



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Jonathan Landry

Jonathan Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (1)LY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (1)LY HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (1)LY HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

All Headers Are Considered 2X10 Beams Unless Otherwise Noted

All Walls Shown Are Considered Load Bearing

Roof Area = 2396.08 sq.ft.
Ridge Line = 81.8 ft.
Hip Line = 0 ft.
Horiz. OH = 151.03 ft.
Raked OH = 241.11 ft.
Decking = 82 sheets

Dimension Notes

- All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
- All interior wall dimensions are to face of stud unless noted otherwise
- All exterior wall to truss dimensions are to face of stud unless noted otherwise

Hatch Legend

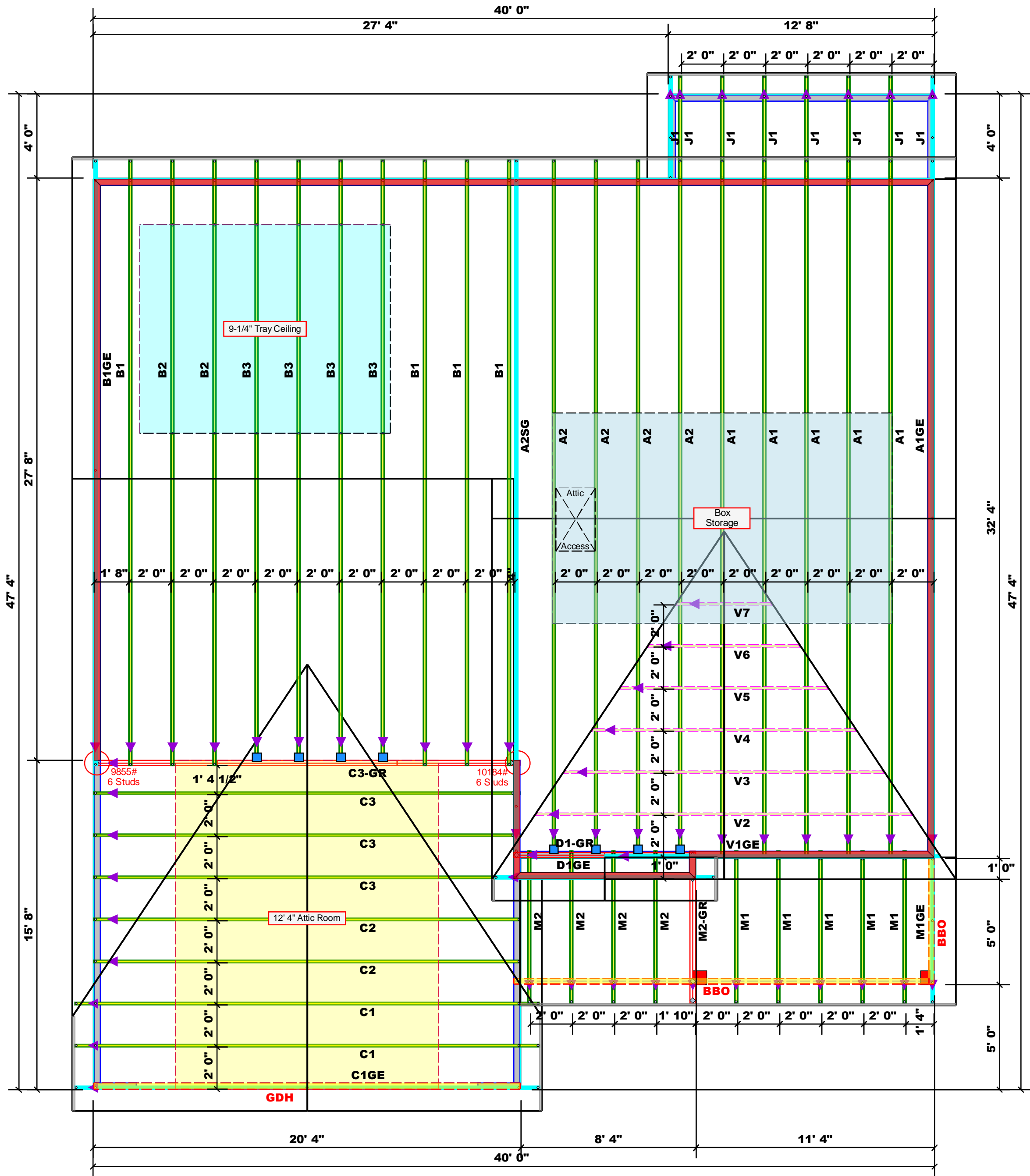
- Second Floor Walls
- Box Storage
- Tray Ceiling
- Drop Beam
- Flush Beam

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
	HUS26	USP	8	NA	16d/3-1/2"	16d/3-1/2"

Products				
PlotID	Length	Product	Plies	Net Qty
BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM2	5' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	13' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM4	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM5	11' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH	21' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

Truss Placement Plan

Scale: 1/4"=1'



△ = Indicates Left End of Truss

(Reference Engineered Truss Drawing)

Do NOT Erect Truss Backwards

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY.
These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: J0424-2426
Delude Residence

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I66592130 thru I66592156

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



July 2, 2024

Gilbert, Eric

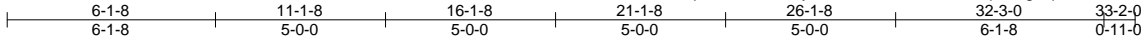
IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	A1	COMMON	5	1	166592130
					Job Reference (optional)

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:38:57 2024 Page 1

ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f



5x5 =

Scale = 1:67.7

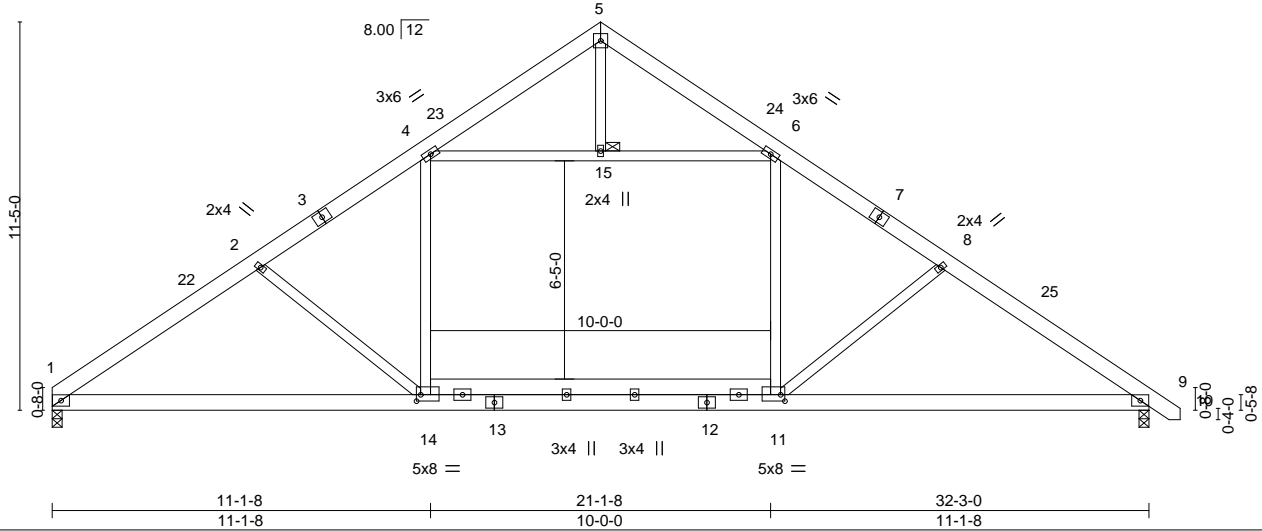


Plate Offsets (X,Y)--		[11:0-1-8,0-2-4], [14:0-1-8,0-2-4]							
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15		TC 0.21		Vert(LL) -0.24 14-18 >999 360		MT20	244/190
TCDL	10.0	Lumber DOL 1.15		BC 0.53		Vert(CT) -0.29 14-18 >999 240			
BCLL	0.0 *	Rep Stress Incr YES		WB 0.49		Horz(CT) 0.05 9 n/a n/a			
BCDL	10.0	Code IRC2015/TPI2014		Matrix-AS		Wind(LL) 0.27 14-18 >999 240		Weight: 252 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
JOINTS 1 Brace at Jt(s): 15

REACTIONS.

(size) 1=0-3-8, 9=0-3-8
Max Horz 1=-354(LC 8)
Max Uplift 1=-219(LC 12), 9=-236(LC 13)
Max Grav 1=1455(LC 19), 9=1499(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2143/726, 2-4=-1937/676, 4-5=-471/260, 5-6=-471/260, 6-8=-1940/669, 8-9=-2140/714
BOT CHORD 1-14=-443/1986, 11-14=-230/1662, 9-11=-427/1714
WEBS 8-11=-445/292, 2-14=-444/292, 4-14=-65/648, 6-11=-56/646, 4-15=-1355/515, 6-15=-1355/515

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 16-1-8, Exterior(2) 16-1-8 to 20-6-5, Interior(1) 20-6-5 to 33-0-1 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- All plates are 4x6 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 219 lb uplift at joint 1 and 236 lb uplift at joint 9.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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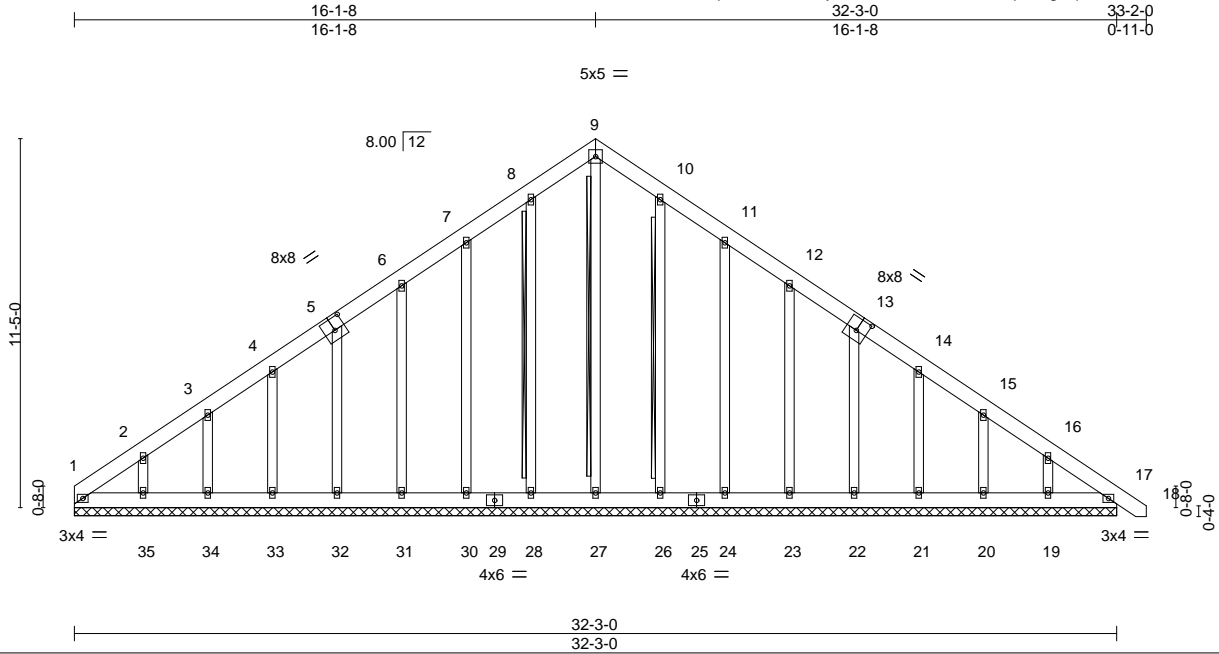
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	A1GE	GABLE	1	1	166592131
					Job Reference (optional)

Comtech, Inc., Fayetteville, NC - 28314,

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:71.3

Plate Offsets (X,Y)--		[5:0-4-0,0-4-8], [13:0-4-0,0-4-8]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCLL 20.0	Plate Grip DOL	1.15	TC 0.06
TCDL 10.0	Lumber DOL	1.15	BC 0.03
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.22
BCDL 10.0	Code	IRC2015/TPI2014	Matrix-S
		DEFL.	in (loc) l/defl L/d
		Vert(LL)	0.00 17 n/r 120
		Vert(CT)	0.00 17 n/r 120
		Horz(CT)	0.01 17 n/a n/a
		PLATES	GRIP
		MT20	244/190
		Weight: 293 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD
BOT CHORD
WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.
T-Brace: 2x4 SPF No.2 - 9-27, 8-28, 10-26
Fasten (2X) T and I braces to narrow edge of web with 10d
(0.131"x3") nails, 6in o.c., with 3in minimum end distance.
Brace must cover 90% of web length.

REACTIONS.

All bearings 32-3-0.
(lb) - Max Horz 1=-444(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 17, 27, 28, 26 except 1=-144(LC 10),
30=-148(LC 12), 31=-140(LC 12), 32=-133(LC 12), 33=-119(LC 12), 34=-129(LC
12), 35=-186(LC 12), 24=-152(LC 13), 23=-140(LC 13), 22=-133(LC 13),
21=-119(LC 13), 20=-132(LC 13), 19=-169(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 17, 28, 30, 31, 32, 33, 34, 35,
26, 24, 23, 22, 21, 20, 19 except 27=285(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-464/343, 2-3=-333/286, 3-4=-275/251, 6-7=-196/281, 7-8=-284/346, 8-9=-331/381,
9-10=-331/381, 10-11=-284/321, 16-17=-356/243
BOT CHORD 1-35=-224/350, 34-35=-224/350, 33-34=-224/350, 32-33=-224/350, 31-32=-230/353,
30-31=-230/353, 28-30=-230/353, 27-28=-230/353, 26-27=-230/353, 24-26=-230/353,
23-24=-230/353, 22-23=-230/353, 21-22=-224/347, 20-21=-224/347, 19-20=-224/347,
17-19=-224/347
WEBS 9-27=-261/171

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 27, 28, 26 except (jt=lb) 1=144, 30=148, 31=140, 32=133, 33=119, 34=129, 35=186, 24=152, 23=140, 22=133, 21=119, 20=132, 19=169.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



July 2,2024

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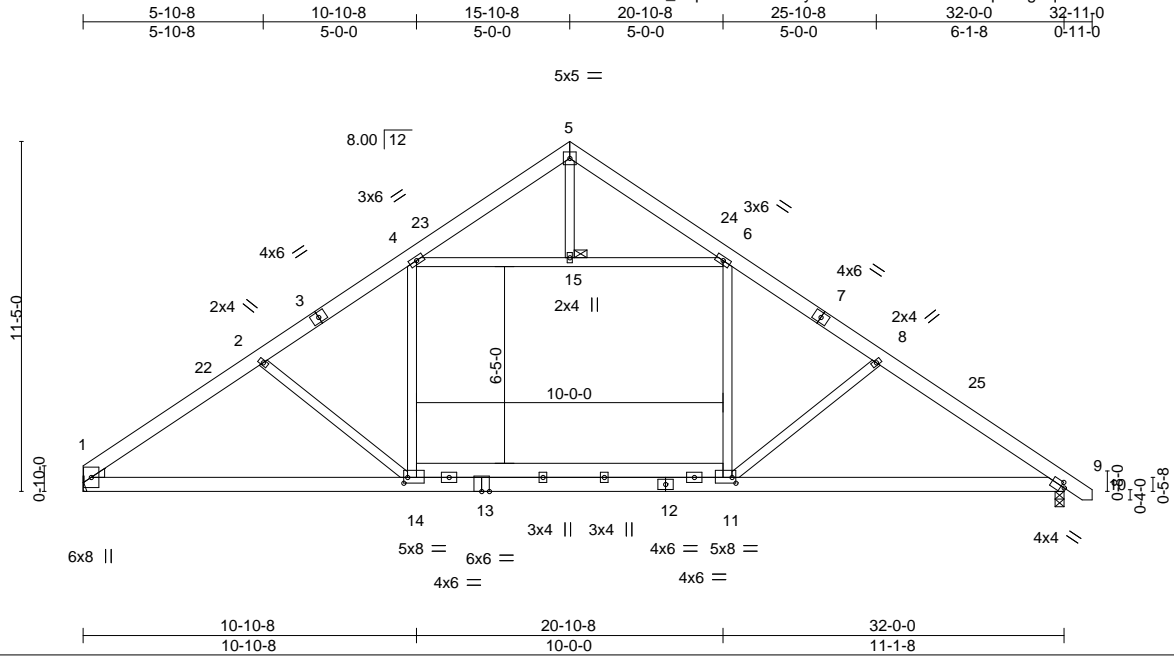
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	A2	COMMON	4	1	166592132
Comtech, Inc., Fayetteville, NC - 28314,					Job Reference (optional)

Comtech, Inc., Fayetteville, NC - 28314,

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Scale = 1:75.2

Plate Offsets (X,Y)--		[9:0-1-5,0-1-12], [11:0-1-8,0-2-4], [14:0-1-8,0-2-4]															
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL.		in (loc)		l/defl		L/d		PLATES		GRIP	
TCLL	20.0	Plate Grip DOL 1.15		TC 0.23		Vert(LL)		-0.24		11-21		>999		360		MT20 244/190	
TCDL	10.0	Lumber DOL 1.15		BC 0.53		Vert(CT)		-0.30		11-21		>999		240			
BCLL	0.0 *	Rep Stress Incr YES		WB 0.48		Horz(CT)		0.05		9		n/a		n/a			
BCDL	10.0	Code IRC2015/TPI2014		Matrix-AS		Wind(LL)		0.27		11-21		>999		240		Weight: 251 lb FT = 25%	

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2
WEDGE
Left: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.
JOINTS 1 Brace at Jt(s): 15

REACTIONS.

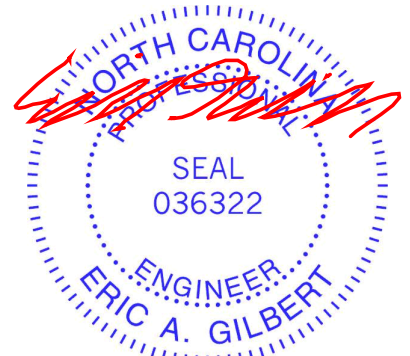
(size) 1=Mechanical, 9=0-3-8
Max Horz 1=-354(LC 8)
Max Uplift 1=-216(LC 12), 9=-235(LC 13)
Max Grav 1=1445(LC 19), 9=1488(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-2080/706, 2-4=-1908/664, 4-5=-471/260, 5-6=-462/258, 6-8=-1911/662,
8-9=-2120/708
BOT CHORD 1-14=-417/1907, 11-14=-223/1635, 9-11=-423/1699
WEBS 8-11=-458/292, 2-14=-375/280, 4-14=-50/596, 6-11=-56/647, 4-15=-1334/510,
6-15=-1334/510

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 15-10-8, Exterior(2) 15-10-8 to 20-3-5, Interior(1) 20-3-5 to 32-9-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=216, 9=235.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2, 2024

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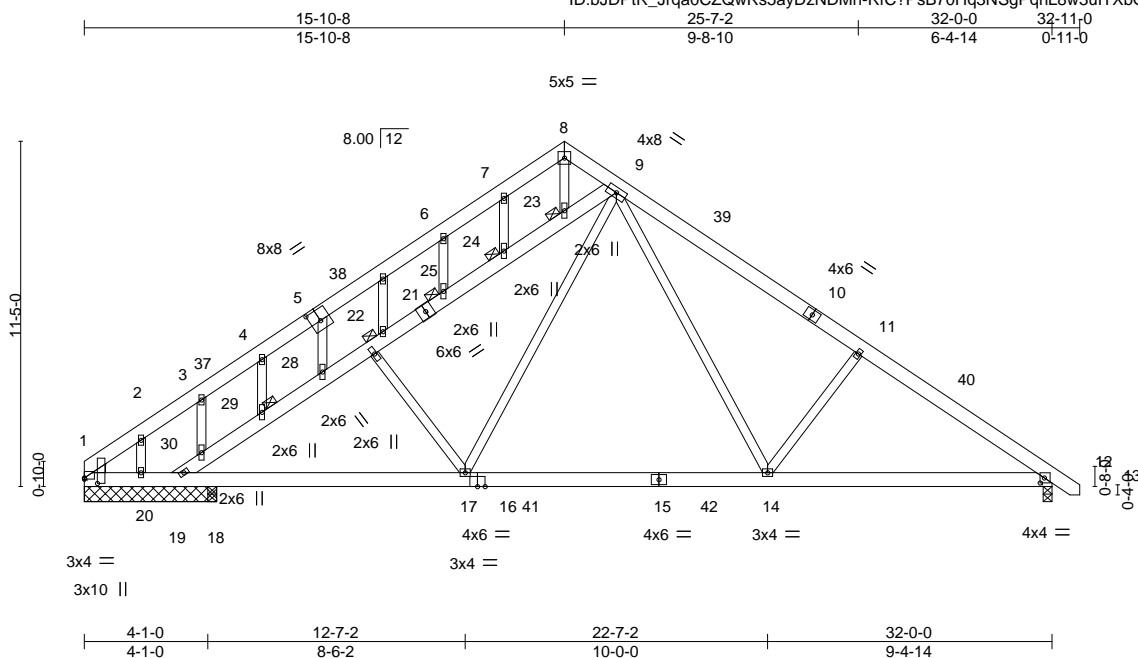


Plate Offsets (X,Y)-- [1:0-0,0-0-7], [1:0-2-2,0-5-3], [5:0-4-0,0-4-8], [12:0-1-11,0-2-0]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	-0.18 14-17 >999 360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.46	Vert(CT)	-0.25 14-17 >999 240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.45	Horz(CT)	0.03 12 n/a n/a		
BCDL	10.0	Code IRC2015/TPI2014		Matrix-AS		Wind(LL)	0.04 14-17 >999 240	Weight: 277 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2 *Except*
19-21,9-21: 2x6 SP No.1
OTHERS 2x4 SP No.2
WEDGE
Left: 2x4 SP No.2

BRACING-

TOP CHORD	Structural wood sheathing directly applied.
BOT CHORD	Rigid ceiling directly applied.
JOINTS	1 Brace at Jt(s): 22, 23, 24, 25, 29

REACTIONS.

REACTIONS. All bearings 4-4-8 except (jt=length) 12=0-3-8, 18=0-3-8.
 (lb) - Max Horz 1=354(LC 8)
 Max Uplift All uplift 100 lb or less at joint(s) except 12=208(LC 13), 19=366(LC 12), 20=216(LC 12)
 Max Grav All reactions 250 lb or less at joint(s) 1, 1 except 12=1297(LC 20), 19=918(LC 19), 20=289(LC 19), 18=409(LC 3)

FORCES.

