

Carter Sanford Component Plant 298 Harvey Faulk Rd Sanford, NC 27332

Phone #:919-775-1450

Builder: A&G Residential

Model: 60 Harnett Lakes/ Havilland

THE PLACEMENT PLAN NOTES:

1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.

2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.

3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.

4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.

5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.

6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.

7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.

8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death. 9. All uplift connectors shown within these documents are recommendations only. Per ANSI/TPI 1, all uplift connectors are the responsibility of the building designer and or contractor.

Approved By: _____

Date: _____

General Notes:	** CUTTING OR DRILLING OF COMPONENTS SHOULD NOT BE DONE WITHOUT CONTACTING COMPONENT SUPPLIER



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	THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual components to be incorporated into the building design at the specification of the building designer. See Individual design sheets for	each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor evetems and for the overall structure. The disign of the triss support structure	including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding the bracing, consult "Bracing	of Wood Truss" available from the Truss Plate Institute, 583 D'Onifrio Drive: Madison, WI 53179					
				8					
	A&G Residential	60 Harnett	Lakes-Roof-Havilland A	ROOF PLACEMENT PLAN					
	A&G Residential	60 Harnett	Lakes-Roof-Havilland A	ROOF PLACEMENT PLAN					
	Scale:	<pre>60 Harnett</pre>	Lakes-Roof-Havilland A	ROOF PLACEMENT PLAN					
	Scale:	60 Harnett	Lakes-Roof-Havilland A	ROOF PLACEMENT PLAN					
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	Scale:	60 Harnett	Lakes-Roof-Havilland A Image: State of the s	ROOF PLACEMENT PLAN					
	Scale:	60 Harnett	Lakes-Roof-Havilland A Image: State of the s	BOF PLACEMENT PLAN					
	Scale:	60 Harnett	Lakes-Roof-Havilland A Image: Stress Stre						
	Scale: Date: 25	60 Harnett	Lakes-Roof-Havilland A Image: State of the st	BOF PLACEMENT PLAN					

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A01	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Tue Feb 04 13:35:11 Page: 1 ID:Ri0J5R6gDYoSCEpKBd7tWWzoaqU-kDpp6vzF2TXp48mQz9cBoUZIsKzWLDtLxAGQUwzoWSU

Structural wood sheathing directly applied or 10-0-0 oc purlins.

Rigid ceiling directly applied or 6-0-0 oc bracing.



Scale = 1:67.5

Plate Offsets (X, Y): [12:3-00,Edge]

Loading	(psf)	Spacing	1-11-04	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.37	Vert(LL)	n/a	-	n/a	999	MT20	244/190	
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.37	Vert(CT)	n/a	-	n/a	999			
TCDL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(CT)	0.01	22	n/a	n/a			
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH									
BCDL	10.0										Weight: 219 lb	FT = 20%	

BRACING TOP CHORD

BOT CHORD

LUMBER

TOP CHORD	2x4 SP No.2
BOT CHORD	2x4 SP No.2
OTHERS	2x4 SP No.3

- REACTIONS All bearings 37-11-00.
 - (lb) Max Horiz 2=127 (LC 14), 42=127 (LC 14)

10-11=-42/251

- Max Uplift All uplift 100 (lb) or less at joint(s) 2, 24, 25, 26, 27, 28, 29, 30, 35, 36, 37, 38, 39, 40, 41, 42
 - Max Grav All reactions 250 (lb) or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 35, 36, 37, 38, 39, 40, 42 except 24=480 (LC 6), 32=475 (LC 6), 22=441 (LC 6), 41=260 (LC 2)
 - 6), 33=441 (LC 5), 41=259 (LC 3)
- FORCES (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

