PLANS DESIGNED TO THE 2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE

MEAN ROOF HEIGHT: 19-9	HEIGHT TO RIDGE: 27 -5						
CLIMATE ZONE	ZONE 3A	ZONE 4A	ZONE 5A				
FENESTRATION U-FACTOR	0.35	0.35	0.35				
SKYLIGHT U-FACTOR	0.55	0.55	0.55				
GLAZED FENESTRATION SHGC	0.30	0.30	0.30				
CEILING R-VALUE	38 or 30ci	38 or 30ci	38 or 30ci				
WALL R-VALUE	15	15	19				
FLOOR R-VALUE	19	19	30				
* BASEMENT WALL R-VALUE	5/13	10/15	10/15				
** SLAB R-VALUE	0	10	10				
* CDANN CDACE WALL D VALUE	E/12	10/15	10/10				

"10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION

** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SLAB 24" OR TO BOTTOM OF FOUNDATION WALL

DESIGNED FOR WIN								
COMPONENT								
MEAN ROOF								
ZONE 1								-16.8
ZONE 2	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 3	14.2	-18.0						-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
								and the state of

DESIGNED FOR WIN	ID SPEED	OF 130 MR	?Н, 3 SECO	OND GUST	(101 FAS	TEST MILE	E) EXPOSU	IRE "B"
COMPONENT								
MEAN ROOF								
								-20.2
								-23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE E	10 2	-24.0	10 1	-25.2	10.0	-26.2	20.4	-26.0

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertically to the floor or *grade* below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a quard

Harnett

R312.2 Height. Required *guards* at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914 mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads.

Exceptions:

Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the

2. Where the top of the guard also serves as a natural of the open stoces stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter.

 The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153

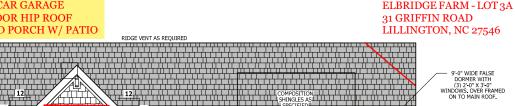
2. Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.

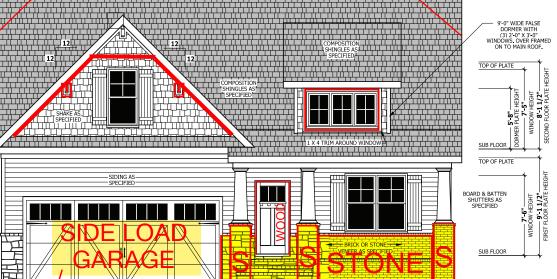
ROOF VENTILATION

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT. NET EREE CROSS VENTUATION NEEDED:

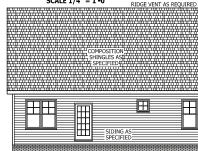
WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ.FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CEILING = 8.26 SQ.FT.

3 CAR GARAGE TUDOR HIP ROOF COVERED PORCH W/ PATIO





FRONT ELEVATION SCALE 1/4" = 1'-0"



AIR LEAKAGE

infiltration. The sealing methods between dissimilar materials shall allow for differential expansion and contraction. For all homes, stripped or otherwise sealed with an air barrier material or solid material consistent with Appendix E-2.4 of this code: 1. Blocking and sealing floor/ceiling systems and under knee walls



RIGHT SIDE ELEVATION

SCALE 1/8" = 1'-0"

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PROCEDURES,
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VARY WITH LOCATION, A LOCAL
DESIGNER, ARCHITECT OR
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BEFORE CONSTRUCTION,
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INSTRUMENTS OF SERVICE AND
AS SUCH SHALL REMAIN
PROPERTY OF THE DESIGNER.

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ELEVATION

SQUARE FOOTAGE HEATED FIRST FLOOR 1766 SQ.FT. HEATED OPTIONAL 148 SQ FT 304 SQ FT 452 SQ FT UNHEATED FRONT PORCH GARAGE 188 90 FT 488 90 FT TOTAL 676 SQ FT.

UNHEATED OPTIONAL

SCREENED FORCH 166 SQ FT.

DECK / PATIO 108 SQ FT.

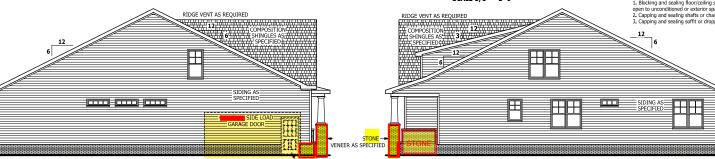
THIRD GARAGE 292 SQ FT.

TOTAL 560 SQ FT.

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REAR ELEVATION

SCALE 1/8" = 1'-0"



XXXXX

WINDOWS WITH SIDE LOAD

LEFT SIDE ELEVATION

SCALE 1/8" = 1'-0'

SQUARE FOOTAGE HEATED FIRST FLOOR PLAYROOM

1766 SQ FT 400 SQ FT 2166 SQ FT TOTAL
HEATED OPTIONAL
CAROLINA ROOM 148 SQ FT.
RECREATION ROOM 304 SQ FT.

452 SQ FT UNHEATED 188 SQ.FT. 488 SQ.FT. FRONT PORCH

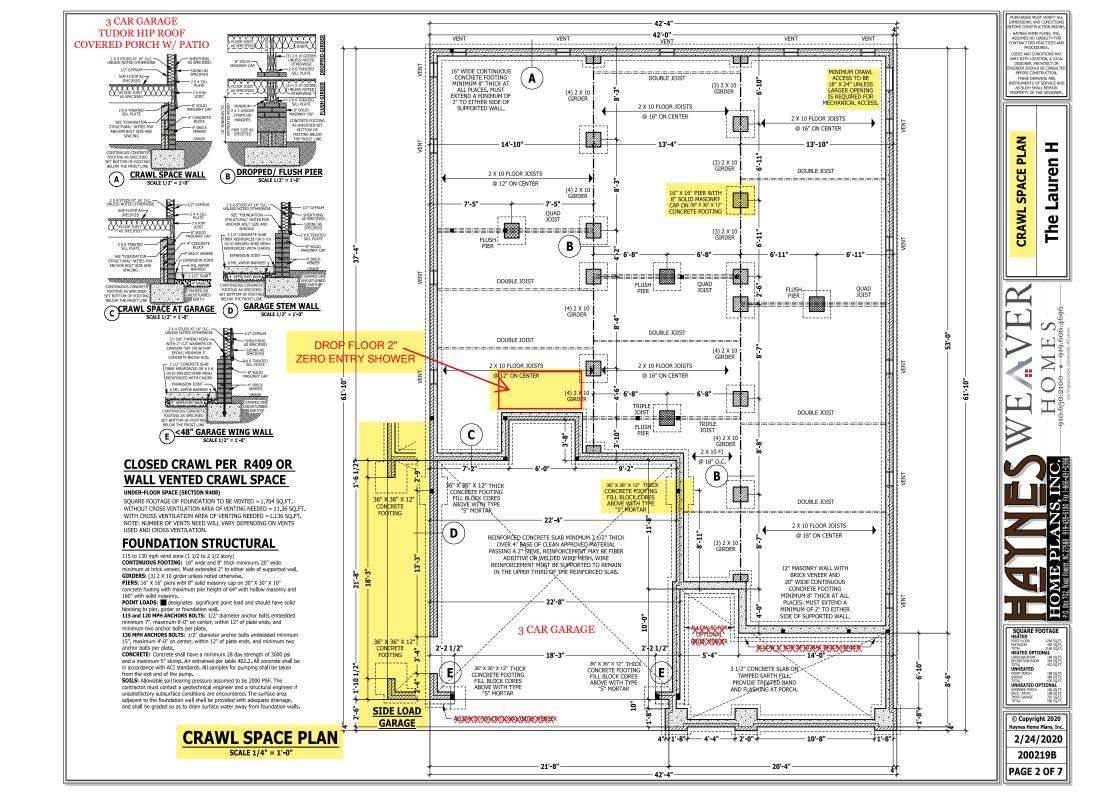
GARAGE UNHEATED OPTIONAL SCREENED PORCH DECK / PATIO 160 SQ FT 108 SQ FT THIRD GARAGE

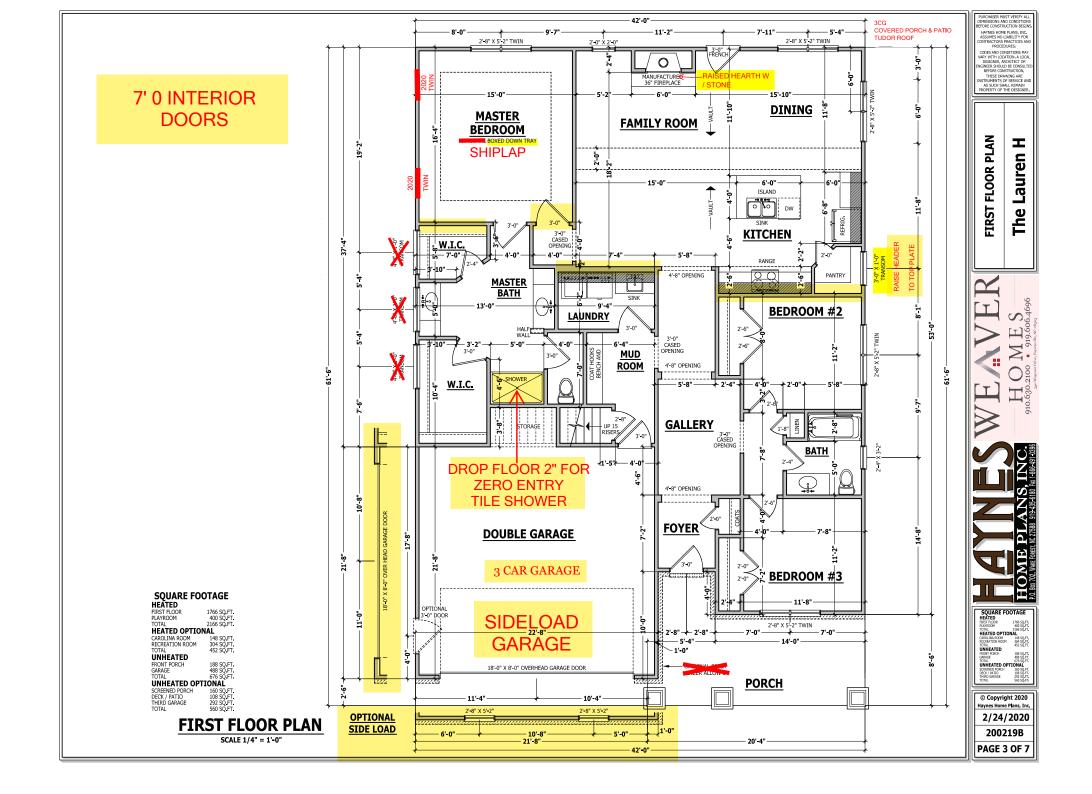
Section N1102.4 N1102.4.1 Building thermal envelope. The building thermal envelope shall be durably sealed with an air barrier system to limit where present, the following shall be caulked, gasketed, weather

open to unconditioned or exterior space.

