PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

MEAN ROOF HEIGHT: 19'-9"					HEIGHT TO RIDGE: 27'-5"			
CLIMATE ZO	NE		ZONE 3A		ZONE 4A		ZOM	VE 5A
FENESTRATION	U-FACT	OR	0	.35	0	.35	0	35
SKYLIGHT U-FA	CTOR		0	.55	0	.55	0	.55
GLAZED FENEST	RATION	SHGC	0	.30	0	.30	0	.30
CEILING R-VALU	JE		38 c	r 30ci	38 o	r 30ci	38 c	r 30ci
WALL R-VALUE				15		15		19
FLOOR R-VALUE				19		19		30
* BASEMENT W	ALL R-V/	ALUE	5	/13	10	/15	10	/15
** SLAB R-VALU	IE			0		10		10
* CRAWL SPACE	WALL F	R-VALUE	5	/13	10	/15	10	/19
***10/13* MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** JINSULATION DEPTH WITH MONOLITHIC SUB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING, INSULATION DEPTH WITH STEM WALL SUB 24" OR TO BOTTOM OF FOUNDATION WALL DEFICIENCE FOR MUNIC DEPTH OF 10 MINU 2 GEFORD CUT VIA EASTERT MILE EXPORTMENT FOR DESCRIPTION OF THE DATA ON 2 GEFORD CUT VIA EASTERT MILE EXPORTMENT FOR DESCRIPTION OF THE DATA ON 2 GEFORD CUT VIA EASTERT MILE EXPORTMENT FOR DESCRIPTION OF THE DATA ON 2 GEFORD CUT VIA EASTERT MILE EXPORTMENT FOR DESCRIPTION OF THE DATA ON 2 GEFORD CUT VIA EASTERT MILE EXPORTMENT FOR DESCRIPTION OF THE DATA ON 2 GEFORD CUT VIA EASTERT FOR THE EXPORTMENT FOR DESCRIPTION OF THE DATA ON THE 2 GEFORD CUT VIA EASTERT DESCRIPTION OF THE DATA ON THE 2 GEFORD CUT VIA EASTERT DESCRIPTION OF THE DATA ON THE FORD DESCRIPTION OF THE DATA ON THE 2 GEFORD CUT VIA EASTERT DESCRIPTION OF THE DATA ON THE DATA								
COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS								
MEAN BOOF	UP T	0.30'	30'-1"	TO 35'	35'-1" TO 40' 40'-1" TO 45'			
ZONE 1	14.2	-15.0	14.9	-15.8	15.5	-16.4	15.9	-16.8
ZONE 2	14.2	-18.0	14.9	18.9	15.5	19.6	15.9	20.2
ZONE 3	14.2	-18.0	14.9	-18.9	15.5	-19.6	15.9	-20.2
ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4
DESIGNED FOR WIND SPEED OF 130 MPH, 3 SECOND GUST (101 FASTEST MILE) EXPOSURE "B"								
COMPONENT	& CLA	DDING	DESIG	NED FC	R THE	FOLLO	WING I	LOADS
MEAN ROOF	UP T	'O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	23.5
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	26.9

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertical to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a *guard*

R312.2 Height, Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914) mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads

Where the top of the guard also serves as a handrail on the open sides of 2: where the top of the guard also serves as a natural of the open sides of stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required guards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter.

Exceptions: The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

 Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.

ROOF VENTILATION

SECTION R806

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT. NET FREE CROSS VENTILATION NEEDED: WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CELLING = 8.26 SO FT

12



SCALE 1/8" = 1'-0'

SCALE 1/8" = 1'-0"

PAGE 1 OF 7





STRUCTURAL NOTES

A construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all the 2018 North Caroina residential building code, prus an local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans,

Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION			
USE	(PSF)	(PSF)	(LL)			
Attics without storage	10	10	L/240			
Attics with limited storage	20	10	L/360			
Attics with fixed stairs	40	10	L/360			
Balconies and decks	40	10	L/360			
Fire escapes	40	10	L/360			
Guardrails and handrails	200		-			
Guardrail in-fil components	50		-			
Passenger vehicle garages	50	10	L/360			
Rooms other than sleeping	40	10	L/360			
Sleeping rooms	30	10	L/360			
Stairs	40		L/360			
Snow	20					

LADDER FRAMED

noted otherwise.