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

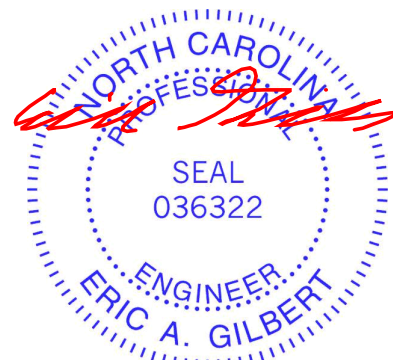
TOP CHORD 1-2=429/283, 2-3=352/65, 3-4=379/0, 4-5=264/40, 5-6=337/72, 6-7=331/163,
7-8=288/197, 8-9=388/280, 9-11=1741/653, 11-12=1874/614

BOT CHORD 1-20=228/430, 19-20=228/430, 18-19=302/1590, 17-18=302/1590, 14-17=36/1040,
12-14=350/1470

WEBS 19-30=1877/878, 29-30=1735/781, 28-29=1810/831, 22-28=1611/696,
22-25=1593/684, 24-25=1420/569, 23-24=1398/556, 9-23=1512/631, 9-14=219/802,
9-17=176/723, 17-22=472/342, 11-14=588/380, 6-25=312/223, 5-28=416/289,
3-30=255/173. 2-20=433/314

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; $V_{ult}=150\text{mph}$ $V_{asd}=119\text{mph}$; $TCDL=6.0\text{psf}$; $BCDL=6.0\text{psf}$; $h=15\text{ft}$; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 15-10-8, Exterior(2) 15-10-8 to 20-3-5, Interior(1) 20-3-5 to 32-9-1 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TP1 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2'-0" oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3'-6" tall by 2'-0" wide will fit between the bottom chord and any other members, with $BCDL = 10.0\text{psf}$.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 208 lb uplift at joint 12, 366 lb uplift at joint 19 and 216 lb uplift at joint 20.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2, 2024



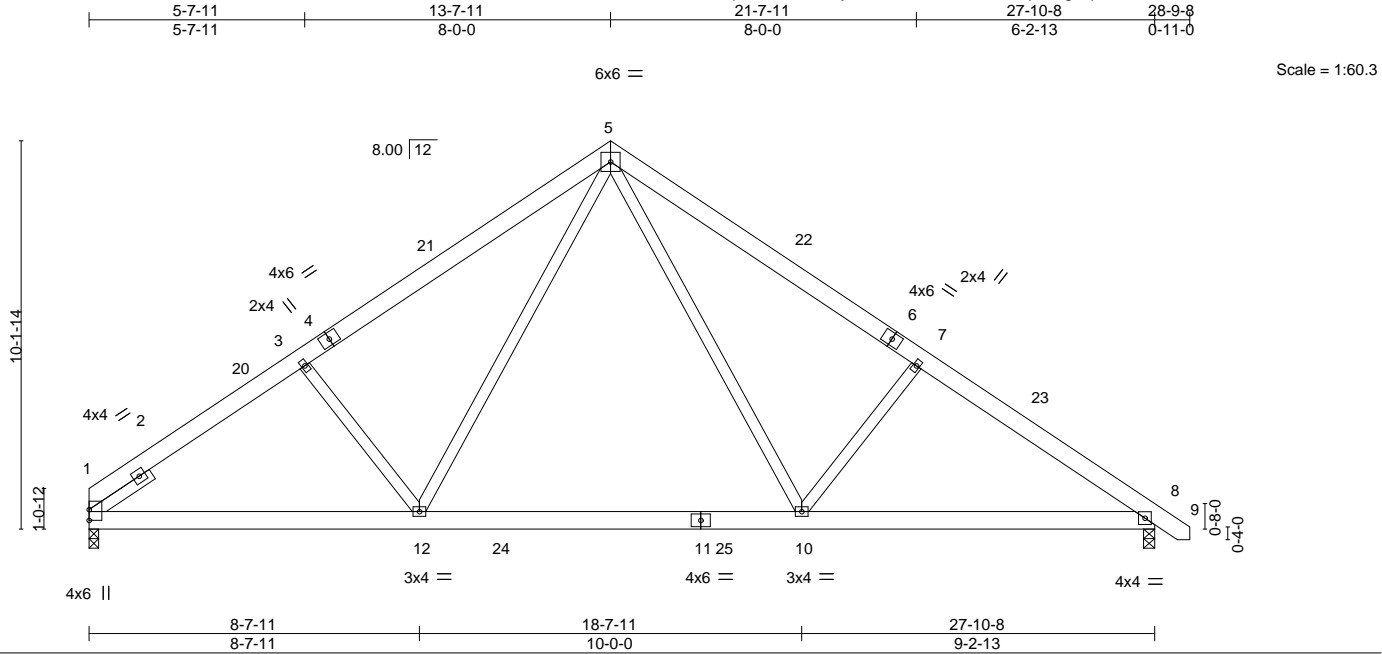
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TP1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Components Association (www.sbcacomponents.com)



818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	B1	COMMON	4	1	66592134

Comtech, Inc., Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:00 2024 Page 1
ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.24	Vert(LL)	-0.18 10-12	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.46	Vert(CT)	-0.26 10-12	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.43	Horz(CT)	0.03 8	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-AS	Wind(LL)	0.04 10-12	>999	240	Weight: 195 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x4 SP No.2
SLIDER Left 2x4 SP No.2 1-11-0

BRACING-

TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

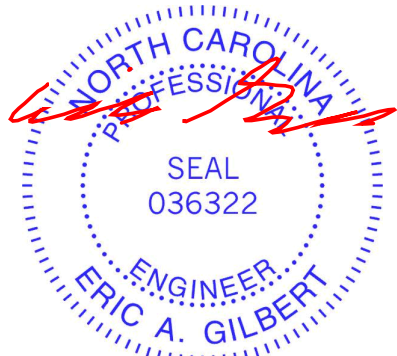
(size) 1=0-3-0, 8=0-3-8
Max Horz 1=-313(LC 8)
Max Uplift 1=-186(LC 12), 8=-207(LC 13)
Max Grav 1=1186(LC 19), 8=1232(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-3=-1685/609, 3-5=-1613/645, 5-7=-1696/658, 7-8=-1783/623
BOT CHORD 1-12=-343/1515, 10-12=-54/995, 8-10=-379/1404
WEBS 3-12=-478/350, 5-12=-179/680, 5-10=-213/805, 7-10=-571/381

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 13-7-11, Exterior(2) 13-7-11 to 18-0-8, Interior(1) 18-0-8 to 28-7-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 186 lb uplift at joint 1 and 207 lb uplift at joint 8.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

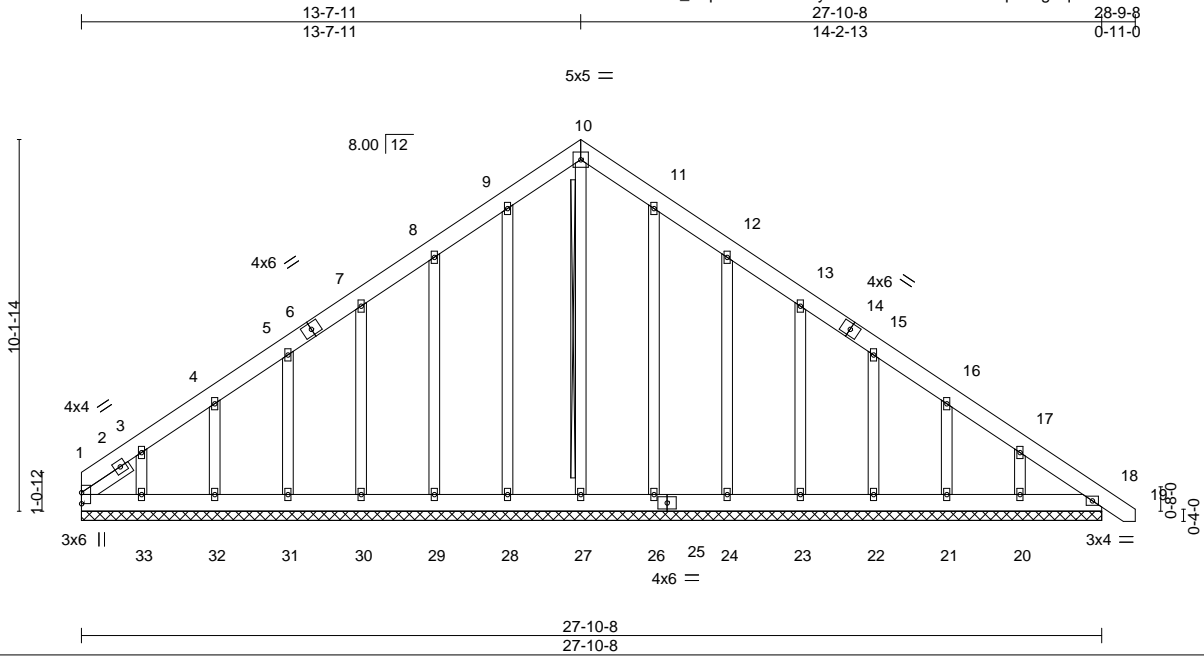
ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	B1GE	GABLE	1	1	166592135
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:02 2024 Page 1
ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:62.9

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.06	Vert(LL)	0.00	18	n/r	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.03	Vert(CT)	0.00	18	n/r		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.18	Horz(CT)	0.01	18	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S						
									Weight: 245 lb FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2
SLIDER Left 2x4 SP No.2 1-6-10

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS T-Brace: 2x4 SPF No.2 - 10-27
Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.
Brace must cover 90% of web length.

REACTIONS.

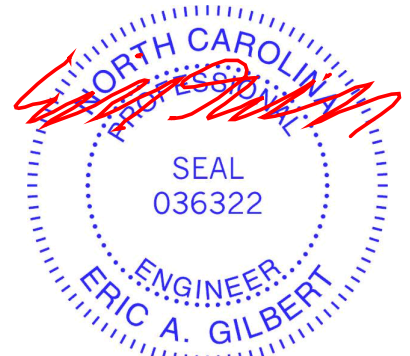
All bearings 27-10-8.
(lb) - Max Horz 1=-394(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 18, 28, 26 except 1=-199(LC 10), 29=-147(LC 12), 30=-132(LC 12), 31=-133(LC 12), 32=-125(LC 12), 33=-265(LC 12), 24=-151(LC 13), 23=-132(LC 13), 22=-132(LC 13), 21=-128(LC 13), 20=-171(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 18, 28, 29, 30, 31, 32, 26, 24, 23, 22, 21, 20 except 1=305(LC 12), 27=251(LC 13), 33=252(LC 19)

FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-3=-450/331, 3-4=-280/245, 8-9=-246/299, 9-10=-298/340, 10-11=-298/340, 11-12=-246/274, 17-18=-309/210
BOT CHORD 1-33=-199/312, 32-33=-199/312, 31-32=-199/312, 30-31=-199/312, 29-30=-199/312, 28-29=-199/312, 27-28=-199/312, 26-27=-199/312, 24-26=-199/312, 23-24=-199/312, 22-23=-199/312, 21-22=-199/312, 20-21=-199/312, 18-20=-199/312
WEBS 3-33=-235/267

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 28, 26 except (jt=lb) 1=199, 29=147, 30=132, 31=133, 32=125, 33=265, 24=151, 23=132, 22=132, 21=128, 20=171.
- Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



July 2,2024

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ENGINEERING BY
TRENCO
A MiTek Affiliate

818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	B2	ROOF SPECIAL	2	1	166592136
Job Reference (optional)					

Comtech, Inc., Fayetteville, NC - 28314,

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITxbGKWrcDoi7J4zJC?f

5-7-11 13-7-11 21-7-11 25-8-8 27-10-8 28-9-8

5-7-11 8-0-0 8-0-0 4-0-13 2-2-0 0-11-0

6x6 =

Scale = 1:62.5

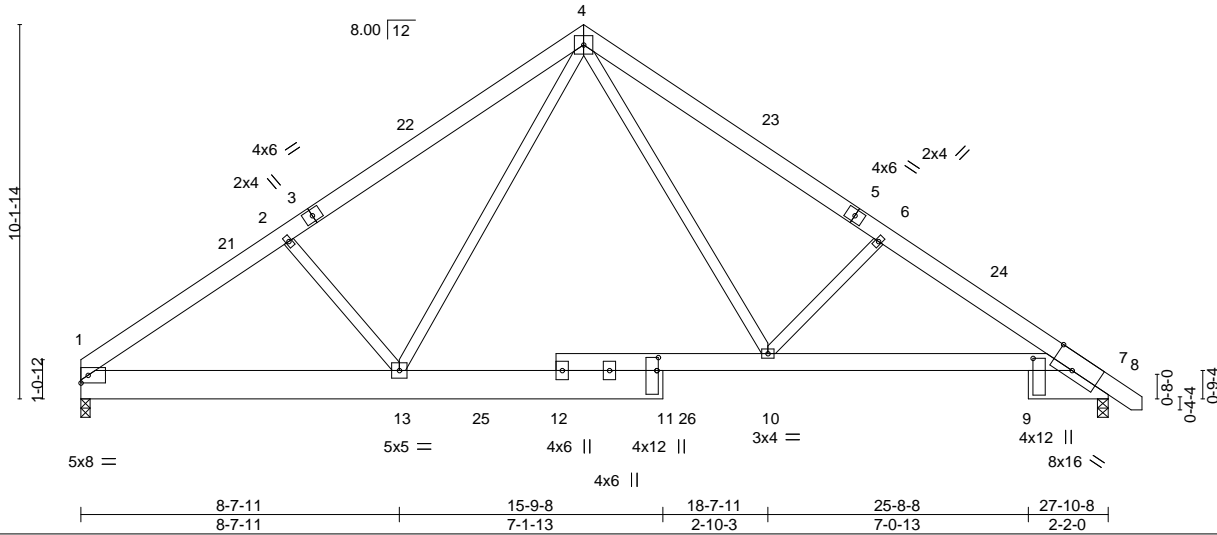


Plate Offsets (X, Y)--		[7:0-7-0,Edge], [11:0-3-15,10-2-8], [11:0-4-4,0-0-8]							
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.36	Vert(LL)	-0.08 10-13	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.48	Vert(CT)	-0.16 10-20	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.45	Horz(CT)	0.05 7	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-AS	Wind(LL)	0.07 10-20	>999	240	Weight: 228 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1

BOT CHORD 2x10 SP No.1 *Except*

7-12: 2x6 SP No.1

WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied.

BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(size) 1=0-3-0, 7=0-3-8

Max Horz 1=-312(LC 8)

Max Uplift 1=-186(LC 12), 7=-210(LC 13)

Max Grav 1=1176(LC 19), 7=1231(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1708/612, 2-4=-1613/640, 4-6=-1886/681, 6-7=-2042/686

BOT CHORD 1-13=-356/1556, 10-13=-55/1039, 7-10=-452/1692

WEBS 4-10=-252/1001, 6-10=-692/406, 4-13=-166/607, 2-13=-534/373

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 13-7-11, Exterior(2) 13-7-11 to 18-0-8, Interior(1) 18-0-8 to 28-7-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=186, 7=210.
- This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2,2024

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence	166592137
J0424-2426	B3	ROOF SPECIAL	4	1	Job Reference (optional)	

Comtech, Inc., Fayetteville, NC - 28314,

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f

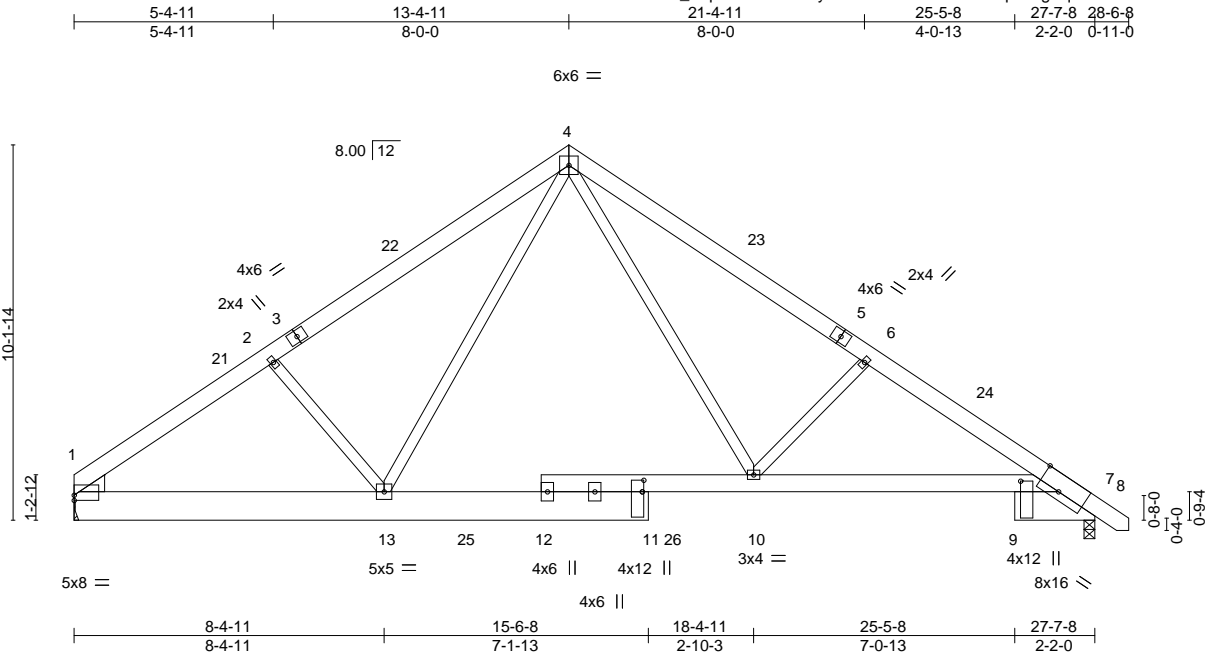


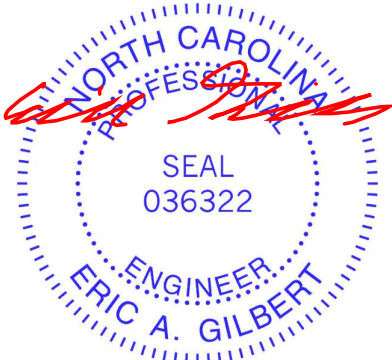
Plate Offsets (X,Y)--		[1:0-0-0,0-1-11], [7:0-7-0,Edge], [11:0-3-7,10-2-14], [11:0-3-12,0-0-8]							
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES GRIP	
TCLL	20.0	Plate Grip DOL	1.15	TC	0.36	Vert(LL)	-0.08 10-13 >999	360	MT20 244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.48	Vert(CT)	-0.15 10-20 >999	240	
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.47	Horz(CT)	-0.06 1 n/a	n/a	
BCDL	10.0	Code IRC2015/TPI2014		Matrix-AS		Wind(LL)	0.08 10-20 >999	240	Weight: 228 lb FT = 25%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied.
BOT CHORD 2x10 SP No.1 *Except*	BOT CHORD Rigid ceiling directly applied.
7-12: 2x6 SP No.1	
WEBS 2x4 SP No.2	
WEDGE	
Left: 2x6 SP No.2	

REACTIONS. (size) 1=Mechanical, 7=0-3-8
Max Horz 7=-312(LC 8)
Max Uplift 1=-183(LC 12), 7=-209(LC 13)
Max Grav 1=1166(LC 19), 7=1220(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-1650/595, 2-4=-1557/623, 4-6=-1863/687, 6-7=-2019/693
BOT CHORD 1-13=-387/1276, 10-13=-99/1010, 7-10=-503/1894
WEBS 4-10=-263/1050, 6-10=-689/409, 4-13=-145/578, 2-13=-512/367

- NOTES-**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-0-0 to 4-4-13, Interior(1) 4-4-13 to 13-4-11, Exterior(2) 13-4-11 to 17-9-8, Interior(1) 17-9-8 to 28-4-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=183, 7=209.
 - This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2,2024

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	C1	ATTIC	2	1	166592138
Job Reference (optional)					

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:04 2024 Page 1

ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcD0i7J4zJC?f

-0-11-0 3-8-12 | 7-2-1 | 10-1-8 | 13-0-15 | 16-6-4 | 20-3-0 21-2-0
0-11-0 3-8-12 | 3-5-5 | 2-11-7 | 2-11-7 | 3-5-5 | 3-8-12 0-11-0

6x8 =

Scale: 1/8"=1'

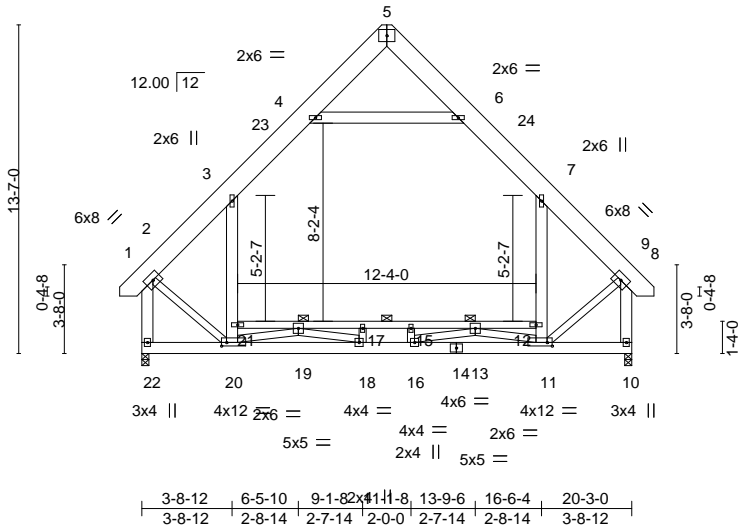


Plate Offsets (X,Y)--		[11:0-2-8,0-1-12], [20:0-2-8,0-1-12]									
LOADING	(psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC 0.27	Vert(LL)	-0.11	16-18	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC 0.76	Vert(CT)	-0.20	16-18	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB 0.35	Horz(CT)	0.03	10	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014		Matrix-AS	Wind(LL)	0.06	18-20	>999	240	Weight: 273 lb	FT = 25%

LUMBER-	BRACING-
TOP CHORD 2x10 SP No.1	TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x6 SP No.1 *Except* 12-21: 2x4 SP No.1	BOT CHORD Rigid ceiling directly applied. Except: 4-0-0 oc bracing: 13-19 10-0-0 oc bracing: 19-21, 12-13
WEBS 2x4 SP No.2 *Except* 7-11,3-20,4-6,2-22,8-10: 2x6 SP No.1	JOINTS 1 Brace at Jt(s): 19, 13

REACTIONS. (size) 22=0-3-8, 10=0-3-8
Max Horz 22=338(LC 11)
Max Grav 22=1608(LC 21), 10=1608(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1210/80, 3-4=-918/271, 6-7=-919/271, 7-8=-1210/80, 2-22=-1666/68,
8-10=-1669/68
BOT CHORD 20-22=-332/376, 18-20=0/2262, 16-18=0/2894, 11-16=0/2193, 19-21=-152/397,
17-19=-2265/0, 15-17=-2265/0, 13-15=-2265/0, 12-13=-168/410
WEBS 11-12=-67/365, 7-12=-28/535, 20-21=-67/365, 3-21=-28/535, 4-6=-960/290,
2-20=0/1027, 8-11=0/1030, 15-16=-261/0, 17-18=-261/0, 19-20=-1933/0, 18-19=0/766,
13-16=0/764, 11-13=-1933/0

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-6-11 to 3-8-4, Interior(1) 3-8-4 to 10-1-8, Exterior(2) 10-1-8 to 14-6-5, Interior(1) 14-6-5 to 20-9-11 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s).7-12, 3-21
 - 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21, 17-19, 15-17, 13-15, 12-13
 - 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - 8) Attic room checked for L/360 deflection.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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818 Soundside Road
Edenton, NC 27932

-0-11-0	3-8-12	7-2-1	10-1-8	13-0-15	16-6-4	20-3-0	21-2-0
0-11-0	3-8-12	3-5-5	2-11-7	2-11-7	3-5-5	3-8-12	0-11-0

 $8 \times 8 =$

Scale: 1/8"=1'

