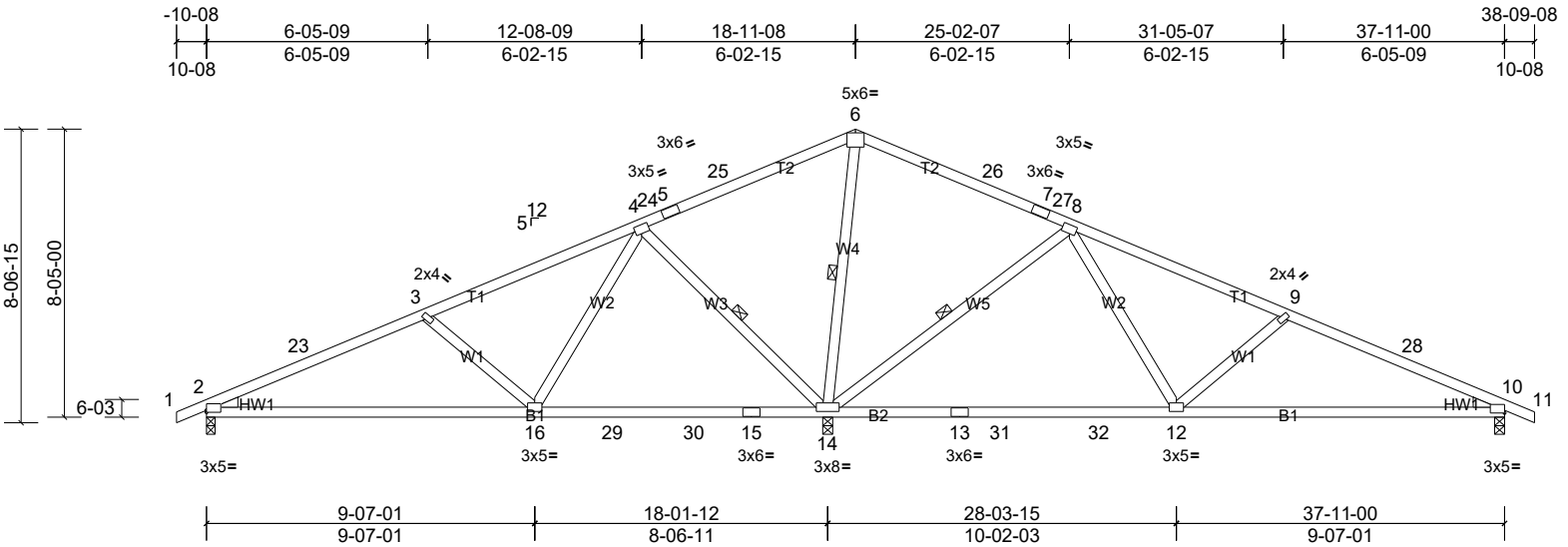
Job 25010296-A	Truss A02	Truss Type Common	Qty 6	Ply 1	60 Harnett Lakes-Roof-Havilland A
Carter Components, Sanford, NC, user					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-11]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.77	Vert(LL)	-0.27	12-14	>893	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.44	12-14	>546	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 189 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 5-1-4 oc purlins.
 Rigid ceiling directly applied or 2-2-0 oc bracing.
 1 Row at midpt 4-14, 6-14, 8-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=538/3-00, (min. 1-08), 10=624/3-08, (min. 1-08),
 14=1977/3-08, (min. 2-09)
 Max Horiz 2=131 (LC 14)
 Max Uplift 2=-81 (LC 14), 10=-110 (LC 15), 14=-168 (LC 14)
 Max Grav 2=628 (LC 38), 10=719 (LC 6), 14=2147 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-23=-835/100, 3-23=-772/125, 3-4=-547/69, 4-24=-38/583, 5-24=-36/595, 5-25=-26/630, 6-25=-21/703, 6-26=0/613,
 7-26=0/540, 7-27=-7/505, 8-27=-9/493, 8-9=-789/134, 9-28=-1002/191, 10-28=-1042/175
 BOT CHORD 2-16=-198/719, 13-14=-28/338, 13-31=-28/338, 31-32=-28/338, 12-32=-28/338, 10-12=-101/926
 WEBS 3-16=-452/194, 4-16=-27/745, 4-14=-896/230, 6-14=-813/138, 8-14=-992/232, 8-12=-7/792, 9-12=-435/194