2. Capping and sealing shafts or chases, including flue shafts. Capping and sealing soffit or dropped ceiling areas.

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STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all the 2018 North Caronna residential Building Code, prus an local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans,

Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		-
Guardrail in-fil components	50		-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Chalma	40		1.7200

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless

ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI Parallel strain Limber (PSL) = PD=200 PSI, PN=290 PSI, E=1,DSXL06 PSI Laminated strain lumber (LSL) Pb=2250 PSI, Fy=400 PSI, E=1,55xL06 PSI Instal all connections per manufactures instructions. TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist

layouts shall be prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2'-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing.

ROOF SHEATHING: OSB or CDX roof sheathing minimum

3/8" thick for 16" on center rafters and 7/16" for 24" on

center rafters.

CONCRETE AND SOILS: See foundation notes.

EXTERIOR HEADERS

(2) 2 X 10

2 JACKS EACH END

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE

- KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16'
KING STUDYS) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE
- LADDER FRAMED

BRACE WALL PANEL NOTES

EXTERIOR WALLS: All exterior walls to be sheathed with CS-WSP or CS-SFB in accordance with section R602.10.3 unless noted otherwise.

GYPSUM: All interior sides of exterior walls and both sides interior walls to have 1/2" gypsum installed. When not using method GB gypsum to be fastened per table R702.3.5. Method GB to be fastened per table R602.10.1.

REQUIRED LENGTH OF BRACING: Required brace wall length for each side of the circumscribed rectangle are interpolated per table R602.10.3. Methods CS-WSP and CS-SFB contribute their actual length. Method GB contributes 0.5 it's actual length. Method PF contributes 1.5 times its actual length.

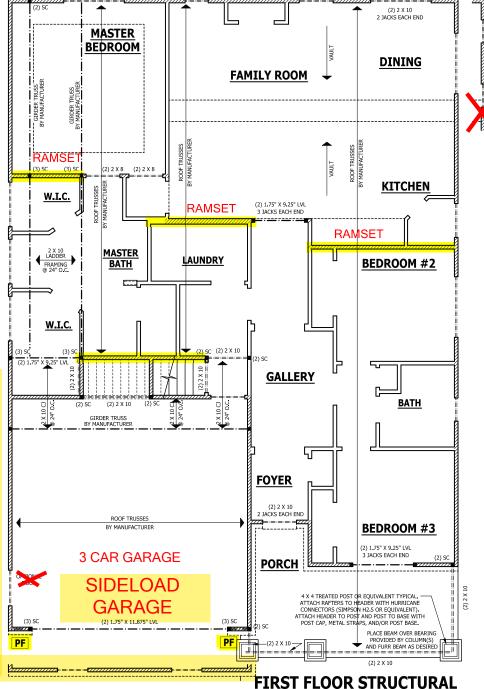
HD: 800 lbs hold down hold down device fastened to the edge

of the brace wall panel closets to the corner.

Methods Per Table R602.10.1 CS-WSP: Shall be minimum 3/8" OSB or CDX nailed at 6" on center at edges and 12" on center at intermediate supports with 6d common nails or 8d(2 1/2" long x 0.113" diameter). CS-SFB: Shall be minimum 1/2" structural fiber board nailed at 3" on center at edges and 3" on center at intermediate supports with 1 1/2" long x 0.12" diameter galvanized roofing

GB: Interior walls show as GB are to have minimum 1/2" gypsum board on both sides of the wall fastened at 7" on center at edges and 7" on center at intermediate supports with minimum 5d cooler nails or #6 screws. PF: Portal fame per figure R602.10.1

PF



COVERED PORCH & PATIO

TUDOR ROOF

HAYNES HOME PLANS, INC.

PROCEDURES.

THESE DRAWING ARE INSTRUMENTS OF SERVICE AN AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

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SQUARE FOOTAGE HEATED

FRONT PORCH 188 SQ.F GARAGE 488 SQ.F TOTAL 676 SQ.F UNHEATED OPTIONAL

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HEATED OPTIONAL

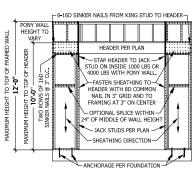
UNHEATED

1766 SQ FT 400 SQ FT

148 50 FT 304 50 FT 452 50 FT

160 SQ FT. 108 SQ FT. 292 SQ FT.

FIRST FLOOR STRUCTURAL



PF PORTAL FRAME AT OPENING

(METHOD PF PER FIGURE AND SECTION R602.10.1) SCALE 1/4" = 1'-0"

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Havnes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer falls to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. **BEARING.** All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

OPTIONAL SIDE LOAD

SCALE 1/4" = 1'-0"

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Havnes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics.

BEARING. All trusses shall be designed for bearing on SPF #2 plates or

ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus al local codes and regulations. This document in no way shall be construed to supersede the code.

JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no

liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	_
Guardrail in-fill components	50	-	_
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360
Cnow	20		

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 ENGINEERED WOOD BEAMS:

Laminated veneer lumber (LVL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x106 PSI
Parallel strand lumber (PSL) = Fb=2900 PSI, Fv=290 PSI, E=2.0x106 PSI
Laminated strand lumber (LSL) Fb=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Install all connections per manufacturers instructions.

TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist layouts shall be

prepared in accordance with this document. Trusses and I-joists shall be installed according to the manufacture's specifications. Any change in truss or I-loist layout shall be coordinated with Haynes Homes Plans. Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9-0" unless noted otherwise. 3 1/2" x 1/4" steel angle with 1/2" bots at 2-0" on center for spans up to 18-0" unless noted otherwise.

FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing, and minimum 3/4" thick for 24" on center joist spacing, ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters.

CONCRETE AND SOILS: See foundation notes.

ATTIC ACCESS

SECTION R807

R807.1 Attic access. An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net clear opening shall not be less than 20 inches by 30 inches (508 mm by 762 mm) and shall be located in a hallway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

Exceptions:

 Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.

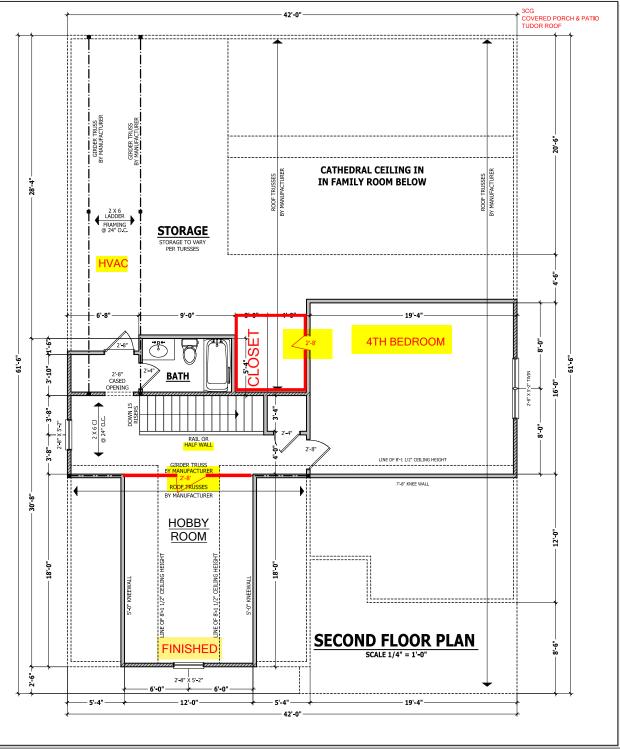
Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE
- KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16' KING STUD(S) 1 2 3 5 6

INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END LINI ESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE



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SECOND FLOOR PLAN The

SQUARE FOOTAGE HEATED 1766 SQ FT 400 SQ FT HEATED OPTIONAL 148 50 FT 304 50 FT 452 SQ FT UNHEATED 160 SQ FT. 108 SQ FT. 292 SQ FT.

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TUDOR HIP ROOF **COVERED PORCH** W/ PATIO 3 CAR GARAGE

BY MANUFACTURER GIRDER TRUSS CATHEDRAL CEILING IN FAMILY ROOM, KITCHEN, AND DINING [6:12] ROOF SUPPORT TO WALL BELOW 2 X 6 LADDER FRAMING @ 24" O.C. ROOF SUPPORT TO WALL BELOW ROOF SUPPORT TO WALL BELOW ROOF SUPPORT TO WALL BELOW 2 X 6 R 2 X 6 R @ 24" O.C. @ 24" O.C. GIRDER TRUSS BY MANUFACTURER OVER FRAME FALSE DORMER ON TO MAIN ROOF WITH 2 X 8 RAFTERS AT 24" ON CENTER FRAME ON TO 2 X 10 FLAT PLATE 12:12 12:12 FRAME 60" HIGH WALL ROOF TRUSSES BY MANUFACTURER 4:12 - 1'-0" OVERHANG TYPICAL

ROOF PLAN
SCALE 1/4" = 1'-0"

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins.

KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and celling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or in lisuation to in oil will be some the first his his time to the some oil to displace the some reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.

HEEL HEIGHT ABOVE FIRST FLOOR PLATE

HEEL HEIGHT ABOVE SECOND FLOOR PLATE

PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS HAYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES.