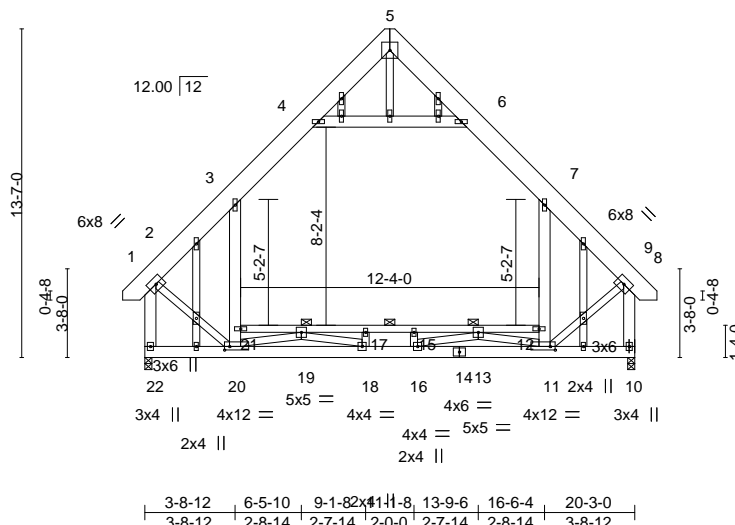


Plate Offsets (X,Y)--		[11:0-2-8,0-1-12], [20:0-2-8,0-1-12]		36:12		26:14		27:14		28:0		29:14		30:14		36:12	
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)		I/defl	L/d	PLATES		GRIP			
TCLL	20.0	Plate Grip DOL	1.15	TC	0.27	Vert(LL)	-0.11	16-18		>999	360	MT20		244/190			
TCDL	10.0	Lumber DOL	1.15	BC	0.76	Vert(CT)	-0.20	16-18		>999	240						
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.35	Horz(CT)	0.03	10		n/a	n/a						
BCDL	10.0	Code IRC2015/TPI2014		Matrix-AS		Wind(LL)	0.06	18-20		>999	240	Weight: 293 lb		FT = 25%			

LUMBER-		BRACING-	
TOP CHORD	2x10 SP No.1	TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	2x6 SP No.1 *Except*	BOT CHORD	Rigid ceiling directly applied. Except:
	12-21: 2x4 SP No.1		4-0-0 oc bracing: 13-19
WEBS	2x4 SP No.2 *Except*		10-0-0 oc bracing: 19-21, 12-13
	7-11,3-20,4-6,2-22,8-10: 2x6 SP No.1	JOINTS	1 Brace at Jt(s): 19, 13
OTHERS	2x4 SP No.2		

REACTIONS. (size) 22=0-3-8, 10=0-3-8
Max Horz 22=338(LC 11)
Max Gray 22=1608(LC 21). 10=1608(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

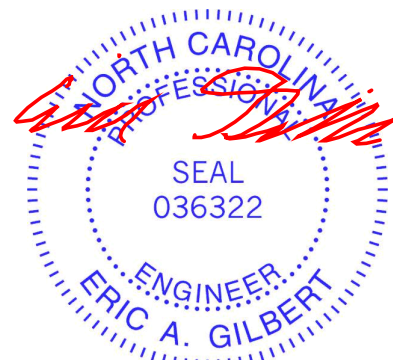
TOP CHORD 2-3=-1210/106, 3-4=-918/311, 6-7=-919/311, 7-8=-1210/106, 2-22=-1666/102, 8-10=-1669/102

BOT CHORD 20-22=-332/376, 18-20=0/2262, 16-18=0/2894, 11-16=0/2193, 19-21=-152/397, 17-19=-2265/0, 15-17=-2265/0, 13-15=-2265/0, 12-13=-168/410

WEBS 11-12=-67/365, 7-12=-28/535, 20-21=-67/365, 3-21=-28/535, 4-6=-960/363, 2-20=0/1027, 8-11=0/1030, 15-16=-261/0, 17-18=-261/0, 19-20=-1933/0, 18-19=0/766, 13-16=0/764, 11-13=-1933/0

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Corner(3) 0-6-11 to 3-8-4, Exterior(2) 3-8-4 to 10-1-8, Corner(3) 10-1-8 to 14-6-5, Exterior(2) 14-6-5 to 20-9-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x6 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Ceiling dead load (10.0 psf) on member(s). 3-4, 6-7, 4-6; Wall dead load (5.0psf) on member(s). 7-12, 3-21
- 9) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 19-21, 17-19, 15-17, 13-15, 12-13
- 10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 11) Attic room checked for L/360 deflection.



July 2, 2024

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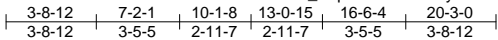
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	C2	ATTIC	2	1	I66592140
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:05 2024 Page 1

ID:bjDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcD0i7J4zJC?f



6x8 =

Scale: 1/8"=1'

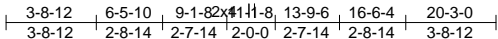
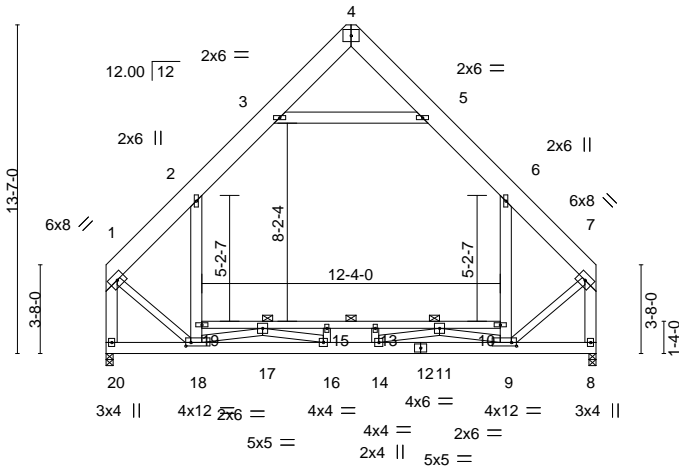


Plate Offsets (X,Y)--	[9:0-2-8,0-1-12], [18:0-2-8,0-1-12]				
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl L/d
TCLL 20.0	Plate Grip DOL	1.15	TC 0.27	Vert(LL)	-0.11 14-16 >999 360
TCDL 10.0	Lumber DOL	1.15	BC 0.76	Vert(CT)	-0.20 14-16 >999 240
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.35	Horz(CT)	0.03 8 n/a n/a
BCDL 10.0	Code IRC2015/TPI2014		Matrix-AS	Wind(LL)	0.07 16-18 >999 240
					Weight: 267 lb FT = 25%

LUMBER-

TOP CHORD 2x10 SP No.1

BOT CHORD 2x6 SP No.1 *Except*
10-19: 2x4 SP No.1

WEBS 2x4 SP No.2 *Except*
6-9,2-18,3-5,1-20,7-8: 2x6 SP No.1

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.

BOT CHORD Rigid ceiling directly applied. Except:
4-0-0 oc bracing: 11-17
10-0-0 oc bracing: 17-19, 10-11

JOINTS
1 Brace at Jt(s): 17, 11

REACTIONS. (size) 20=0-3-8, 8=0-3-8
Max Horz 20=-391(LC 8)
Max Grav 20=1580(LC 21), 8=1580(LC 20)

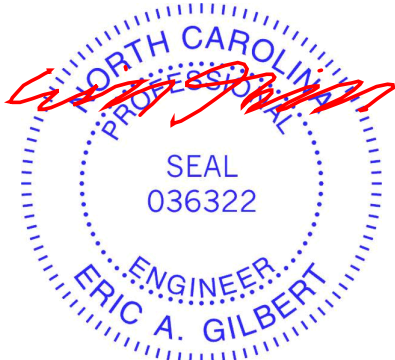
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-1221/64, 2-3=-932/298, 5-6=-933/298, 6-7=-1222/64, 1-20=-1647/46,
7-8=-1650/46

BOT CHORD 18-20=-406/432, 16-18=0/2302, 14-16=0/2895, 9-14=0/2194, 17-19=-208/436,
15-17=-2265/0, 13-15=-2265/0, 11-13=-2265/0, 10-11=-228/451

WEBS 9-10=-70/363, 6-10=-31/533, 18-19=-70/363, 2-19=-31/533, 3-5=-961/340,
1-18=-19/1049, 7-9=-21/1052, 13-14=-266/13, 15-16=-265/10, 17-18=-1933/0,
16-17=0/791, 11-14=0/789, 9-11=-1933/0

- NOTES-**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 5) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-5; Wall dead load (5.0psf) on member(s).6-10, 2-19
 - 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-19, 15-17, 13-15, 11-13, 10-11
 - 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
 - 8) Attic room checked for L/360 deflection.



July 2,2024

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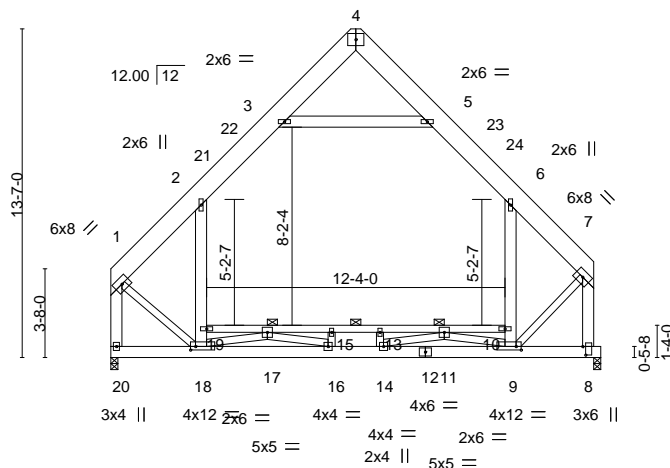


818 Soundside Road
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3-8-12	7-2-1	10-1-8	13-0-15	16-6-4	20-3-0
3-8-12	3-5-5	2-11-7	2-11-7	3-5-5	3-8-12

 $6 \times 8 =$

Scale: 1/8"=1'



3-8-12	6-5-10	9-1-82x41-1-8	13-9-6	16-6-4	20-3-0
3-8-12	2-8-14	2-7-14	2-0-0	2-7-14	2-8-14
					3-8-12

Plate Offsets (X,Y)--		[8:0-4,-0,0-18], [9:0-2-8,0-1-12], [18:0-2-8,0-1-12]				
LOADING (psf)		SPACING- 2-0-0	CSI.	DEFL. in (loc) l/defl L/d	PLATES	GRIP
TCLL 20.0		Plate Grip DOL 1.15	TC 0.28	Vert(LL) -0.11 14-16 >999 360	MT20	244/190
TCDL 10.0		Lumber DOL 1.15	BC 0.76	Vert(CT) -0.19 14-16 >999 240		
BCLL 0.0 *		Rep Stress Incr YES	WB 0.35	Horz(CT) 0.03 8 n/a n/a		
BCDL 10.0		Code IRC2015/TPI2014	Matrix-AS	Wind(LL) 0.06 16-18 >999 240	Weight: 266 lb	FT = 25%

LUMBER-

TOP CHORD	2x10 SP No.1
BOT CHORD	2x6 SP No.1 *Except*
	10-19: 2x4 SP No.1
WEBS	2x4 SP No.2 *Except*
	6-9,2-18,3-5,1-20,7-8: 2x6 SP No.1

BRACING-

TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	Rigid ceiling directly applied. Except: 4-0-0 oc bracing: 11-17 6-0-0 oc bracing: 17-19 10-0-0 oc bracing: 10-11
JOINTS	1 Brace at Jt(s): 17, 11

REACTIONS.

(size) 20=0-3-8, 8=0-3-8
Max Horz 20=312(LC 9)
Max Grav 20=1556(LC 21), 8=1584(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	1-2=-1193/39, 2-3=-891/262, 5-6=-909/264, 6-7=-1139/44, 1-20=-1605/12, 7-8=-1667/6
BOT CHORD	18-20=-322/348, 16-18=0/2245, 14-16=0/2865, 9-14=0/2143, 17-19=-161/349, 15-17=-2258/0, 13-15=-2258/0, 11-13=-2258/0, 10-11=-157/444
WEBS	9-10=-121/311, 6-10=-84/480, 18-19=-71/361, 2-19=-29/534, 3-5=-919/272, 1-18=0/985, 7-9=0/1068, 13-14=-270/0, 15-16=-252/0, 17-18=-1914/0, 16-17=0/732, 11-14=0/786, 9-11=-1941/0

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDF=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 10-1-8, Exterior(2) 10-1-8 to 14-6-5, Interior(1) 14-6-5 to 19-8-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Ceiling dead load (10.0 psf) on member(s). 2-3, 5-6, 3-5; Wall dead load (5.0psf) on member(s).6-10, 2-19
- 6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 17-19, 15-17, 13-15, 11-13, 10-11
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.
- 8) Attic room checked for L/360 deflection.



July 2, 2024

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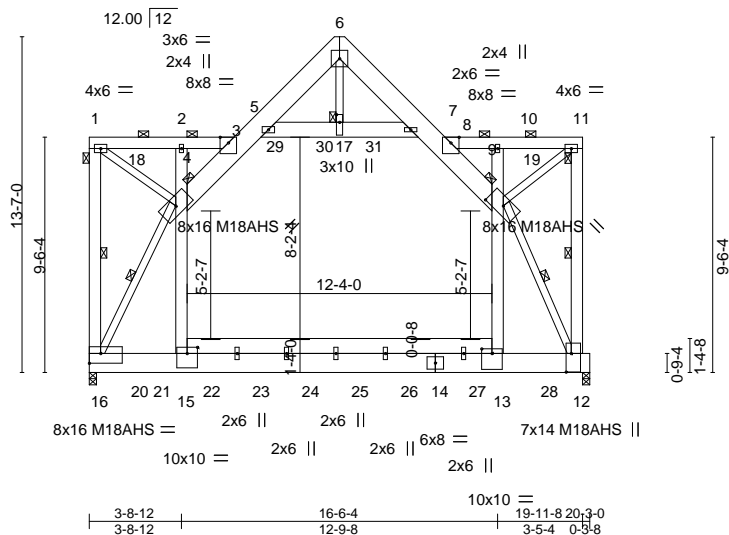
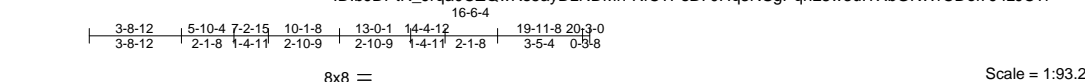


Plate Offsets (X,Y)-- [9:0-8,4,0-4,0], [12:0-9,0,0-2,8], [13:0-5,0,0-2,4], [15:0-5,0,0-3,0], [16:Edge,0-4,12], [17:0-3,12,0-1,8]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.19 13-15 >999 360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.43	Vert(CT)	-0.26 13-15 >891 240	M18AHS	186/179
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.98	Horz(CT)	0.02 12 n/a n/a		
BCDL	10.0	Code IRC2015/TPI2014		Matrix-MS		Wind(LL)	0.04 13-15 >999 240	Weight: 699 lb	FT = 25%

LUMBER-		BRACING-	
TOP CHORD	2x6 SP No.1 *Except* 4-6,6-9: 2x10 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 1-3, 3-4, 8-9, 8-11.
BOT CHORD	2x10 SP 2400F 2.0E *Except* 13-15: 2x8 SP 2400F 2.0E	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.2 *Except* 1-16,11-12,2-15,10-13: 2x6 SP No.1, 5-7: 2x8 SP No.1	WEBS	1 Row at midpt 1-16, 11-12, 4-16, 9-12
		JOINTS	1 Brace at Jt(s): 1, 4, 9, 11, 17

REACTIONS. (size) 16=0-3-8 (req. 0-4-1), 12=0-3-8 (req. 0-4-3)
 Max Horz 16=125(LC 35)
 Max Uplift 16=-1108(LC 9), 12=-1045(LC 8)
 Max Grav 16=9855(LC 2), 12=10184(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD
1-16=1388/632, 1-2=1317/795, 2-3=1103/909, 3-4=6529/985, 3-5=5887/748,
5-6=3615/644, 6-7=3567/523, 7-8=6408/833, 8-9=6783/984, 8-10=699/1127,
10-11=800/995, 11-12=1790/720

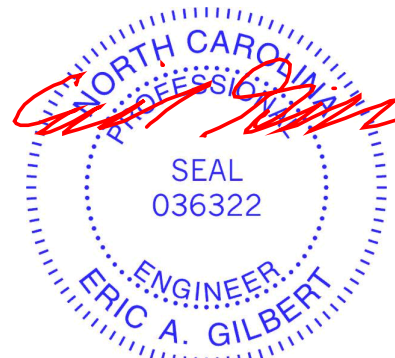
BOT CHORD
15-16=456/4676, 13-15=471/4614, 12-13=458/4673

WEBS
1-4=941/1487, 4-15=0/3402, 2-4=2074/408, 5-17=2301/165, 7-17=2301/165,
9-13=0/3359, 9-10=2181/423, 9-11=1205/942, 4-16=9523/938, 9-12=10152/994,
6-17=498/4171

NOTES-

- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x10 - 2 rows staggered at 0-5-0 oc.
Bottom chords connected as follows: 2x10 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc, 2x8 - 3 rows staggered at 0-2-0 oc.
- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- 3) Unbalanced roof live loads have been considered for this design.
- 4) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- 5) Provide adequate drainage to prevent water ponding.
- 6) All plates are MT20 plates unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Ceiling dead load (10.0 psf) on member(s). 2-3, 3-5, 7-8, 8-10, 5-17, 7-17; Wall dead load (5.0psf) on member(s). 4-15, 2-4, 9-13, 9-10
- 10) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 13-15
- 11) WARNING: Required bearing size at joint(s) 16, 12 greater than input bearing size.

SUPPLEMENTARY BEARING PLATES, SPECIAL ANCHORAGE, OR OTHER MEANS TO ALLOW FOR THE MINIMUM REQUIRED SUPPORT WIDTH (SUCH AS COLUMN CAPS, BEARING BLOCKS, ETC.) ARE THE RESPONSIBILITY OF THE TRUSS MANUFACTURER OR THE BUILDING DESIGNER.



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Continued on page 2



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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	C3-GR	Attic Girder	1	2	I66592142
					Job Reference (optional)

- NOTES-**
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 16=1108, 12=1045.
 - 13) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - 14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 329 lb down and 226 lb up at 0-2-12, 1074 lb down and 216 lb up at 1-8-12, 1068 lb down and 216 lb up at 3-8-12, 1068 lb down and 223 lb up at 6-1-8, 1074 lb down and 216 lb up at 15-8-12, and 1074 lb down and 216 lb up at 17-8-12, and 1092 lb down and 203 lb up at 19-8-12 on top chord, and 633 lb down at 0-9-4, 629 lb down at 2-9-4, 629 lb down at 4-9-4, 336 lb down and 109 lb up at 6-9-4, 336 lb down and 109 lb up at 8-9-4, 336 lb down and 109 lb up at 10-9-4, 336 lb down and 109 lb up at 12-9-4, 336 lb down and 109 lb up at 14-5-4, and 336 lb down and 109 lb up at 16-5-4, and 336 lb down and 109 lb up at 18-5-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 15) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-80, 3-5=-80, 5-6=-60, 6-7=-60, 7-8=-80, 8-10=-80, 10-11=-60, 15-16=-20, 13-15=-40, 12-13=-20, 5-7=-20

Drag: 2-15=-10, 10-13=-10

Concentrated Loads (lb)

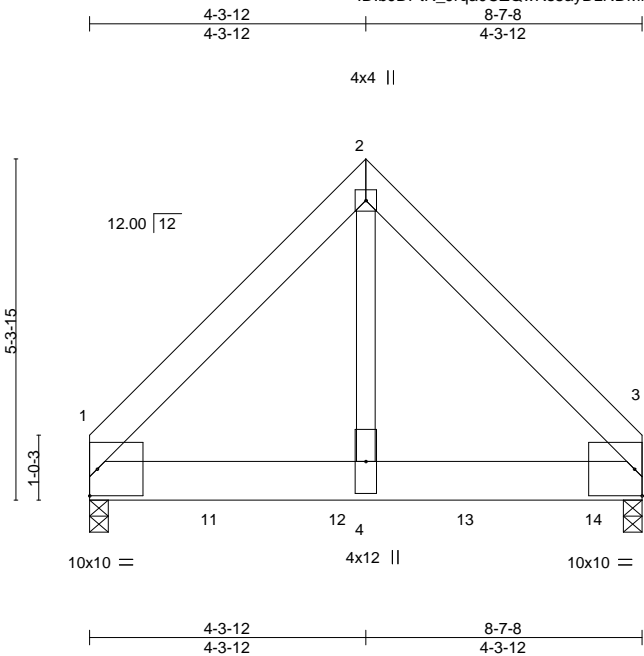
Vert: 1=-42 3=-1028(F) 2=-1028(F) 13=-38(B) 7=-1166(F) 10=-1034(F) 11=-1072(F) 18=-1034(F) 19=-1034(F) 20=-146(B) 21=-141(B) 22=-141(B) 23=-38(B) 24=-38(B) 25=-38(B) 26=-38(B) 27=-38(B) 28=-38(B) 29=-1166(F) 30=-1166(F) 31=-1166(F)

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	D1-GR	Common Girder	1	2	I66592143

Comtech, Inc., Fayetteville, NC - 28314,

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Scale = 1:36.0

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.07	Vert(LL)	-0.01 4-10	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.33	Vert(CT)	-0.03 4-10	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.38	Horz(CT)	0.00 1	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-MP	Wind(LL)	0.01 4-10	>999	240		
									Weight: 127 lb FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x8 SP No.1
WEBS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=0-3-8, 3=0-3-8
Max Horz 1=-138(LC 4)
Max Uplift 1=-446(LC 9), 3=-568(LC 8)
Max Grav 1=2607(LC 2), 3=3331(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

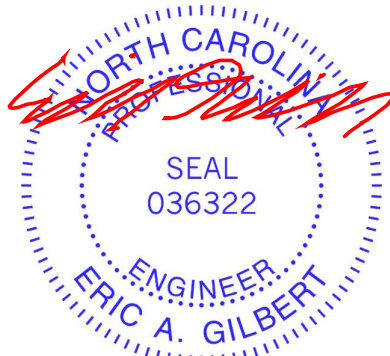
TOP CHORD 1-2=-2399/465, 2-3=-2390/463
BOT CHORD 1-4=-278/1654, 3-4=-278/1654
WEBS 2-4=-524/3098

NOTES-

- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x8 - 2 rows staggered at 0-5-0 oc.
Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.
- All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=446, 3=568.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 1333 lb down and 236 lb up at 1-10-12, 1333 lb down and 236 lb up at 3-10-12, and 1333 lb down and 236 lb up at 5-10-12, and 1336 lb down and 233 lb up at 7-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-2=-60, 2-3=-60, 5-8=-20
Concentrated Loads (lb)
Vert: 11=-1259(B) 12=-1259(B) 13=-1259(B) 14=-1262(B)



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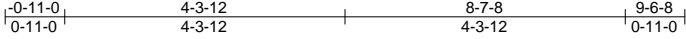
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	D1GE	COMMON SUPPORTED GAB	1	1	166592144
					Job Reference (optional)

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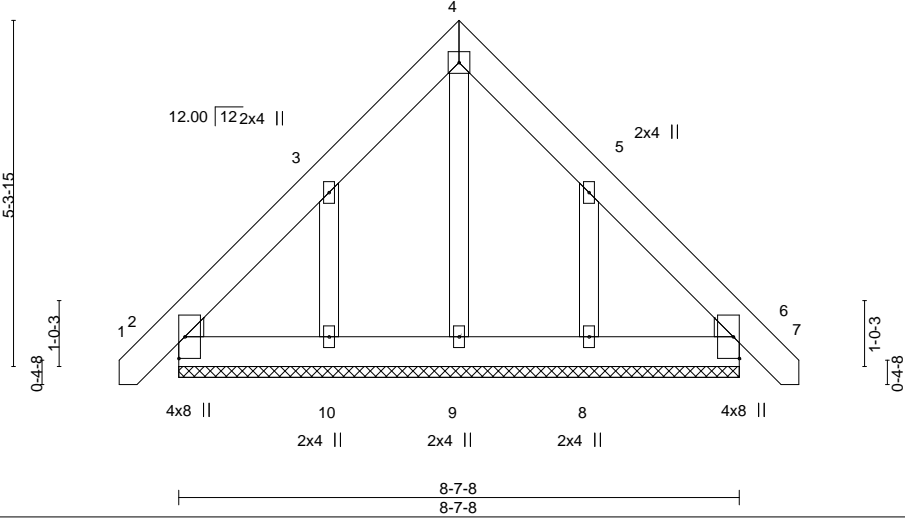
8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:08 2024 Page 1

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4x4 =

Scale = 1:35.5



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.05	Vert(LL)	0.00	6	n/r	120	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.02	Vert(CT)	0.00	6	n/r	120		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.06	Horz(CT)	0.00	6	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-P						Weight: 70 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2
WEDGE
Left: 2x4 SP No.2 , Right: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

All bearings 8-7-8.
(lb) - Max Horz 2=-203(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 2, 6 except 10=-290(LC 12), 8=-286(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 2, 6, 9 except 10=271(LC 19), 8=266(LC 20)

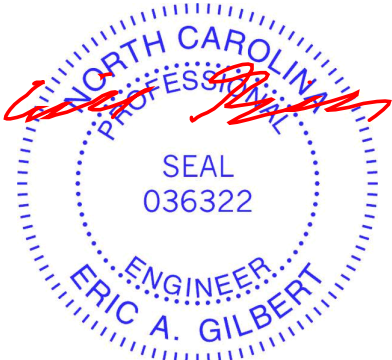
FORCES.