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

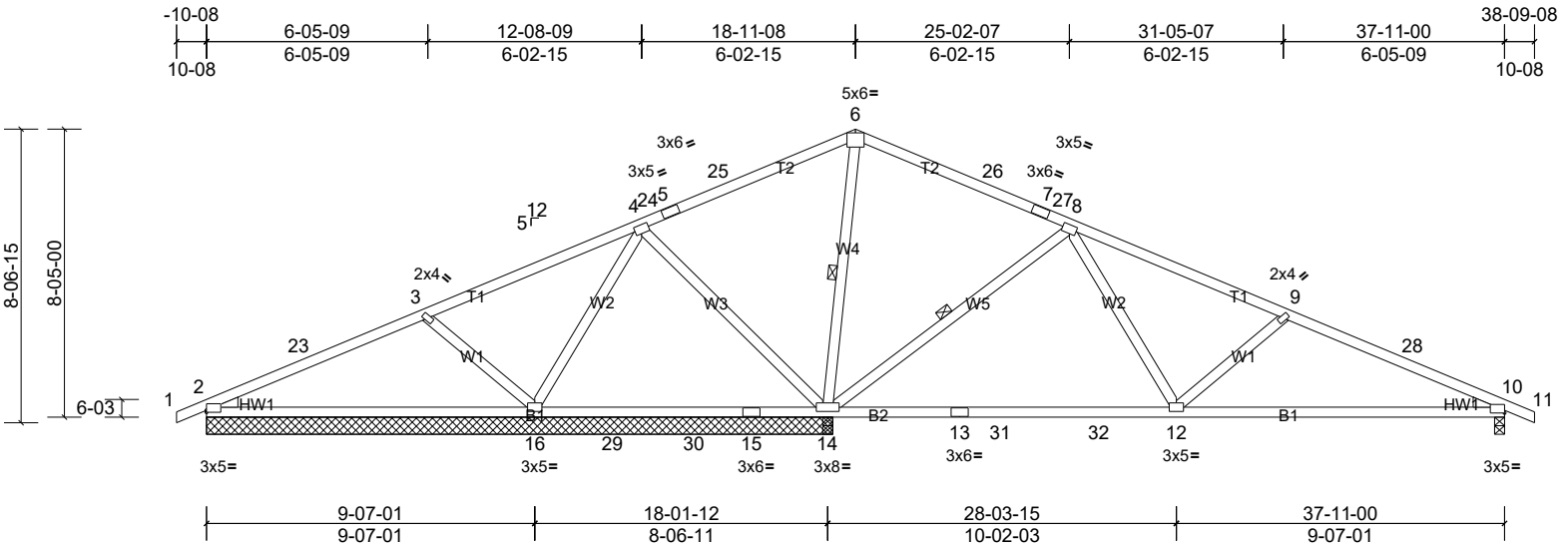
Job 25010296-A	Truss A03	Truss Type Common	Qty 1	Ply 1	60 Harnett Lakes-Roof-Havilland A
Carter Components, Sanford, NC, user					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-11]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.27	12-14	>883	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.44	12-14	>536	180		
TCDL	10.0	Rep Stress Incr	YES	WB	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 189 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 4-11-8 oc purlins.
 Rigid ceiling directly applied or 2-2-0 oc bracing.
 1 Row at midpt 8-14, 6-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 18-03-08. except 10=3-08

(lb) - Max Horiz 2=131 (LC 14), 17=131 (LC 14)
 Max Uplift All uplift 100 (lb) or less at joint(s) 2, 17 except 10=-102 (LC 15), 14=-169 (LC 15), 16=-136 (LC 14)
 Max Grav All reactions 250 (lb) or less at joint(s) except 2=488 (LC 38), 10=740 (LC 6), 14=1892 (LC 6), 16=502 (LC 36), 17=488 (LC 38)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-23=-491/15, 3-23=-450/40, 4-24=0/435, 5-24=0/443, 5-25=0/466, 6-25=0/539, 6-26=0/495, 7-26=0/389, 7-27=-5/354, 8-27=-6/342, 8-9=-889/114, 9-28=-1144/170, 10-28=-1196/155
 BOT CHORD 2-16=-178/435, 13-14=0/373, 13-31=0/373, 31-32=0/373, 12-32=0/373, 10-12=-83/1047
 WEBS 8-14=-990/232, 8-12=-8/786, 9-12=-430/195, 4-16=-35/449, 3-16=-462/198, 6-14=-700/124, 4-14=-609/129

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, 16, and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