PROCEDURES.
CODES AND CONDITIONS MAY
VARY WITH LOCATION. A LOCAL
DESIGNES, ARCHITECT OR
ENGINEER SHOULD BE CONSULTED
BEFORE CONSTRUCTION.
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INSTRUMENTS OF SERVICE AND
AS SUCH SHALL REMAIN
PROPERTY OF THE DESIGNER.

I **ROOF PLAN**

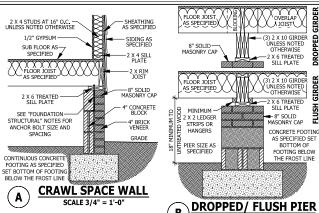
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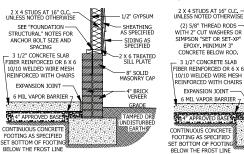
SQUARE FOOTAGE
HEATED
FIRST FLOOR 1766 SQ FT.
PLAYROOM 400 SQ FT. 1766 SQ FT 400 SQ FT 2166 SQ FT HEATED OPTIONAL 148 SQ FT. 304 SQ FT. 452 SQ FT. TOTAL UNHEATED FRONT PORCH 160 SQ FT. 108 SQ FT. 292 SQ FT. 560 SQ FT.

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DECK STAIR NOTES

SECTION AM110

GARAGE STEM WALL

SCALE 3/4" = 1'-0"

AM110.1 Stairs shall be constructed per Figure AM110. Stringer spans shall be no greater than 7 foot span between supports. Spacing between stringers shall be based upon decking material used per AM107.1. Each Stringer shall have minimum 3 1/2 inches between step cut and back of stringer If used, suspended headers shall shall be attached with 3/8 inch galvanized bolts with nuts and washers to securely support stringers at the top.

DECK BRACING

SECTION AM109

AM109.1 Deck bracing. Decks shall be braced to provide lateral stability. The following are acceptable means to

provide lateral stability.

AM109.1.1, When the deck floor height is less than 4'-0" above finished grade per Figure AM109 and the deck is attached to the structure in accordance with Section AM104, lateral bracing is not required.

AM109.1.2. 4 x 4 wood knee braces may be provided on each column in both directions. The knee braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be angled between 45 degrees and 60 degrees from the horizontal. Knee braces shall be botted to the post and the girder/double band with one 5/8 inch hot dipped galvanized bolt with nut and washer at both ends of the

AM109.1.3. For freestanding decks without knee braces or AS SPECIFIED diagonal bracing, lateral stability may be provided by embedding the post in accordance with Figure AM109.2

POST SIZE	MÁX TRIBUTARY ÁREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	DIAMETER	
4 X 4	48 SF	4'-0"	2'-6"	1'-0"	
6 X 6	120 SF	6'-0"	3'-6"	1'-8"	

AM109.1.4. 2 x 6 diagonal vertical cross bracing may be provided in two perpendicular directions for freestanding decks or parallel to the structure at the exterior column line for attached decks. The 2 x 6's shall be attached to the posts with one 5/8 inch hot dipped galvanized bolt with nut and washer at each end of each bracing member per Figure AM109 3

AM109.1.5. For embedment of piles in Coastal Regions see Chapter 45.

WEEP SCREED SCALE 3/4" = 1'-0"

SHEATHING AS SPECIFIED

SEE FOUNDATION

FOR FOLINDATION

SILL PLATE (3) 2 X 10 GIRDER UNLESS NOTED OTHERWISE 2 X 6 TREATED SILL PLATE 8" SOLID MASONRY CAR CONCRETE FOOTING AS SPECIFIED SET BOTTOM OF FOOTING BELOW THE FROST LINE DROPPED/ FLUSH PIER В SCALE 3/4" = 1'-0" 2 X 4 STUDS AT 16" O.C. — UNLESS NOTED OTHERWISE 1/2" GYPSUM (2) 5/8" THREAD RODS - SHEATHING AS SPECIFIED WITH 2" CUT WASHERS OR SIMPSON "SET OR SET-XP" SIDING AS SPECIFIED CONCRETE BELOW ROD. 2 Y 6 TREATER

<48" GARAGE WING WALL SCALE 3/4" = 1'-0"

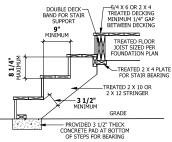


FIGURE AM110 TYPICAL DECK STAIR DETAIL SCALE 3/4" = 1'-0'

-STONE VEENER

AS SPECIFIED

VAPOR BARRIER

-WEEP SCREED

MINIMUM 4" TO

GROUND OR 2"

-TO PAVEMENT

GRADE

WEEP SCREEDS

— 8" SOLID MASONRY CAP

TAMPED OR

S EARTHS

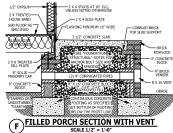
All weep screeds and stone veneer to be installed per manufactures instructions and per the 2012 North Carolina Residential Building code

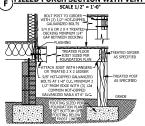
R703.6.2.1 - A minimum 0.019-inch (0.5 mm) (No. 26 galvanized sheet gage), corrosion-resistant weep screed or plastic ween screed with a minimum vertical attachment flange of 31/2 inches (89 mm) shall be provided at or below the foundation plate line on exterior stud walls in accordance with ASTM C 926. The weep screed shall be placed a minimum of 4 inches (102 mm) above the earth or 2 inches (51 mm) above payed areas and shall be of a type that will allow trapped water to drain to the exterior of the building. The weather-resistant barrier shall lap the attachment flange. The exterior lath

attachment flange of the weep screed.

2 X 4 STUDS AT 16" O.C. -UNLESS NOTED OTHERWISE 1/2" GYPSUM SEE POOF PLAN OR SUB FLOOR AS-ELEVATION SPECIFIED OR PITCH AS SPECIFIED ROOF TRUSSES BY 4" CONCRETE BLOCK 2 X 6 TREATED PORCH HEADER PER --4" BRICK VENEER SEE "FOUNDATION-PLAN INSTALLED OVER EXPANSION JOINT STRUCTURAL" NOTES FOR CENTER OF COLUMN BASE ANCHOR BOLT SIZE AND —6 MIL VAPOR BARRIER BLOCKING INSTALLED-SPACING ON BOTH SIDES & UNDER 3 1/2" SLAB HEADER AS DESIRED CONTINUOUS CONCRETE 1 Χ ΜΔΤΕΡΙΔΙ TAMPED OR FOOTING AS SPECIFIED CENTER LINE OF HEADER SET BOTTOM OF FOOTI JNDISTURBED AND COLUMN ₩ EARTH BELOW THE FROST LINE **PORCH HEADER WITH** CRAWL SPACE AT GARAGE **TAPERED COLUMN**

C SCALE 3/4" = 1'-0"





F DECK ATTACHMENT

SMOKE ALARMS

SECTION R314

R314.1 Smoke detection and notification. All smoke alarms shall be listed in accordance with UL 217 and installed in accordance with the provisions of this code and the household fire warning equipment provisions of NFPA 72.

R314.2 Smoke detection systems. Household fire alarm systems installed in accordance with NEPA 72 that include smoke alarms, or a combination of smoke detector and audible notification device installed as required by this section for smoke alarms, shall be permitted. The household fire alarm system shall provide the same level of smoke detection and alarm as required by this section for smoke alarms. Where a household fire warning system is installed using a combination of smoke detector and audible notification device(s), it shall become a permanent fixture of the occupancy and owned by the homeowner. The system shall be monitored by an approved supervising station and be maintained in accordance with NEPA 72

Exception: Where smoke alarms are provided meeting the ements of Section R314.4.

R314.3 Location. Smoke alarms shall be installed in the following

 In each sleeping room. 2. Outside each separate sleeping area in the immediate vicinity of

the bedrooms.

3. On each additional story of the dwelling, including basements and habitable attics (finished) but not including crawl spaces. uninhabitable (unfinished) attics and uninhabitable (unfinished) attic-stories. In dwellings or dwelling units with split levels and without an intervening door between the adjacent levels, a smoke alarm installed on the upper level shall suffice for the adjacent lower level provided that the lower level is less than one full *story* below the upper level.

When more than one smoke alarm is required to be installed within an individual dwelling unit the alarm devices shall be interconnected in such a manner that the actuation of one alarm will activate all of the alarms in the individual unit

R314.4 Power source. Smoke alarms shall receive their primary power from the building wiring when such wiring is served from a commercial source, and when primary power is interrupted, shall receive power from a battery. Wiring shall be permanent and without a disconnecting switch other than those required for overcurrent protection. Smoke alarms shall be interconnected.

CARBON MONOXIDE ALARMS

EDGED OR PORCH FLOOR

- SHINGLES AS SPECIFIED

SHEATHING AS SPECIFIED

- 15# BUILDING FELT

2 X 6 SUB FASCIA

VINYL OR HARDIE SOFFIT

INSTALLED PER MANUFACTURERS

INSTRUCTIONS

TARERED COLLIMN OVER

MASONRY BASE ATTACHED TO HEADER

WITH POST CAP

SECTION R315

R315.1 Carbon monoxide alarms. In new construction, dwelling units shall be provided with an approved carbon monoxide alarm installed outside of each separate sleeping area in the immediate vicinity of the bedroom(s) as directed by the alarm manufacturer.

R315.2 Where required in existing dwellings. In existing dwellings, where interior alterations, repairs, fuel-fired appliance replacements, or additions requiring a permit occurs, or where one or more sleeping rooms are added or reated, carbon monoxide alarms shall be provided in accordance with Section

R315.3 Alarm requirements. The required carbon monoxide alarms shall be audible in all bedrooms over background noise levels with all intervening doors closed. Single station carbon monoxide alarms shall be listed as complying with UL 2034 and shall be installed in accordance with this code and the manufacturer's installation instructions.

STAIRWAY NOTES

shall not be less than 6 feet 8 inches (2032 mm) measured vertically from the sloped line adjoining the tread nosing or from the floor surface of the landing or platform on that portion of the stainway.