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 3-10=-350/325, 5-8=-351/326

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCCL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6 except (jt=lb) 10=290, 8=286.
- 9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.



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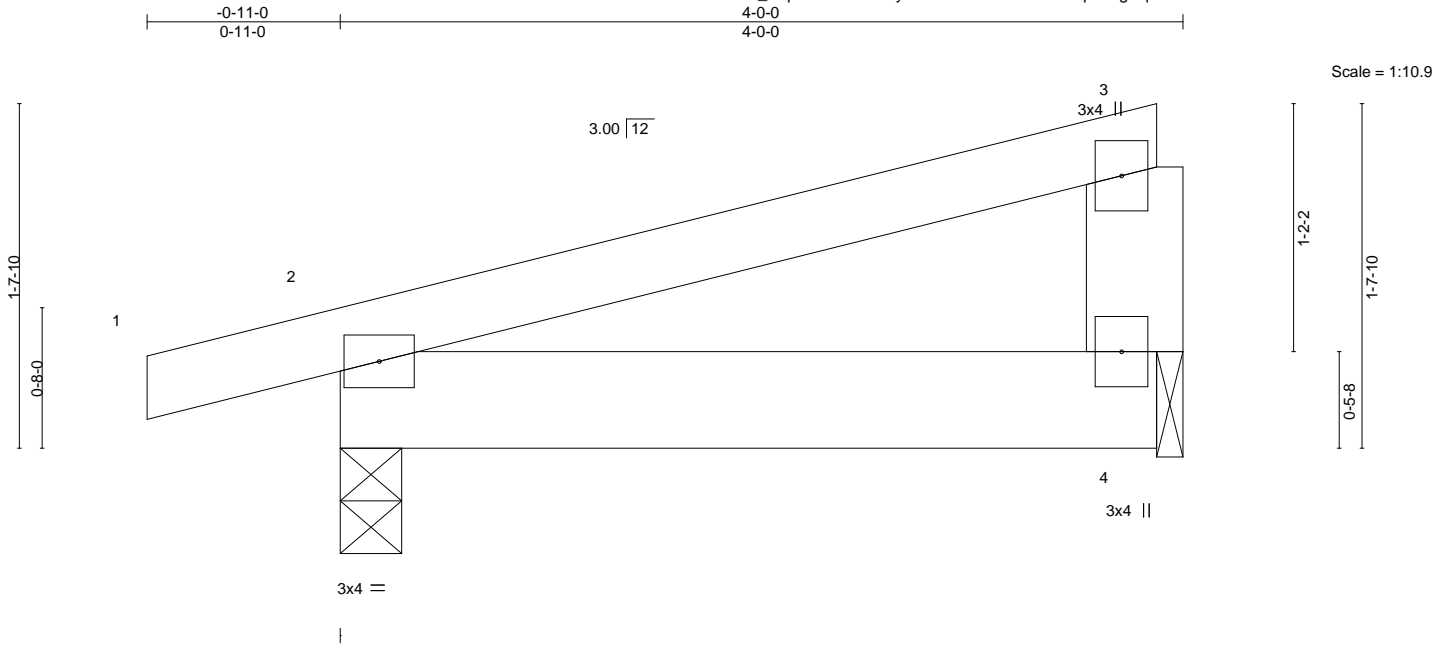
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence	I66592145
J0424-2426	J1	Monopitch	8	1	Job Reference (optional)	

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.11	Vert(LL)	0.01	4-7	>999	240	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.07	Vert(CT)	-0.01	4-7	>999	240		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-AS						Weight: 19 lb	FT = 25%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x6 SP No.1
WEBS 2x6 SP No.1

BRACING-

TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS.

(size) 2=0-3-8, 4=0-1-8
Max Horz 2=78(LC 8)
Max Uplift 2=133(LC 8), 4=80(LC 12)
Max Grav 2=213(LC 1), 4=144(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2=133.
- 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



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Job	Truss	Truss Type	Qty	Ply	Delude Residence	I66592147
J0424-2426	M1GE	GABLE	1	1	Job Reference (optional)	

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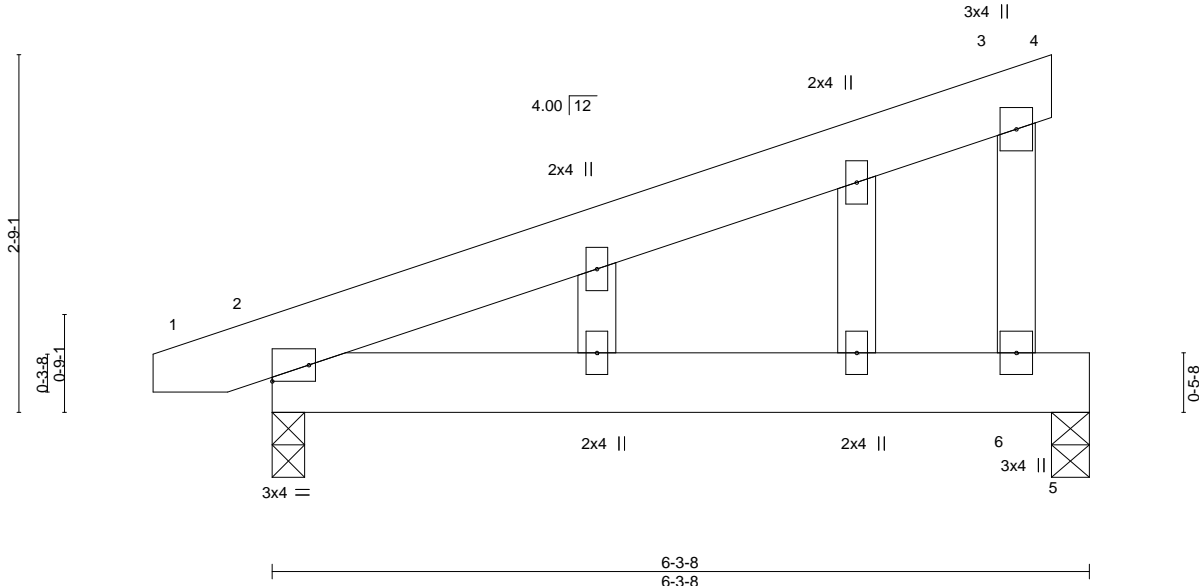


Plate Offsets (X,Y)--		[2:Edge,0-1-8]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCLL 20.0	Plate Grip DOL	1.15	TC 0.21
TCDL 10.0	Lumber DOL	1.15	BC 0.24
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.00
BCDL 10.0	Code	IRC2015/TPI2014	Matrix-AS
DEFL. in (loc) l/defl L/d			
Vert(LL)	0.07	6-13	>999 240
Vert(CT)	-0.03	6-13	>999 240
Horz(CT)	-0.01	2	n/a n/a
PLATES		GRIP	
MT20		244/190	
Weight: 38 lb		FT = 25%	

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1	TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x6 SP No.1	BOT CHORD Rigid ceiling directly applied.
WEBS 2x4 SP No.2	
OTHERS 2x4 SP No.2	

REACTIONS. (size) 2=0-3-0, 5=0-3-8
Max Horz 2=141(LC 8)
Max Uplift 2=230(LC 8), 5=223(LC 8)
Max Grav 2=285(LC 1), 5=225(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-6=-165/288

- NOTES-
- 1) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 - 3) Gable studs spaced at 2-0-0 oc.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=230, 5=223.
 - 7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



July 2,2024

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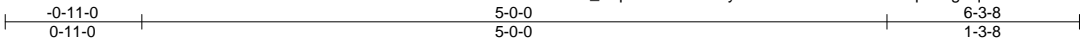
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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence	I66592148
J0424-2426	M2	HALF HIP	4	1	Job Reference (optional)	

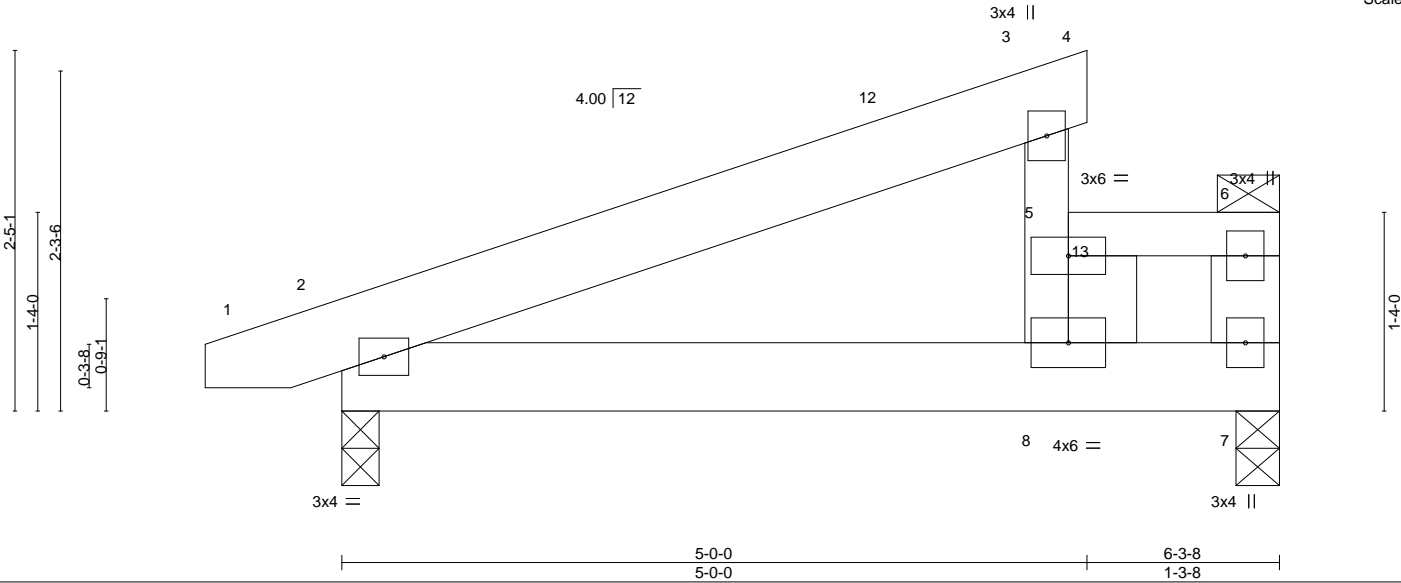
Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:11 2024 Page 1

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Scale = 1:15.5



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.32	Vert(LL)	-0.00	8-11	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.15	Vert(CT)	-0.01	8-11	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.00	Horz(CT)	-0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-MR	Wind(LL)	0.01	8-11	>999	240	Weight: 36 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1 *Except*
5-6: 2x4 SP No.1

BOT CHORD 2x6 SP No.1

WEBS 2x6 SP No.1 *Except*
3-8: 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-8, 5-6. Except:
6-0-0 oc bracing: 3-5

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 7=0-3-8, 2=0-3-0
Max Horz 2=126(LC 12)
Max Uplift 7=111(LC 12), 2=89(LC 8)
Max Grav 7=420(LC 1), 2=318(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

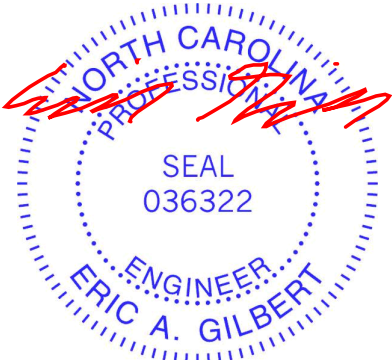
TOP CHORD 2-3=-321/182, 5-6=-260/237
BOT CHORD 2-8=-307/322, 7-8=-237/260

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-7-9 to 3-9-4, Interior(1) 3-9-4 to 6-0-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 7=111.
- Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) . The design/selection of such connection device(s) is the responsibility of others.
- In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
- Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-20, 5-6=-60, 7-9=-20
- Concentrated Loads (lb)
Vert: 13=-210(F)



July 2,2024

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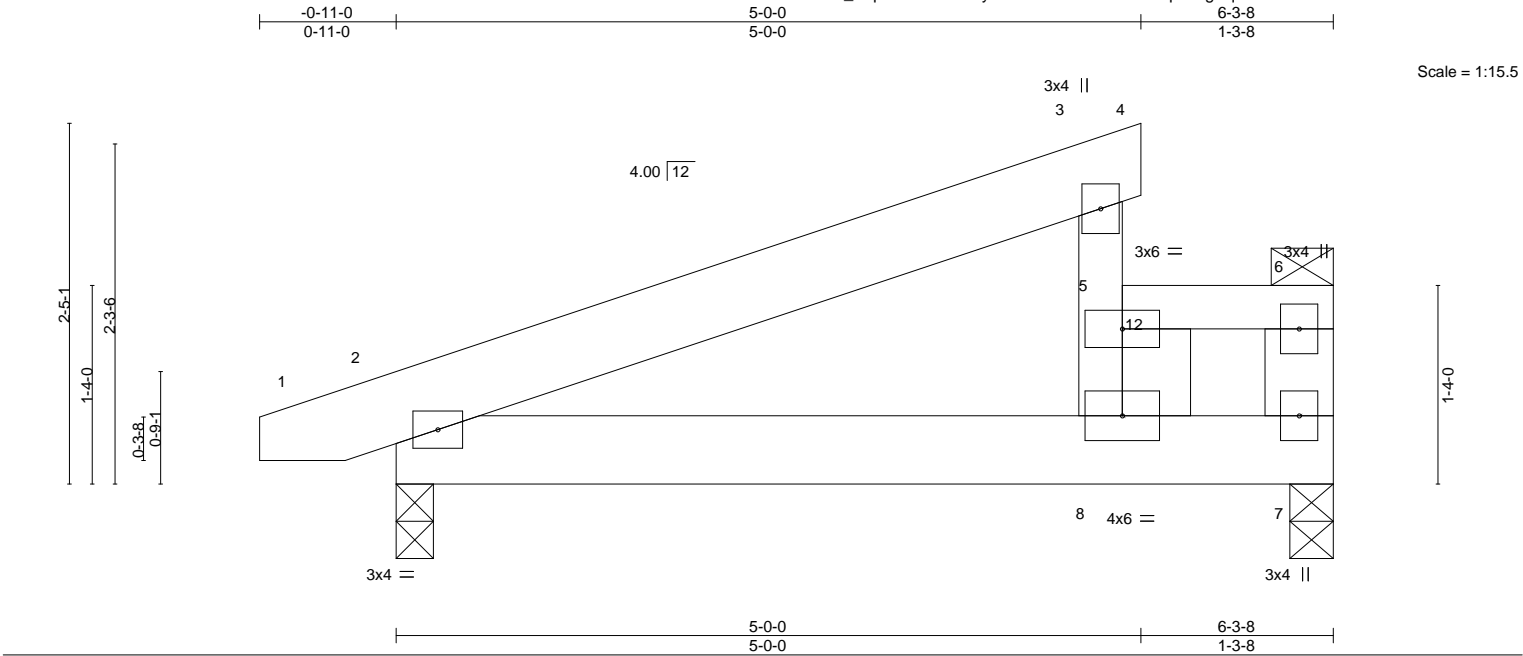
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence	I66592149
J0424-2426	M2-GR	HALF HIP	1	2	Job Reference (optional)	

Comtech, Inc., Fayetteville, NC - 28314,

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ID:bjDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrcD0i7J4zJC?f



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.16	Vert(LL)	-0.00	8	>999	360	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.08	Vert(CT)	-0.01	8-11	>999	240		
BCLL 0.0 *	Rep Stress Incr	NO	WB 0.00	Horz(CT)	0.00	2	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-MR	Wind(LL)	0.00	8	>999	240	Weight: 73 lb	FT = 25%

LUMBER-	BRACING-
TOP CHORD 2x6 SP No.1 *Except* 5-6: 2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-8, 5-6.
BOT CHORD 2x6 SP No.1	Except:
WEBS 2x6 SP No.1 *Except* 3-8: 2x4 SP No.2	6-0-0 oc bracing: 3-5
	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 7=0-3-8, 2=0-3-0
Max Horz 2=126(LC 8)
Max Uplift 7=111(LC 8), 2=89(LC 4)
Max Grav 7=420(LC 1), 2=318(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-320/59
BOT CHORD 2-8=-100/281

- NOTES-**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Webs connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TC DL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); Lumber DOL=1.60 plate grip DOL=1.60
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 7=111.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) . The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-60, 3-4=-20, 5-6=-60, 7-9=-20



July 2,2024

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	M2-GR	HALF HIP	1	2	I66592149
					Job Reference (optional)

LOAD CASE(S) Standard
Concentrated Loads (lb)
Vert: 12=-210(F)

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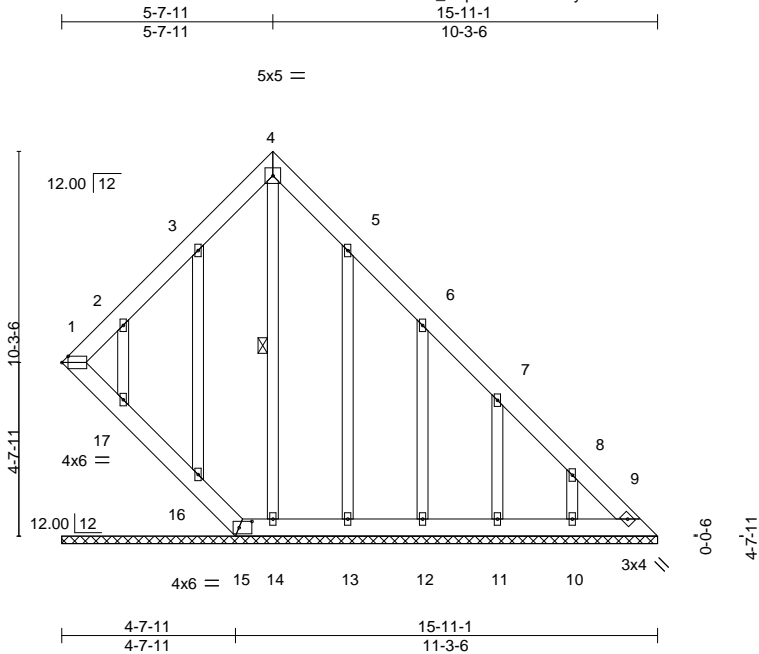


818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	V1GE	GABLE	1	1	I66592150
					Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcD0i7J4zJC?f



Scale = 1:61.6

Plate Offsets (X,Y)--		[1:0-2-0,Edge], [15:0-4-2,0-2-0]								
LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.05	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.04	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.19	Horz(CT)	0.01	9	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S						Weight: 145 lb	FT = 25%

LUMBER-

TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 4-14

REACTIONS.

All bearings 15-11-1.
(lb) - Max Horz 1=-418(LC 13)
Max Uplift All uplift 100 lb or less at joint(s) 9, 14 except 15=-348(LC 13), 16=-175(LC 12), 17=-206(LC 12),
13=-151(LC 13), 12=-223(LC 13), 11=-197(LC 13), 10=-206(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 15, 14, 16, 17, 13, 12, 11, 10 except 1=303(LC 13),
9=299(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 3-4=-234/268, 4-5=-238/253, 7-8=-289/188, 8-9=-456/321
BOT CHORD 1-17=-381/524, 16-17=-379/531, 15-16=-365/524, 14-15=-258/369, 13-14=-258/369,
12-13=-258/369, 11-12=-257/368, 10-11=-256/367, 9-10=-254/365
WEBS 6-12=-254/246

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Bearing at joint(s) 1, 16, 17 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 14 except (jt=lb) 15=348, 16=175, 17=206, 13=151, 12=223, 11=197, 10=206.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 1, 16, 17.



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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	V2	VALLEY	1	1	166592151
					Job Reference (optional)

Comtech, Inc.

Fayetteville, NC - 28314,

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f

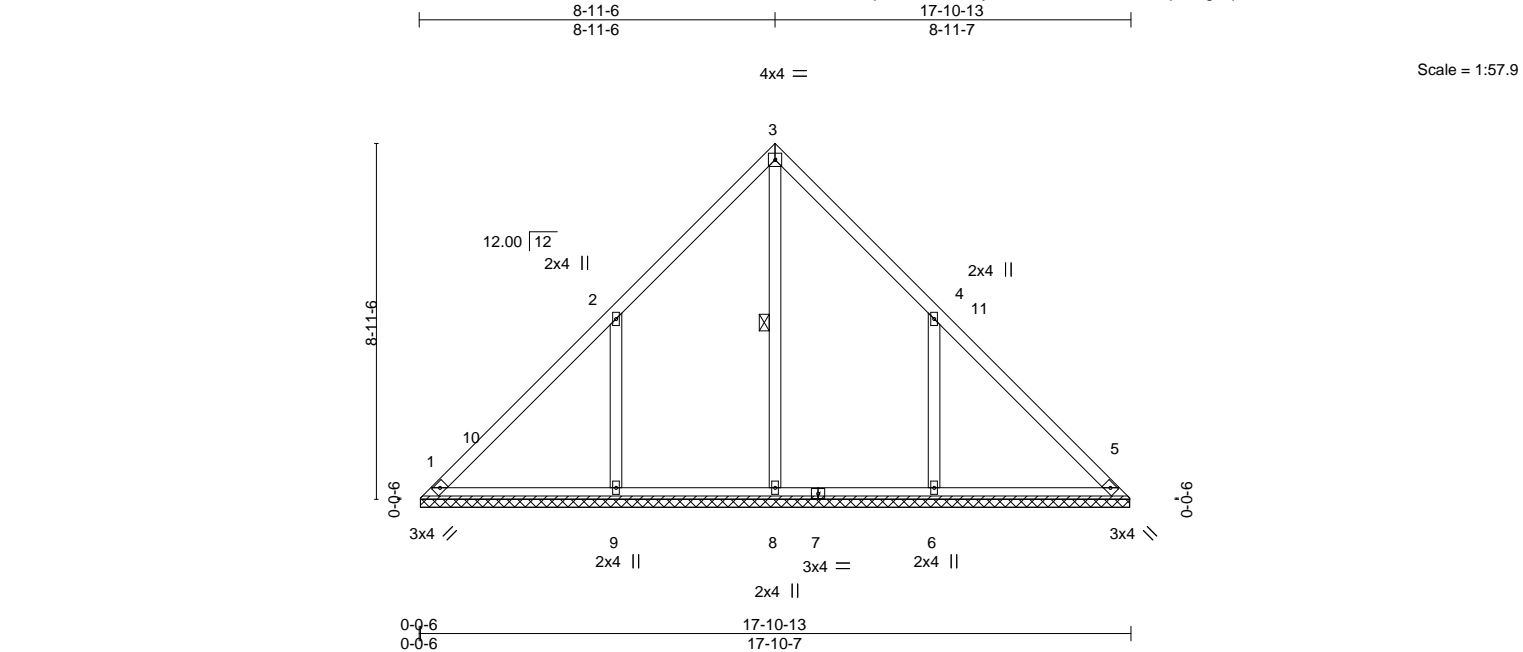


Plate Offsets (X,Y)--		[4:0-0-0,0-0-0]										
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d				PLATES GRIP		
TCLL	20.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.21	Horz(CT)	0.01	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014		Matrix-S							Weight: 88 lb	FT = 25%

LUMBER-

TOP CHORD 2x4 SP No.1

BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

WEBS 1 Row at midpt 3-8

REACTIONS.