TOP CHORD

WEBS 11-33=-325/32, 13-32=-370/32, 21-24=-277/144

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-11-8, Exterior(2N) 2-11-8 to 14-11-8, Corner(3R) 14-11-8 to 22-11-8, Exterior(2N) 22-11-8 to 34-11-8, Corner(3E) 34-11-8 to 38-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) Truss designed for wind loads in the plane of the truss only. For study exposed to wind (normal to the fare) sconstander loads in two constants. Constant Constants are placed to wind (normal to the fare).
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 5) Unbalanced snow loads have been considered for this design.
- 6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 7) All plates are 2x4 MT20 unless otherwise indicated.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 35, 36, 37, 38, 39, 40, 41, 30, 29, 28, 27, 26, 25, 24, 2.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A02	Common	6	1	Job Reference (optional)

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-11]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.77	Vert(LL)	-0.27	12-14	>893	240	MT20	244/190	
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.44	12-14	>546	180			
TCDL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.02	2	n/a	n/a			
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH									
BCDL	10.0										Weight: 189 lb	FT = 20%	

LUMBER			BRACING	
TOP CHORI	D 2x4 SP	No.2	TOP CHORD	Structural wood sheathing directly applied or 5-1-4 oc purlins.
BOT CHORI	D 2x4 SP	No.2	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2x4 SP	No.3	WEBS	1 Row at midpt 4-14, 6-14, 8-14
WEDGE Left: 2x4 SP No.3		4 SP No.3		MiTek recommends that Stabilizers and required cross bracing be
	Right: 2	x4 SP No.3		installed during truss erection, in accordance with Stabilizer
REACTION	S (lb/size)	2=538/3-00, (min. 1-08), 10=624/3-08, (min. 1-08),		Installation guide.
	Max Horiz	14 - 1977/3 - 00, (11111, 2 - 09) 2 - 131/1 - 01/3		
	Max Liplift	2 = 131 (LC 14) 2 = .81 (LC 14) 10 = .110 (LC 15) 14 = .168 (LC 14)		
	Max Grav	2=616 (LC 14), 10=710 (LC 6), 14=7100 (LC 14)		
TOP CHOP	- (DI)	Max. Comp./Max. 1 en All forces 250 (ib) of less except when snow $-256/400 + 250-370/405 + 24-547/60 + 24-38/592 + 24-36/505$		01/202 6 26-0/612
TOP CHORI	D 2-23 7 26	-033/100, 3-23172/123, 3-4347/09, 4-2430/303, 3-2430/393, -0/540, 7, 37-, 7/505, 9, 37-, 0/402, 9, 0-, 780/124, 0, 38-, 1003/101, 10	, 3-2320/030, 0-232 29- 1042/175	21/703, 0-20=0/013,
	7-20	-0/340, 7-277/303, 0-279/493, 0-9709/134, 9-201002/191, 10 - 108/710, 13, 1/- 28/338, 13, 31- 28/338, 31, 3228/338, 12, 3228	-201042/170 2/338 10 12- 101/026	
WERS	2-10	-150/113, 15-14-20/330, 15-31-20/330, 31-32-20/330, 12-32-20	730, 10 - 12 - 101/920	- 435/104
VLDS	5-10	452/194, 4-1027/745, 4-14890/250, 0-14815/158, 8-14992/	232, 0-121/192, 9-12-	
NOTES				
1) Unbala	nced roof live	e loads have been considered for this design.		
2) Wind: A	ASCE 7-16; \	/ult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.	0psf; h=25ft; Cat. II; Ex	p B; Enclosed; MWFRS (envelope) exterior zone and C-C
Exterio	r(2E) -0-10-8	to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0,	Interior (1) 22-9-0 to 35	b-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and
right ex	posea; ena	vertical left and right exposed; U-U for members and forces & MWFRS	for reactions shown; L	umber DOL=1.60 plate grip DOL=1.60
3) TOLL: /	ASCE 7-16; I	2r=20.0 pst (root LL: Lum DOL=1.15 Plate DOL=1.15); Pt=20.0 pst (Li	um DOL=1.15 Plate DC	DL=1.15); IS=1.0; Rough Cat B; Fully Exp.; Ce=0.9; CS=1.00;
4) Unhala	J naad anaw la	ada have been considered for this design		
 4) Unbala 5) This true 		adds have been considered for this design.	reafland of 20.0 peter	a avarbanza nan aanavraat with athar live laada
6) This tru	iss nas been	designed for a 10.0 per better abord live load of 12.0 psi of 1.00 times that	roor load of 20.0 psi of	n overhangs non-concurrent with other live loads.
7) * This tru	iss has been	designed for a 10.0 psi bollom chold live load honconcurrent with any		