R311.7.4 Stair treads and risers. Stair treads and risers shall meet the requirements of this section. For the purposes of this section all dimensions and dimensioned surfaces shall be exclusive of carpets, rugs or runners. R311.7.4.1 Riser height. The maximum riser height shall be 8 1/4 inches (210 mm). The riser shall be measured vertically between leading edges of

the adjacent treads. R311,7,4,2 Tread depth. The minimum tread depth shall be 9 inches (229 mm). The tread depth shall be measured horizontally between the vertical planes of the foremost projection of adjacent treads and at a right angle to the tread's leading edge. Winder treads shall have a minimum tread depth of 9 inches (229 mm) measured as above at a point 12 inches (305 mm) from the side where the treads are narrower. Winder treads shall have a

minimum tread depth of 4 inches (102 mm) at any point.

R311.7.4.3 Profile. The radius of curvature at the nosing shall be no greater than 9/16 inch (14 mm). A nosing not less than 3/4 inch (19 mm) but not more than 1 1/4 inches (32 mm) shall be provided on stairways with solid

R311.7.7 Handrails. Handrails shall be provided on at least one side of each continuous run of treads or flight with four or more risers.

R311.7.7.1 Height. Handrail height, measured vertically from the sloped

plane adjoining the tread nosing, or finish surface of ramp slope, shall be not less than 34 inches (864 mm)and not more than 38 inches (965 mm). Exceptions:

The use of a volute, turnout or starting easing shall be allowed over the

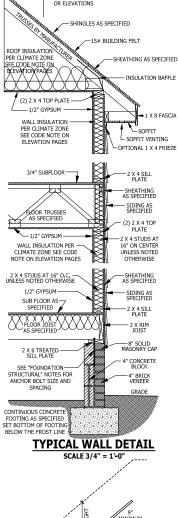
 When handrail fittings or bendings are used to provide continuous. transition between flights, the transition from handrail to guardrail, or used at the start of a flight, the handrail height at the fittings or bendings shall be permitted to exceed the maximum height.

R311.7.7.2 Continuity. Handrails for stairways shall be continuous for the

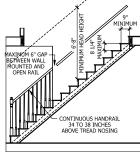
full length of the flight, from a point directly above the top riser of the flight to a point directly above the lowest riser of the flight. Handrail ends shall be returned or shall terminate in newel posts or safety terminals. Handrails adjacent to a wall shall have a space of not less than 11/2 inch (38 mm). between the wall and the handrails

 Handralls shall be permitted to be interrupted by a newel post. 2. The use of a volute, turnout, starting easing or starting newel shall be

allowed over the lowest tread. 3. Two or more senarate rails shall be considered continuous if the termination of the rails occurs within 6 inches (152 mm) of each other. If transitioning between a wall-mounted handrail and a guardrail/handrail, the wall-mounted rail must return into the wall.



TTCH PER ROOF PLAN



TYPICAL STAIR DETAIL

160 SQ FT 108 SQ FT 292 SQ FT © Copyright 2020 Haynes Home Plans, Inc 2/24/2020

SQUARE FOOTAGE

HEATED OPTIONAL

UNHEATED OPTIONAL

UNHEATED

1766 SQ FT 400 SQ FT

148 50 FT 304 50 FT 452 SQ FT

HAYNES HOME PLANS, INC.

PROCEDURES.

CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNER, ARCHITECT OR NGINER SHOULD BE CONSULTE BEFORE CONSTRUCTION.

THESE DRAWING ARE STRUMENTS OF SERVICE AN

AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

DETAILS

LYPICAL

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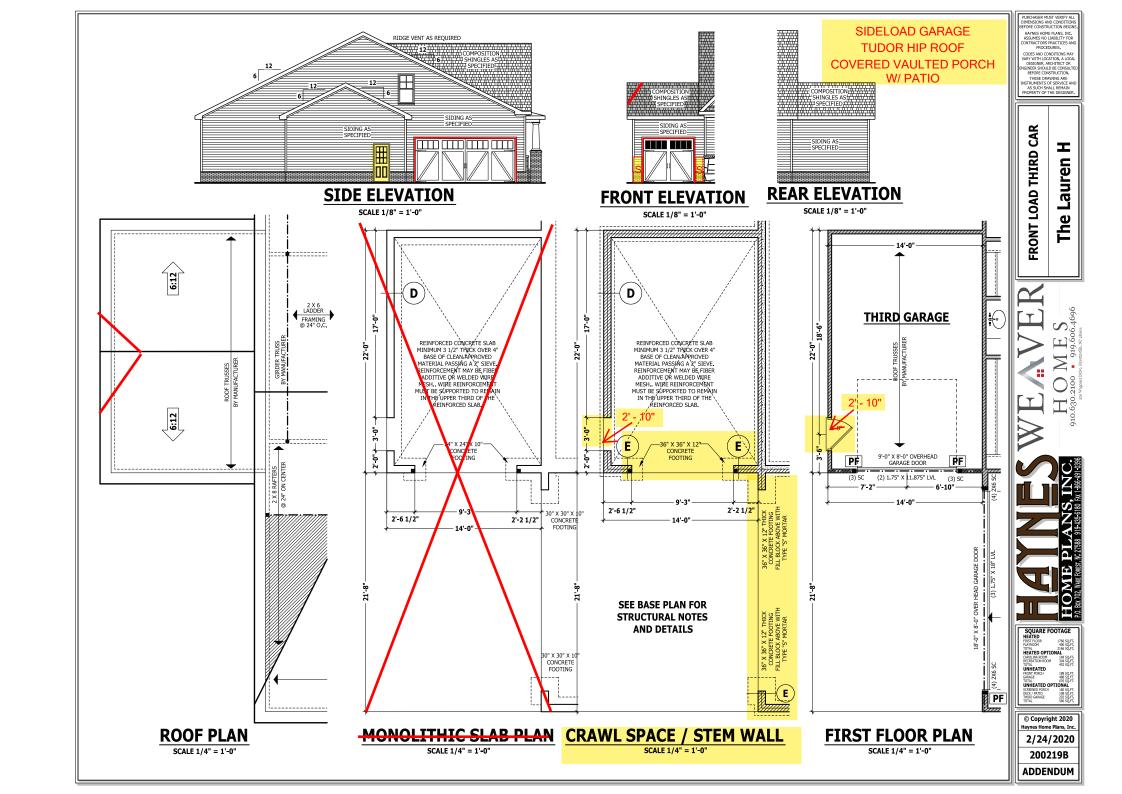
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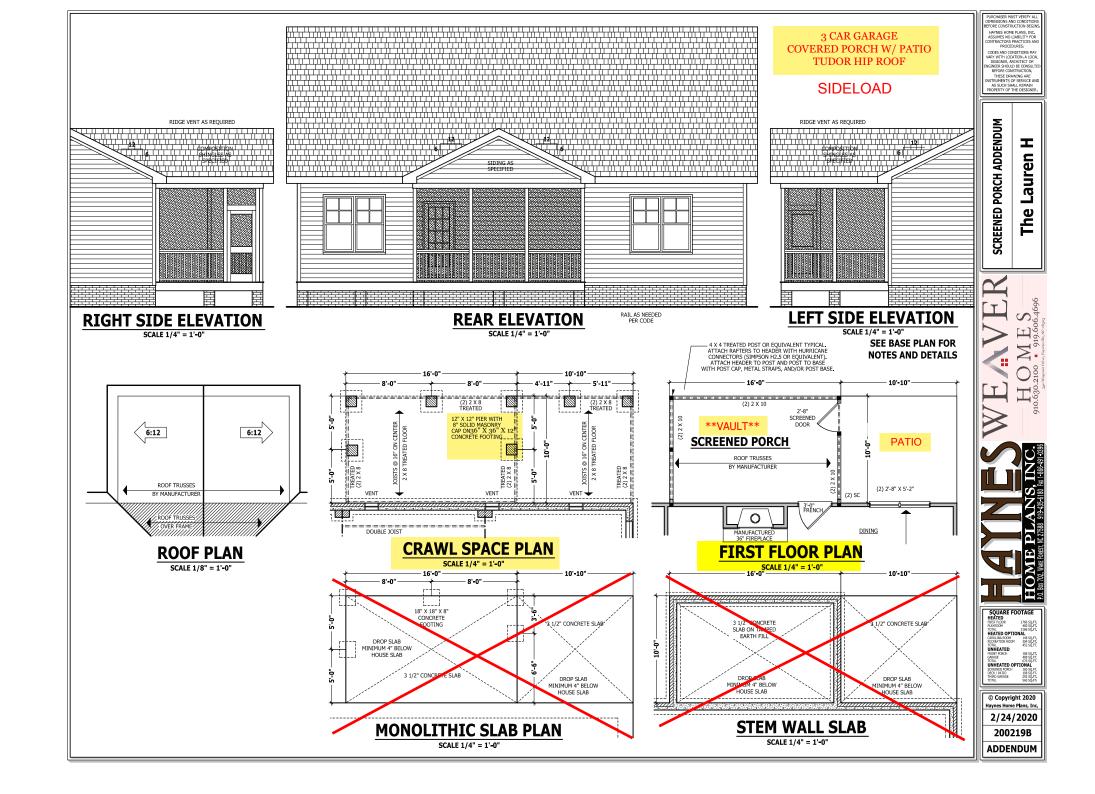
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The

200219B PAGE 7 OF 7







Client: Project: Address: Weaver Homes

Date: 1/24/2025 Input by: Curtis Quick

Job Name: The Lauren H Beams

Page 1 of 14

0

0

Ld. Comb.

D+S

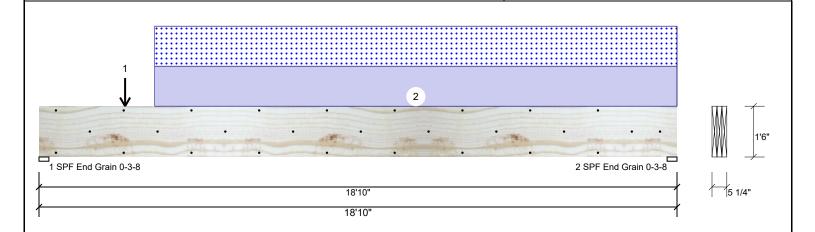
D+S

Project #:

Kerto-S LVL GDH (Side Load)

1.750" X 18.000"

3-Ply - PASSED Level: Level



Reactions UNPATTERNED Ib (Uplift) Application: Direction Live Wind Type: Brg Dead Snow Const Plies: 3 Design Method: ASD 0 4291 4093 0 Vertical 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 0 3868 3670 0 Deflection LL: 360 Load Sharing: Yes Deflection TL: 240 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F

Bearings Bearing Length

End Grain

End Grain

1 - SPF 3.500"

2 - SPF 3.500"

Dir.