All bearings 17-10-1.

(lb) - Max Horz 1=-274(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 9=-331(LC 12), 6=-331(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=426(LC 22), 9=600(LC 19), 6=599(LC 20)

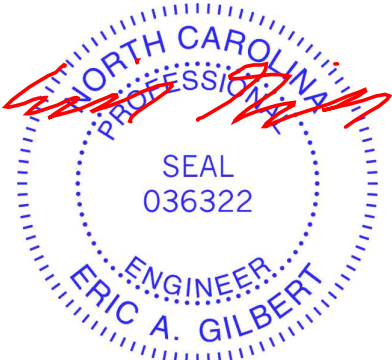
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-257/246, 3-4=-257/246

WEBS 2-9=-589/479, 4-6=-589/479

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TC DL=6.0psf; BC DL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-11-6, Interior(1) 4-11-6 to 8-11-6, Exterior(2) 8-11-6 to 13-4-3, Interior(1) 13-4-3 to 17-6-9 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BC DL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 9=331, 6=331.
- Non Standard bearing condition. Review required.



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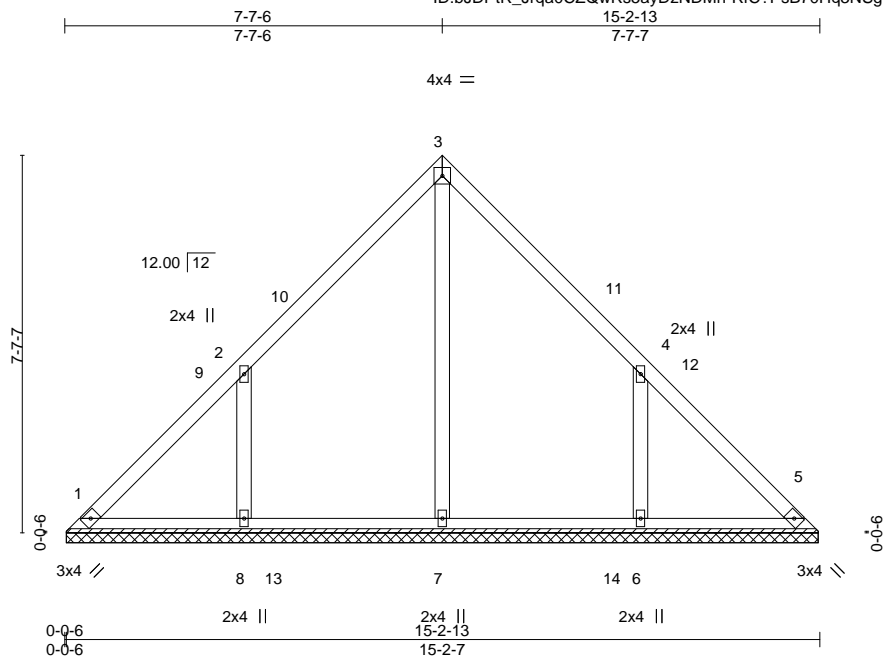


Plate Offsets (X,Y)-- [4:0-0,0,0-0]												
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d				PLATES GRIP		
TCLL	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.18	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014		Matrix-S							Weight: 73 lb	FT = 25%

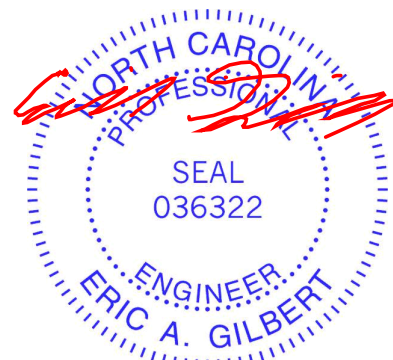
LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.2		

REACTIONS. All bearings 15-2-1.
(lb) - Max Horz 1=232(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=276(LC 12), 6=275(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=421(LC 22), 8=488(LC 19), 6=488(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-497/423, 4-6=-497/423

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Interior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 7-7-6, Exterior(2) 7-7-6 to 12-0-3, Interior(1) 12-0-3 to 14-10-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=276, 6=275.
- 6) Non Standard bearing condition. Review required.



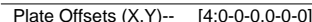
July 2, 2024

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Edenton, NC 27932

Scale = 1:40.3

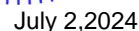
LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.1	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
OTHERS	2x4 SP No.2		

REACTIONS. All bearings 12-6-1.
(lb) - Max Horz 1=189(LC 11)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 8=242(LC 12), 6=242(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 1, 5, 7 except 8=370(LC 19), 6=370(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-8=-448/403, 4-6=-448/403

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 4-9-0, Interior(1) 4-9-0 to 6-3-6, Exterior(2) 6-3-6 to 10-8-3, Interior(1) 10-8-3 to 12-2-9 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=242, 6=242.
- 6) Non Standard bearing condition. Review required.



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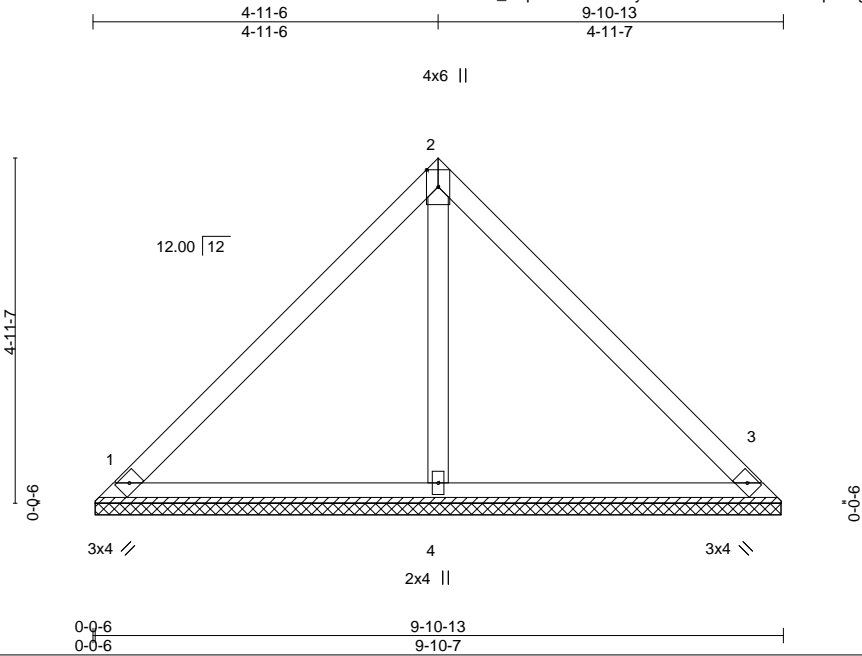


818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	V5	VALLEY	1	1	I66592154
					Job Reference (optional)

Comtech, Inc., Fayetteville, NC - 28314,

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ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.29	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.16	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.07	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-S						Weight: 40 lb	FT = 25%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=9-10-1, 3=9-10-1, 4=9-10-1
Max Horz 1=147(LC 8)
Max Uplift 1=55(LC 13), 3=55(LC 13), 4=19(LC 12)
Max Grav 1=209(LC 1), 3=209(LC 1), 4=319(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.
- 6) Non Standard bearing condition. Review required.



July 2, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate

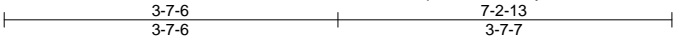
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	V6	VALLEY	1	1	I66592155
					Job Reference (optional)

Comtech, Inc., Fayetteville, NC - 28314,

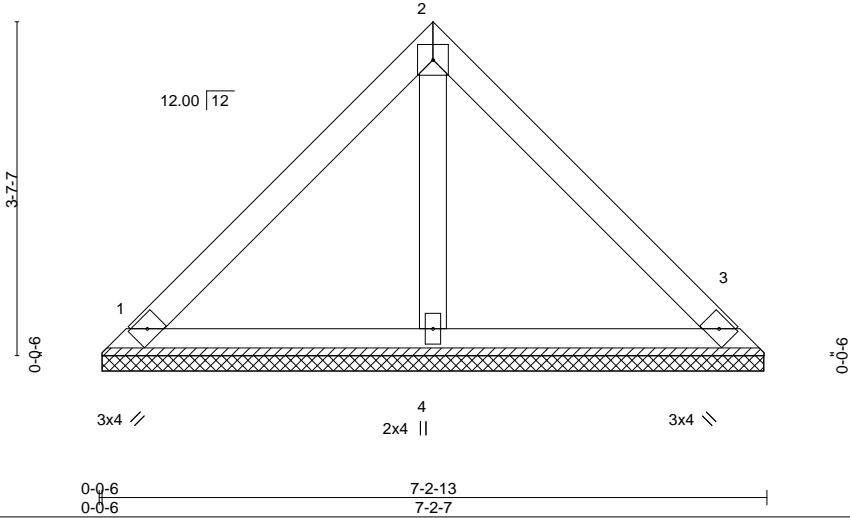
8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:15 2024 Page 1

ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcDoi7J4zJC?f



4x4 =

Scale = 1:25.0



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.23	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.08	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.02	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-P						Weight: 29 lb	FT = 25%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=7-2-1, 3=7-2-1, 4=7-2-1
Max Horz 1=-104(LC 8)
Max Uplift 1=-52(LC 13), 3=-52(LC 13)
Max Grav 1=159(LC 1), 3=159(LC 1), 4=204(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) Non Standard bearing condition. Review required.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

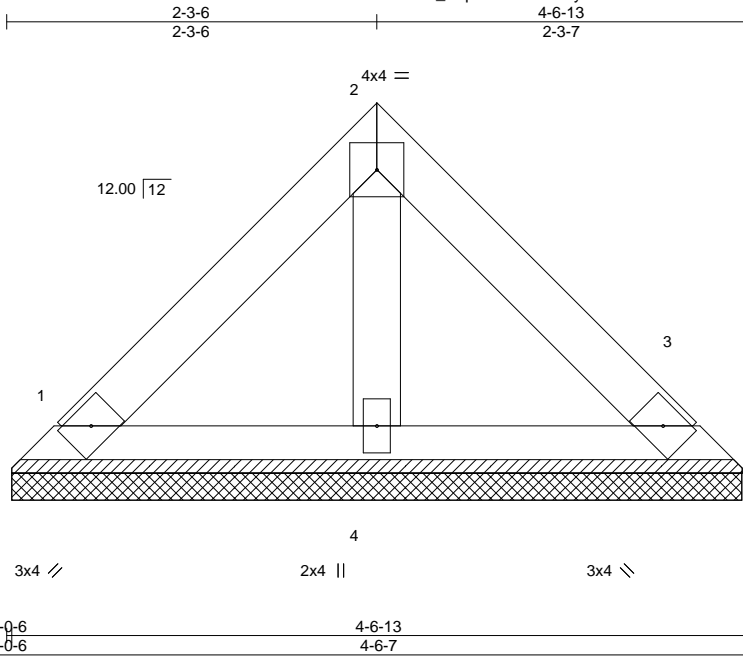
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2426	V7	VALLEY	1	1	I66592156
Comtech, Inc., Fayetteville, NC - 28314,					Job Reference (optional)

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 09:39:15 2024 Page 1
ID:bJDPtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrcD0i7J4zJC?f



Scale = 1:14.2

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	TC 0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.15	BC 0.03	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0 *	Rep Stress Incr	YES	WB 0.01	Horz(CT)	0.00	3	n/a	n/a		
BCDL 10.0	Code IRC2015/TPI2014		Matrix-P						Weight: 17 lb	FT = 25%

LUMBER-

TOP CHORD 2x4 SP No.1
BOT CHORD 2x4 SP No.1
OTHERS 2x4 SP No.2

BRACING-

TOP CHORD Structural wood sheathing directly applied or 4-6-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS.

(size) 1=4-6-1, 3=4-6-1, 4=4-6-1
Max Horz 1=-62(LC 8)
Max Uplift 1=-31(LC 13), 3=-30(LC 13)
Max Grav 1=94(LC 1), 3=94(LC 1), 4=121(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-10; Vult=150mph Vasd=119mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- Non Standard bearing condition. Review required.



July 2, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

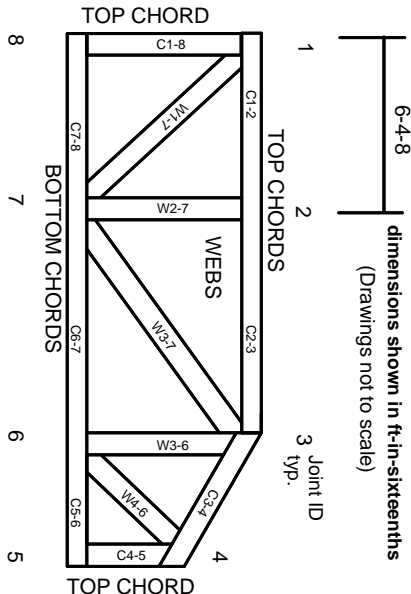


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

MITek

ENGINEERING BY
TRENCO
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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023



ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park
Fayetteville, N.C. 28309
Phone: (910) 864-8787
Fax: (910) 864-4444

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature Jonathan Landry

Jonathan Landry

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION (UP TO)	REQ'D STUDS FOR (1) X HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (1) X HEADER	END REACTION (UP TO)	REQ'D STUDS FOR (1) X HEADER
1700	1	2550	1	3400	1
3400	2	5100	2	6800	2
5100	3	7650	3	10200	3
6800	4	10200	4	13600	4
8500	5	12750	5	17000	5
10200	6	15300	6		
11900	7				
13600	8				
15300	9				

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.
-- Denotes Reaction Greater than 3,000 lbs. Reaction / # of Studs

All Headers Are Considered 2X10 Beams Unless Otherwise Noted

All Walls Shown Are Considered Load Bearing

Plumbing Drop Notes

1. Plumbing drop locations shown are NOT exact.
2. Contractor to verify ALL plumbing drop locations prior to setting Floor Trusses.
3. Adjust spacing as needed not to exceed 24" oc.

Dimension Notes

1. All exterior wall to wall dimensions are to face of sheathing unless noted otherwise
2. All interior wall dimensions are to face of stud unless noted otherwise
3. All exterior wall to truss dimensions are to face of stud unless noted otherwise

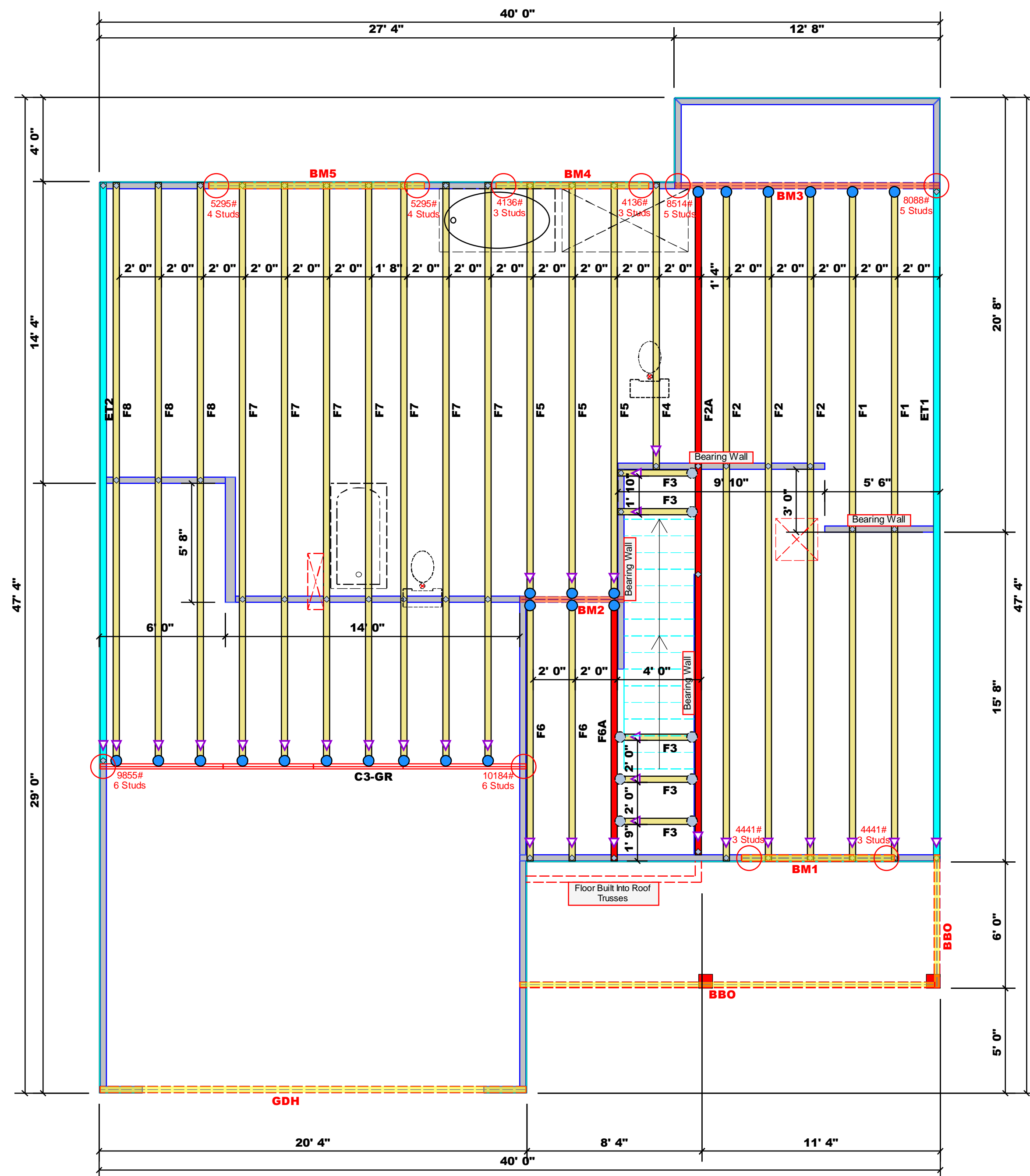
Hatch Legend

- Second Floor Walls
- Box Storage
- Tray Ceiling
- Drop Beam
- Flush Beam

Connector Information					Nail Information	
Sym	Product	Manuf	Qty	Supported Member	Header	Truss
●	HUS410	USP	22	NA	16d/3-1/2"	16d/3-1/2"
●	MSH422	USP	8	Varies	10d/3"	10d/3"

Products				
PlotID	Length	Product	Plies	Net Qty
BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM2	5' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM3	13' 0"	1-3/4"x 16" LVL Kerto-S	2	2
BM4	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2
BM5	11' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2
GDH	21' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2

1 Truss Placement Plan
Scale: 1/4"=1'

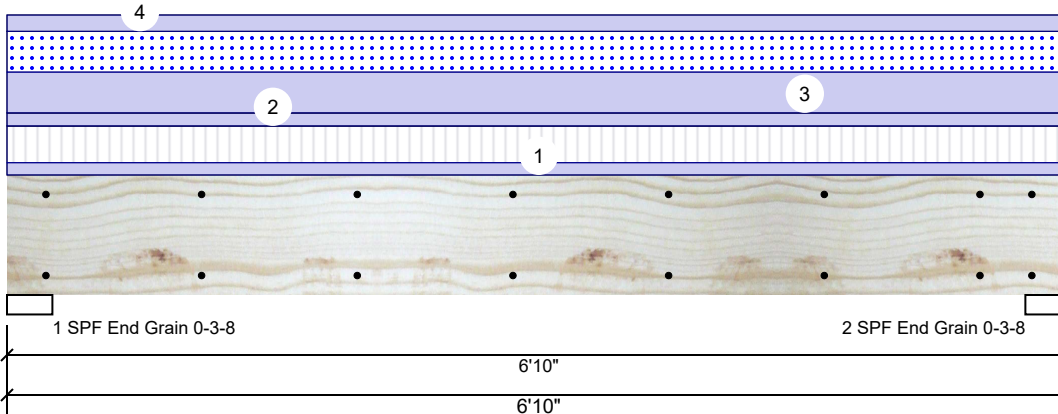


△ = Indicates Left End of Truss
(Reference Engineered Truss Drawing)
Do NOT Erect Truss Backwards

THIS IS A TRUSS PLACEMENT DIAGRAM ONLY. These trusses are designed as individual building components to be incorporated into the building design at the specification of the building designer. See individual design sheets for each truss design identified on the placement drawing. The building designer is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package or online @ sbcindustry.com

BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 360
Importance: Normal - II
Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015
Load Sharing: No
Deck: Not Checked
Ceiling: Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1155	2614	1281	0	0
2	Vertical	1155	2614	1281	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	43%	2614 / 1827	4441	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	Vert	43%	2614 / 1827	4441	L	D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6604 ft-lb	3'5"	14423 ft-lb	0.458 (46%)	D+0.75(L+S)	L
Unbraced	6604 ft-lb	3'5"	10210 ft-lb	0.647 (65%)	D+0.75(L+S)	L
Shear	3066 lb	1' 3/4"	7943 lb	0.386 (39%)	D+0.75(L+S)	L
LL Defl inch	0.053 (L/1451)	3'5"	0.159 (L/480)	0.331 (33%)	0.75(L+S)	L
TL Defl inch	0.128 (L/597)	3'5"	0.212 (L/360)	0.603 (60%)	D+0.75(L+S)	L

Design Notes

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- Refer to last page of calculations for fasteners required for specified loads.
- Girders are designed to be supported on the bottom edge only.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at end bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	113 PLF	338 PLF	0 PLF	0 PLF	0 PLF	F2
2	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Top	375 PLF	0 PLF	375 PLF	0 PLF	0 PLF	A1
4	Uniform			Top	150 PLF	0 PLF	0 PLF	0 PLF	0 PLF	V1GE
	Self Weight				7 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Comtech, Inc.
1001 S Reilly Road
Fayetteville
Cumberland
28314



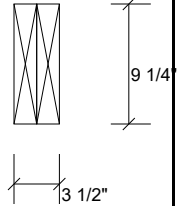
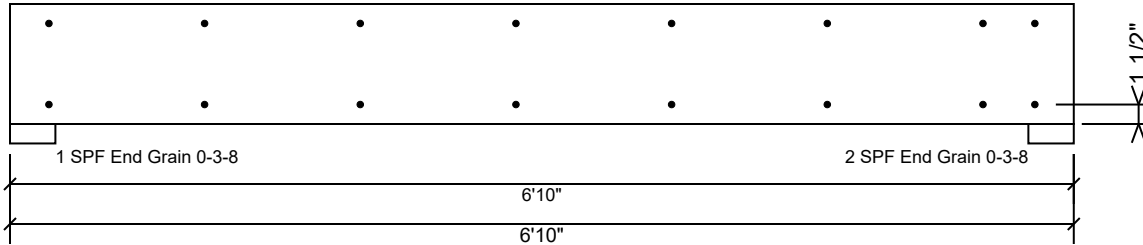
Client: Gammon Construction
Project: The Armstrong
Address:

Date: 7/1/2024
Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

Page 2 of 12

BM1 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
C _m	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