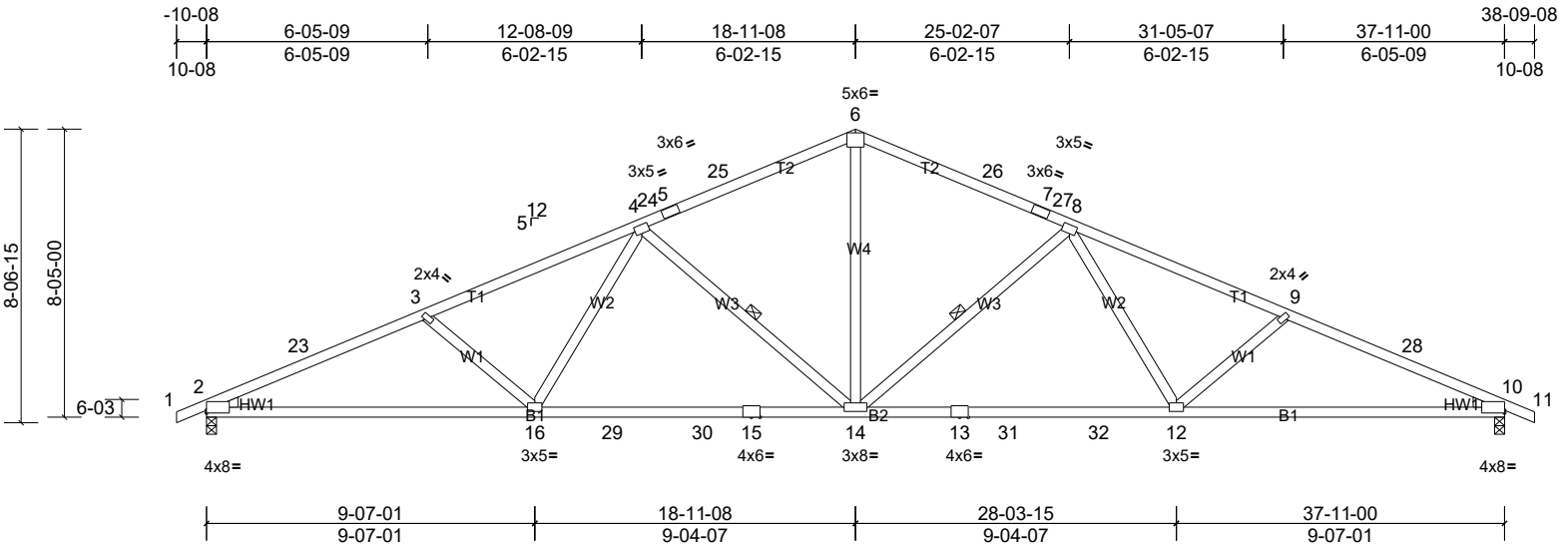
Job 25010296-A	Truss A04	Truss Type Common	Qty 5	Ply 1	60 Harnett Lakes-Roof-Havilland A
					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-15], [10:Edge,0-15]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	-0.28	14-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.50	14-16	>917	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(CT)	0.12	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											Weight: 189 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-10 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 8-14, 4-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1569/3-08, (min. 1-08), 10=1569/3-08, (min. 1-08)
Max Horiz 2=131 (LC 14)
Max Uplift 2=-169 (LC 14), 10=-169 (LC 15)
Max Grav 2=1695 (LC 3), 10=1695 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-23=-3803/322, 3-23=-3733/347, 3-4=-3533/322, 4-24=-2518/292, 5-24=-2510/295, 5-25=-2471/304, 6-25=-2452/321, 6-26=-2452/321, 7-26=-2471/304, 7-27=-2510/295, 8-27=-2518/292, 8-9=-3533/322, 9-28=-3733/347, 10-28=-3803/322
BOT CHORD 2-16=-361/3431, 16-29=-216/2855, 29-30=-216/2855, 15-30=-216/2855, 14-15=-216/2855, 13-14=-164/2855, 13-31=-164/2855, 31-32=-164/2855, 12-32=-164/2855, 10-12=-237/3431
WEBS 6-14=-80/1546, 8-14=-881/227, 8-12=-9/642, 9-12=-346/189, 4-14=-881/227, 4-16=-9/642, 3-16=-346/188