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. 7)

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connection is for uplift only and does not 8) consider lateral forces.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A03	Common	1	1	Job Reference (optional)

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Structural wood sheathing directly applied or 4-11-8 oc purlins.

8-14. 6-14 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

Rigid ceiling directly applied or 2-2-0 oc bracing.

1 Row at midpt

Installation guide.



Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-11]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.75	Vert(LL)	-0.27	12-14	>883	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.99	Vert(CT)	-0.44	12-14	>536	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.70	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 189 lb	FT = 20%

BRACING TOP CHORD

WFBS

BOT CHORD

LUMBER		
TOP CHORD	2x4 SP No.2	
BOT CHORD	2x4 SP No.2	
WEBS	2x4 SP No.3	
WEDGE	Left: 2x4 SP No.3	
	Right: 2x4 SP No.3	

REACTIONS All bearings 18-03-08. except 10=3-08

(lb) - Max Horiz 2=131 (LC 14), 17=131 (LC 14)

Max Uplift All uplift 100 (lb) or less at joint(s) 2, 17 except 10=-102 (LC

15), 14=-169 (LC 15), 16=-136 (LC 14)

Max Grav All reactions 250 (lb) or less at joint(s) except 2=488 (LC 38), 10=740 (LC 6), 14=1892 (LC 6), 16=502 (LC 36), 17=488 (LC

38)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-23=-491/15, 3-23=-450/40, 4-24=0/435, 5-24=0/443, 5-25=0/466, 6-25=0/539, 6-26=0/495, 7-26=0/389, 7-27=-5/354,

8-27=-6/342, 8-9=-889/114, 9-28=-1144/170, 10-28=-1196/155

BOT CHORD 2-16=-178/435, 13-14=0/373, 13-31=0/373, 31-32=0/373, 12-32=0/373, 10-12=-83/1047

WEBS 8-14=-990/232, 8-12=-8/786, 9-12=-430/195, 4-16=-35/449, 3-16=-462/198, 6-14=-700/124, 4-14=-609/129

NOTES

Unbalanced roof live loads have been considered for this design. 1)

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; 3) Ct=1.10

4) Unbalanced snow loads have been considered for this design.

This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. 5)

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 6)

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and 7) any other members, with BCDL = 10.0psf.

One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, 16, and 10. This connection is for uplift only and does 8) not consider lateral forces.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A04	Common	5	1	Job Reference (optional)

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Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-15], [10:Edge,0-15]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	-0.28	14-16	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.50	14-16	>917	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.57	Horz(CT)	0.12	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 189 lb	FT = 20%

