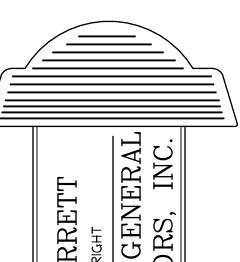


SCALE 1/8" = 1'-0"



INC

CONRACTORS

PROBUILT

GARRETT THE

> ∞ 9 $\overline{}$ #

1678 249 244 526 H H H H FIRST FLOOR UNF REC ROOM FRONT PORCH GARAGE

CTHEATHER HALL
35 HEATHERSTONE C
BENSON NC 27504
(919) 207-1403 165

H SQUARED HOME DESIGN, INC.

ANY DEVIATION OF THE SPECIFIED MEASUREMENTS OR DIMENSIONS VOIDS H SQUARED HOME DESIGN, INC.'S LIABILITY.

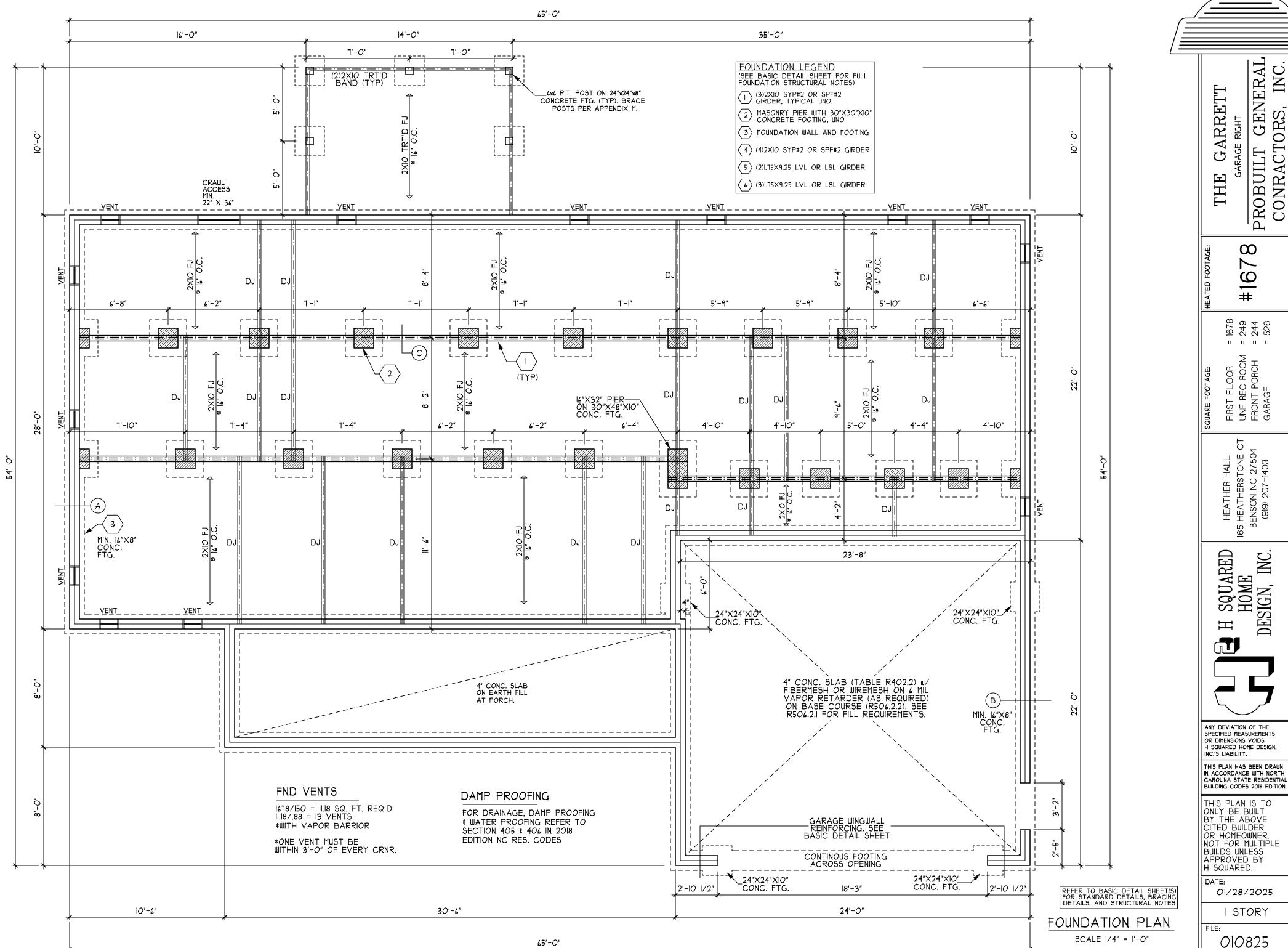
THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 2018 EDITION.