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise

ENGINEERED WOOD BEAMS :

ENGINEERED WOUD BEARS: Laminated venera lumber (UVL) = F0=2600 PSI, Fv=265 PSI, E=1.9x106 PSI Parallel strand lumber (ISL) = F0=2500 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (ISL) F0=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Instal all connections per manufactures instructions. TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist

layouts shall be prepared in accordance with this document. Trusses and L-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. CONCRETE AND SOILS: See foundation notes.



ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Havnes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer. ANCHORAGE. All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. BEARING, All trusses shall be designed for bearing on SPF #2 plates or edgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.



URCHASER MUST VERIFY A

HAYNES HOME PLANS, INC.

SUMES NO LIABILITY FOR

PROCEDURES. PROLEDURES CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCA DESIGNER, ARCHITECT OR VGINEER SHOULD BE CONSULTI BEFORE CONSTRUCTION.

THESE DRAWING ARE INSTRUMENTS OF SERVICE AN AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

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Lauren

The

SQUARE FOOTAGE

FRONT PORCH 188 SQ F GARAGE 488 SQ F TOTAL 676 SQ F UNHEATED OPTIONAL

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2/24/2020

200219B

PAGE 4 OF 7

Haynes Home Plans, Inc

HEATED OPTIONAL

UNHEATED

1766 SQ FT 400 SQ FT

148 50 FT 304 50 FT 452 SQ FT

160 SQ FT 108 SQ FT 292 SQ FT

FIRST FLOOR STRUCTURAL

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brough to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CELING HEIGHTS. All finished knee wall heights and calling heights are shown furred down 10° from root decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated hele heights, finished knee wall heights, or finished celling heights shown on these drawings the finished square footage may avar, Any discrepancy must be brought to Haynes home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer. ANCHORAGE, All required anchors for trusses due to uplift or bearing

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or

ledgers unless noted otherwise. Plate Heights & Floor Systems. See elevation page(s) for plate heights

and floor system thicknesses.

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Cardina Residential Bulking Code, byte Sal local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. tasks no responsibility for the contractor's failure to carry

Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LUAD	DEAD LOAD	DEFLECTION				
USE	(PSF)	(PSF)	(LL)				
Attics without storage	10		L/240				
Attics with limited storage	20	10	L/360				
Attics with fixed stairs	40	10	L/360				
Balconies and decks	40	10	L/360				
Fire escapes	40	10	L/360				
Guardrails and handrails	200	-	-				
Guardrail in-fill components	50	-	-				
Passenger vehicle garages	50	10	L/360				
Rooms other than sleeping	40	10	L/360				
Sleeping rooms	30	10	L/360				
Stairs	40 -		L/360				
Snow	20	-	-				

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS :

Enclineted where fumber (VL) = Fb=2600 PSI, Fv=28S PSI, E=1.9x108 PSI Parallel strand lumber (PSI) = Fb=2600 PSI, Fv=209 PSI, E=2.0x108 PSI Laminated strand lumber (LSI) = Fb=2200 FSI, Fv=209 PSI, E=2.0x108 PSI Instal al connections per manufactures instructions. **TRUSS AND** -1031T **MEMEERS**: All roof truss and Fjoidt Jayouts shall be

prepared in accordance with this document. Trusses and 1-joist shall be instaled according to the manufacture's specifications. Any change in truss or 1-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS; Fink Linkes Shall be 31/2* 31/2* 1/14* stale angle for up to 6-0° span. 6° x 4° x 5/16° steel angle with 6° leg vertical for spans up to 9-0° unless noted otherwise. 31/2* x 1/2* x 1/2* x 1/2* to 1/2* thick for 15° on center joist spacing, minimum 5/6* thick for 15.2* on center joist spacing, minimum 5/6* thick for 15.2* on center joist spacing, minimum 5/6* thick for 15.2* on center joist constraints. One of secting minimum 3/6* thick for 16° on center rathers and 7/16* for 24* on do sheating minimum 3/6* thick for 16° on enter rathers and 7/16* for 24* on do sheating minimum 3/6* thick for 16° on enter rathers and 7/16* for 24* on do sheating minimum 3/6* thick for 16°.