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

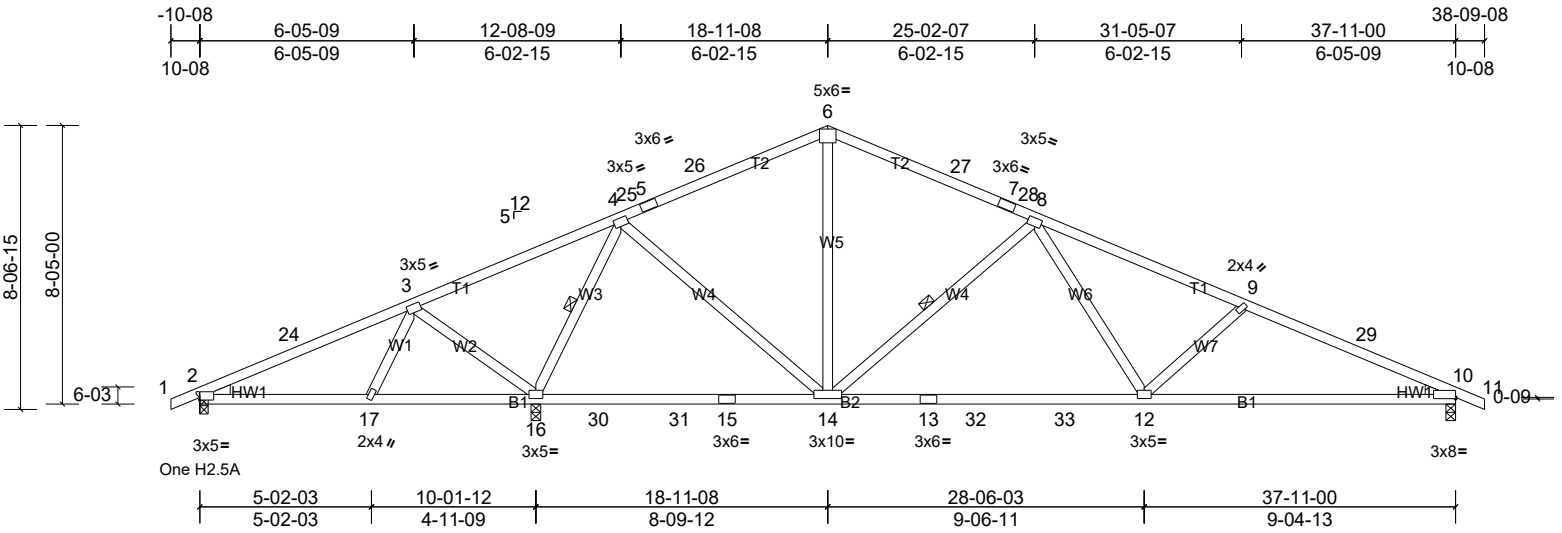
Job 25010296-A	Truss A05	Truss Type Common	Qty 3	Ply 1	60 Harnett Lakes-Roof-Havilland A
Carter Components, Sanford, NC, user					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:69.9

Plate Offsets (X, Y): [10:Edge,0-07]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP		
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.73	Vert(LL)	-0.22	12-14	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.99	Vert(CT)	-0.37	12-14	>891	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(CT)	0.04	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
										Weight: 193 lb	FT = 20%	

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.1 *Except* B2:2x4 SP No.2
 WEBS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-7-11 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 2-2-0 oc bracing.
 WEBS 1 Row at midpt 4-16, 8-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=197/3-00, (min. 1-08), 10=1068/3-08, (min. 1-08),
 16=1873/3-08, (min. 2-07)
 Max Horiz 2=131 (LC 14)
 Max Uplift 2=-107 (LC 10), 10=-152 (LC 15), 16=-155 (LC 14)
 Max Grav 2=296 (LC 36), 10=1174 (LC 6), 16=2058 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-24=-174/287, 3-24=-109/333, 3-4=-61/791, 4-25=-958/152, 5-25=-950/155, 5-26=-910/164, 6-26=-892/181,
 6-27=-892/181, 7-27=-911/164, 7-28=-951/155, 8-28=-958/153, 8-9=-2064/243, 9-29=-2282/292, 10-29=-2341/266
 BOT CHORD 2-17=-265/179, 16-17=-336/158, 13-14=-48/1438, 13-32=-48/1438, 32-33=-48/1438, 12-33=-48/1438, 10-12=-193/2094
 WEBS 3-16=-567/246, 4-16=-1809/240, 4-14=-20/957, 6-14=-26/343, 8-14=-911/229, 8-12=-14/725, 9-12=-386/188

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 16, and 10. This connection is for uplift only and does not consider lateral forces.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.
- Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