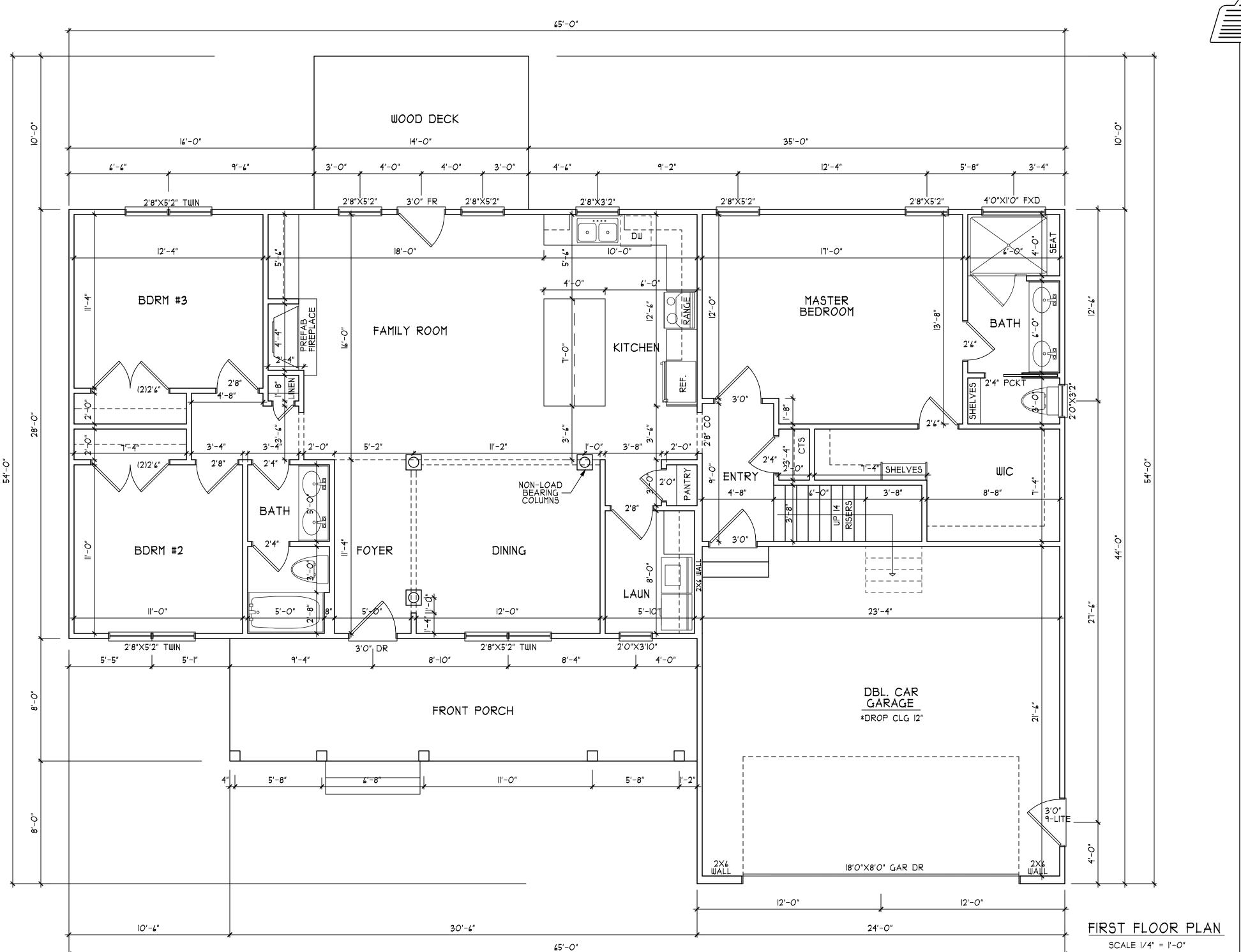
THIS PLAN IS TO ONLY BE BUILT BY THE ABOVE CITED BUILDER OR HOMEOWNER.
NOT FOR MULTIPLE
BUILDS UNLESS
APPROVED BY H SQUARED.

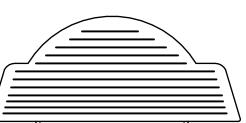
DATE: 01/28/2025

I STORY FILE:

010825







GARRETT

GENERAL PROBUILT GEN CONRACTORS, THE

 ∞ #167

= 1678 = 249 = 244 = 526 FIRST FLOOR UNF REC ROOM FRONT PORCH GARAGE

HEATHER HALL
165 HEATHERSTONE CT
BENSON NC 27504
(919) 207-1403

H SQUARED HOME DESIGN, INC. H

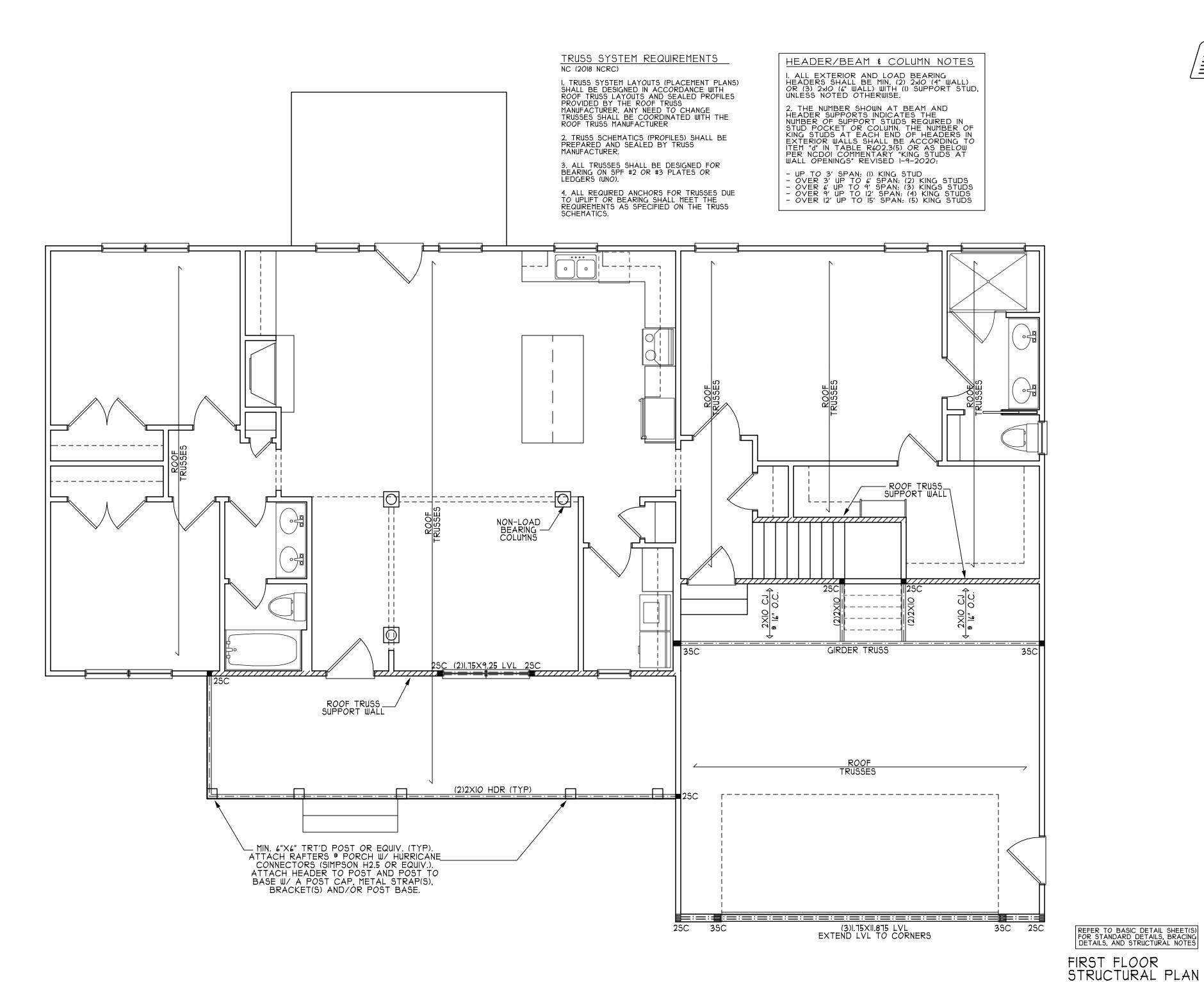
ANY DEVIATION OF THE SPECIFIED MEASUREMENTS OR DIMENSIONS VOIDS H SQUARED HOME DESIGN, INC.'S LIABILITY.

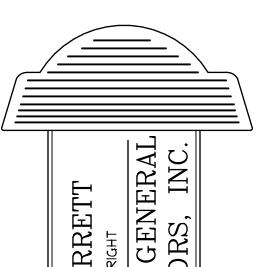
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DATE: 01/28/2025

I STORY





GARRETT

PROBUILT

CONRACTORS

 ∞ 0 ___ #

1678 249 244 526 H H H H FIRST FLOOR UNF REC ROOM FRONT PORCH GARAGE

CT HEATHER HALL
165 HEATHERSTONE CT
BENSON NC 27504
(919) 207-1403

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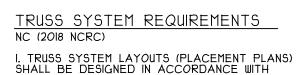
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BUILDS UNLESS
APPROVED BY H SQUARED.

DATE: 01/28/2025

I STORY

SCALE 1/4" = 1'-0"



I. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS)
SHALL BE DESIGNED IN ACCORDANCE WITH
ROOF TRUSS LAYOUTS AND SEALED PROFILES
PROVIDED BY THE ROOF TRUSS
MANUFACTURER. ANY NEED TO CHANGE
TRUSSES SHALL BE COORDINATED WITH THE
ROOF TRUSS MANUFACTURER

2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.