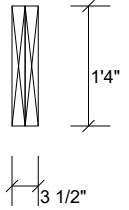
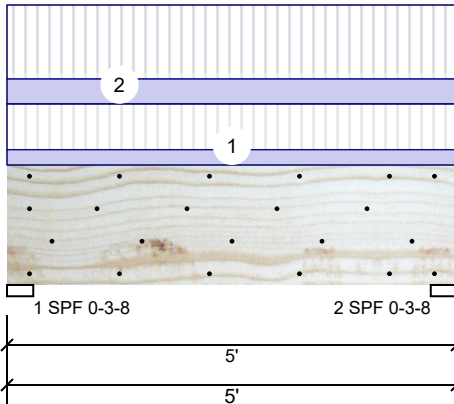
Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Comtech, Inc.
1001 S Reilly Road
Fayetteville
Cumberland
28314

This design is valid until 6/28/2026

BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 360
Importance: Normal - II
Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015
Load Sharing: No
Deck: Not Checked
Ceiling: Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1625	576	0	0	0
2	Vertical	1625	576	0	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF	3.500"	Vert	42%	576 / 1625	2201	L	D+L
2 - SPF	3.500"	Vert	42%	576 / 1625	2201	L	D+L

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2291 ft-lb	2'6"	34565 ft-lb	0.066 (7%)	D+L	L
Unbraced	2291 ft-lb	2'6"	24270 ft-lb	0.094 (9%)	D+L	L
Shear	1944 lb	1'7 1/2"	11947 lb	0.163 (16%)	D+L	L
LL Defl inch	0.006 (L/8929)	2'6"	0.114 (L/480)	0.054 (5%)	L	L
TL Defl inch	0.008 (L/6592)	2'6"	0.152 (L/360)	0.055 (5%)	D+L	L

Design Notes

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- Refer to last page of calculations for fasteners required for specified loads.
- Girders are designed to be supported on the bottom edge only.
- Top must be laterally braced at end bearings.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Near Face	83 PLF	249 PLF	0 PLF	0 PLF	0 PLF	F6
2	Uniform			Far Face	135 PLF	401 PLF	0 PLF	0 PLF	0 PLF	F5
	Self Weight				12 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

Metsä Wood
301 Merritt 7 Building, 2nd Floor
Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

Comtech, Inc.
1001 S Reilly Road
Fayetteville
Cumberland
28314



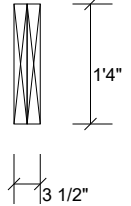
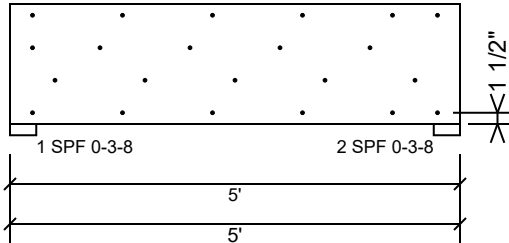
Client: Gammon Construction
Project: The Armstrong
Address:

Date: 7/1/2024
Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

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BM2 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	81.9 %
Load	268.0 PLF
Yield Limit per Foot	327.4 PLF
Yield Limit per Fastener	81.9 lb.
C _m	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

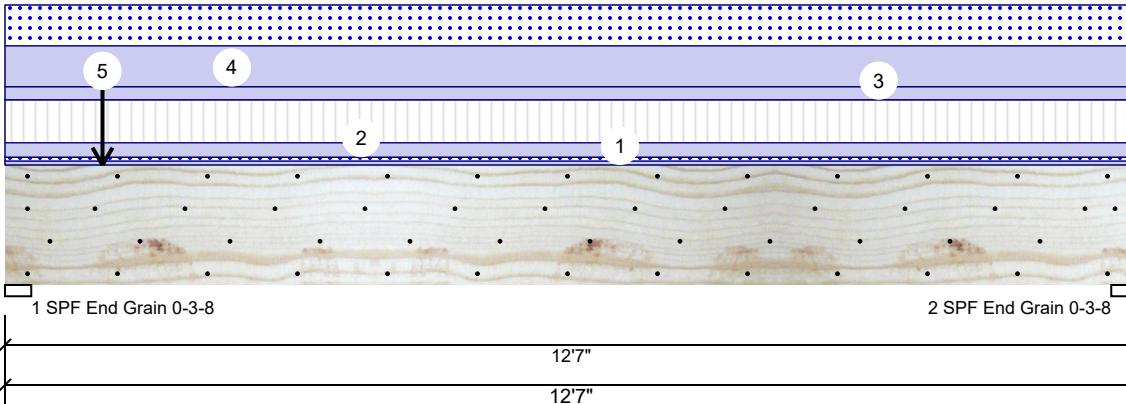
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Norwalk, CT 06851
(800) 622-5850
www.metsawood.com/us

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1001 S Reilly Road
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This design is valid until 6/28/2026

BM3 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Member Information

Type: Girder
Plies: 2
Moisture Condition: Dry
Deflection LL: 480
Deflection TL: 360
Importance: Normal - II
Temperature: Temp <= 100°F

Application: Floor
Design Method: ASD
Building Code: IBC/IRC 2015
Load Sharing: No
Deck: Not Checked
Ceiling: Gypsum 1/2"

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	2911	4392	2586	0	0
2	Vertical	2518	4261	2586	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	83%	4392 / 4122	8514	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	Vert	79%	4261 / 3828	8088	L	D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	23817 ft-lb	6'3 3/16"	39750 ft-lb	0.599 (60%)	D+0.75(L+S)	L
Unbraced	23817 ft-lb	6'3 3/16"	23866 ft-lb	0.998 (100%)	D+0.75(L+S)	L
Shear	6330 lb	1'7 1/2"	11947 lb	0.530 (53%)	D+L	L
LL Defl inch	0.149 (L/978)	6'3 5/16"	0.304 (L/480)	0.491 (49%)	0.75(L+S)	L
TL Defl inch	0.314 (L/464)	6'3 3/8"	0.405 (L/360)	0.776 (78%)	D+0.75(L+S)	L

Design Notes

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- Refer to last page of calculations for fasteners required for specified loads.
- Girders are designed to be supported on the bottom edge only.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 4'9 1/2" o.c.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	36 PLF	0 PLF	36 PLF	0 PLF	0 PLF	J1
2	Uniform			Near Face	132 PLF	395 PLF	0 PLF	0 PLF	0 PLF	F1
3	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
4	Uniform			Top	375 PLF	0 PLF	375 PLF	0 PLF	0 PLF	A1
5	Point	1-1-0		Near Face	153 lb	458 lb	0 lb	0 lb	0 lb	F2A
	Self Weight				12 PLF					

Notes

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Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

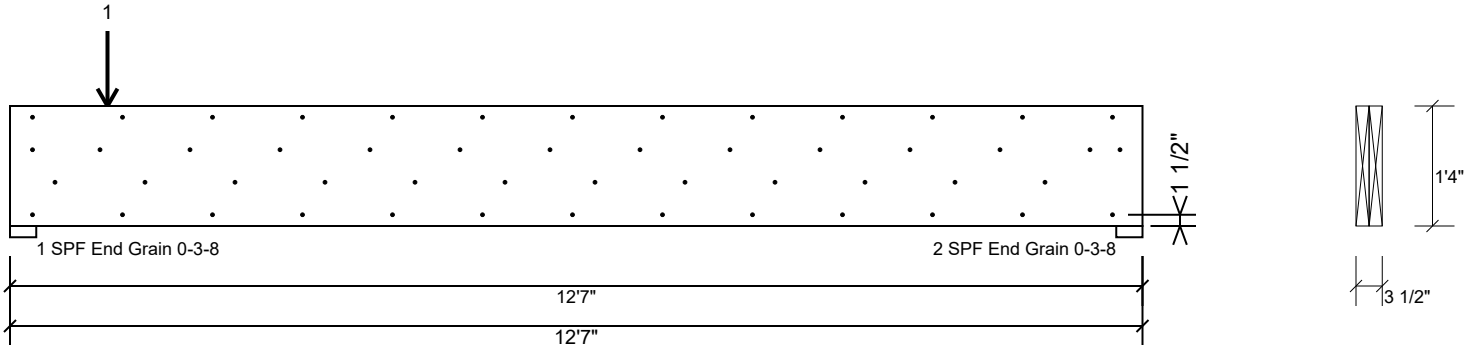
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BM3 Kerto-S LVL 1.750" X 16.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 4 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	80.5 %
Load	263.5 PLF
Yield Limit per Foot	327.4 PLF
Yield Limit per Fastener	81.9 lb.
C _m	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	D+L
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

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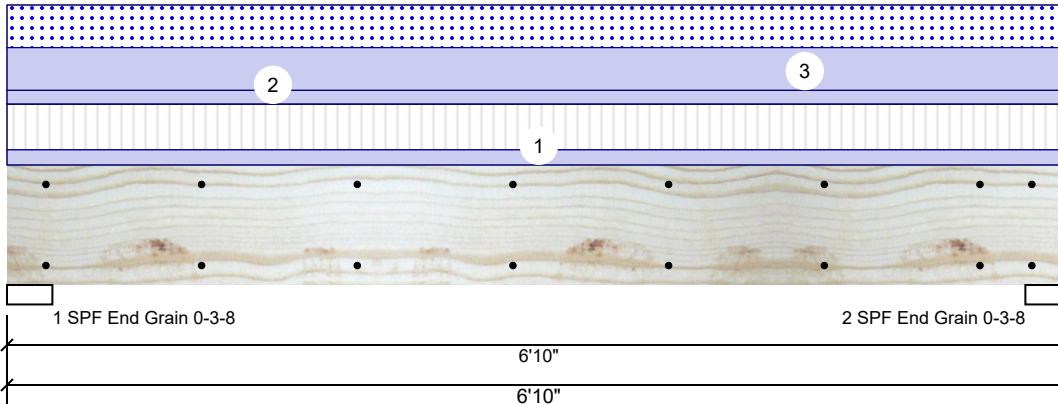
Client: Gammon Construction
Project: The Armstrong
Address:

Date: 7/1/2024
Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

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BM4 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II	Ceiling:	Gypsum 1/2"
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1363	2160	1271	0	0
2	Vertical	1363	2160	1271	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	40%	2160 / 1976	4136	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	Vert	40%	2160 / 1976	4136	L	D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6149 ft-lb	3'5"	14423 ft-lb	0.426 (43%)	D+0.75(L+S)	L
Unbraced	6149 ft-lb	3'5"	10210 ft-lb	0.602 (60%)	D+0.75(L+S)	L
Shear	2855 lb	5'9 1/4"	7943 lb	0.359 (36%)	D+0.75(L+S)	L
LL Defl inch	0.057 (L/1342)	3'5"	0.159 (L/480)	0.358 (36%)	0.75(L+S)	L
TL Defl inch	0.119 (L/641)	3'5"	0.212 (L/360)	0.561 (56%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	133 PLF	399 PLF	0 PLF	0 PLF	0 PLF	F5
2	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Top	372 PLF	0 PLF	372 PLF	0 PLF	0 PLF	A2
	Self Weight				7 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

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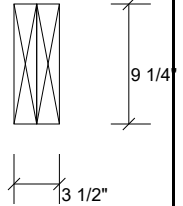
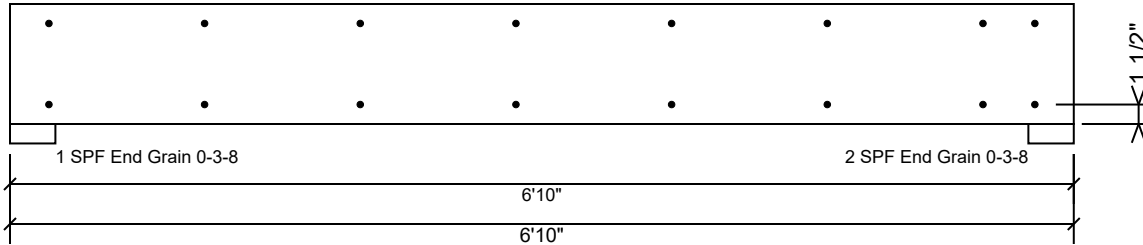
Client: Gammon Construction
Project: The Armstrong
Address:

Date: 7/1/2024
Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

Page 8 of 12

BM4 Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
C _m	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

Manufacturer Info

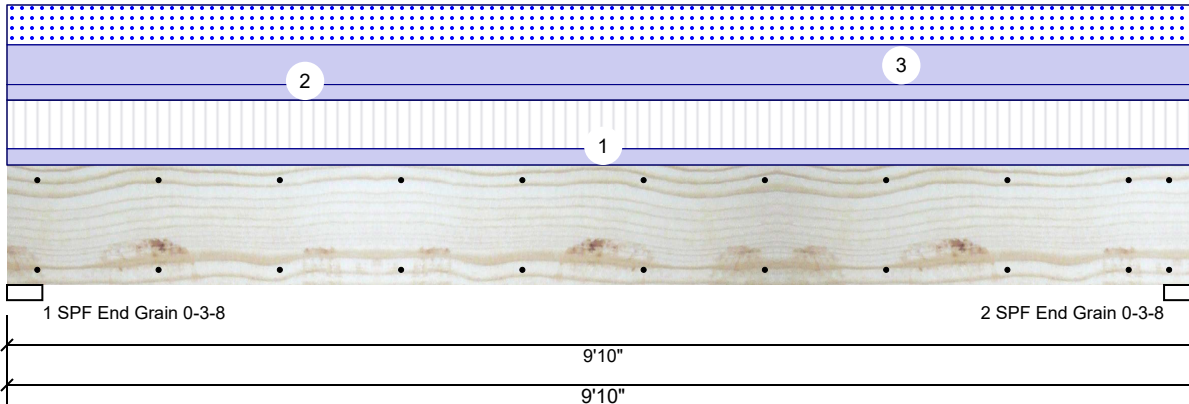
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This design is valid until 6/28/2026

BM5 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II	Ceiling:	Gypsum 1/2"
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	1854	2769	1514	0	0
2	Vertical	1854	2769	1514	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	51%	2769 / 2526	5295	L	D+0.75(L+S)
2 - SPF End Grain	3.500"	Vert	51%	2769 / 2526	5295	L	D+0.75(L+S)

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	10330 ft-lb	4'11"	19911 ft-lb	0.519 (52%)	D+L	L
Unbraced	11832 ft-lb	4'11"	11839 ft-lb	0.999 (100%)	D+0.75(L+S)	L
Shear	3427 lb	1'3 3/8"	8867 lb	0.387 (39%)	D+L	L
LL Defl inch	0.107 (L/1051)	4'11"	0.234 (L/480)	0.457 (46%)	0.75(L+S)	L
TL Defl inch	0.224 (L/501)	4'11"	0.312 (L/360)	0.718 (72%)	D+0.75(L+S)	L

Design Notes

- Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- Refer to last page of calculations for fasteners required for specified loads.
- Girders are designed to be supported on the bottom edge only.
- Top loads must be supported equally by all plies.
- Top must be laterally braced at a maximum of 7'4 5/16" o.c.
- Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	126 PLF	377 PLF	0 PLF	0 PLF	0 PLF	F7
2	Uniform			Top	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	Wall
3	Uniform			Top	308 PLF	0 PLF	308 PLF	0 PLF	0 PLF	B2
	Self Weight				9 PLF					

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- Dry service conditions, unless noted otherwise
- LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

- LVL beams must not be cut or drilled
- Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
- Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info

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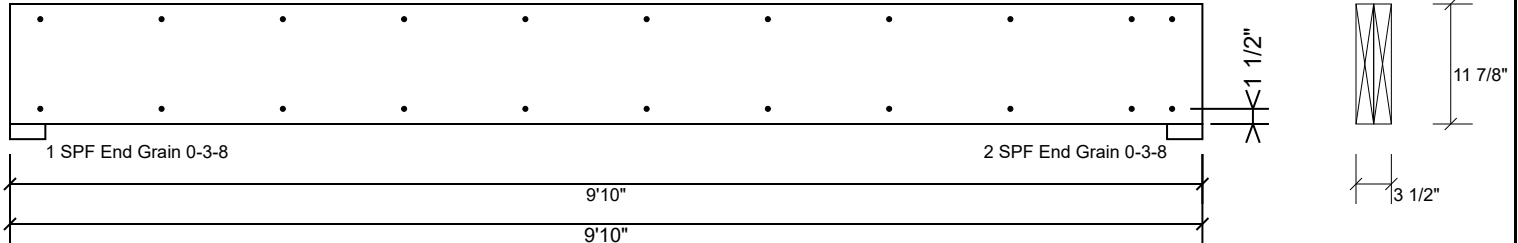
Client: Gammon Construction
Project: The Armstrong
Address:

Date: 7/1/2024
Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

Page 10 of 12

BM5 Kerto-S LVL 1.750" X 11.875" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
C _m	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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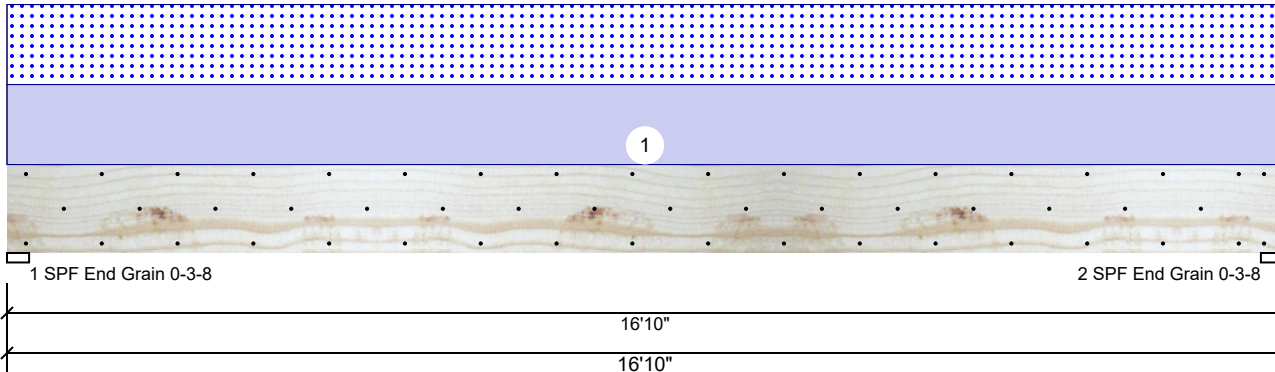
Client: Gammon Construction
Project: The Armstrong
Address:

Date: 7/1/2024
Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

Page 11 of 12

GDH Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Member Information

Type:	Girder	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC/IRC 2015
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Deck:	Not Checked
Importance:	Normal - II	Ceiling:	Gypsum 1/2"
Temperature:	Temp <= 100°F		

Reactions UNPATTERNED lb (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1859	1768	0	0
2	Vertical	0	1859	1768	0	0

Bearings

Bearing	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	35%	1859 / 1768	3627	L	D+S
2 - SPF End Grain	3.500"	Vert	35%	1859 / 1768	3627	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	14442 ft-lb	8'5"	31049 ft-lb	0.465 (47%)	D+S	L
Unbraced	14442 ft-lb	8'5"	14446 ft-lb	1.000 (100%)	D+S	L
Shear	3011 lb	1'5 1/2"	12021 lb	0.250 (25%)	D+S	L
LL Defl inch	0.229 (L/859)	8'5 1/16"	0.409 (L/480)	0.559 (56%)	S	L
TL Defl inch	0.469 (L/419)	8'5 1/16"	0.546 (L/360)	0.860 (86%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 7'1 3/16" o.c.
- 7 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Top	210 PLF	0 PLF	210 PLF	0 PLF	0 PLF	C1GE
	Self Weight				11 PLF					

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive chemicals

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

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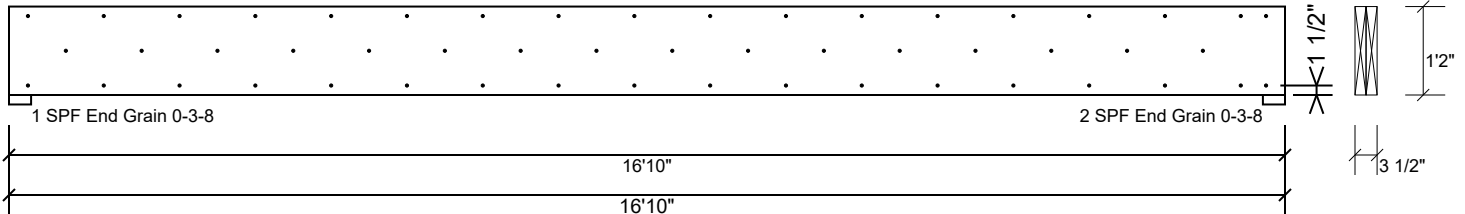
Client: Gammon Construction
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Input by: Jonathan Landry
Job Name: Delude Residence
Project #: J0424-2427

Page 12 of 12

GDH Kerto-S LVL 1.750" X 14.000" 2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	245.6 PLF
Yield Limit per Fastener	81.9 lb.
C _m	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structural Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

1. Dry service conditions, unless noted otherwise
2. LVL not to be treated with fire retardant or corrosive

chemicals

Handling & Installation

1. LVL beams must not be cut or drilled
2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
3. Damaged Beams must not be used
4. Design assumes top edge is laterally restrained
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6. For flat roofs provide proper drainage to prevent ponding

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Cumberland
28314

Trenco
818 Soundside Rd
Edenton, NC 27932

Re: J0424-2427
Delude Residence

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Comtech, Inc - Fayetteville.

Pages or sheets covered by this seal: I66593256 thru I66593267

My license renewal date for the state of North Carolina is December 31, 2024.