ATTIC ACCESS

SECTION R807

R807.1 Attic access An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net dear opening shall not be less than 20 inches by 30 inches (306 mb y 762 mm) and shall be located in a halway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

 Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
 Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16'

KING STUD(S) 1 2 3 5 6 INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH 1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED



TUDOR HIP ROOF COVERED PORCH W/ PATIO 3 CAR GARAGE



PURCHASER MUST VERIFY ALL DIMENSIONS AND CONDITIONS BEFORE CONSTRUCTION BEGINS HATYNES HOME PLANS, INC. ASSUMES NO LIABILITY FOR CONTRACTORS PRACTICES AND PROCEDURES. PROCEDURES. CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCAL DESIGNES, ARCHITECT OR ENGINEER SHOULD BE CONSTRUCTION, THESE DRAWING ARE INSTRUMENTS OF SERVICE AND AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER. Т The Lauren **ROOF PLAN** M E S -0 Г
 SQUARE FOOTAGE

 HEATED

 FIRST FLOOR

 PLAYEOCM

 900 SQLFT.
 1766 SQ FT 400 SQ FT 2166 SQ FT TOTAL 216 HEATED OPTIONAL 148 SO FT. 304 SQ FT. 452 SQ FT. SCREATION ROOM TOTAL UNHEATED FRONT PORCH UNHEATED FRONT PORCH 188 SQ.FT. GARAGE 488 SQ.FT. TOTAL 675 SQ.FT. UNHEATED OPTIONAL 160 SQ FT 108 SQ FT 292 SQ FT 540 SQ FT SCREENED PORCI DECK / PATIO THIRD GARAGE © Copyright 2020 Haynes Home Plans, Inc. 2/24/2020 200219B

PAGE 6 OF 7

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. **KNEE WALL AND CEILING HEIGHTS.** All finished knee wall heights and celling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or Installation 1: In or any isegment to dusts instructure raises the instead of the exceed designated head heights, or finished accelling heights, or finished accelling heights, and the set and the set of the exceed heights and the set of the s reasonability of the truss manufacturer.

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or ledgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses. HEEL HEIGHT ABOVE

HEEL HEIGHT ABOVE









	Client:	Weaver Homes		Date:	1/24/2025	Page 2 of 14
	Project:			Input by:	Curtis Quick	
Ispesign	Address:			Job Name	e: The Lauren H Beams	
				Project #:		
GDH (Side Load)	Kerto-S LVL	1.750" X 18.0	000" 3-Ply -	PASSED	Level: Level	
• •	٠	• •	•	•	• •	
		•		•		. 2
						$\overline{\mathbf{v}}$ \mathbb{W} $1^{16^{\circ}}$
1 SPE End Grain 0-3-8	•	• •	•	•	• • •	
/		18	3'10"			5 1/4"
/		18	5'10"			/
Multi-Ply Analysis						
Fasten all plies using 3 r	ows of SDW22500 a	t 24" o.c Maximu	m end distance r	ot to exceed	12".	
Capacity	0.0 %					
Load Vield Limit per Foot	0.0 PLF 382 5 PLF					
Yield Limit per Fastener	255 0 lb					
См	1					
Yield Mode	Lookup					
Edge Distance	1 1/2"					
Min. End Distance	6"					
Load Combination	4.00					
Duration Factor	1.00					
Neter	abanaia-1-	~	For flat roofs and the sec	drainago to accord	Manufacturer Info	
Notes Calculated Structured Designs is responsible	only of the Handling & Installa	e. ation	For flat roots provide proper ponding	urainage to prevent	Metsä Wood	1
structural adequacy of this component bas design criteria and loadings shown	ed on the 1. LVL beams must not b It is the 2. Refer to manufact	e cut or drilled turer's product information			301 Merritt 7 Building, 2nd Floor	
responsibility of the customer and/or the co ensure the component suitability of the	e intended factoring datalla base	in requirements, multi-ply			Norwalk, CT 06851 (800) 622-5850	
application, and to verify the dimensions and	loads. approvals	t not be used			www.metsawood.com/us	
Lumber 1. Dry service conditions, unless noted other	4. Design assumes top e wise 5. Provide lateral current	dge is laterally restrained				
2. LVL not to be treated with fire retardant of	or corrosive 3. Provide lateral suppor lateral displacement a	nd rotation	This design is valid unti	6/28/2026		