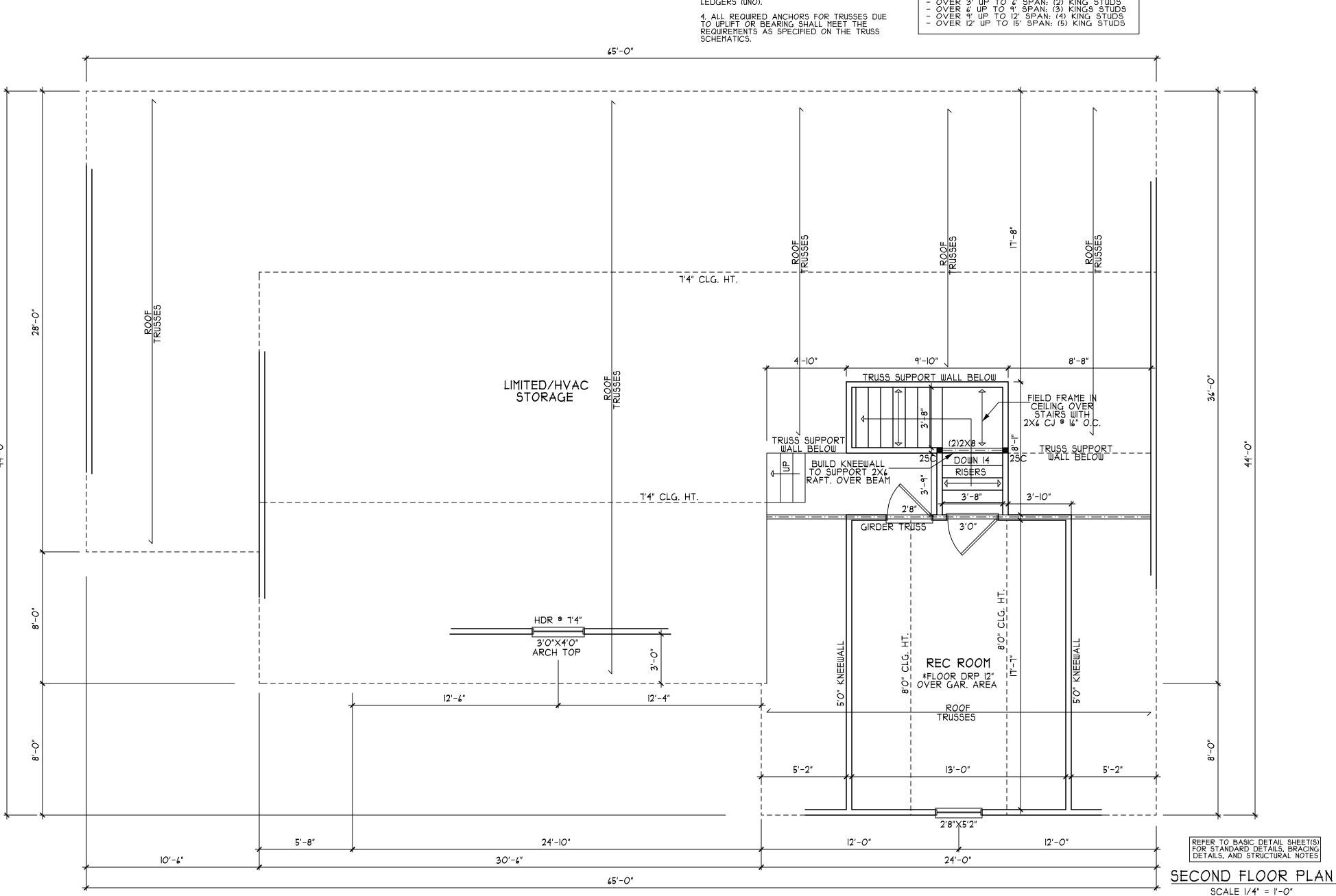
3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNO).

HEADER/BEAM & COLUMN NOTES

I. ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 2x10 (4" WALL) OR (3) 2x10 (6" WALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.

2. THE NUMBER SHOWN AT BEAM AND HEADER SUPPORTS INDICATES THE NUMBER OF SUPPORT STUDS REQUIRED IN STUD POCKET OR COLUMN. THE NUMBER OF KING STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO ITEM "d" IN TABLE R602.3(5) OR AS BELOW PER NCDOI COMMENTARY "KING STUDS AT WALL OPENINGS" REVISED 1-9-2020:

- UP TO 3' SPAN: (1) KING STUD - OVER 3' UP TO 6' SPAN: (2) KING STUDS - OVER 6' UP TO 9' SPAN: (3) KINGS STUDS - OVER 9' UP TO 12' SPAN: (4) KING STUDS - OVER 12' UP TO 15' SPAN: (5) KING STUDS



NC

GENERAL GARRETT PROBUILT GEI CONRACTORS THE

 ∞

H H H H FIRST FLOOR UNF REC ROOM FRONT PORCH GARAGE

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165 HEATHERSTONE CT
BENSON NC 27504
(919) 207-1403

SQUARED HOME INC DESIGN, H

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NOT FOR MULTIPLE
BUILDS UNLESS
APPROVED BY H SQUARED.

DATE: 01/28/2025

I STORY

I. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS)
SHALL BE DESIGNED IN ACCORDANCE WITH
ROOF TRUSS LAYOUTS AND SEALED PROFILES
PROVIDED BY THE ROOF TRUSS
MANUFACTURER. ANY NEED TO CHANGE
TRUSSES SHALL BE COORDINATED WITH THE
ROOF TRUSS MANUFACTURER

2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.

3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNO).

4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UPLIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS.

ROOF TRUSS SUPPORT WALL ROOF TRUSS SUPPORT WALK ROOF TRUSS GIRDER TRUSS <u>ROOF TRUSS SUPPORT WALL</u> HATCHED AREAS FRAMED WITH 2X6 RAFTERS 9 16" O.C. ROOF TRUSSES

REFER TO BASIC DETAIL SHEET(S) FOR STANDARD DETAILS, BRACING DETAILS, AND STRUCTURAL NOTES

ROOF PLAN

SCALE 1/4" = 1'-0"

GENERAL

GARRETT

THE

PROBUILT GEN CONRACTORS, ∞ #16

FIRST FLOOR UNF REC ROOM FRONT PORCH GARAGE

HEATHER HALL
165 HEATHERSTONE CT
BENSON NC 27504
(919) 207-1403

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IN ACCORDANCE WITH NORTH
CAROLINA STATE RESIDENTIAL
BUILDING CODES 2018 EDITION.

THIS PLAN IS TO
ONLY BE BUILT
BY THE ABOVE
CITED BUILDER
OR HOMEOWNER.
NOT FOR MULTIPLE
BUILDS UNLESS
APPROVED BY H SQUARED.