North Carolina COA: C-0844



July 2, 2024

Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	ET1	GABLE	1	1	I66593256

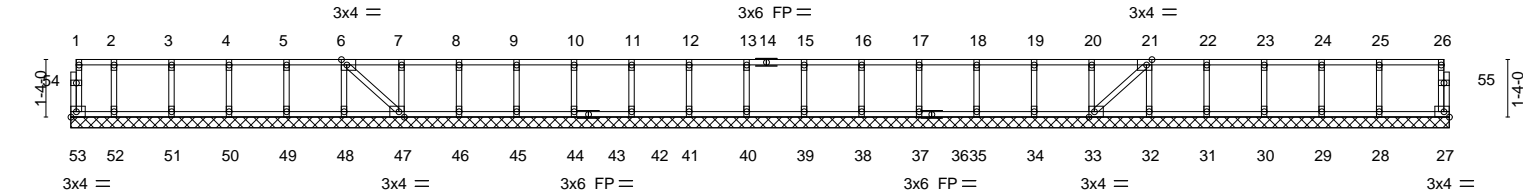
Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:12 2024 Page 1
ID:bJDPTR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

0-1-8

0-1-8

Scale = 1:53.4



1-0-0	2-4-0	3-8-0	5-0-0	6-4-0	7-8-0	9-0-0	10-4-0	11-8-0	13-0-0	14-4-0	15-8-0	17-0-0	18-4-0	19-8-0	21-0-0	22-4-0	23-8-0	25-0-0	26-4-0	27-8-0	29-0-0	30-4-0	31-11-8
1-0-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-4-0	1-7-8
Plate Offsets (X,Y)-- [6:0-1-8,Edge], [21:0-1-8,Edge], [33:0-1-8,Edge], [47:0-1-8,Edge]																							

LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.08	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.01	Vert(CT)	n/a	-	n/a	999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.04	Horz(CT)	-0.00	33	n/a	n/a		
BCDL 5.0	Code IRC2015/TPI2014		Matrix-S						Weight: 142 lb	FT = 20%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.1(flat)	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	
OTHERS 2x4 SP No.3(flat)	

REACTIONS. All bearings 31-11-8.
(lb) - Max Grav All reactions 250 lb or less at joint(s) 53, 27, 52, 51, 50, 49, 48, 47, 46, 45, 44, 42, 41, 40, 39, 38, 37, 35, 34, 33, 32, 31, 30, 29, 28

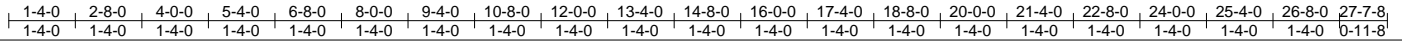
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES-**
- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
 - 2) Plates checked for a plus or minus 1 degree rotation about its center.
 - 3) Gable requires continuous bottom chord bearing.
 - 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - 5) Gable studs spaced at 1-4-0 oc.
 - 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



July 2,2024

Scale = 1:46.2



July 2, 2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI 1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbccomponents.com)

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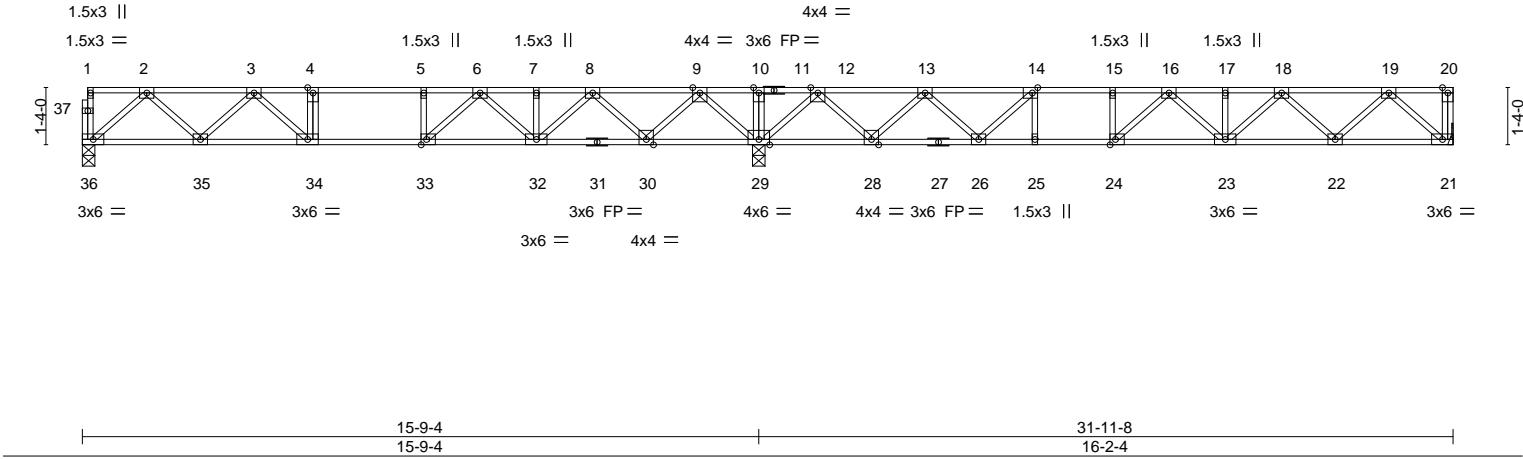
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F1	Floor	2	1	I66593258

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:13 2024 Page 1

ID:bJDPTR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.69	Vert(LL)	-0.18 23-24 >999 480	MT20		244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.94	Vert(CT)	-0.24 23-24 >790 360				
BCLL	0.0	Rep Stress Incr	YES	WB	0.53	Horz(CT)	0.04 21 n/a n/a				
BCDL	5.0	Code	IRC2015/TPI2014	Matrix-S							
								Weight: 167 lb		FT = 20%F, 11%E	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1(flat)	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.1(flat)	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2x4 SP No.3(flat)		

REACTIONS. (size) 36=0-3-8, 29=0-3-8, 21=Mechanical
Max Grav 36=734(LC 3), 29=2081(LC 1), 21=790(LC 4)

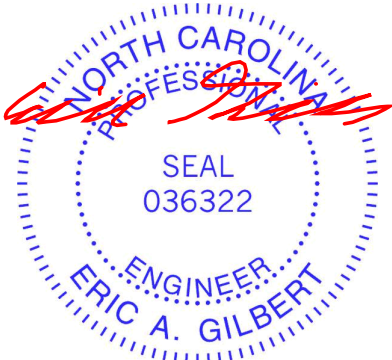
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1260/0, 3-4=-2013/0, 4-5=-2013/0, 5-6=-2013/0, 6-7=-1431/158, 7-8=-1431/158, 8-9=-227/612, 9-10=0/2258, 10-12=0/2258, 12-13=-537/871, 13-14=-1681/364, 14-15=-2218/57, 15-16=-2218/57, 16-17=-2175/0, 17-18=-2175/0, 18-19=-1370/0

BOT CHORD 35-36=0/786, 34-35=0/1711, 33-34=0/2013, 32-33=0/1789, 30-32=-369/930, 29-30=-1236/0, 28-29=-1166/0, 26-28=-607/1226, 25-26=-57/2218, 24-25=-57/2218, 23-24=0/2324, 22-23=0/1881, 21-22=0/840

WEBS 2-36=-1044/0, 2-35=0/659, 3-35=-627/0, 3-34=-67/403, 9-29=-1460/0, 9-30=0/1081, 8-30=-1046/0, 8-32=0/753, 6-32=-565/0, 6-33=0/656, 5-33=-339/0, 12-29=-1503/0, 12-28=0/1110, 13-28=-1061/0, 13-26=0/762, 19-21=-1118/0, 19-22=0/738, 18-22=-710/0, 18-23=-15/400, 16-24=-504/8, 14-26=-957/0, 14-25=0/309

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x4 MT20 unless otherwise indicated.
 - 3) Plates checked for a plus or minus 1 degree rotation about its center.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 6) CAUTION, Do not erect truss backwards.

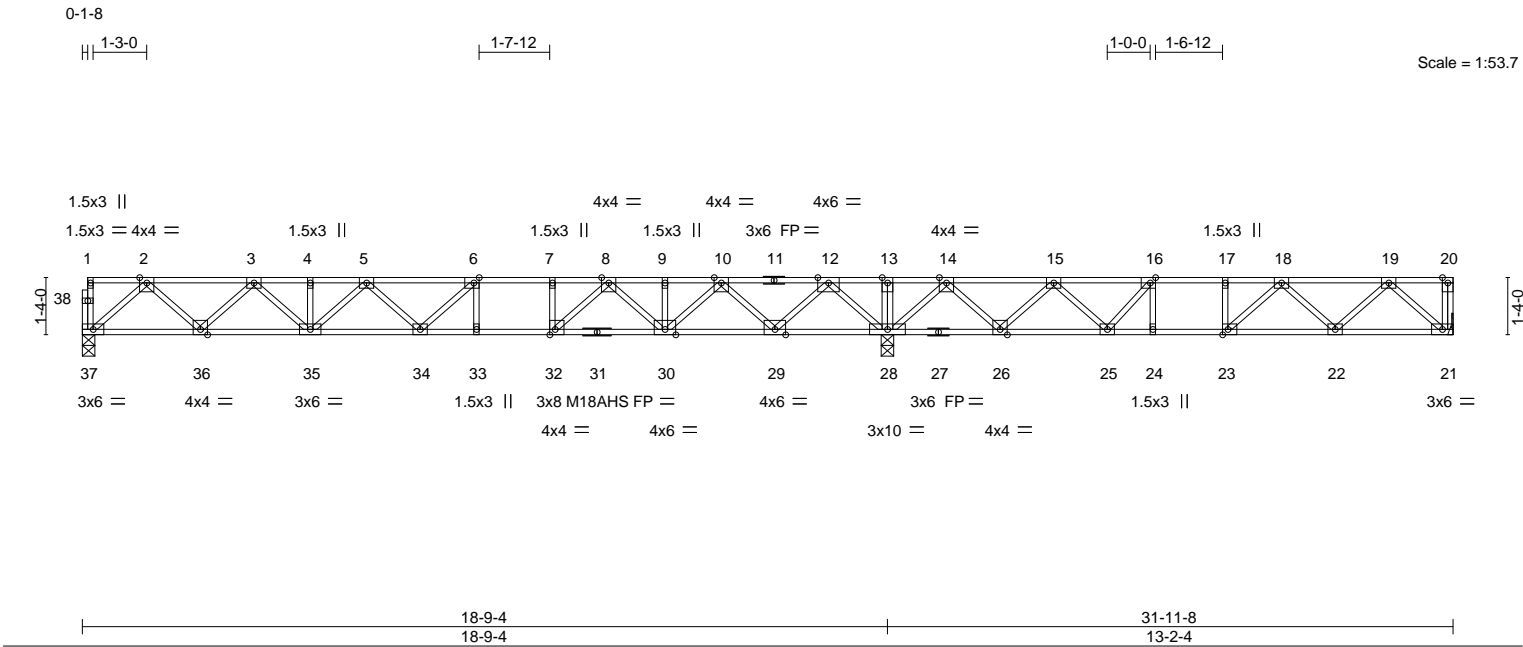


July 2,2024

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F2	Floor	3	1	I66593259

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:14 2024 Page 1
ID:bJDPTR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



LOADING (psf)	SPACING-	2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.72	Vert(LL)	-0.23 33-34	>952	480	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.71	Vert(CT)	-0.32 33-34	>705	360	M18AHS	186/179
BCLL 0.0	Rep Stress Incr	YES	WB 0.62	Horz(CT)	0.04 28	n/a	n/a		
BCDL 5.0	Code IRC2015/TPI2014		Matrix-S						
								Weight: 168 lb	FT = 20%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP 2400F 2.0E(flat)	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	

REACTIONS. (size) 37=0-3-8, 21=Mechanical, 28=0-3-8
Max Grav 37=900(LC 3), 21=611(LC 4), 28=2115(LC 1)

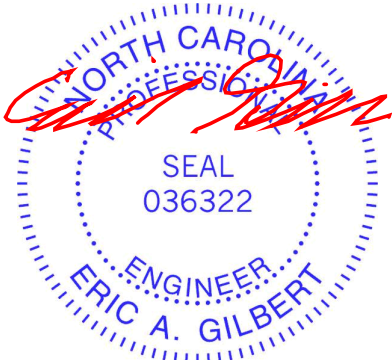
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1627/0, 3-4=-2648/0, 4-5=-2648/0, 5-6=-3038/0, 6-7=-2935/0, 7-8=-2935/0, 8-9=-1837/0, 9-10=-1837/0, 10-12=-310/473, 12-13=0/2326, 13-14=0/2326, 14-15=-296/1130, 15-16=-1129/604, 16-17=-1379/321, 17-18=-1379/321, 18-19=-991/14

BOT CHORD 36-37=0/973, 35-36=0/2249, 34-35=0/3002, 33-34=0/2935, 32-33=0/2935, 30-32=0/2404, 29-30=-205/1168, 28-29=-1062/0, 26-28=-1434/0, 25-26=-858/839, 24-25=-321/1379, 23-24=-321/1379, 22-23=-108/1302, 21-22=0/641

WEBS 2-37=-1293/0, 2-36=0/910, 3-36=-865/0, 3-35=0/543, 12-28=-1682/0, 12-29=0/1294, 10-29=-1241/0, 10-30=0/957, 8-30=-825/0, 8-32=0/942, 5-35=-481/0, 6-34=-134/433, 6-33=-325/0, 7-32=-370/0, 19-21=-853/0, 19-22=-37/487, 18-22=-433/131, 18-23=-335/104, 14-28=-1316/0, 14-26=0/944, 15-26=-902/0, 15-25=0/599, 16-25=-728/0, 16-24=0/285

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are MT20 plates unless otherwise indicated.
 - 3) All plates are 3x4 MT20 unless otherwise indicated.
 - 4) Plates checked for a plus or minus 1 degree rotation about its center.
 - 5) Refer to girder(s) for truss to truss connections.
 - 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 7) CAUTION, Do not erect truss backwards.



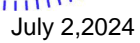
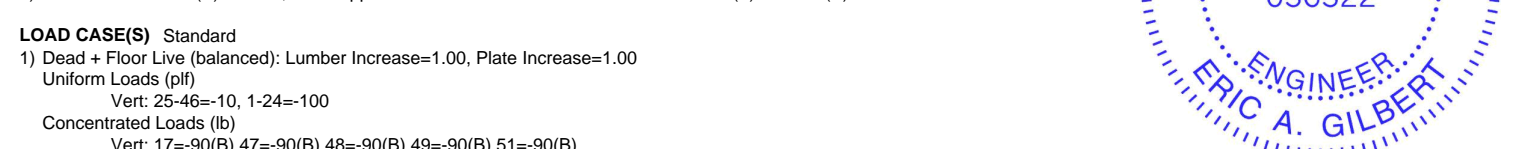
July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompoments.com)

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A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Comtech, Inc. Fayetteville, NC - 28314, 8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:15 2024 Page 1
ID:bJDpTR_Jrqa0CZQwRs5ayDzNDmN-RfC?PsB70Hq3NSgPqnL8w3uITxbGKwRCDoi7J4zJC?f



Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F3	Floor	5	1	I66593261

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:16 2024 Page 1

ID:bJDPTR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWwCDoi7J4zJC?f

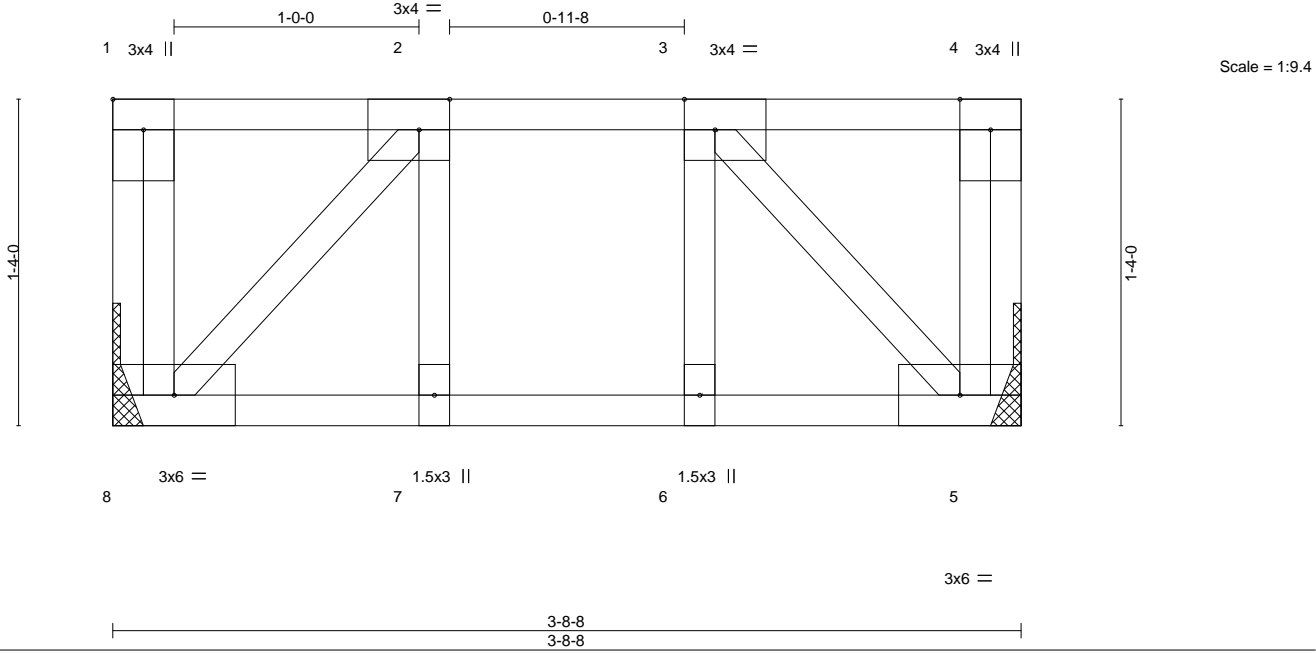


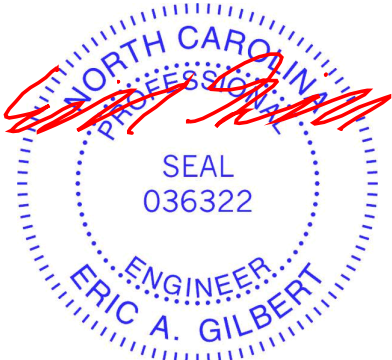
Plate Offsets (X,Y)-- [1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,Edge]									
LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06	Vert(LL)	-0.00	7	>999
TCDL	10.0	Lumber DOL	1.00	BC	0.05	Vert(CT)	-0.00	7	>999
BCLL	0.0	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00	5	n/a
BCDL	5.0	Code	IRC2015/TPI2014	Matrix-S					
							PLATES	GRIP	
							MT20	244/190	
							Weight: 25 lb	FT = 20%F, 11%E	

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1(flat)	TOP CHORD	Structural wood sheathing directly applied or 3-8-8 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.1(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3(flat)		

REACTIONS. (size) 8=Mechanical, 5=Mechanical
Max Grav 8=190(LC 1), 5=190(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced floor live loads have been considered for this design.
2) Plates checked for a plus or minus 1 degree rotation about its center.
3) Refer to girder(s) for truss to truss connections.
4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails.
Strongbacks to be attached to walls at their outer ends or restrained by other means.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
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TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F4	Floor	1	1	166593262

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:16 2024 Page 1
ID:bJDPTR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

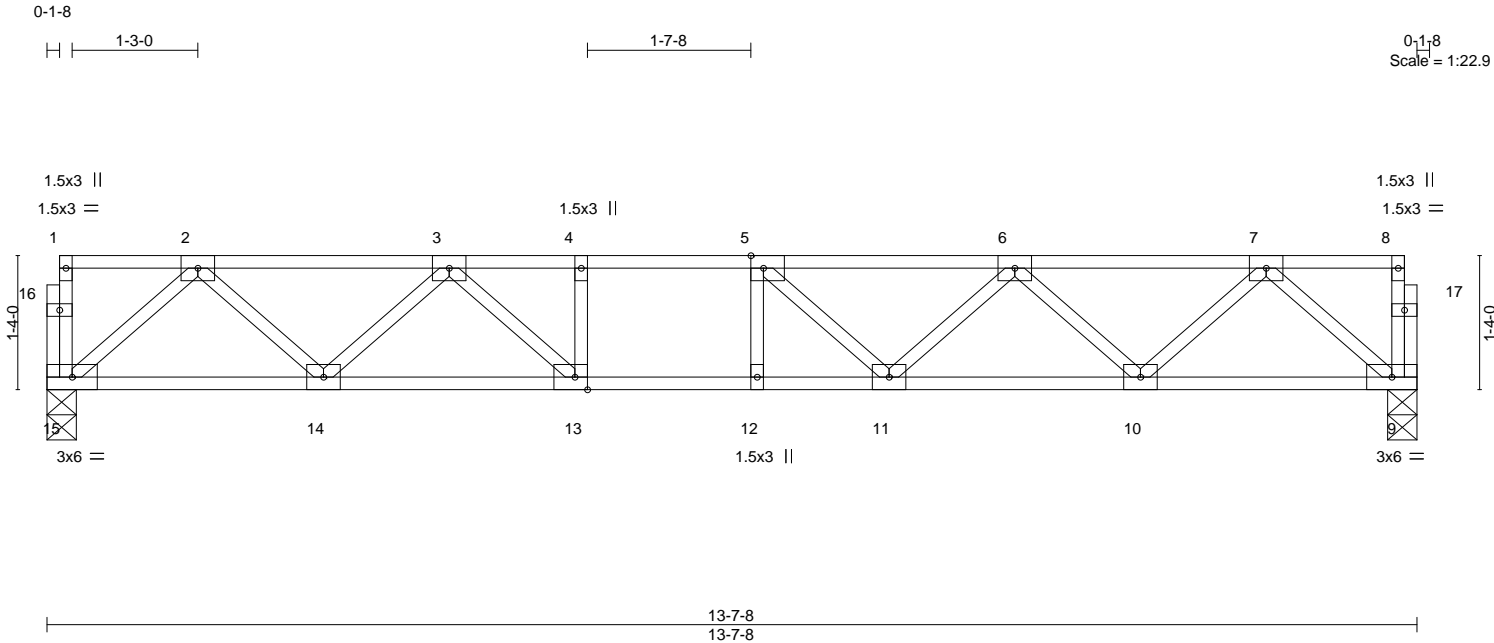


Plate Offsets (X,Y)--		[5:0-1-8,Edge], [13:0-1-8,Edge]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCLL 40.0	Plate Grip DOL	1.00	TC 0.40
TCDL 10.0	Lumber DOL	1.00	BC 0.66
BCLL 0.0	Rep Stress Incr	YES	WB 0.32
BCDL 5.0	Code	IRC2015/TPI2014	Matrix-S
			DEFL.
			in (loc) l/defl L/d
			Vert(LL) -0.11 11-12 >999 480
			Vert(CT) -0.14 11-12 >999 360
			Horz(CT) 0.03 9 n/a n/a
			PLATES GRIP
			MT20 244/190
			Weight: 71 lb FT = 20%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.1(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	

REACTIONS. (size) 15=0-3-8, 9=0-3-8
Max Grav 15=729(LC 1), 9=729(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1247/0, 3-4=-1999/0, 4-5=-1999/0, 5-6=-1892/0, 6-7=-1261/0
BOT CHORD 14-15=0/779, 13-14=0/1705, 12-13=0/1999, 11-12=0/1999, 10-11=0/1725, 9-10=0/772
WEBS 7-9=-1025/0, 2-15=-1034/0, 7-10=0/680, 2-14=0/651, 6-10=-645/0, 3-14=-637/0, 6-11=0/306, 3-13=0/554, 5-11=-324/47

NOTES-

- Unbalanced floor live loads have been considered for this design.
- All plates are 3x4 MT20 unless otherwise indicated.
- Plates checked for a plus or minus 1 degree rotation about its center.
- Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

ENGINEERING BY
TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F5	Floor	3	1	I66593263

Comtech, Inc., Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:17 2024 Page 1

ID:bJDPTR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

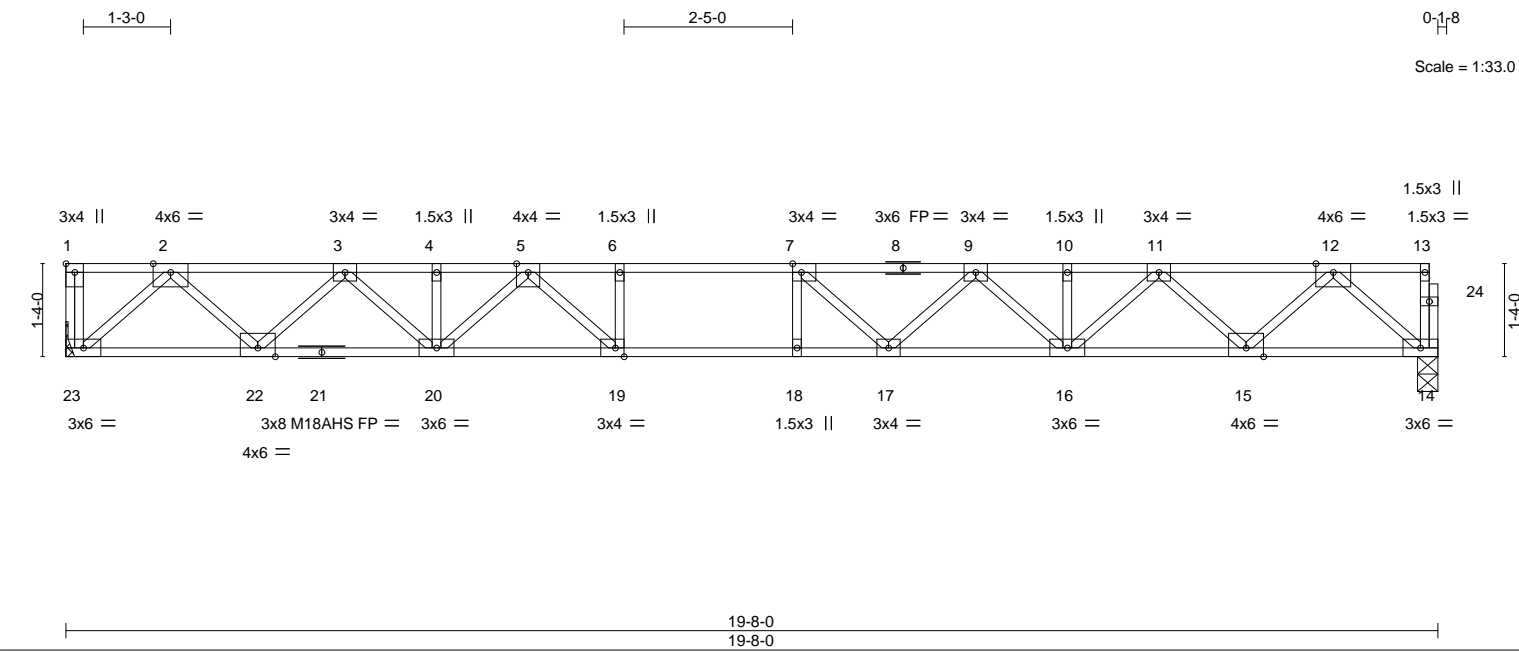


Plate Offsets (X,Y)-- [1:Edge,0-1-8], [7:0-1-8,Edge], [19:0-1-8,Edge]									
LOADING (psf)		SPACING- 2-0-0		CSI.		DEFL. in (loc) l/defl L/d		PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.53	Vert(LL)	-0.29 17-18 >804 480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.73	Vert(CT)	-0.40 17-18 >588 360	M18AHS	186/179
BCLL	0.0	Rep Stress Incr	YES	WB	0.54	Horz(CT)	0.06 14 n/a n/a		
BCDL	5.0	Code IRC2015/TPI2014		Matrix-S				Weight: 103 lb	FT = 20%F, 11%E

LUMBER-			BRACING-	
TOP CHORD	2x4	SP 2400F 2.0E(flat)	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4	SP 2400F 2.0E(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4	SP No.3(flat)		

REACTIONS. (size) 23=Mechanical, 14=0-3-8
Max Grav 23=1068(LC 1), 14=1062(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1977/0, 3-4=-3351/0, 4-5=-3351/0, 5-6=-4227/0, 6-7=-4227/0, 7-9=-4074/0, 9-10=-3346/0, 10-11=-3346/0, 11-12=-1976/0

BOT CHORD 22-23=0/1158, 20-22=0/2760, 19-20=0/3815, 18-19=0/4227, 17-18=0/4227, 16-17=0/3853, 15-16=0/2765, 14-15=0/1156

WEBS 2-23=-1542/0, 2-22=0/1139, 3-22=-1089/0, 3-20=0/803, 12-14=-1536/0, 12-15=0/1141, 11-15=-1098/0, 11-16=0/790, 5-20=-632/0, 5-19=0/857, 9-16=-690/0, 9-17=0/475, 7-17=-563/148, 6-19=-398/0

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are MT20 plates unless otherwise indicated.
 - 3) Plates checked for a plus or minus 1 degree rotation about its center.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 6) CAUTION, Do not erect truss backwards.