		Client: Weaver Home	s	Date:	1/24/2025	Page	4 of 14
		Project:		Input by:	Curtis Quick		
isDesig	gn	Address:		Job Nam	e: The Lauren H Beams		
· · · · · · · · · · · · · · · · · · ·				Project #			
		4 750" V 44 0			l evel: l evel		
GDH-1 Kei	rto-5 LVL	1./50° X 11.8	75° 2-Piy	PASSED	2010		
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							11 7/8"
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							≁—
1 SPF End Grain	0-3-8				2 SPF End Grain 0-3-8	I	
<i> </i>			10'		· · · · · · · · · · · · · · · · · · ·	3 1/2"	
,			10			10112	
1			10'		,	1	
Multi-Ply Analysis	5						
Eacton all plics usin	a 2 rows of SDV	N/22228 at 24" o.c. Max	vimum and distand	co not to ovcoo	4 12"		
l'asteri all piles usiri	19 2 10WS 01 3DV	V22550 at 24 0.c 101a					
Load Vield Limit per Feet	0.0 PLF	F					
Yield Limit per Fool	200.0 PL	F					
	200.0 10.						
Vield Mode	Lookun						
Edge Distance	1 1/2"						
Min End Distance	6"						
Load Combination	C C						
Duration Factor	1 00						
	1.00						
Notos	cho	micals	6. For flat roofe provide	proper drainage to prevent	Manufacturer Info		
Calculated Structured Designs is re-	sponsible only of the Hand	ling & Installation	ponding		Metsä Wood	1	
structural adequacy of this compo design criteria and loadings	onent based on the 1. LVL shown. It is the 2. Det	beams must not be cut or drilled	ation		301 Merritt 7 Building, 2nd Floor		
responsibility of the customer and/	or the contractor to regard	arding installation requirements, mult	ti-ply		Norwalk, CT 06851 (800) 622-5850		
application, and to verify the dimensi	sions and loads. app	ening details, beam strength values, and c rovals	code		www.metsawood.com/us		
Lumber	3. Dan 4. Des	naged Beams must not be used ign assumes top edge is laterally restrained					
 Dry service conditions, unless no LVL not to be treated with fire re 	etardant or corrosive 5. Provilater	vide lateral support at bearing points to a ral displacement and rotation	avoid	Luntil 6/29/2026			
	10101	,	i nis desidn is valid	1 UTUL 0/20/2020		1	





Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the interded application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	Handling & Installation 1. UVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	



Multi-Ply Analysis

Fasten all plies using 2 rows of SDW22338 at 24" o.c.. except for regions covered by concentrated load fastening. Maximum

end distance not to exceed	12".
----------------------------	------

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	255.0 PLF	
Yield Limit per Fastener	255.0 lb.	
См	1	
Yield Mode	Lookup	
Edge Distance	1 1/2"	
Min. End Distance	6"	
Load Combination		
Duration Factor	1.00	

Concentrated Load

Fasten at concentrated side load at 2-0-0 with a

minimum of (4) – SDW22338 in the pattern shown.

All fasteners shall be installed with the head on the cide of the applied load

side of the applied load.	
Capacity	92.8 %
Load	1088.0lb.
Total Yield Limit	1173.0 lb.
Cg	1.0000
См	1
Yield Limit per Fastener	293.3 lb.
Yield Mode	Lookup
Load Combination	D+S
Duration Factor	1.15

chemicals

3

Handling & Installation

Min/Max fastener distances for Concentrated Side Loads

This design is valid until 6/28/2026



1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive 5

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Notes

Lumber



Multi-Ply Analysis

Concentrated Load

Fasten at concentrated side load at 6-10-8 with a minimum of (4) – SDW22338 in the pattern shown. All fasteners shall be installed with the head on the side of the applied load.