DATE: 01/28/2025

I STORY

2)	DESIGN LOADS (R301.4)	LIVE LOAD	DEAD LOAD	DEFLECTION
		(PSF)	(PSF)	(LL)
	ROOMS OTHER THAN SLEEPING RO	OMS 40	10	L/360
	SLEEPING ROOMS	30	10	L/360
	ATTIC WITH PERMANENT STAIR	40	10	L/360
	ATTIC WITH OUT PERMANENT STAIR	20	10	L/360
	ATTIC WITH OUT STORAGE	10	10	L/240
	STAIRS	40		L/360
	EXTERIOR BALCONIES	60	10	L/360
	DECKS	40	10	L/360
	GUARDRAILS AND HANDRAILS	200		
	PASSENGER VEHICLE GARAGES	50	10	L/360
	FIRE ESCAPES	40	10	L/360

WIND LOAD (BASED ON 1115/120 MPH WIND VELOCITY & EXPOSURE B)

3) WALL BRACING: BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO SECTION R602.10.3.

THE AMOUNT AND LOCATION OF BRACING SHALL COMPLY WITH TABLE R602.10.1.

THE LENGTH OF BRACED PANELS SHALL BE DETERMINED BY SECTION R602.10.4.

LATERAL BRACING SHALL BE SATISFIED PER METHOD 3 BY CONTINUOUSLY

SHEATHING WALLS WITH STRUCTURAL SHEATHING PER SECTION R602.10.3.

NOTE THAT ANY SPECIFIC BRACED WALL DETAIL SHALL BE INSTALLED AS SPECIFIED.

4) CONCRETE SHALL HAVE A MINIMUM 28 DAY STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF 5 INCHES UNLESS NOTED OTHERWISE (UNO). AIR ENTRAINED PER TABLE 402.2. ALL CONCRETE SHALL BE PROPORTIONED, MIXED, HANDLED, SAMPLED, TESTED, AND PLACED IN ACCORDANCE WITH ACI STANDARDS. ALL SAMPLES FOR PUMPING SHALL BE TAKEN FROM THE EXIT END OF THE PUMP.

5) ALLOWABLE SOIL BEARING PRESSURE ASSUMED TO BE 2000 PSF. THE CONTRACTOR MUST CONTACT A GEOTECHNICAL ENGINEER AND THE STRUCTUAL ENGINEER IF UNSATISFACTORY SUBSURFACE CONDITIONS ARE ENCOUNTERED. THE SURFACE AREA ADJACENT TO THE FOUNDATION WALL SHALL BE PROVIDED WITH ADEQUATE DRAINAGE, AND SHALL BE GRADED SO AS TO DRAINSURFACE WATER AWAY FROM FOUNDATION WALLS.

6) ALL FRAMING LUMBER SHALL BE SPF #2 (Fb = 875 PSI) UNLESS NOTED OTHERWISE (UNO). ALL TREATED LUMBER SHALL BE SYP # 2 (Fb=975 PSI). PLATE MATERIAL MAY BE SPF # 3 OR SYP #3 (Fc(perp) = 425 PSI - MIN).

1) ALL WOODEN BEAMS AND HEADERS SHALL HAVE THE FOLLOWING END SUPPORTS: (1) 2x4 STUD COLUMN FOR 6'-O" MAX. BEAM SPAN (UNO), (2) 2X4 STUDS FOR BEAM SPAN GREATER THAN 6'-O" (UNO).

8) L.V.L. SHALL BE LAMINATED VENEER LUMBER: Fb=2400 PSI, Fv=285 PSI, E=1.9xl0⁴ PSI. P.S.L. SHALL BE PARALLEL STRAND LUMBER: Fb=2900 PSI, Fv=290 PSI, E=2.0xl0⁴ PSI. L.S.L. SHALL BE LAMINATED STRAND LUMBER: Fb=2250 PSI, Fv=400 PSI, E=1.55xl0⁴ PSI. INSTALL ALL CONNECTIONS PER MANUFACTURERS INSTRUCTIONS.

9) ALL ROOF TRUSS AND I-JOIST LAYOUTS SHALL BE PREPARED IN ACCORDANCE WITH ANY SEALED STRUCTURAL DRAWINGS. TRUSSES AND I-JOISTS SHALL BE INSTALLED ACCORDING TO THE MANUFACTURE'S SPECIFICATIONS. ANY CHANGE IN TRUSS OR I-JOIST LAYOUT SHALL BE COORDINATED WITH DESIGNER OR ENGINEER.

IO) ALL STRUCTURAL STEEL SHALL BE ASTM A-36. STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" INCHES AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (I/2" DIAMETER x 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOIST ARE TOE NAILED TO THE SOLE PLATE, AND SOLE PLATE IS NAILED OR BOLTED TO THE BEAM FLANGE 9 48" O.C. . ALL STEEL TUBING SHALL BE ASTM A500.

II) REBAR SHALL BE DEFORMED STEEL, ASTM615, GRADE 60.

I2) FLITCH BEAMS SHALL BE BOLTED TOGETHER USING (2) ROWS OF I/2" DIAMETER BOLTS (ASTM A301) WITH WASHERS PLACED UNDER THE THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24" O.C. (MAX), AND STAGGERED AT THE TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH 2 BOLTS LOCATED AT 4" FROM EACH END.

13) BRICK LINTELS SHALL BE 3 1/2"x3 1/2"x1/4" STEEL ANGLE FOR UP TO 6'-O" SPAN AND 6"x4"x5/16" STEEL ANGLE WITH 6" LEG VERTICAL FOR SPANS UP TO 9'-O". SEE PLANS FOR SPANS OVER 9'-O".