LUMBER TOP CHO BOT CHO WEBS WEDGE	2 DRD 2x4 SP 2400F 2.0E DRD 2x4 SP 2400F 2.0E 2x4 SP No.3 Left: 2x4 SP No.3 Right: 2x4 SP No.3	BRACING TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly applied or 4-0-10 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 8-14, 4-14 MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer
REACTIC	 DNS (lb/size) 2=1569/3-08, (min. 1-08), 10=1569/3-08, (min. 1-08) Max Horiz 2=131 (LC 14) Max Uplift 2=-169 (LC 14), 10=-169 (LC 15) Max Grav 2=1695 (LC 3), 10=1695 (LC 3) 	Installation guide.	
FORCES TOP CHO	 (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except whe 2-23=-3803/322, 3-23=-3733/347, 3-4=-3533/322, 4-24=-2518/292, 6-26=-2452/321, 7-26=-2471/304, 7-27=-2510/295, 8-27=-2518/292 	n shown. 5-24=-2510/295, 5-25=-247 2, 8-9=-3533/322, 9-28=-373	/1/304, 6-25=-2452/321, 33/347, 10-28=-3803/322
BOT CHO	DRD 2-16=-361/3431, 16-29=-216/2855, 29-30=-216/2855, 15-30=-216/2 13-31=-164/2855, 31-32=-164/2855, 12-32=-164/2855, 10-12=-237	2855, 14-15=-216/2855, 13-1 /3431	14=-164/2855,
WEBS	6-14=-80/1546, 8-14=-881/227, 8-12=-9/642, 9-12=-346/189, 4-14=	-881/227, 4-16=-9/642, 3-16	5=-346/188
NOTES			
1) Unb	alanced root live loads have been considered for this design.	DI -6 Opef: b-25ft: Cat. II: E	Exp B: Enclosed: MWERS (anyclone) exterior zone and C C
Z) Wild Exte	erior(2E) -0-10-8 to 2-11-0. Interior (1) 2-11-0 to 15-2-0. Exterior(2R) 15-2-0 to 2	22-9-0. Interior (1) 22-9-0 to	35-0-0. Exterior(2E) 35-0-0 to 38-9-8 zone: cantilever left and
right	exposed ; end vertical left and right exposed;C-C for members and forces & N	IWFRS for reactions shown;	; Lumber DOL=1.60 plate grip DOL=1.60
3) TCL Ct=1	L: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 1 10	psf (Lum DOL=1.15 Plate E	DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00;
4) Unb	alanced snow loads have been considered for this design.		
5) This	truss has been designed for greater of min roof live load of 12.0 psf or 1.00 tin	nes flat roof load of 20.0 psf	on overhangs non-concurrent with other live loads.
6) This	truss has been designed for a 10.0 psf bottom chord live load nonconcurrent v	with any other live loads.	
7) * Th	is truss has been designed for a live load of 20.0psf on the bottom chord in all	areas where a rectangle 3-0	6-00 tall by 2-00-00 wide will fit between the bottom chord and
any	other members, with BCDL = 10.0pst.	ring walls due to LIPLIET at i	t(s) 2 and 10. This connection is for unlift only and does not

 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A05	Common	3	1	Job Reference (optional)

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Scale = 1:69.9

Plate Offsets (X, Y): [10:Edge,0-07]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.73	Vert(LL)	-0.22	12-14	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.99	Vert(CT)	-0.37	12-14	>891	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.43	Horz(CT)	0.04	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 193 lb	FT = 20%

LUMBER TOP CHO BOT CHO WEBS WEDGE	RD 2x4 SP No.2 RD 2x4 SP No.1 *Except* B2:2x4 SP No.2 2x4 SP No.3 Left: 2x4 SP No.3 Right: 2x4 SP No.3	BRACING TOP CHORD BOT CHORD WEBS	Structural wood sheathing directly applied or 3-7-11 oc purlins. Rigid ceiling directly applied or 2-2-0 oc bracing. <u>1 Row at midpt</u> <u>4-16, 8-14</u> MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer						
REACTIO	NS (lb/size) 2=197/3-00, (min. 1-08), 10=1068/3-08, (min. 1-08), 16=1873/3-08, (min. 2-07) Max Horiz 2=131 (LC 14) Max Uplift 2=-107 (LC 10), 10=-152 (LC 15), 16=-155 (LC 14) Max Grav 2=296 (LC 36), 10=1174 (LC 6), 16=2058 (LC 3)		Installation guide.						
FORCES (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-24=-174/287, 3-24=-109/333, 3-4=-61/791, 4-25=-958/152, 5-25=-950/155, 5-26=-910/164, 6-26=-892/181, 6-27=-892/181, 7-27=-911/164, 7-28=-951/155, 8-28=-958/153, 8-9=-2064/243, 9-29=-2282/292, 10-29=-2341/266 BOT CHORD 2-17=-265/179, 16-17=-336/158, 13-14=-48/1438, 13-32=-48/1438, 32-33=-48/1438, 12-33=-48/1438, 10-12=-193/2094 WEBS 3-16=-567/246, 4-16=-1809/240, 4-14=-20/957, 6-14=-26/343, 8-14=-911/229, 8-12=-14/725, 9-12=-386/188									
NOTES 1) Unba 2) Wind Exter right 3) TCLL Ct=1.	JTES Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00;								
 4) Unba 5) This t 6) This t 7) * This any o 	lanced snow loads have been considered for this design. russ has been designed for greater of min roof live load of 12.0 psf or 1.00 times fla russ has been designed for a 10.0 psf bottom chord live load nonconcurrent with ar s truss has been designed for a live load of 20.0psf on the bottom chord in all areas ther members, with BCDL = 10.0psf.	t roof load of 20.0 psf o ny other live loads. where a rectangle 3-06	n overhangs non-concurrent with other live loads. -00 tall by 2-00-00 wide will fit between the bottom chord and						