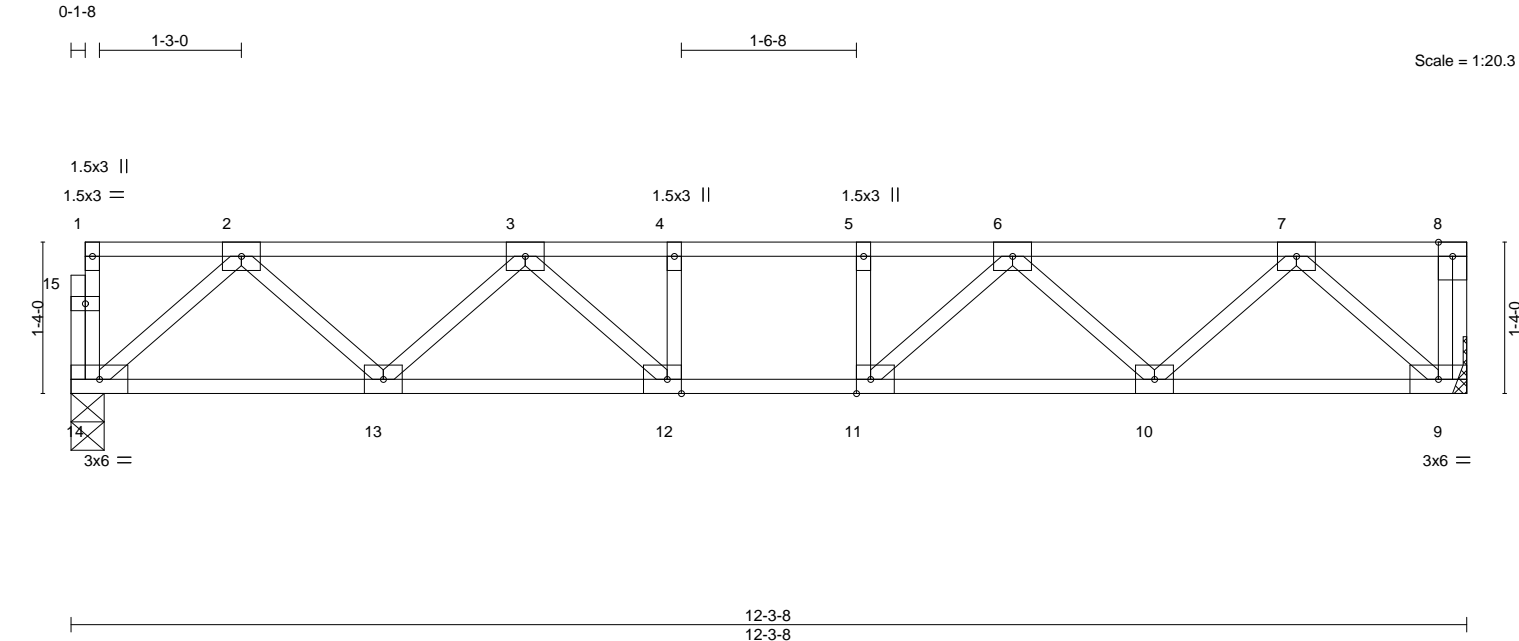


July 2,2024

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F6	Floor	2	1	166593264

Comtech, Inc, Fayetteville, NC - 28314,

8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:18 2024 Page 1
ID:bJDPTtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES		GRIP	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.26	Vert(LL)	-0.06 10-11 >999 480	MT20		244/190	
TCDL	10.0	Lumber DOL	1.00	BC	0.35	Vert(CT)	-0.08 10-11 >999 360				
BCLL	0.0	Rep Stress Incr	YES	WB	0.26	Horz(CT)	0.02 9 n/a n/a				
BCDL	5.0	Code	IRC2015/TPI2014	Matrix-S							
								Weight: 65 lb FT = 20%F, 11%E			

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1(flat)	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.1(flat)	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3(flat)		

REACTIONS. (size) 14=0-3-8, 9=Mechanical
Max Grav 14=656(LC 1), 9=662(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1097/0, 3-4=-1627/0, 4-5=-1627/0, 5-6=-1627/0, 6-7=-1097/0
BOT CHORD 13-14=0/697, 12-13=0/1466, 11-12=0/1627, 10-11=0/1466, 9-10=0/698
WEBS 2-14=-926/0, 2-13=0/556, 3-13=-513/0, 7-9=-929/0, 7-10=0/556, 6-10=-513/0, 6-11=0/392, 3-12=0/392

- NOTES-**
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x4 MT20 unless otherwise indicated.
 - 3) Plates checked for a plus or minus 1 degree rotation about its center.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 6) CAUTION, Do not erect truss backwards.



July 2,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F6A	Floor	1	1	I66593265

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8.430 s Jan 6 2022 MiTek Industries, Inc. Mon Jul 1 10:02:18 2024 Page 1

ID:bJDPTtR_Jrqa0CZQwRs5ayDzNDMn-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

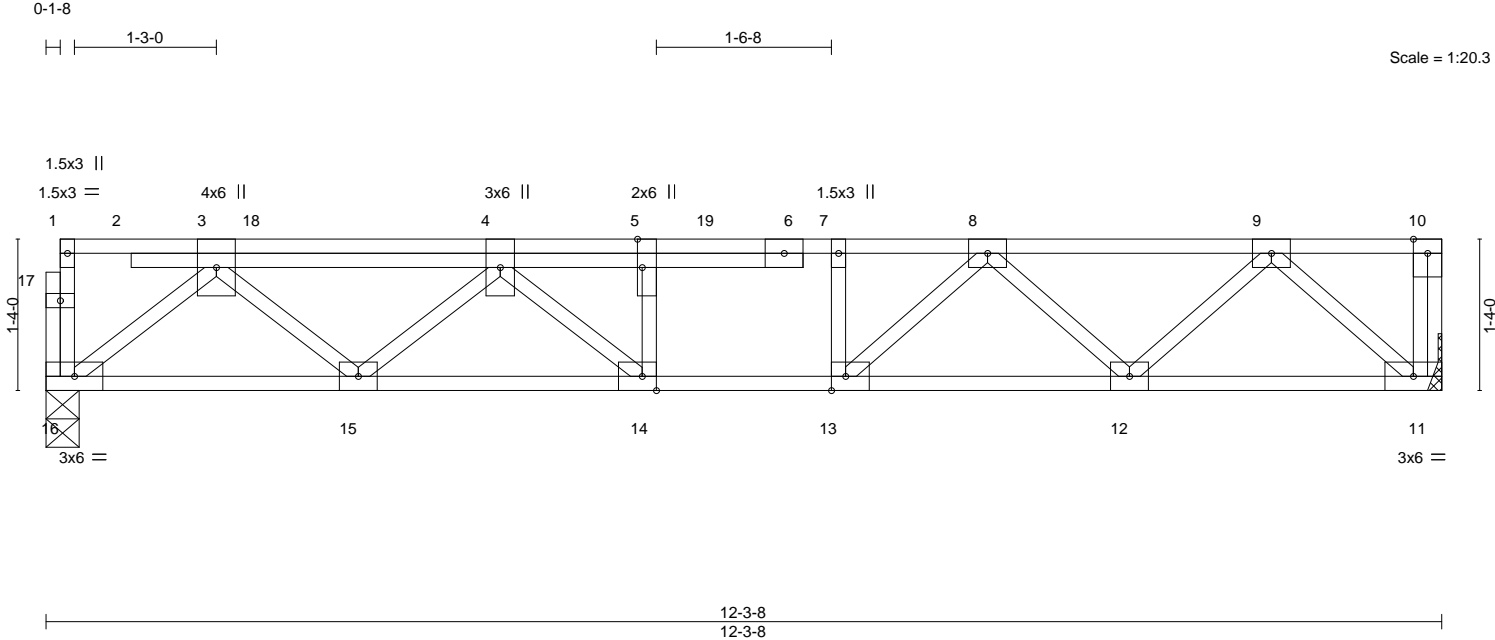


Plate Offsets (X,Y)--		[5:0-3-0,Edge], [13:0-1-8,Edge], [14:0-1-8,Edge]	
LOADING (psf)	SPACING-	2-0-0	CSI.
TCLL 40.0	Plate Grip DOL	1.00	TC 0.40
TCDL 10.0	Lumber DOL	1.00	BC 0.52
BCLL 0.0	Rep Stress Incr	NO	WB 0.35
BCDL 5.0	Code	IRC2015/TPI2014	Matrix-S
DEFL.	in (loc)	l/defl	L/d
Vert(LL)	-0.07 14-15	>999	480
Vert(CT)	-0.10 14-15	>999	360
Horz(CT)	0.03 11	n/a	n/a
PLATES	GRIP		
MT20	244/190		
Weight: 74 lb	FT = 20%F, 11%E		

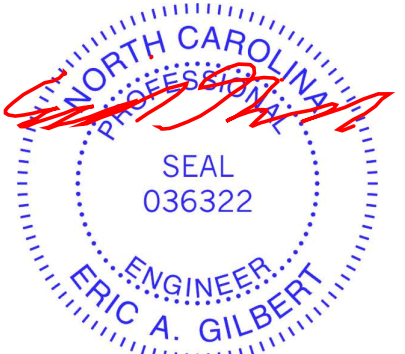
LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.1(flat)	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	

REACTIONS. (size) 16=0-3-8, 11=Mechanical
Max Grav 16=841(LC 1), 11=748(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-1505/0, 4-5=-2040/0, 5-7=-2042/0, 7-8=-2040/0, 8-9=-1276/0
BOT CHORD 15-16=0/959, 14-15=0/2005, 13-14=0/2040, 12-13=0/1741, 11-12=0/794
WEBS 3-16=-1246/0, 3-15=0/741, 4-15=-678/0, 9-11=-1057/0, 9-12=0/670, 8-12=-647/0, 8-13=0/543, 7-13=-289/0

- NOTES-
- Unbalanced floor live loads have been considered for this design.
 - All plates are 3x4 MT20 unless otherwise indicated.
 - Plates checked for a plus or minus 1 degree rotation about its center.
 - Refer to girder(s) for truss to truss connections.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION, Do not erect truss backwards.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 170 lb down at 1-10-12, and 170 lb down at 3-10-12, and 107 lb down at 5-10-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00
Uniform Loads (plf)
Vert: 11-16=-10, 1-10=-100
Concentrated Loads (lb)
Vert: 4=-90(F) 18=-90(F) 19=-90(F)



July 2,2024

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Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacompnents.com)

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TRENCO
A MiTek Affiliate
818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F7	Floor	7	1	I66593266

Comtech, Inc., Fayetteville, NC - 28314,

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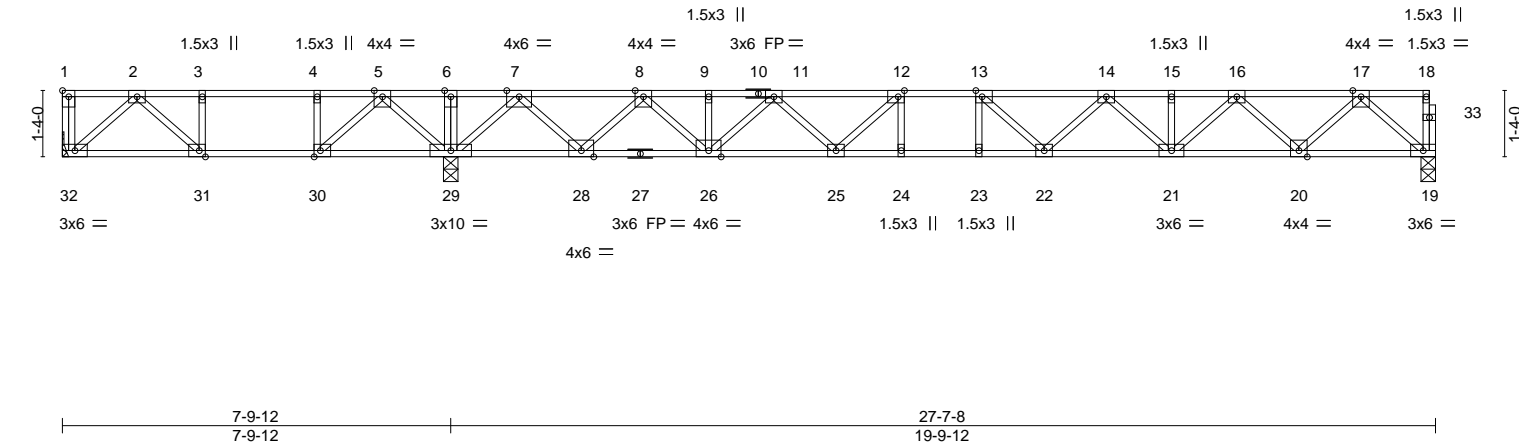


Plate Offsets (X,Y)-- [1:Edge,0-1-8], [12:0-1-8,Edge], [13:0-1-8,Edge], [30:0-1-8,Edge], [31:0-1-8,Edge]									
LOADING (psf)		SPACING-		CSI.		DEFL.		PLATES	
TCLL	40.0	Plate Grip DOL	1.00	TC	0.93	Vert(LL)	-0.29 23 >810 480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.99	Vert(CT)	-0.40 23 >592 360		
BCLL	0.0	Rep Stress Incr	YES	WB	0.62	Horz(CT)	0.07 19 n/a n/a		
BCDL	5.0	Code	IRC2015/TP12014	Matrix-S				Weight: 145 lb	FT = 20%F, 11%E

LUMBER-		BRACING-	
TOP CHORD	2x4 SP No.1(flat)	TOP CHORD	Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.1(flat)	BOT CHORD	Rigid ceiling directly applied or 2-2-0 oc bracing.
WEBS	2x4 SP No.3(flat)		

REACTIONS. (size) 32=Mechanical, 29=0-3-8, 19=0-3-8
Max Uplift 32=-89(LC 4)
Max Grav 32=356(LC 3), 29=1802(LC 1), 19=1003(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-433/468, 3-4=-433/468, 4-5=-433/468, 5-6=0/1456, 6-7=0/1456, 7-8=-974/0, 8-9=-2535/0, 9-11=-2535/0, 11-12=-3444/0, 12-13=-3802/0, 13-14=-3696/0, 14-15=-3095/0, 15-16=-3095/0, 16-17=-1849/0

BOT CHORD 31-32=-136/322, 30-31=-468/433, 29-30=-957/53, 26-28=0/1856, 25-26=0/3115, 24-25=0/3802, 23-24=0/3802, 22-23=0/3802, 21-22=0/3538, 20-21=0/2578, 19-20=0/1090

WEBS 2-32=-428/181, 2-31=-451/151, 5-29=-833/0, 5-30=0/924, 4-30=-477/0, 17-19=-1448/0, 17-20=0/1056, 16-20=-1014/0, 16-21=0/704, 7-29=-1668/0, 7-28=0/1303, 8-28=-1253/0, 8-26=0/949, 11-26=-810/0, 11-25=0/549, 14-21=-601/0, 14-22=-8/362, 13-22=-421/200, 12-25=-691/0, 12-24=-112/250

- NOTES-**
- Unbalanced floor live loads have been considered for this design.
 - All plates are 3x4 MT20 unless otherwise indicated.
 - Plates checked for a plus or minus 1 degree rotation about its center.
 - Refer to girder(s) for truss to truss connections.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 32.
 - Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - CAUTION, Do not erect truss backwards.



July 2,2024

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818 Soundside Road
Edenton, NC 27932

Job	Truss	Truss Type	Qty	Ply	Delude Residence
J0424-2427	F8	Floor	3	1	

I66593267

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Job Reference (optional)

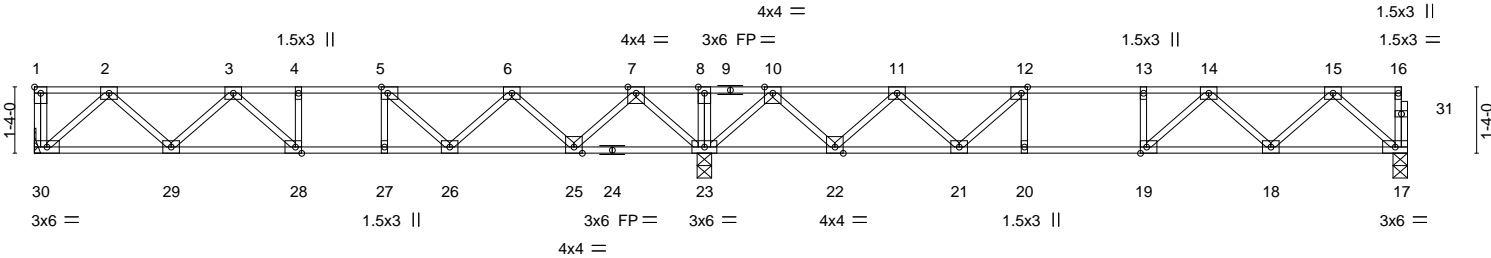


Plate Offsets (X,Y)--	[1:Edge,0-1-8], [5:0-1-8,Edge], [12:0-1-8,Edge], [19:0-1-8,Edge], [28:0-1-8,Edge]
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LOADING (psf)	SPACING-		CSI.	DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL 1.00		TC 0.50	Vert(LL)	-0.11	18-19	>999	480	MT20	244/190
TCDL 10.0	Lumber DOL 1.00		BC 0.63	Vert(CT)	-0.14	18-19	>999	360		
BCLL 0.0	Rep Stress Incr YES		WB 0.43	Horz(CT)	0.03	17	n/a	n/a		
BCDL 5.0	Code IRC2015/TPI2014		Matrix-S							
									Weight: 142 lb	FT = 20%F, 11%E

LUMBER-	BRACING-
TOP CHORD 2x4 SP No.1(flat)	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.1(flat)	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3(flat)	

REACTIONS. (size) 30=Mechanical, 23=0-3-8, 17=0-3-8
Max Grav 30=649(LC 3), 23=1791(LC 1), 17=670(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-3=-1068/0, 3-4=-1564/0, 4-5=-1564/0, 5-6=-1307/150, 6-7=-495/531, 7-8=0/1689, 8-10=0/1689, 10-11=-447/402, 11-12=-1357/34, 12-13=-1686/0, 13-14=-1686/0, 14-15=-1130/0
BOT CHORD 29-30=0/682, 28-29=0/1424, 27-28=0/1564, 26-27=0/1564, 25-26=-322/1040, 23-25=-769/0, 22-23=-811/0, 21-22=-202/1034, 20-21=0/1686, 19-20=0/1686, 18-19=0/1510, 17-18=0/715
WEBS 2-30=-908/0, 2-29=0/536, 3-29=-495/4, 7-23=-1243/0, 7-25=0/885, 6-25=-848/0, 6-26=0/486, 5-26=-576/0, 10-23=-1277/0, 10-22=0/911, 11-22=-872/0, 11-21=0/527, 15-17=-949/0, 15-18=0/577, 14-18=-529/0, 14-19=-83/265, 12-21=-625/0

- NOTES-
- 1) Unbalanced floor live loads have been considered for this design.
 - 2) All plates are 3x4 MT20 unless otherwise indicated.
 - 3) Plates checked for a plus or minus 1 degree rotation about its center.
 - 4) Refer to girder(s) for truss to truss connections.
 - 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
 - 6) CAUTION, Do not erect truss backwards.



July 2,2024

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Edenton, NC 27932

Symbols

PLATE LOCATION AND ORIENTATION



* Plate location details available in MITek software or upon request.

PLATE SIZE

4 X 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

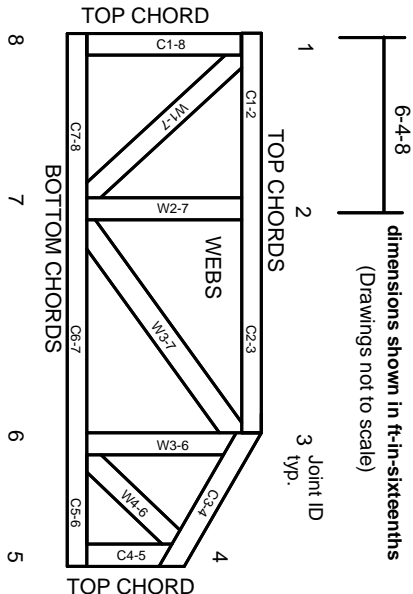


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TP1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-22: Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282
ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TP1 1 section 6.3. These truss designs rely on lumber values established by others.

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TP1 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TP1 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TP1 1 Quality Criteria.
21. The design does not take into account any dynamic or other loads other than those expressly stated.

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MITek Engineering Reference Sheet: MII-7473 rev. 1/2/2023