Capacity	92.8 %	
Load	1088.0lb.	
Total Yield Limit	1173.0 lb.	
Cg	1.0000	
См	1	
Yield Limit per Fastener	293.3 lb.	
Yield Mode	Lookup	
Load Combination	D+S	
Duration Factor	1.15	

Min/Max fastener distances for Concentrated Side Loads





		С	lient:	Weaver Hom	es			Date:	1/24/202	25				Page 9 of 14
	_	P	roject:					Input by:	Curtis Q	uick				
is	Design	A	ddress:					Job Name	e: The Lau	iren H Be	eams			
								Project #:						
BM2	Kerto-S L\	/L 1	.750"	X 9.25	50"	2-Ply -	PAS	SSED	Level: Leve	1				
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					C. C	ACCOUNT A COMPANY		1					V V	
1 SPF	End Grain 0-3-8				2 SPF	End Grain 0-3	3-8	ĺ						
			5'4"				,	ł					1	3 1/2"
/			5'4"				,	ł						
Mombor In	formation						Poor	tions UN			(IInlift)			
	Girder		Applicat	ion: F	loor		Bro	Direction	Live		Dead	Snow	Wind	Const
Plies:	2		Design	Method: A	SD		1	Vertical	C)	1678	1659	0	0
Moisture Cond	dition: Dry		Building	Code: II	BC 2012		2	Vertical	C)	1678	1659	0	0
Deflection LL:	360		Load Sh	naring: N	lo									
Deflection TL:	240 Normal - II		Deck:	Ν	lot Checke	d								
Temperature:	Temp <= 100°	F												
	·						Bear	rings						
							Bea	aring Lengtl	h Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - 3	SPF 3.500"	Vert	32%	1678 / 1659	3337	L	D+S
Analysis Re	sults						Gra	in						
Analysis	Actual	Location A	llowed	Capacity	Comb.	Case	2-	SPF 3.500"	Vert	32%	1678 / 1659	3337	L	D+S
Moment	3717 ft-lb	2'8" 14	4423 ft-lb	0.258 (26%	5) D+S	L	Gra	i in						
Unbraced	3717 ft-lb	2'8" 11	1811 ft-lb	0.315 (31%	5) D+S	L								
Shear	2013 lb	4'3 1/4" 79	943 lb	0.253 (25%	b) D+S	L								
LL Defl inch	0.024 (L/2469)	2'8" 0.	162 (L/360) 0.146 (15%	5) S	L								
TL Defi inch	0.048 (L/1227)	2'8" 0.	244 (L/240	0) 0.196 (20%	b) D+S	L	┥							
Design Not	es	- movement	and rotatio	n at the and h		atoral aupport	4							
may also be	e required at the inter	rior bearings l	by the build	ling code.	eannys. La	aterar support								
2 Fasten all p 12"	blies using 2 rows of 8	SDW22338 at	t 24" o.c. N	laximum end	distance no	ot to exceed								
3 Refer to las	st page of calculations	s for fasteners	s required	for specified lo	oads.									
4 Simpson fa	steners applied from	a single side	of the mer	nber use tip va ne only	alues wher	e published.								
6 Top loads n	nust be supported eq	ually by all pl	ies.	je only.										
7 Top must b	e laterally braced at e	end bearings.												
9 Lateral sler	nderness ratio based	on single ply	width.											
ID	Load Type	Lo	ocation	Trib Width	Side	Dead 0.9		Live 1 Sno	ow 1.15	Wind 1	I.6 Const. 1	.25 Co	mments	
1	Uniform				Тор	622 PLF		0 PLF 6	622 PLF	0 P	LF OF	PLF A3		
	Self Weight					7 PLF								
								I	Manufactu	rer Info				
Notes Calculated Structured	Designs is responsible only of	chemicals the Handling	& Installati	on	6. Fo po	r flat roofs provide p nding	proper drair	nage to prevent	Metsä Wood	d		-		
structural adequacy of design criteria and responsibility of the	of this component based on I loadings shown. It is customer and/or the contractor	the 1. LVL beam the 2. Refer to	s must not be co manufacture	ut or drilled r's product infor	mation				301 Merritt Norwalk, C1	7 Building F 06851	, 2nd Floor			
ensure the compon application, and to ver	ify the dimensions and loads.	ded fastening approvals	Installation details, beam	requirements, m strength values, and	uiti-ply i code				(800) 622-5 www.metsa	850 wood.con	n/us			
Lumber 1. Dry service conditi	ions, unless noted otherwise	 Damaged Design as 	Beams must no sumes top edge	t be used is laterally restraine	d									
2. LVL not to be trea	ated with fire retardant or corros	sive lateral dis	placement and r	otation	avoid	nis design is valio	d until 6/2	28/2026						