 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 16, and 10. This connection is for uplift only and does not consider lateral forces.

9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

10) Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A06	Common Supported Gable	1	1	Job Reference (optional)

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Tue Feb 04 13:35:1: Page: 1 ID:Ri0J5R6gDYoSCEpKBd7tWWzoaqU-CPNBJF_tonfgilKcWs8QLi5xtkKo4gGUAq?_1NzoWST



Scale = 1:67.5

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.24	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.37	Horz(CT)	-0.01	24	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 230 lb	FT = 20%

LUMBER TOP CHORD BOT CHORD OTHERS REACTIONS All (lb) - Ma Ma	2x4 SP No.2 2x4 SP No.2 2x4 SP No.3 bearings 37-11-00. x Horiz 2=131 (LC 14), 43=131 (LC 14) x Uplift All uplift 100 (lb) or less at joint(s) 2, 24, 25, 26, 27, 28, 29, 30, 32, 34, 36, 37, 38, 39, 40, 41, 42, 43 x Grav All reactions 250 (lb) or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 32, 34, 36, 37, 38, 39, 40, 41, 43 except 24=389 (LC 1), 33=329 (LC 22), 42=260 (LC 1)	BRACING TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 10-0-0 oc purlins. Rigid ceiling directly applied or 6-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.
FORCES TOP CHORD WEBS	(lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown 2-3=-131/280, 3-4=-86/259, 4-5=-56/266, 5-6=-22/264, 6-7=0/266, 7-8=-10 11-12=-55/286, 12-13=-55/282, 13-14=-40/279, 14-15=-20/279, 16-17=-10, 19-20=-45/271, 21-22=-110/290 12-33=-288/0	n. /273, 9-10=-20/280, 10 /266, 17-18=0/267, 18-	0-11=-40/289, -19=-19/265,
NOTES			

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-8, Interior (1) 2-11-8 to 14-11-8, Exterior(2R) 14-11-8 to 22-11-8, Interior (1) 22-11-8 to 34-11-8, Exterior(2E) 34-11-8 to 38-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

5) Unbalanced snow loads have been considered for this design.

6) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

9) Gable studs spaced at 2-0-0 oc.

10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 34, 36, 37, 38, 39, 40, 41, 42, 32, 30, 29, 28, 27, 26, 25, 24, 2.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A02H	Common	1	1	Job Reference (optional)

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Tue Feb 04 13:35:1: Page: 1 ID:1Y18ZNqcDgNaSMJuWpvROSzoaTb-hbwZXb?VZ5nXKSvo4Zfftvexp8cRp0HePUIXZpzoWSS