Vert

Vert

Cap. React D/L lb

54%

4291 / 4093

3868 / 3670

Total Ld. Case

8383 L

7538 L

Analysis Results

Member Information

•						
Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	35313 ft-lb	9' 3/8"	77108 ft-lb	0.458 (46%)	D+S	L
Unbraced	35313 ft-lb	9' 3/8"	35414 ft-lb	0.997 (100%)	D+S	L
Shear	8377 lb	1'9 1/2"	23184 lb	0.361 (36%)	D+S	L
LL Defl inch	0.229 (L/964)	9'3 3/8"	0.613 (L/360)	0.373 (37%)	S	L
TL Defl inch	0.470 (L/470)	9'3 3/8"	0.920 (L/240)	0.511 (51%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of SDW22500 at 24" o.c. Maximum end distance not to exceed
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at a maximum of 5'7 1/4" o.c.
- 8 Bottom must be laterally braced at end bearings.
- 9 Lateral slenderness ratio based on single ply width

						l					
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Point	2-6-8		Тор	1997 lb	0 lb	1997 lb	0 lb	0 lb	B4	
	Bearing Length	0-3-8									
2	Part. Uniform	3-5-0 to 18-10-0		Тор	374 PLF	0 PLF	374 PLF	0 PLF	0 PLF	B3	
	Self Weight				21 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Client: Weaver Homes 1/24/2025 Page 2 of 14 Project: Input by: Curtis Quick isDesign Address: Job Name: The Lauren H Beams Project #: 1.750" X 18.000" 3-Ply - PASSED Level: Level Kerto-S LVL **GDH (Side Load)** 1 SPF End Grain 0-3-8 2 SPF End Grain 0-3-8 18'10" 18'10" Multi-Ply Analysis Fasten all plies using 3 rows of SDW22500 at 24" o.c.. Maximum end distance not to exceed 12". Capacity 0.0 % 0.0 PLF Yield Limit per Foot 382.5 PLF Yield Limit per Fastener 255.0 lb. См Yield Mode Lookup Edge Distance 1 1/2" Min. End Distance 6" Load Combination **Duration Factor** 1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

L. UVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals
 Damaged Beams must not be used

Danaged Beams must not be used
Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation

This design is valid until 6/28/2026

For flat roofs provide proper drainage to prevent ponding

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us



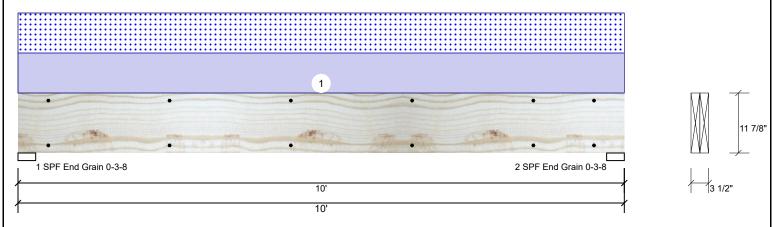
Project: Address: Date: 1/24/2025 Input by:

Curtis Quick Job Name: The Lauren H Beams Page 3 of 14

Project #:

1.750" X 11.875" 2-Ply - PASSED **Kerto-S LVL** GDH-1

Level: Level



Member Infori	mation						Reac	tion	s UNP	ATTERN	ED I	b (Uplift)			
Type:	Girder		Applicati	on:	Floor		Brg	Dire	ction	Live		Dead	Snow	Wind	Const
Plies:	2		Design N	/lethod:	ASD		1	Verti	cal	0		1196	1150	0	0
Moisture Condition	: Dry		Building	Code:	IBC 2012		2	Verti	cal	0		1196	1150	0	0
Deflection LL:	360		Load Sha	aring:	No										
Deflection TL:	240		Deck:		Not Checked										
Importance:	Normal - II														
Temperature:	Temp <= 100	°F					<u> </u>								
							Bear	ings							
							Bea	ring	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - S End		3.500"	Vert	23%	1196 / 1150	2346	L	D+S
Analysis Result	ts						Gra	in							
	tual	Location	Allowed	Capacity	Comb.	Case	2 - 5		3.500"	Vert	23%	1196 / 1150	2346	L	D+S
Moment 53	40 ft-lb	5'	22897 ft-lb	0.233 (23	%) D+S	L	End Gra								
Unbraced 53	40 ft-lb	5'	9721 ft-lb	0.549 (55	%) D+S	L									

L

Design Notes

Shear

1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.

8'8 5/8" 10197 lb

0.172 (17%) D+S

5' 0.318 (L/360) 0.161 (16%) S

5' 0.477 (L/240) 0.219 (22%) D+S

- 2 Fasten all plies using 2 rows of SDW22338 at 24" o.c. Maximum end distance not to exceed 12".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at end bearings.

1754 lb

LL Defl inch 0.051 (L/2238)

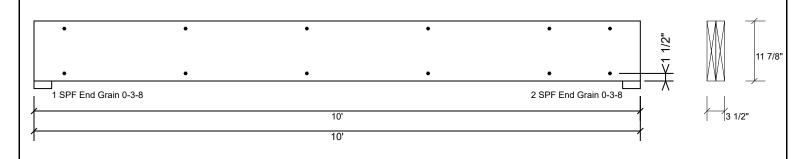
TL Defl inch 0.104 (L/1097)

- 8 Bottom must be laterally braced at end bearings.
- 9 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	230 PLF	0 PLF	230 PLF	0 PLF	0 PLF	G1	
	Self Weight				9 PLF						

Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended	LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply	ponding	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	
ensure the Component statement of the fine application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid		www.metsawood.com/us	
	lateral displacement and fotation	This design is valid until 6/28/2026		

Client: Weaver Homes Date: 1/24/2025 Page 4 of 14 Project: Input by: Curtis Quick isDesign Address: Job Name: The Lauren H Beams Project #: 1.750" X 11.875" Level: Level **Kerto-S LVL** 2-Ply - PASSED GDH-1



Multi-Ply Analysis

Fasten all plies using 2 rows of SDW22338 at 24" o.c.. Maximum end distance not to exceed 12".

Capacity 0.0 PLF 255.0 PLF Yield Limit per Foot Yield Limit per Fastener 255.0 lb. См Yield Mode Lookup Edge Distance 1 1/2" Min. End Distance 6" Load Combination **Duration Factor** 1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info	
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	



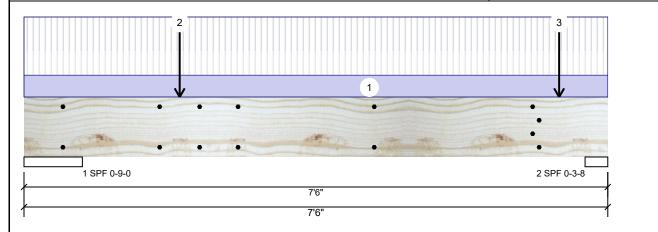
Project: Address: Date: 1/24/2025 Input by:

Curtis Quick Job Name: The Lauren H Beams Page 5 of 14

Project #:

1.750" X 9.250" 2-Ply - PASSED Kerto-S LVL BM1

Level: Level



Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance:

Normal - II Temperature: Temp <= 100°F

Application: Floor Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck: Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	159	1025	937	0	0
2	Vertical	141	1318	1239	0	0

Bearings

Bearing	Length	Dir.	Cap. I	React D/L lb	Total	Ld. Case	Ld. Comb
1 - SPF	9.000"	Vert	15%	1025 / 937	1961	L	D+S
2 - SPF	3.500"	Vert	49%	1318 / 1239	2557	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	2535 ft-lb	2'	14423 ft-lb	0.176 (18%)	D+S	L
Unbraced	2535 ft-lb	2'	10012 ft-lb	0.253 (25%)	D+S	L
Shear	2539 lb	6'5 1/4"	7943 lb	0.320 (32%)	D+S	L
LL Defl inch	0.022 (L/3655)	3'7 1/4"	0.219 (L/360)	0.098 (10%)	S	L
TL Defl inch	0.046 (L/1730)	3'7 9/16"	0.329 (L/240)	0.139 (14%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of SDW22338 at 24" o.c. Maximum end distance not to exceed 12".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Concentrated load fastener specification is in addition to hanger fasteners if a hanger is present.
- 5 Simpson fasteners applied from a single side of the member use tip values where published.
- 6 Girders are designed to be supported on the bottom edge only.
- 7 Top loads must be supported equally by all plies.
- 8 Top must be laterally braced at end bearings.
- 9 Bottom must be laterally braced at end bearings.
- 10 Lateral slenderness ratio based on single ply width.

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation

For flat roofs provide proper drainage to prevent ponding



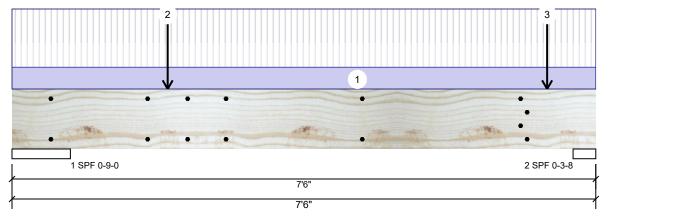
Project: Address: Date: 1/24/2025 Input by: Curtis Quick

Job Name: The Lauren H Beams

Project #:

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED BM₁

Level: Level



	9 1/4
3 1/	2"

Page 6 of 14

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	15 PLF	40 PLF	0 PLF	0 PLF	0 PLF	Floor
2	Point	2-0-0		Far Face	1088 lb	0 lb	1088 lb	0 lb	0 lb	A6
3	Point	6-10-8		Far Face	1088 lb	0 lb	1088 lb	0 lb	0 lb	A6
	Self Weight				7 PLF					

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. IVI. beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

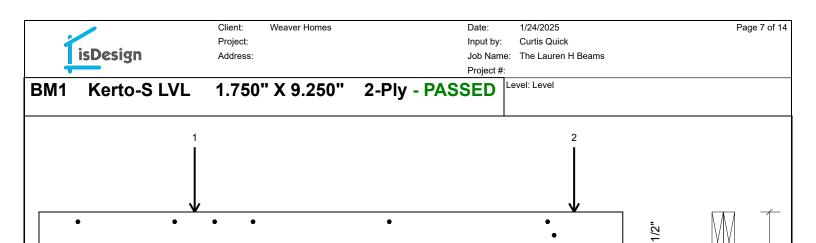
3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026



Multi-Ply Analysis

. 1 SPF 0-9-0

Fasten all plies using 2 rows of SDW22338 at 24" o.c.. except for regions covered by concentrated load fastening. Maximum end distance not to exceed 12".