LOAD CASE(S) Standard

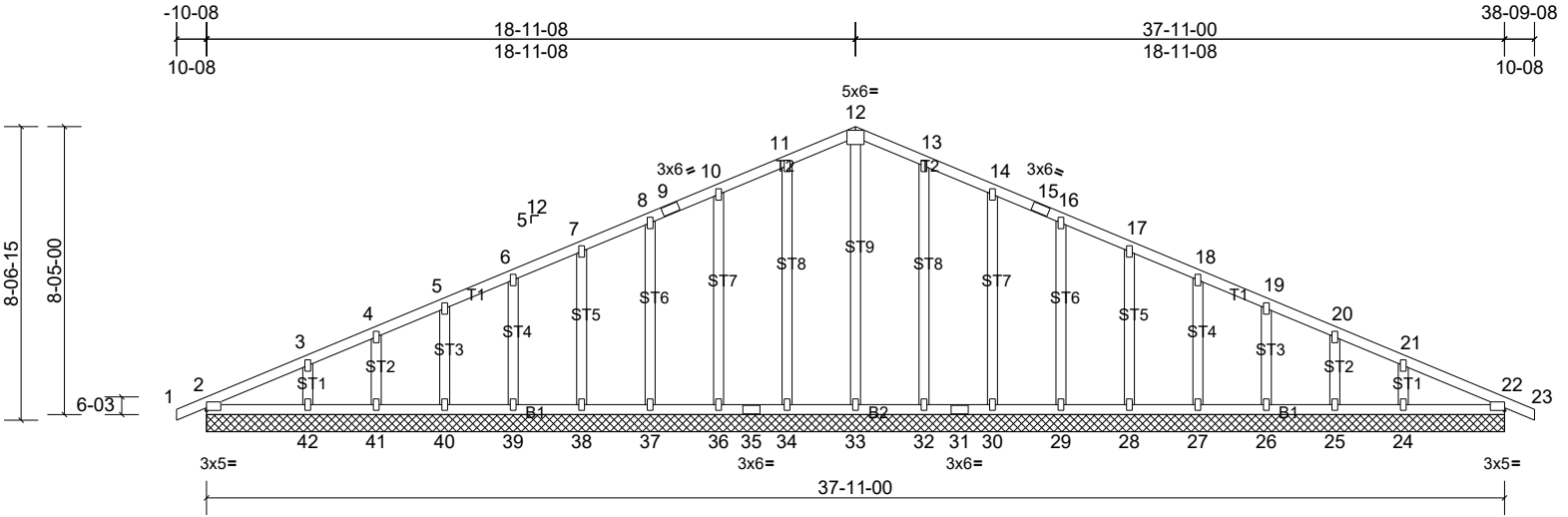
Job 25010296-A	Truss A06	Truss Type Common Supported Gable	Qty 1	Ply 1	60 Harnett Lakes-Roof-Havilland A Job Reference (optional)
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Carter Components, Sanford, NC, user

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Scale = 1:67.5

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.24	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(CT)	-0.01	24	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 230 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
OTHERS 2x4 SP No.3

BRACING
TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 10-0-0 oc purlins.
Rigid ceiling directly applied or 6-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS All bearings 37-11-00.
(lb) - Max Horiz 2=131 (LC 14), 43=131 (LC 14)
Max Uplift All uplift 100 (lb) or less at joint(s) 2, 24, 25, 26, 27, 28, 29, 30, 32, 34, 36, 37, 38, 39, 40, 41, 42, 43
Max Grav All reactions 250 (lb) or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 32, 34, 36, 37, 38, 39, 40, 41, 43 except 24=389 (LC 1), 33=329 (LC 22), 42=260 (LC 1)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-131/280, 3-4=-86/259, 4-5=-56/266, 5-6=-22/264, 6-7=0/266, 7-8=-10/273, 9-10=-20/280, 10-11=-40/289, 11-12=-55/286, 12-13=-55/282, 13-14=-40/279, 14-15=-20/279, 16-17=-10/266, 17-18=0/267, 18-19=-19/265, 19-20=-45/271, 21-22=-110/290

WEBS 12-33=-288/0

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-8, Interior (1) 2-11-8 to 14-11-8, Exterior(2R) 14-11-8 to 22-11-8, Interior (1) 22-11-8 to 34-11-8, Exterior(2E) 34-11-8 to 38-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 34, 36, 37, 38, 39, 40, 41, 42, 32, 30, 29, 28, 27, 26, 25, 24, 2.