Scale = 1:67.5

Plate	Offsets (X	K, Y): [2:Edge,0-1	5], [8:6-00,3-08], [10:6	6-00,7-08], [11:6-00,7-00]	, [13:Edge,0-15]	, [20:3-08,3	3-00], [23:4-0	08,2-04],	[25:2-0	8,2-08]			
Load TCLI Snov TCD BCLI BCD	ling ∟ (roof) v (Pf) L L L	(psf) 20.0 20.0 10.0 0.0 10.0	 Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code 	2-00-00 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MSH	0.98 0.47 0.77	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.29 -0.57 0.10	(loc) 16 16-17 13	l/defl >999 >793 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 250 lb	GRIP 244/190 FT = 20%
LUN TOP BOT WEE WEE	IBER CHORD CHORD CHORD 3S DGE	2x4 SP 2400F 2x4 SP 2400F 2x4 SP No.3 *I Left: 2x4 SP N Right: 2x4 SP	2.0E *Except* T3:2x4 2.0E Except* W8:2x6 SP N o.3 No.3	SP No.1 o.2		BRACIN TOP CH BOT CH WEBS	G ORD ORD	Structu Rigid ca 1 Row a MiTek installe	ral wood eiling dir at midpt recomm ed during	d sheath rectly ap nends th g truss e	ing dir plied c at Stat	ectly applied or 3 or 10-0-0 oc brac 7-19 pilizers and requi n, in accordance	3-0-5 oc purlins. ing. ired cross bracing be with Stabilizer
REA	ACTIONS (Ib/size) 2=1569/3-08, (min. 1-08), 13=1569/3-08, (min. 1-08) Max Horiz 2=131 (LC 14) Max Uplift 2=-169 (LC 14), 13=-169 (LC 15)												
FOR TOP BOT WEE	CHORD CHORD	(lb) - Max. (2-32=-3153 6-7=-675/4 11-12=-309 2-21=-367/2 15-16=-153 19-22=0/30 6-20=-227/	Comp./Max. Ten All 1 /328, 3-32=-3086/354 16, 7-8=-684/296, 8-35 2/325, 12-37=-3095/3 2835, 20-21=-206/234 /2145, 13-15=-248/28 2, 22-23=0/324, 7-23= 1051, 6-23=-2121/483	forces 250 (lb) or less exc , 3-4=-2872/325, 4-33=-2 5=-2247/346, 9-35=-2257, 46, 13-37=-3161/327 0, 19-20=-131/2201, 18-1 44 -85/543, 4-21=-26/476, 3 , 20-23=-795/274, 10-25=	cept when show 316/332, 5-33=- /336, 9-36=-223 9=-128/2167, 1 3-21=-371/197, 8 138/1185, 11-2	n. 2308/333, 8/329, 10-3 7-18=-128/ 9-23=-2462 25=-1063/2	5-34=-2300/ 36=-2287/32 2167, 16-17 //576, 4-20=- 20, 15-25=-{	335, 6-3 0, 10-11 =-128/21 613/175 99/786	4=-2258 =-2917/ 67,	3/346, 338,			
NOT	ES		,,	, ,	,		-,						
1) 2) 3)	Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-11-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00;												
4) 5) 6) 7)	Ut= 1.10 Unbalanced snow loads have been considered for this design. This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads. This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.												

 One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A02W	Common	2	1	Job Reference (optional)

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Tue Feb 04 13:35:13 Page: 1 $\label{eq:linear} ID: 2gI0XXyaSg2L0GxMy1SvzhzoauY-hbwZXb?VZ5nXKSvo4Zfftve_68USp6vePUIXZpzoWSS$



Scale = 1:67.5

Plate Offsets (X, Y): [2:Edge,0-11]