7'6' 7'6'

cira distarred rist to exceed	
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	255.0 PLF
Yield Limit per Fastener	255.0 lb.
CM	1
Yield Mode	Lookup
Edge Distance	1 1/2"
Min. End Distance	6"
Load Combination	
Duration Factor	1.00

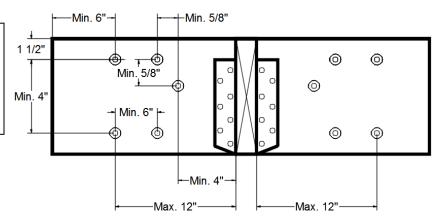
Concentrated Load

Fasten at concentrated side load at 2-0-0 with a minimum of (4) – SDW22338 in the pattern shown. All fasteners shall be installed with the head on the

side of the applied load.

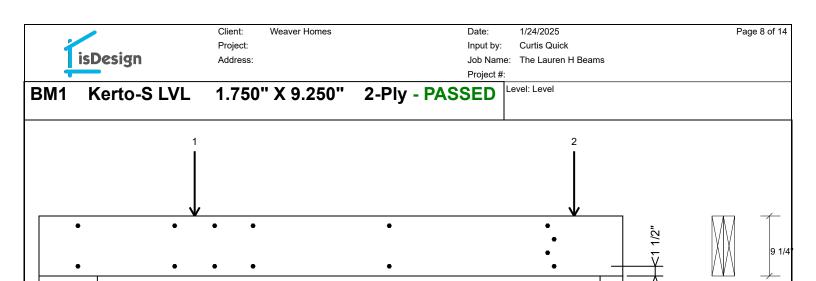
side of the applied load.	
Capacity	92.8 %
Load	1088.0lb.
Total Yield Limit	1173.0 lb.
Cg	1.0000
См	1
Yield Limit per Fastener	293.3 lb.
Yield Mode	Lookup
Load Combination	D+S
Duration Factor	1.15

Min/Max fastener distances for Concentrated Side Loads



2 SPF 0-3-8

Notes Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component sublibility of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive chemicals 6. For flat roofs provide proper drainage to preven ponding 6. For flat roofs provide proper drainage to prevent ponding 6. For flat roofs provide proper drainage to prevent ponding 6. For flat roofs provide proper drainage to prevent ponding Metså Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us 1. LVL beams must not be used 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at labering points to avoid lateral displacement and rotation This design is valid until 6/28/2026



7'6' 7'6"

Multi-Ply Analysis

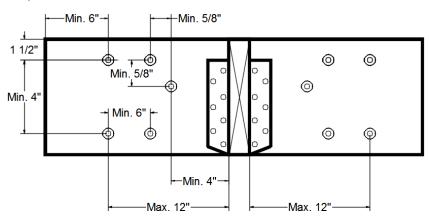
. 1 SPF 0-9-0

Concentrated Load

Fasten at concentrated side load at 6-10-8 with a minimum of (4) – SDW22338 in the pattern shown. All fasteners shall be installed with the head on the side of the applied load

side of the applied load.	
Capacity	92.8 %
Load	1088.0lb.
Total Yield Limit	1173.0 lb.
Cg	1.0000
См	1
Yield Limit per Fastener	293.3 lb.
Yield Mode	Lookup
Load Combination	D+S
Duration Factor	1.15

Min/Max fastener distances for Concentrated Side Loads



2 SPF 0-3-8

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

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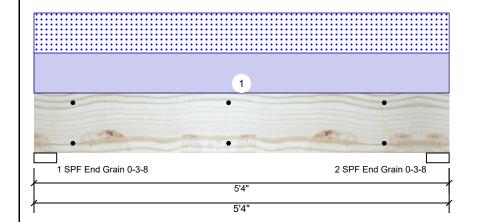
Project: Address: Date: 1/24/2025 Input by:

Curtis Quick Job Name: The Lauren H Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM₂

Level: Level



Application:

Design Method:

Building Code:

Load Sharing:

Deck:

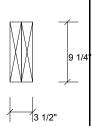
Floor

ASD

No

IBC 2012

Not Checked



Page 9 of 14

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal - II

Temp <= 100°F Temperature:

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1678	1659	0	0
2	Vertical	0	1678	1659	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	3717 ft-lb	2'8"	14423 ft-lb	0.258 (26%)	D+S	L
Unbraced	3717 ft-lb	2'8"	11811 ft-lb	0.315 (31%)	D+S	L
Shear	2013 lb	4'3 1/4"	7943 lb	0.253 (25%)	D+S	L
LL Defl inch	0.024 (L/2469)	2'8"	0.162 (L/360)	0.146 (15%)	S	L
TL Defl inch	0.048 (L/1227)	2'8"	0.244 (L/240)	0.196 (20%)	D+S	L

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	32%	1678 / 1659	3337	L	D+S
2 - SPF End Grain	3.500"	Vert	32%	1678 / 1659	3337	L	D+S

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of SDW22338 at 24" o.c. Maximum end distance not to exceed 12".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at end bearings.

Self Weight

- 8 Bottom must be laterally braced at end bearings.
- 9 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	622 PLF	0 PLF	622 PLF	0 PLF	0 PLF	A3

Notes

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

Handling & Installation

- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- Damaged Beams must not be used beanis must be used to be used to
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

7 PLF

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

Dry service conditions, unless noted otherwise LVL not to be treated with fire retardant or corrosive	Provide lat lateral displ

Version 23.40.705 Powered by iStruct™ Dataset: 24051401.1529

isDesign

Client: Weaver Homes

Project: Address: Date: 1/24/2025

Input by: Curtis Quick Job Name: The Lauren H Beams

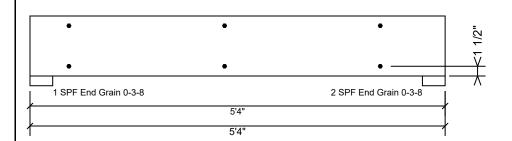
Project #:

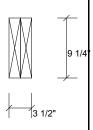
Level: Level

Kerto-S LVL BM2

1.750" X 9.250"

2-Ply - PASSED





Page 10 of 14

Multi-Ply Analysis

Fasten all plies using 2 rows of SDW22338 at 24" o.c.. Maximum end distance not to exceed 12".

rasterrain piles asing 2 rows	01 3D W22330 at 21 0.c Wit
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	255.0 PLF
Yield Limit per Fastener	255.0 lb.
См	1
Yield Mode	Lookup
Edge Distance	1 1/2"
Min. End Distance	6"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850 www.metsawood.com/us

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



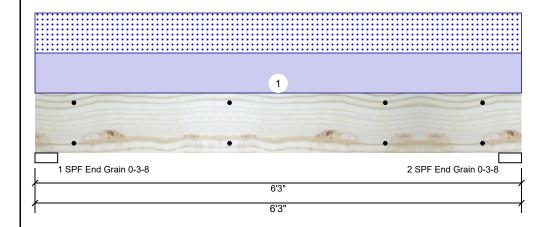
Project: Address: Date: 1/24/2025

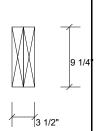
Input by: Curtis Quick Job Name: The Lauren H Beams

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM₃

Level: Level





Page 11 of 14

Member Information

rype:	neader
Plies:	2
Moisture Condition:	Dry
Deflection LL:	360
Deflection TL:	240
Importance:	Normal - II
Temperature:	Temp <= 100°F

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No **Header Supports** No Glass: Deck: Not Checked Reactions UNPATTERNED Ib (Uplift) Wind Brg Direction Live Dead Snow Const 0 2401 2378 0 Vertical 0 1 2 Vertical 0 2401 2378 0 0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	6412 ft-lb	3'1 1/2"	14423 ft-lb	0.445 (44%)	D+S	L
Unbraced	6412 ft-lb	3'1 1/2"	10779 ft-lb	0.595 (59%)	D+S	L
Shear	3160 lb	1' 3/4"	7943 lb	0.398 (40%)	D+S	L
LL Defl inch	0.053 (L/1309)	3'1 1/2"	0.193 (L/360)	0.275 (27%)	S	L
TL Defl inch	0.107 (L/652)	3'1 1/2"	0.290 (L/240)	0.368 (37%)	D+S	L

Bearings

Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	46%	2401 / 2378	4779	L	D+S
2 - SPF End Grain	3.500"	Vert	46%	2401 / 2378	4779	L	D+S

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of SDW22338 at 24" o.c. Maximum end distance not to exceed 12".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top loads must be supported equally by all plies.
- 7 Top must be laterally braced at end bearings.

Self Weight

- 8 Bottom must be laterally braced at end bearings.
- 9 Lateral slenderness ratio based on single ply width.