LOAD CASE(S) Standard

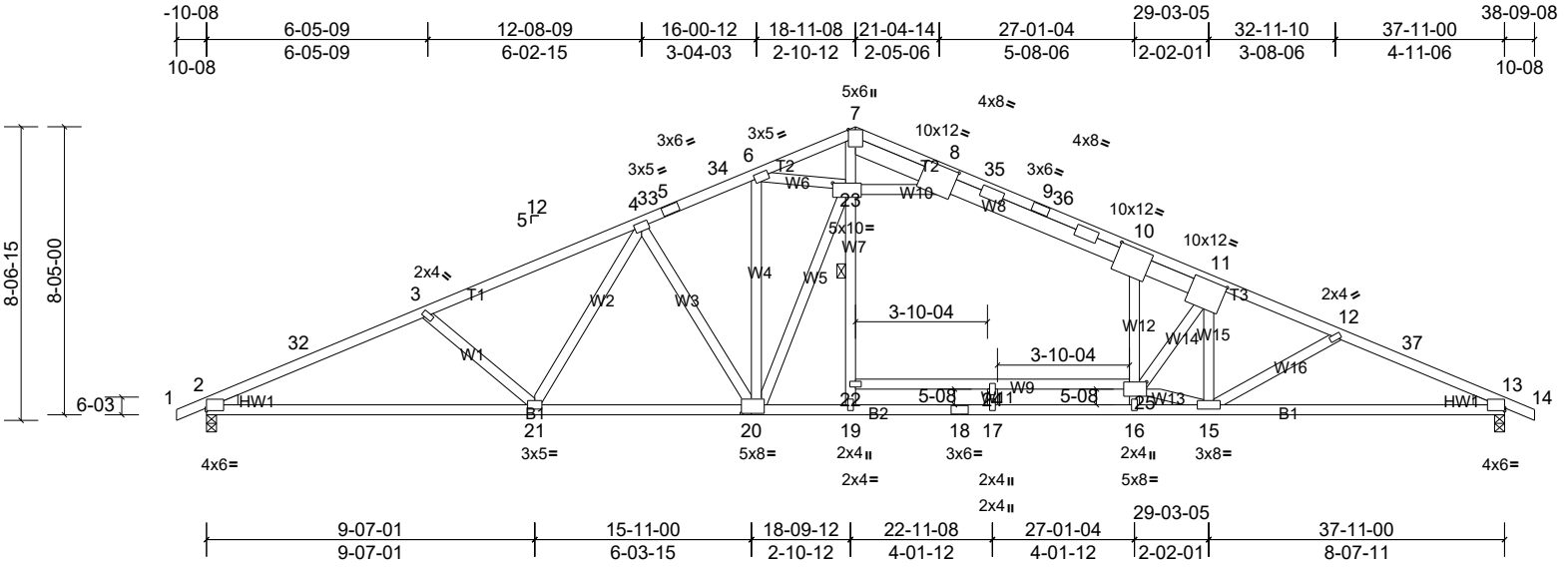
Job 25010296-A	Truss A02H	Truss Type Common	Qty 1	Ply 1	60 Harnett Lakes-Roof-Havilland A
Carter Components, Sanford, NC, user					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-15], [8:6-00,3-08], [10:6-00,7-08], [11:6-00,7-00], [13:Edge,0-15], [20:3-08,3-00], [23:4-08,2-04], [25:2-08,2-08]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.98	Vert(LL)	-0.29	16	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.47	Vert(CT)	-0.57	16-17	>793	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.77	Horz(CT)	0.10	13	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 250 lb FT = 20%

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* T3:2x4 SP No.1
 BOT CHORD 2x4 SP 2400F 2.0E
 WEBS 2x4 SP No.3 *Except* W8:2x6 SP No.2
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 3-0-5 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
 WEBS 1 Row at midpt 7-19

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1569/3-08, (min. 1-08), 13=1569/3-08, (min. 1-08)
 Max Horiz 2=131 (LC 14)
 Max Uplift 2=-169 (LC 14), 13=-169 (LC 15)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-32=-3153/328, 3-32=-3086/354, 3-4=-2872/325, 4-33=-2316/332, 5-33=-2308/333, 5-34=-2300/335, 6-34=-2258/346, 6-7=-675/416, 7-8=-684/296, 8-35=-2247/346, 9-35=-2257/336, 9-36=-2238/329, 10-36=-2287/320, 10-11=-2917/338, 11-12=-3092/325, 12-37=-3095/346, 13-37=-3161/327
 BOT CHORD 2-21=-367/2835, 20-21=-206/2340, 19-20=-131/2201, 18-19=-128/2167, 17-18=-128/2167, 16-17=-128/2167, 15-16=-153/2145, 13-15=-248/2844
 WEBS 19-22=0/302, 22-23=0/324, 7-23=-85/543, 4-21=-26/476, 3-21=-371/197, 8-23=-2462/576, 4-20=-613/175, 6-20=-227/1051, 6-23=-2121/483, 20-23=-795/274, 10-25=-138/1185, 11-25=-1063/220, 15-25=-99/786

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BC DL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