14) THE POSITIVE AND NEGATIVE DESIGN PRESSURE FOR DOORS AND WINDOWS FOR A MEAN ROOF HEIGHT OF 35 FEET OR LESS SHALL BE 25 PSF.

15) THE POSITIVE AND NEGATIVE DESIGN PRESSURES REQUIRED FOR ANY ROOF OR WALL CLADDING APPLICATION NOT SPECIFICALLY ADDRESSED IN THE NORTH CAROLINA STATE RESIDENTIAL CODE - 2018 EDITION SHALL BE AS FOLLOWS:

ROOF: 45.4 PSF - 2.25:12 PITCH OR LESS 34.8 PSF - 2.25:12 TO 1:12 PITCH 21 PSF - 1:12 TO 12:12 PITCH WALLS:

24.1 PSF - WALLS SEE ALSO SECTION R103.1.3 LINTELS FOUNDATION STRUCTURAL NOTES:
NC (2018 NCRC): Wind: 115-120 MPH

 $\langle 1 \rangle$ (3) 2xIO SYP #2 OR SPF#2 GIRDER, TYPICAL UNO.

CONCRETE BLOCK PIER SIZE SHALL BE:

SIZE HOLLOW MASONRY SOLID MASONRY

8 × 16 UP TO 32" HIGH UP TO 5'-0" HIGH

12 × 16 UP TO 48" HIGH UP TO 9'-0" HIGH

16 × 16 UP TO 64" HIGH UP TO 12'-0" HIGH

24 × 24 UP TO 96" HIGH

WITH 30" × 30" × 10" CONCRETE FOOTING, UNO.

WALL FOOTING AS FOLLOWS:
DEPTH: 8" - UP TO 2-1/2 STORY

10" - 3 STORY

WIDTH: SIDING (OR EQUAL)

- 16" - UP TO 2-1/2 STORY

- 20" - 3 STORY BRICK VENEER - 16" - 1 STORY - 20" - 2 STORY - 24" - 3 STORY

FOR FOUNDATION WALL HEIGHT AND BACKFILL REQUIREMENTS, REFER TO NORTH CAROLINA RESIDENTIAL CODE TABLE R404.1.1 (I THRU 4) NOTE: ASSUMED SOIL BEARING CAPACITY = 2000 PSF. CONTRACTOR MUST VERIFY SITE CONDITIONS AND CONTACT SOILS ENGINEER IF MARGINAL OR UNSTABLE SOILS ARE ENCOUNTERED.

 $\overline{\langle 4 \rangle}$ (4) 2xIO SYP#2 OR SPF#2 GIRDER.

(5) (2) 1.75X9.25 LVL OR LSL GIRDER

 $\langle \overline{6} \rangle$ (3) 1.75X9.25 LVL OR LSL GIRDER

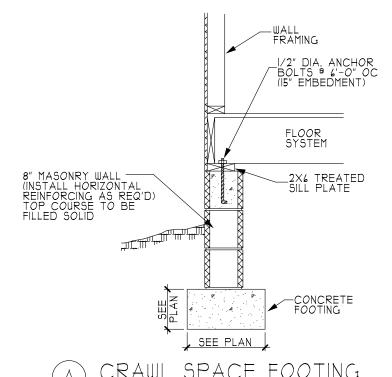
7. " DESIGNATES A SIGNIFICANT POINT LOAD TO HAVE SOLID BLOCKING TO PIER. SOLID BLOCK ALL BEAM BEARING POINTS NOTED TO HAVE THREE OR MORE STUDS TO FND, TYPICAL.

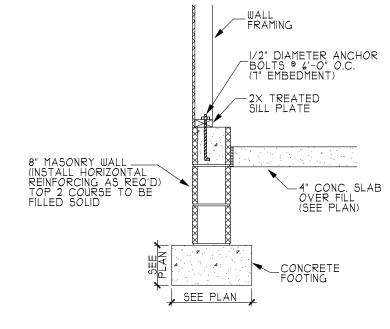
8. ABBREVIATIONS:

"SJ" = SINGLE JOIST

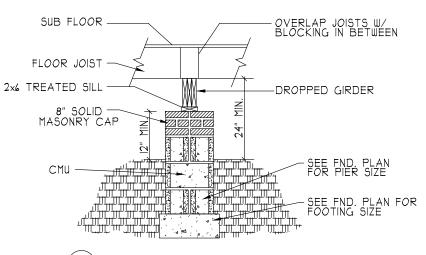
"DJ" = DOUBLE JOIST

"TJ" = TRIPLE JOIST





(B) GARAGE WALL FOOTING



(C) <u>dropped girder</u>

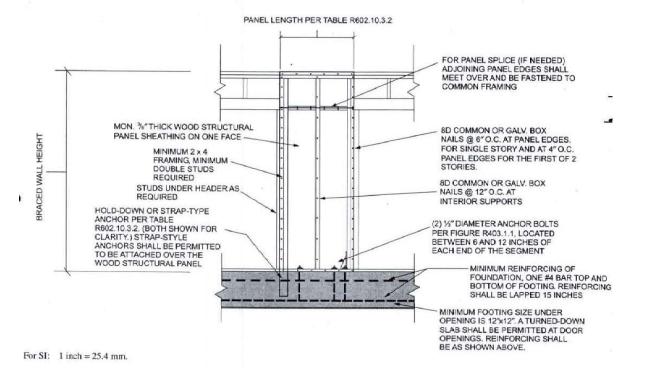


FIGURE R602.10.3.2 ALTERNATE BRACED WALL PANEL

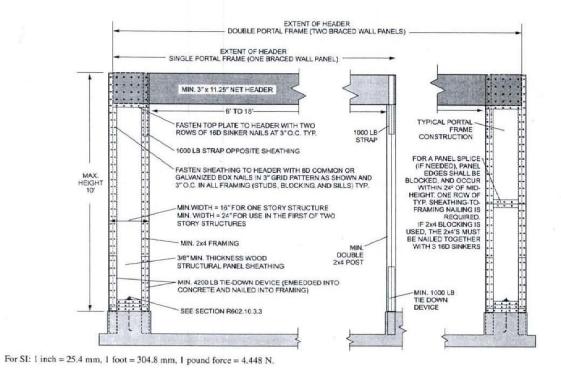


FIGURE R602.10.3.3 METHOD PFH: PORTAL FRAME WITH HOLD-DOWNS TRUSS SYSTEM REQUIREMENTS
NC (2018 NCRC):

I. TRUSS SYSTEM LAYOUTS (PLACEMENT PLANS)
SHALL BE DESIGNED IN ACCORDANCE WITH
SEALED STRUCTURAL PLANS. ANY NEED TO
CHANGE TRUSSES SHALL BE COORDINATED
WITH SOUTHERN ENGINEERS.

2. TRUSS SCHEMATICS (PROFILES) SHALL BE PREPARED AND SEALED BY TRUSS MANUFACTURER.

3. ALL TRUSSES SHALL BE DESIGNED FOR BEARING ON SPF #2 OR #3 PLATES OR LEDGERS (UNO).

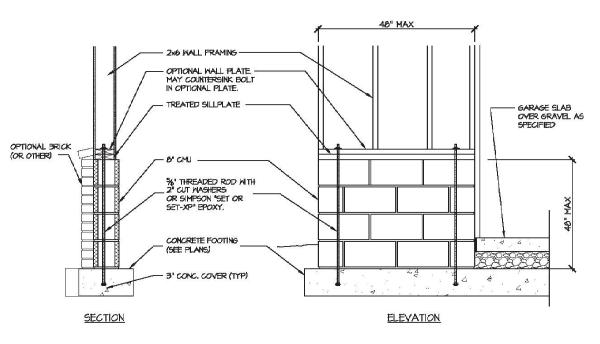
4. ALL REQUIRED ANCHORS FOR TRUSSES DUE TO UPLIFT OR BEARING SHALL MEET THE REQUIREMENTS AS SPECIFIED ON THE TRUSS SCHEMATICS.

HEADER/BEAM & COLUMN NOTES

I. ALL EXTERIOR AND LOAD BEARING HEADERS SHALL BE MIN. (2) 2×10 (4" WALL) OR (3) 2×10 (6" WALL) WITH (1) SUPPORT STUD, UNLESS NOTED OTHERWISE.

2. THE NUMBER SHOWN AT BEAM AND HEADER SUPPORTS INDICATES THE NUMBER OF SUPPORT STUDS REQUIRED IN STUD POCKET OR COLUMN. THE NUMBER OF KING STUDS AT EACH END OF HEADERS IN EXTERIOR WALLS SHALL BE ACCORDING TO TABLE R602.1.5 OR AS BELOW:

- UP TO 3' SPAN: (1) KING STUD - OVER 3' UP TO 6' SPAN: (2) KING STUDS - OVER 6' UP TO 9' SPAN: (3) KINGS STUDS - OVER 9' UP TO 12' SPAN: (4) KING STUDS - OVER 12' UP TO 15' SPAN: (5) KING STUDS



GARAGE 'WING WALL' REINFORCING PER IRC FIGURE R602.10.43

BASIC BUILDING
DETAIL SHEET
(115/120 MPH)

PLEASE NOTE THAT NOT ALL DETAILS APPLY TO EVERY PLAN.

HEATHER HALL
165 HEATHERSTONE C
BENSON NC 27504
(919) 207-1403

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THIS PLAN HAS BEEN DRAWN IN ACCORDANCE WITH NORTH CAROLINA STATE RESIDENTIAL BUILDING CODES 2018 EDITION.

DATE:



Client: **PROBUILT**

Project: Address:

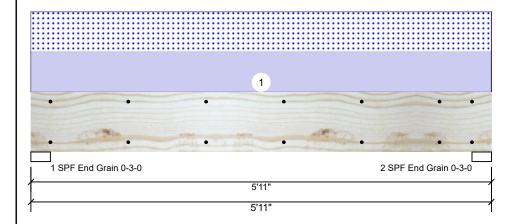
1/26/2025 Input by: LENNY NORRIS Job Name: GARRETT

Project #:

Date:

Kerto-S LVL 2-Ply - PASSED 1.750" X 9.250" **2852 TWIN**

Level: Level



Application:

Design Method:

Building Code:

Load Sharing:

Deck:

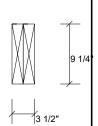
Floor

ASD

No

IRC 2018

Not Checked



Page 1 of 2

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal -

Ш Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1645	1624	0	0
2	Vertical	0	1645	1624	0	0