Loading	(psf)	Spacing	2-00-00	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.77	Vert(LL)	-0.27	12-14	>893	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.98	Vert(CT)	-0.44	12-14	>546	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0										Weight: 189 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	0 2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 5-1-4 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt 4-14, 6-14, 8-14
WEDGE	Left: 2x4 SP No.3		MiTek recommends that Stabilizers and required cross bracing be
	Right: 2x4 SP No.3		installed during truss proction in accordance with Stabilizer
DEACTIONO	(ll /===) 0=500/2 00 /==== 4 00) 40=604/2 00 /==== 4 00)		Installed during truss election, in accordance with Stabilizer
REACTIONS	(ID/SIZE) 2=538/3-00, (MIN. 1-08), 10=624/3-08, (MIN. 1-08),		
	14 = 1977/3 = 000, (111111, 2 = 0.09)		
	Max = Harrist = 2 = 131 (LC + 14) $Max = Harrist = 2 = 131 (LC + 14) = 140 (LC + 15) = 14 = 460 (LC + 14)$		
	$\operatorname{Max} \operatorname{Oplim} 2=-81 (\operatorname{LC} 14), 10=-110 (\operatorname{LC} 15), 14=-168 (\operatorname{LC} 14)$		
	Max Grav 2=628 (LC 38), 10=719 (LC 6), 14=2147 (LC 3)		
FORCES	(lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when show	'n.	
TOP CHORD	2-23=-835/100, 3-23=-772/125, 3-4=-547/69, 4-24=-38/583, 5-24=-36/595	, 5-25=-26/630, 6-25=-2	21/703, 6-26=0/613,
	7-26=0/540, 7-27=-7/505, 8-27=-9/493, 8-9=-789/134, 9-28=-1002/191, 10	-28=-1042/175	
BOT CHORD	D 2-16=-198/719, 13-14=-28/338, 13-31=-28/338, 31-32=-28/338, 12-32=-28	8/338, 10-12=-101/926	
WEBS	3-16=-452/194, 4-16=-27/745, 4-14=-896/230, 6-14=-813/138, 8-14=-992/2	232, 8-12=-7/792, 9-12	=-435/194
NOTES			
1) Unbalan	nced roof live loads have been considered for this design		
2) Wind: AS	SCE 7-16: Vult=130mph (3-second gust) Vasd=103mph; TCDI =6.0psf; BCDI =6.	Onsf: h=25ft: Cat_II: Ex	n B: Enclosed: MWERS (envelope) exterior zone and C-C
Exterior((2E) -0.10-8 to 2-11.0 Interior (1) 2-11.0 to 15-2-0 Exterior(2R) 15-2-0 to 22-9-0	Interior (1) 22-9-0 to 34	5-0-0 Exterior(2E) 35-0-0 to 38-9-8 zone: cantilever left and
right eyn	nosed : end vertical left and right exposed C.C for members and forces & MWERS	for reactions shown: I	μ mber DOI =1.60 plate grip DOI =1.60
3) TCLL · A	Δ SCE 7-16: Pr=20.0 nsf (roof LL: Lum DOL =1.15 Plate DOL =1.15): Pf=20.0 nsf (L	μ m DOI =1 15 Plate DC	1 = 1.15): $1 = 1.00$ plate grip BOE = 1.00
Ct=1 10)		52-1.10), 13-1.0, Rough Out D, 1 uny Exp., 00-0.0, 03-1.00,
4) Unhalan	, nced snow loads have been considered for this design		
5) This true	ss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat	roof load of 20.0 psf or	overbangs pon-concurrent with other live loads
6) This true	ss has been designed for a 10.0 nsf bottom chord live load nonconcurrent with an	v other live loads	r overhangs non-concurrent with other live loads.
	as has been designed for a 10.0 psi bottom chord live load honconcurrent with an	y other live loads.	

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf. One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 14, and 10. This connection is for uplift only and does not 7)

8) consider lateral forces.