isDesign	Client: Weaver Homes Project: Address:	Date: Input by: Job Nam Project #	1/24/2025 Curtis Quick he: The Lauren H Beams	Page 10 of 1
BM2 Kerto-S LVL	1.750" X 9.250"	2-Ply - PASSED	Level: Level	
•	•	• ≺1 1/2"		9 1/
1 SPF End Grain 0-3-8	2 5'4" 5'4"	SPF End Grain 0-3-8		3 1/2"
Multi-Ply Analysis				
Load 0.0 PL Yield Limit per Foot 255.0 I Yield Limit per Fastener 255.0 I Cm 1 Yield Mode Looku Edge Distance 6" Load Combination Duration Factor 1.00	F PLF Ib. p			
Notes Calculated Structured Designs is responsible only of the design criteria and loadings shown. It is the 2. r responsibility of the customer and/or the contractor 2. of application, and to verify the dimensions and loads of	chemicals ndling & Installation VL beams must not be cut or drilled refer to manufacturer's product information regarding distallation requirements, multi-ply grading distallations, beam strength volues, and code	 For flat roofs provide proper drainage to prevent ponding 	Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850	
Lumber 3. 1. Dry service conditions, unless noted otherwise 4. 2. LVL not to be treated with fire retardant or corrosive 5.	approvents Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid ateral displacement and rotation	This design is valid until 6/28/2026	www.metsaw000.com/uS	