ID Load Type Location Trib Width Side Dead 0.9 Snow 1.15 Wind 1.6 Const. 1.25 Comments Live 1 1 Uniform Top 761 PLF 0 PLF 761 PLF 0 PLF 0 PLF

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

7 PLF

isDesign

BM₃

Client: Weaver Homes

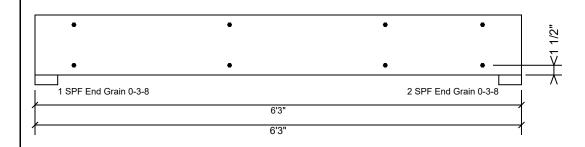
Project: Address: Date:

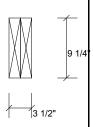
1/24/2025 Input by: Curtis Quick Job Name: The Lauren H Beams

Project #:

Kerto-S LVL 1.750" X 9.250" 2-Ply - PASSED

Level: Level





Page 12 of 14

Multi-Ply Analysis

Fasten all plies using 2 rows of SDW22338 at 24" o.c.. Maximum end distance not to exceed 12".

rasterran pries asing 2 rove	3 01 3D 11 LL 330 at L 1	0.0 1110
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	255.0 PLF	
Yield Limit per Fastener	255.0 lb.	
См	1	
Yield Mode	Lookup	
Edge Distance	1 1/2"	
Min. End Distance	6"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Version 23.40.705 Powered by iStruct™ Dataset: 24051401.1529

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us

Manufacturer Info



Client: Project: Address:

Weaver Homes

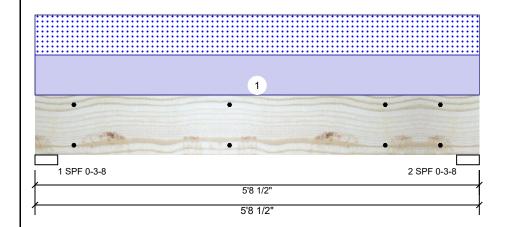
Date: 1/24/2025

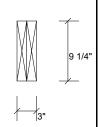
Input by: Curtis Quick Job Name: The Lauren H Beams

Project #:

2.000" X 10.000" 2-Ply - PASSED S-P-F #2

Level: Level





Page 13 of 14

Member Information

Type: Plies: Moisture Condition: Dry Deflection LL: 360 Deflection TL: 240 Importance: Normal - II Temp <= 100°F Temperature:

Application: Design Method: ASD **Building Code:** IBC 2012 Load Sharing: No Deck:

Not Checked

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	782	782	0	0
2	Vertical	0	782	782	0	0

Bearings

Bearing	Length	Dir.	Cap. R	eact D/L lb	act D/L lb Total		Ld. Comb.
1 - SPF	3.500"	Vert	35%	782 / 782	1564	L	D+S
2 - SPF	3.500"	Vert	35%	782 / 782	1564	L	D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	1888 ft-lb	2'10 1/4"	3946 ft-lb	0.478 (48%)	D+S	L
Unbraced	1888 ft-lb	2'10 1/4"	3629 ft-lb	0.520 (52%)	D+S	L
Shear	1404 lb	1' 3/4"	2872 lb	0.489 (49%)	D+S	L
LL Defl inch	0.017 (L/3726)	2'10 1/4"	0.175 (L/360)	0.097 (10%)	S	L
TL Defl inch	0.034 (L/1863)	2'10 1/4"	0.262 (L/240)	0.129 (13%)	D+S	L

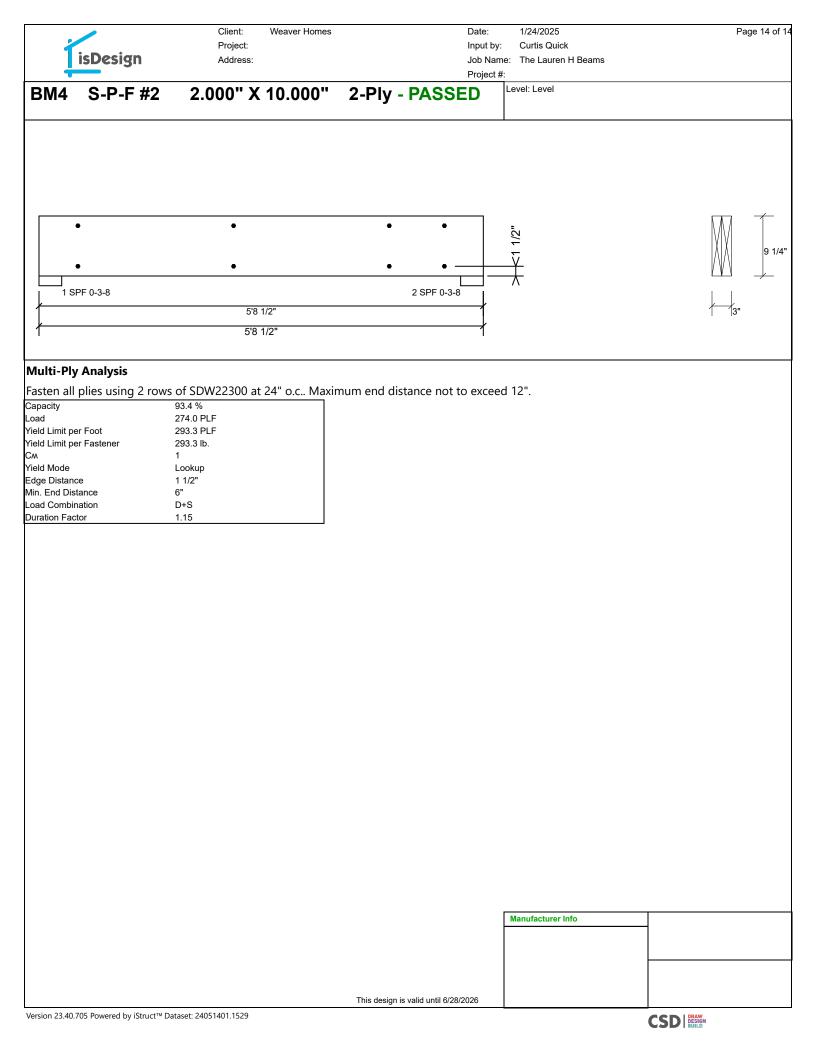
Design Notes

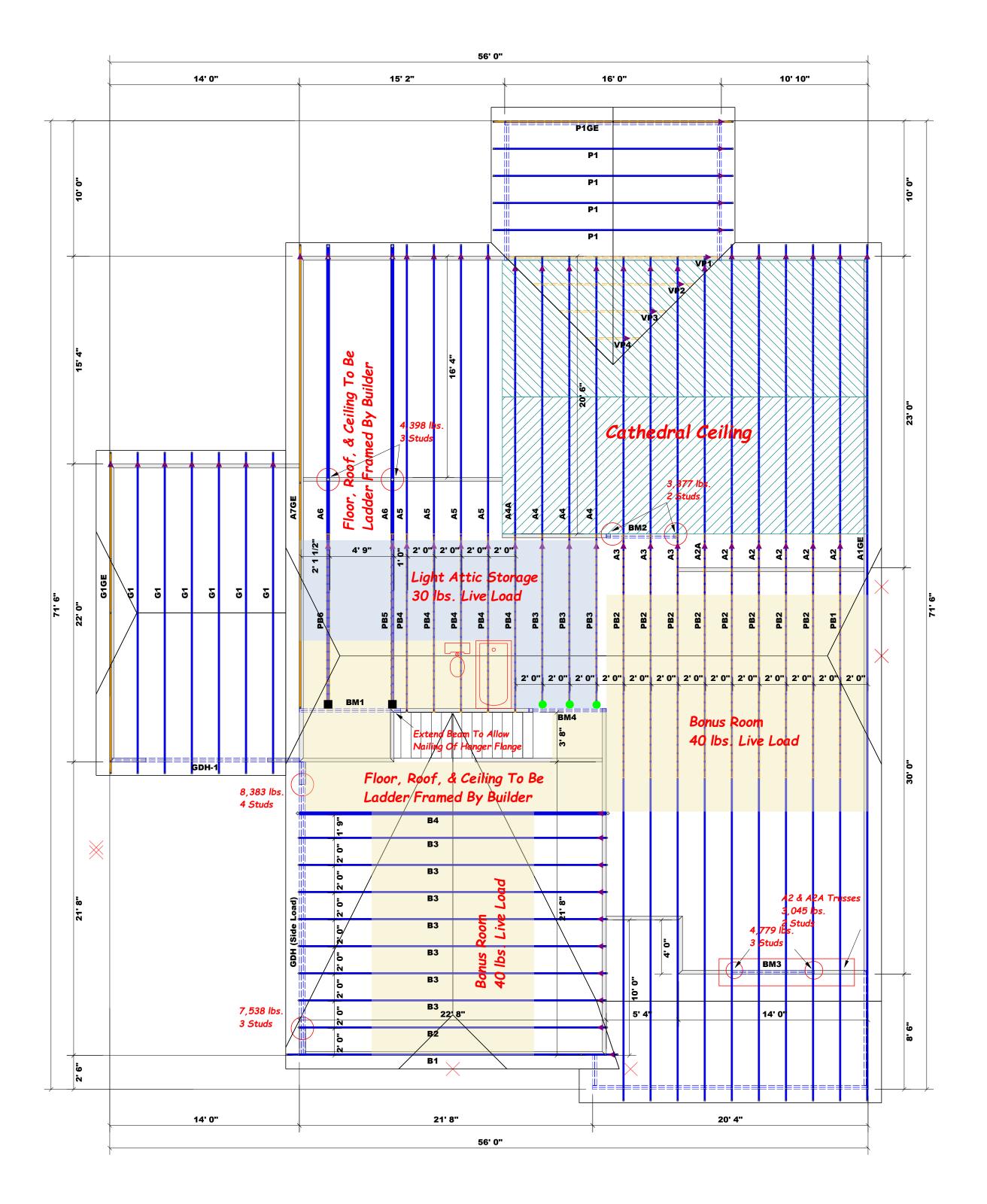
- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of SDW22300 at 24" o.c. Maximum end distance not to exceed 12".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Simpson fasteners applied from a single side of the member use tip values where published.
- 5 Girders are designed to be supported on the bottom edge only.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Far Face	274 DI E	0 DI E	274 DI E	0 DI E	0 DI E	Δ.4

This design is valid until 6/28/2026

Manufacturer Info		





HANGER LEGEND = USP THD28-2 / Double 2x Hanger = USP HUS26 / Single 2x Hanger

▲ = Denotes Left End of Truss (Reference Engineered Truss Drawing) Do Not Erect Trusses Backwards