Job	Truss	Truss Type	Qty	Ply	60 Harnett Lakes-Roof-Havilland A
25010296-A	A04H	Common	6	1	Job Reference (optional)

Run: 8.73 S Jul 11 2024 Print: 8.730 S Jul 11 2024 MiTek Industries, Inc. Tue Feb 04 13:35:13 Page: 1 ID:1Y18ZNqcDgNaSMJuWpvROSzoaTb-hbwZXb?VZ5nXKSvo4Zfftve368cyp?PePUIXZpzoWSS



Scale = 1:67.5

Plate	e Offsets (X, Y): [2:Edge,0-15],	[8:6-00,3-08], [10:6-0	00,7-08], [11:6-00,7-00]	, [13:Edge,0-07]	, [20:3-08,	3-00], [23:4-0	08,2-00],	[25:2-0	8,2-08]			
Loa TCL Sno TCC BCL	ding L (roof) w (Pf) DL L	(psf) 20.0 20.0 10.0 0.0*	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code	2-00-00 1.15 1.15 YES IRC2021/TPI2014	CSI TC BC WB Matrix-MSH	0.45 0.50 0.89	DEFL Vert(LL) Vert(CT) Horz(CT)	in -0.25 -0.60 0.11	(loc) 16 16-17 13	l/defl >999 >761 n/a	L/d 240 180 n/a	PLATES MT20	GRIP 244/190
BCL		10.0		-								Weight: 250 lb	FT = 20%
LUN TOF BO WE WE	LUMBER TOP CHORD 2x4 SP 2400F 2.0E BOT CHORD 2x4 SP 2400F 2.0E WEBS 2x4 SP No.3 *Except* W8:2x6 SP 2400F 2.0E WEDGE Left: 2x4 SP No.3 Right: 2x4 SP No.3						G ORD ORD	Structu Rigid ce <u>1 Row a</u> MiTek installe	Structural wood sheathing directly applied or 4-0-12 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. <u>1 Row at midpt</u> MiTek recommends that Stabilizers and required cross bracing be installed during trues exerction in accordance with Stabilizer.				
REACTIONS (lb/size) 2=1648/3-08, (min. 1-08), 13=1690/3-08, (min. 1-08) Installatin Max Horiz 2=131 (LC 18) Installatin Max Uplift 2=-90 (LC 14), 13=-48 (LC 15) Installatin								ation gui	de.		,		
FORCES (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD 2-32=-3342/139, 3-32=-3274/164, 3-4=-3064/132, 4-33=-2513/136, 5-33=-2506/136, 5-34=-2497/138, 6-34=-2455/150, 6-7=-691/381, 7-8=-708/257, 8-35=-2476/103, 9-35=-2486/101, 9-36=-2458/98, 10-36=-2507/87, 10-11=-3265/0, 11-12=-3398/20, 12-37=-3368/65, 13-37=-3438/46 BOT CHORD 2-21=-213/3008, 20-21=-26/2521, 19-20=0/2434, 19-38=0/2392, 18-38=0/2392, 17-18=0/2392, 17-39=0/2392.													
WEBS 19-22=0/428, 22-23=0/458, 7-23=-37/587, 4-21=-32/470, 3-21=-365/203, 8-23=-2666/303, 4 6-20=-121/1130, 6-23=-2264/281, 20-23=-921/108, 16-25=0/286, 10-25=0/1446, 11-15=-29! 15-25=0/927							/303, 4-20=- 15=-295/0, 1	611/178 1-25=-11	28/172				
NO 1) 2) 3)	 FS Unbalanced roof live loads have been considered for this design. Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-11-0, Interior (1) 2-10-0 to 15-2-0, Exterior(2R) 15-2-0 to 22-9-0, Interior (1) 22-9-0 to 35-0-0, Exterior(2E) 35-0-0 to 38-9-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10 												
4) 5)	Unbaian This trus	cea show loads have	for greater of min ro	this design.	or 1 00 times flat	roof load o	of 20.0 nsf.or	overhai	nas non	-concur	ent wit	th other live load	c

on overhangs non-concurrent with other live loads.

6) 200.0lb AC unit load placed on the bottom chord, 22-11-8 from left end, supported at two points, 5-0-0 apart.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and 8) any other members.

9) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 13. This connection is for uplift only and does not consider lateral forces.