		(Client [.]	Weaver Ho	mes			Da	te [.]	1/24/2	025				Page 11 of 1
2		F	Project:	Would he	11100			Inp	ut bv:	Curtis	Quick				r ugo rr or i
1 1 i	isDesign		Address:					Job	Name	The L	auren H E	leams			
								Pro	oiect #:						
DM2	Karta C I		750"	VOO	E0" 4			200		evel: Le	vel				
BIVI 3	Nerto-5 L	VL 1	1.750	X 9.2	50 4	2-Piy -	PA	3 35	ש						
			•••••	<u></u>					<u></u>						
				1											
•			•			•			•					M	1 1
														X)	0 1/
	a ritter	in the second	•		and the state		-		-					///	91/
														<u> </u>	
1 SP	7 End Grain 0-3-8			0.01		28	SPF En	d Grain	0-3-8						
				6'3"										1	3 1/2"
1				6'3"											
Momharl	nformation						Par	ction				h (n ;f+)			
Type:	Header		Applicati	on:	Floor		Bro	Dire	ction			Dead	Snow	Wind	Const
Plies:	2		Design N	/lethod:	ASD			Vertic	al	L1	0	2401	2378	0	001131
Moisture Co	ondition: Dry		Building	Code:	IBC 2012			Vertic	al		0	2401	2378	0	0
Deflection L	L: 360		Load Sh	aring:	No		1	veru	ai		0	2401	2070	0	0
Deflection T	TL: 240		Header	Supports	No										
Importance:	: Normal - II		Glass:												
Temperature	e: Temp <= 100	0°F	Deck:		Not Checked	ł									
·							Bea	rings							
							Be	aring	Lenath	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1-	SPF	3.500"	Vert	46%	2401 / 2378	4779	L	D+S
							_ Ėn	d	0.000		1070	210172010		-	2.0
Analysis R	Results						Gra	ain							
Analysis	Actual	Location A	Allowed	Capacity	/ Comb.	Case	2-	SPF	3.500"	Vert	46%	2401 / 2378	4779	L	D+S
Moment	6412 ft-lb	3'1 1/2" 1	14423 ft-lb	0.445 (44	I%) D+S	L	En Gra	d ain							
Unbraced	6412 ft-lb	3'1 1/2" 1	10779 ft-lb	0.595 (59	9%) D+S	L		411.1							
Shear	3160 lb	1' 3/4" 7	7943 lb	0.398 (40)%) D+S	L									
LL Defl inc	ch 0.053 (1/1309)	3'1 1/2" () 193 (I /360	0 275 (27	/%) S	I									
TL Defl inc	ch 0.107 (L/652)	3'1 1/2" ().290 (L/240) 0.368 (37	/%) D+S	L									
Desian No	otes		, , , , , , , , , , , , , , , , , , ,	, (,		1								
1 Provide s	support to prevent late	eral movement	t and rotation	at the end	bearings. Lat	teral support	1								
may also	be required at the int	terior bearings	by the build	ing code.											
2 Fasten a 12"	Il plies using 2 rows o	f SDW22338 a	at 24" o.c. M	aximum en	d distance no	t to exceed									
3 Refer to I	last page of calculatio	ons for fastene	rs required f	or specified	loads.										
4 Simpson	fasteners applied from	m a single side	e of the mem	nber use tip	values where	published.									
5 Girders a	are designed to be sup	pported on the	bottom edg	e only.											
6 lop loads	s must be supported e	equally by all p t and boarings	olies.												
8 Bottom n	nust be laterally braced a	ed at end bearings	nas.												
9 Lateral s	lenderness ratio base	d on single ply	/ width.												
ID	Load Type	L	ocation	Frib Width	Side	Dead 0.9		Live 1	Snov	w 1.15	Wind	1.6 Const. 1	.25 Com	ments	
1	Uniform				Тор	761 PLF		0 PLF	70	61 PLF	0 F	PLF 0	PLF A2		
	Self Weight					7 PLF									
Notes		chemica	ls		6. For	flat roofs provide	proper dra	nage to p	revent	Manufac	turer Info				
Calculated Structu structural adequad	red Designs is responsible only cy of this component based of	of the Handling	g & Installation ms must not be current.	n t or drilled	pono	aniy				Metsä Wo 301 Merri	ood tt 7 Buildin	a. 2nd Floor			
design criteria responsibility of th	and loadings shown. It is he customer and/or the contract	s the 2. Refer	to manufacturer g installation	's product in requirements	formation multi-plv					Norwalk,	CT 06851	3, 2 1 1001			
ensure the com application, and to	ponent suitability of the int verify the dimensions and loads	ended fastening approva	g details, beam s ls	trength values, a	and code					(800) 622 www.met	-5850 sawood.co	m/us			
Lumber	nditions, unless noted otherwise	 Damage Design a 	d Beams must not assumes top edge	be used is laterally restrai	ned										
2. LVL not to be t	treated with fire retardant or cor	rosive 5. Provide lateral di	splacement and ro	bearing points	το avoid Thi	s design is valio	d until 6/	28/2026							