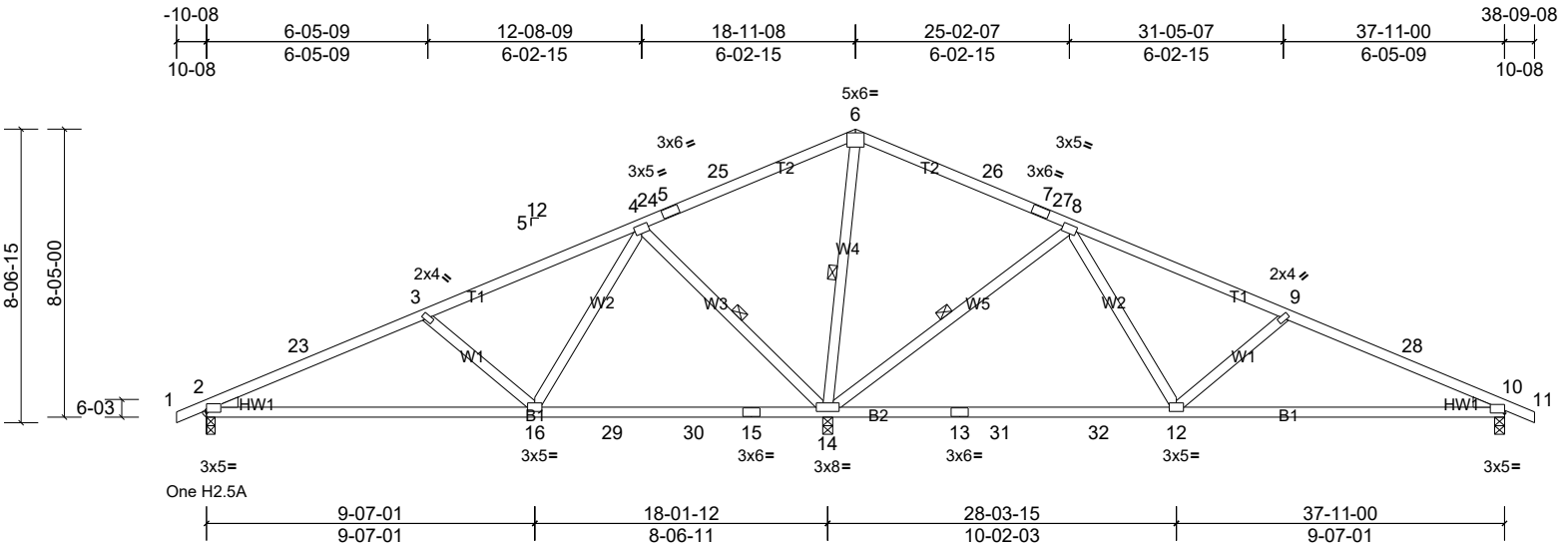
Job 25010296-A	Truss A02W	Truss Type Common	Qty 2	Ply 1	60 Harnett Lakes-Roof-Havilland A
					Job Reference (optional)

Carter Components, Sanford, NC, user

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-11]

Loading	(psf)	Spacing	2-00-00	CSI	0.77	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.77	Vert(LL)	-0.27	12-14	>893	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.44	12-14	>546	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 189 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3
 WEDGE Left: 2x4 SP No.3
 Right: 2x4 SP No.3

BRACING

TOP CHORD
 BOT CHORD
 WEBS

Structural wood sheathing directly applied or 5-1-4 oc purlins.
 Rigid ceiling directly applied or 2-2-0 oc bracing.
 1 Row at midpt 4-14, 6-14, 8-14

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=538/3-00, (min. 1-08), 10=624/3-08, (min. 1-08),
 14=1977/3-08, (min. 2-09)
 Max Horiz 2=131 (LC 14)
 Max Uplift 2=-81 (LC 14), 10=-110 (LC 15), 14=-168 (LC 14)
 Max Grav 2=628 (LC 38), 10=719 (LC 6), 14=2147 (LC 3)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 2-23=-835/100, 3-23=-772/125, 3-4=-547/69, 4-24=-38/583, 5-24=-36/595, 5-25=-26/630, 6-25=-21/703, 6-26=0/613,
 7-26=0/540, 7-27=-7/505, 8-27=-9/493, 8-9=-789/134, 9-28=-1002/191, 10-28=-1042/175
 BOT CHORD 2-16=-198/719, 13-14=-28/338, 13-31=-28/338, 31-32=-28/338, 12-32=-28/338, 10-12=-101/926
 WEBS 3-16=-452/194, 4-16=-27/745, 4-14=-896/230, 6-14=-813/138, 8-14=-992/232, 8-12=-7/792, 9-12=-435/194