END REACTION
(UP TO)
REQ'D STUDS FOR
(4) PLY HEADER

3400 1

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))
NUMBER OF JACK STUDS REQUIRED @ EA END OF
HEADER/GIRDER

END REACTION
(UP TC)
REQ'D STUDS FOR
(3) PLY HEADER

5100 2

7650 3

10200 4

12750 5

15300 6

1700 1

3400 2

5100 3

6800 4

8500 5

10200 6

11900 7

13600 8

15300 9

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan

Beam Legend					
Length	Product	Plies	Net Qty	Fab Type	
8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF	
22' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF	
6' 0"	2x10 SPF No.2	2	2	FF	
	8' 0" 7' 0" 6' 0" 14' 0" 22' 0"	Length Product 8' 0" 1-3/4"x 9-1/4" LVL Kerto-S 7' 0" 1-3/4"x 9-1/4" LVL Kerto-S 6' 0" 1-3/4"x 9-1/4" LVL Kerto-S 14' 0" 1-3/4"x 11-7/8" LVL Kerto-S 22' 0" 1-3/4"x 18" LVL Kerto-S	LengthProductPlies8' 0"1-3/4"x 9-1/4" LVL Kerto-S27' 0"1-3/4"x 9-1/4" LVL Kerto-S26' 0"1-3/4"x 9-1/4" LVL Kerto-S214' 0"1-3/4"x 11-7/8" LVL Kerto-S222' 0"1-3/4"x 18" LVL Kerto-S3	LengthProductPliesNet Qty8' 0"1-3/4"x 9-1/4" LVL Kerto-S227' 0"1-3/4"x 9-1/4" LVL Kerto-S226' 0"1-3/4"x 9-1/4" LVL Kerto-S2214' 0"1-3/4"x 11-7/8" LVL Kerto-S2222' 0"1-3/4"x 18" LVL Kerto-S33	

ses	Backwards			SCALE: 3/16" = 1'	BM4	6' 0"	
В	UILDER	Weaver Homes, Inc.	CITY / CO.	Lillington / Harnett		THIS IS A TR These trusses a the building des sheets for each is responsible fe the overall struc walls, and colum regarding bracin	
J	OB NAME	Lot 3A Elbridge Farm	ADDRESS	90 Larime Lane			
Pl	-AN	Lauren H / 3 Car / SL / Tudor	MODEL	Roof		or online @ sbc Bearing reaction prescriptive Co	
S	EAL DATE	2/24/20	DATE REV.	01/08/25		(derived from foundation size than 3000# but be retained to	
Q	UOTE#	Quote #	DRAWN BY	Curtis Quick		specified in the retained to des	
J	OB #	J0125-0034	SALES REP.	Lenny Norris		Signature_	

A TRUSS PLACEMENT DIAGRAM ONLY.

ses are designed as individual building components to be incorporated into the design at the specification of the building designer. See individual design each truss design identified on the placement drawing. The building designer ole for temporary and permanent bracing of the roof and floor system and for structure. The design of the truss support structure including headers, beams, olumns is the responsibility of the building designer. For general guidance racing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package sbcindustry.com

actions less than or equal to 3000# are deemed to comply with the a Code requirements. The contractor shall refer to the attached Tables om the prescriptive Code requirements) to determine the minimum size and number of wood studs required to support reactions greater but not greater than 15000#. A registered design professional shall to design the support system for any reaction that exceeds those in the attached Tables. A registered design professional shall be design the support system for all reactions that exceed 15000#.

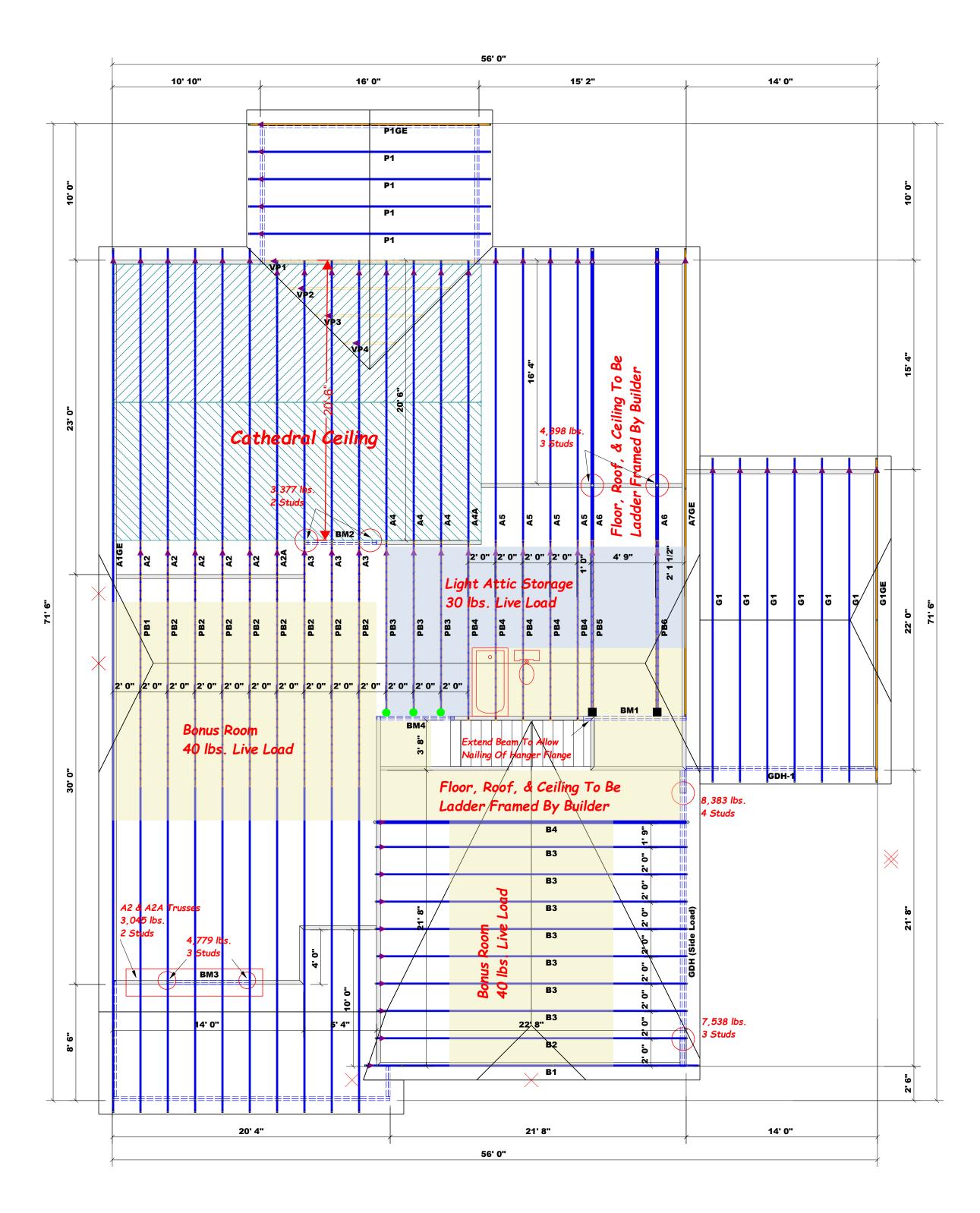
Curtis Quick

TRUSSES & BEAMS Curtis Quick

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

соттесн

ROOF & FLOOR



HANGER LEGEND

= USP THD28-2 / Double 2x Hanger

= USP HUS26 / Single 2x Hanger

= Denotes Left End of Truss
 (Reference Engineered Truss Drawing)
 Do Not Erect Trusses Backwards

END REACTION (UP TO) REQ'D STUDS FOR (4) PLY HEADER

3400 1

6800 2

10200 3

13600 4

17000 5

LOAD CHART FOR JACK STUDS

(BASED ON TABLES R502.5(1) & (b))

NUMBER OF JACK STUDS REQUIRED @ EA END OF HEADER/GIRDER

END REACTION
(UP TC)
REQ'D STUDS FOR
(3) PLY HEADER

5100 2

7650 3

10200 4

12750 5

15300 6

1700 1 3400 2

5100 3

6800 4

8500 5

10200 6

11900 7

13600 8

15300 9

All Truss Reactions are Less than 3,000 lbs. Unless Noted Otherwise.

-- Denotes Reaction Greater than 3,000 lbs.

Truss Placement Plan SCALE: 3/16" = 1'

Beam Legend					
PlotID	Length	Product	Plies	Net Qty	Fab Type
BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
GDH (Side Load)	22' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF
BM4	6' 0"	2x10 SPF No.2	2	2	FF

				JOALL. 3/10 = 1		
BUIL	_DER	Weaver Homes, Inc.	CITY / CO.	Lillington / Harnett	THIS IS A TRUSS PLACE These trusses are designed at the building design at the spec sheets for each truss design ic	
JOB	NAME	Lot 3A Elbridge Farm	ADDRESS	90 Larime Lane	is responsible for temporary a the overall structure. The designable walls, and columns is the respondering bracing, consult BC	
PLAN	7	Lauren H / 3 Car / SL / Tudor	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than prescriptive Code requirem	
SEAL	L DATE	2/24/20	DATE REV.	01/08/25	(derived from the prescript foundation size and numbe than 3000# but not greater be retained to design the si	
QUO	TE#	Quote #	DRAWN BY	Curtis Quick	specified in the attached Ta retained to design the supp	
JOB	#	J0125-0034	SALES REP.	Lenny Norris	Signature	

is IS A TRUSS PLACEMENT DIAGRAM ONLY.

The trusses are designed as individual building components to be incorporated into utilding design at the specification of the building designer. See individual design is for each truss design identified on the placement drawing. The building designer ponsible for temporary and permanent bracing of the roof and floor system and for verall structure. The design of the truss support structure including headers, beams, and columns is the responsibility of the building designer. For general guidance ding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package line @ sbcindustry.com

Ingreactions less than or equal to 3000# are deemed to comply with the criptive Code requirements. The contractor shall refer to the attached Tables are defined to the prescriptive Code requirements to determine the minimum

Curtis Quick

Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.

Signature

ROOF & FLOOR TRUSSES & BEAMS

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