Analysis Results

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4243 ft-lb	2'11 1/2"	14423 ft-lb	0.294 (29%)	D+S	L
Unbraced	4243 ft-lb	2'11 1/2"	11027 ft-lb	0.385 (38%)	D+S	L
Shear	2147 lb	4'10 3/4"	7943 lb	0.270 (27%)	D+S	L
LL Defl inch	0.033 (L/2032)	2'11 1/2"	0.139 (L/480)	0.236 (24%)	S	L
TL Defl inch	0.066 (L/1009)	2'11 1/2"	0.185 (L/360)	0.357 (36%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

l	Bearings	5						
ſ	Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb.
ı	1 - SPF End Grain	3.000"	Vert	37%	1645 / 1624	3270	L	D+S
ı	2 - SPF Fnd	3.000"	Vert	37%	1645 / 1624	3270	L	D+S

ID Location Trib Width Load Type Side Dead 0.9 Live 1 Snow 1.15 Wind 1.6 Const. 1.25 Comments 0 PLF 1 Uniform Тор 549 PLF 549 PLF 0 PLF 0 PLF A1 TRUSS 7 PLF Self Weight

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown, It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

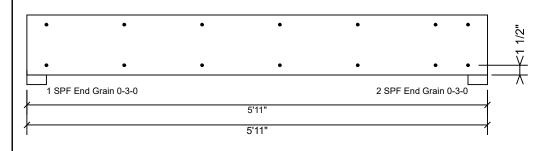
- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
 - LVL beams must not be out or drilled
 Refer to manufacturer's product information
 regarding installation requirements, multi-ply
 fastening details, beam strength values, and code
 approvals
 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

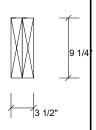
Grain

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

This design is valid until 5/29/2026

PROBUILT Client: Date: 1/26/2025 Project: Input by: LENNY NORRIS isDesign Address: Job Name: GARRETT Project #: **Kerto-S LVL** 1.750" X 9.250" 2-Ply - PASSED Level: Level **2852 TWIN**





Page 2 of 2

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. IVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

This design is valid until 5/29/2026

(800) 622-5850 www.metsawood.com/us

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



Client: **PROBUILT**

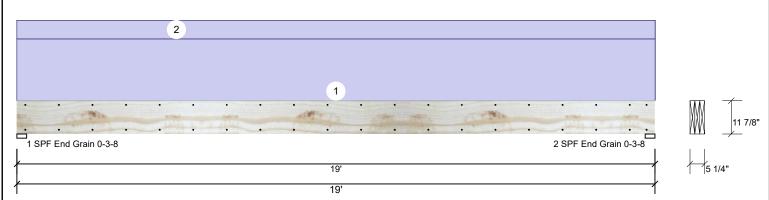
Project: Address: Date: 1/26/2025

Input by: LENNY NORRIS Job Name: GARRETT

Page 1 of 2

Project #:

1.750" X 11.875" Kerto-S LVL GDH 18' FL 3-Ply - PASSED Level: Level



Member Inf	ormation						Reac	tions	UNPA	ATTERN	IED II	b (Uplift)			
Туре:	Girder		Application	n: Fl	oor		Brg	Direct	ion	Live		Dead	Snow	Wind	Const
Plies:	3		Design M	ethod: A	SD		1	Vertica	al	0		2602	0	0	0
Moisture Condi	ition: Dry		Building (Code: IF	RC 2018		2	Vertica	al	0		2602	0	0	0
Deflection LL:	480		Load Sha	ring: Ye	es										
Deflection TL:	360		Deck:	N	ot Checked										
Importance:	Normal - II														
Temperature:	Temp <= 10	0°F													
							Bear	ings							
							Bea	ring L	ength	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - 8	SPF 3	.500"	Vert	17%	2602 / 0	2602	Uniform	D
							End								
Analysis Res	ults						Grai								
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case	2-5		.500"	Vert	17%	2602 / 0	2602	Uniform	D
Moment	11769 ft-lb	9'6"	27954 ft-lb	0.421 (42%) D	Uniform	End Grai								
Unbraced	11769 ft-lb	9'6"	11788 ft-lb	0.998 (100%)	D	Uniform									
Shear	2264 lb	1'3 3/8"	11970 lb	0.189 (19%) D	Uniform									
LL Defl inch	0.000 (L/999)	0	999.000 (L/0)	0.000 (0%)											

Uniform

Design Notes

TL Defl inch 0.519 (L/429)

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

9'6 1/16" 0.618 (L/360) 0.839 (84%) D

- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". Nail from both sides.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 12'2 7/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width

o Eateral sichaemess ratio based on single pry width.											
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	200 PLF	0 PLF	0 PLF	0 PLF	0 PLF	GABLE END	
2	Uniform			Тор	60 PLF	0 PLF	0 PLF	0 PLF	0 PLF	DEAD WALL	
	Self Weight				14 PLF						

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used.

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

This design is valid until 5/29/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us



PROBUILT Client:

Project: Address: Date: 1/26/2025

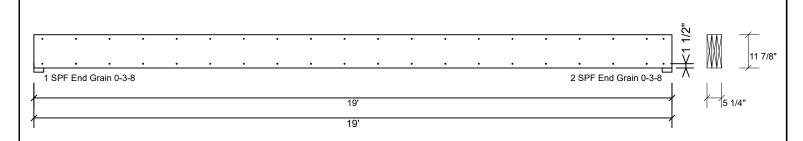
LENNY NORRIS Job Name: GARRETT

Page 2 of 2

Project #:

Input by:

Kerto-S LVL 1.750" X 11.875" **GDH 18' FL** 3-Ply - PASSED Level: Level



Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
См	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding

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