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard

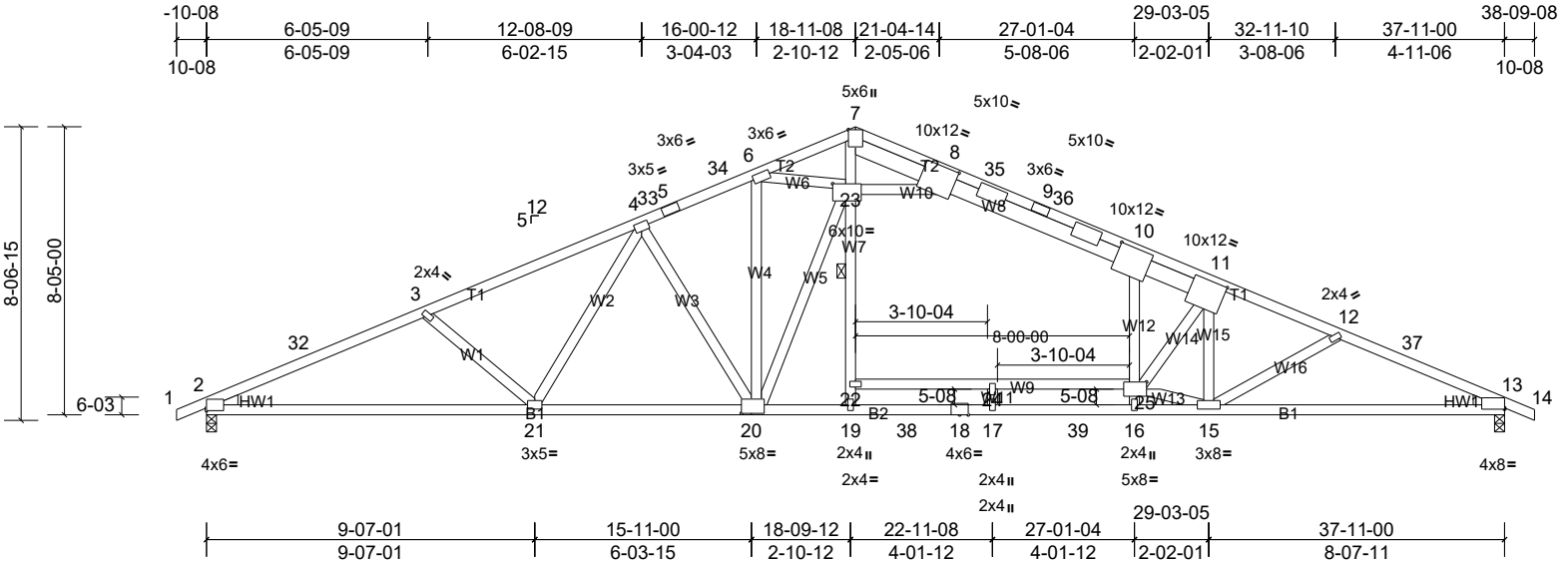
Job 25010296-A	Truss A04H	Truss Type Common	Qty 6	Ply 1	60 Harnett Lakes-Roof-Havilland A
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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-15], [8:6-00,3-08], [10:6-00,7-08], [11:6-00,7-00], [13:Edge,0-07], [20:3-08,3-00], [23:4-08,2-00], [25:2-08,2-08]

Loading	(psf)	Spacing	2-00-00	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	-0.25	16	>999	240	MT20 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.50	Vert(CT)	-0.60	16-17	>761	180	
TCDL	10.0	Rep Stress Incr	YES	WB	0.89	Horz(CT)	0.11	13	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0										Weight: 250 lb FT = 20%

LUMBER
TOP CHORD 2x4 SP 2400F 2.0E
BOT CHORD 2x4 SP 2400F 2.0E
WEBS 2x4 SP No.3 *Except* W8:2x6 SP 2400F 2.0E
WEDGE Left: 2x4 SP No.3
Right: 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-0-12 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 7-19

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS (lb/size) 2=1648/3-08, (min. 1-08), 13=1690/3-08, (min. 1-08)
Max Horiz 2=131 (LC 18)
Max Uplift 2=-90 (LC 14), 13=-48 (LC 15)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-32=-3342/139, 3-32=-3274/164, 3-4=-3064/132, 4-33=-2513/136, 5-33=-2506/136, 5-34=-2497/138, 6-34=-2455/150, 6-7=-691/381, 7-8=-708/257, 8-35=-2476/103, 9-35=-2486/101, 9-36=-2458/98, 10-36=-2507/87, 10-11=-3265/0, 11-12=-3398/20, 12-37=-3368/65, 13-37=-3438/46
BOT CHORD 2-21=-213/3008, 20-21=-26/2521, 19-20=0/2434, 19-38=0/2392, 18-38=0/2392, 17-18=0/2392, 17-39=0/2392, 16-39=0/2392, 15-16=0/2304, 13-15=0/3094
WEBS 19-22=0/428, 22-23=0/458, 7-23=-37/587, 4-21=-32/470, 3-21=-365/203, 8-23=-2666/303, 4-20=-611/178, 6-20=-121/1130, 6-23=-2264/281, 20-23=-921/108, 16-25=0/286, 10-25=0/1446, 11-15=-295/0, 11-25=-1128/172, 15-25=0/927

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TC DL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
 - 200.0lb AC unit load placed on the bottom chord, 22-11-8 from left end, supported at two points, 5-0-0 apart.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.

LOAD CASE(S) Standard