		Client: W	eaver Homes		Date:	1/24/2025	Page 12 of 1
2		Project:			Input by:	Curtis Quick	1 490 12 01 1
Ĩ	isDesign	Address:			Job Name:	The Lauren H Beams	
-					Project #:		
RM3	Korto-SIV	/ 1 750")	(9250"2	Ply - PASS		evel: Level	
			X J.230 Z	-1 19 - 1 AOC			
	_	_		_			
	•	•		•	•	.2	N/N/I I
						~	9 1/4
	•	•		•	• —	<u> </u>	$/ \vee \vee$
18	SPF End Grain 0-3-8			2 SPF End G	rain 0-3-8		
		6	'3"				3 1/2"
∤		6	'3"			\rightarrow	
						'	
Multi-Pl	y Analysis						
Fasten al	I plies using 2 rows	of SDW22338 at 24	' o.c Maximum er	d distance not to	exceed [·]	12".	
Capacity		0.0 %					
Load	F	0.0 PLF					
Yield Limit p	ber Fool	255.0 PLF 255.0 lb.					
См		1					
Yield Mode		Lookup					
Edge Distar Min, End Di	nce	1 1/2" 6"					
Load Combi	ination	0					
Duration Fa	ctor	1.00					
					- I -	•	1
Notes	atured Depleter is seen at the second	chemicals	6. For fla pondine	roofs provide proper drainage	to prevent	nanutacturer Into	4
Calculated Stru structural adeq	ictured Designs is responsible only of t quacy of this component based on t	he 1. LVL beams must not be cut or	drilled		N B	//etsa vvood /01 Merritt 7 Building, 2nd Floor	
responsibility of ensure the content	a and loadings shown. It is t if the customer and/or the contractor component suitability of the intend	to regarding installation reg	product information uirements, multi-ply		N	lorwalk, CT 06851 800) 622-5850	
application, and	to verify the dimensions and loads.	 rastening details, beam stren approvals Damaged Boards must not be 	gui values, and code		v	/ww.metsawood.com/us	
1. Dry service	conditions, unless noted otherwise	2. Design assumes top edge is la 5. Provide lateral support at b	terally restrained				
2. LVL not to b	be treated with fire retardant or corrosi	ve lateral displacement and rotati	on This	lesian is valid until 6/28/2	026		

F	Client: Weaver Homes	Date: 1/24/2025 Page 13 of 1
M	^o roject:	Input by: Curtis Quick
isDesign A	Address:	Job Name: The Lauren H Beams
		Project #:
BM4 S-P-F #2 2.00	0" X 10 000" 2-Ply - P	PASSED Level
	$\frac{1}{2}$	
•••••••••••••••••••••••••••••••••••••••		*****
	1	
•	•	• 1
and the second	and alter the state	9 1/4"
	2	
1 3FF 0-3-0	۲.	
	5'8 1/2"	1 13"
1	5'8 1/2"	<u> </u>
iviember information	1	
Type: Girder	Application: Floor	Brg Direction Live Dead Snow Wind Const
Plies: 2	Design Method: ASD	1 Vertical 0 782 782 0 0
Deflection LL 260	Building Code: IBC 2012	2 Vertical 0 782 782 0 0
Deflection LL: 360	Load Sharing: No	
Importance: Normal - II	Deck. Not Checked	
Temperature: Temp <= 100°F		
		Bearings
		Bearing Length Dir Can React D/Lib Total Ld Case Ld Comb
		1 - SPE 3 500" Vert 35% 782 / 782 1564 L D+S
		2 SPE 3 500" Vert 35% 782 / 782 1564 L D+S
Analysis Results		
Analysis Actual Location A	Allowed Capacity Comb. Case	
Moment 1888 ft-lb 2'10 1/4" 3	3946 ft-lb 0.478 (48%) D+S L	
Unbraced 1888 ft-lb 2'10 1/4" 3	3629 ft-lb 0.520 (52%) D+S L	
Shear 1404 lb 1' 3/4" 2	2872 lb 0.489 (49%) D+S L	
LL Defl inch 0.017 (L/3726) 2'10 1/4" 0).175 (L/360) 0.097 (10%) S L	
TL Defl inch 0.034 (L/1863) 2'10 1/4" 0).262 (L/240) 0.129 (13%) D+S L	
Design Natas		-
Design Notes	t and rotation at the and hearings. Lateral support	4
may also be required at the interior bearings	by the building code.	
2 Fasten all plies using 2 rows of SDW22300 a	at 24" o.c. Maximum end distance not to exceed	
12". 2 Poter to last page of calculations for fastone	re required for specified leads	
4 Simpson fasteners applied from a single side	e of the member use tip values where published.	
5 Girders are designed to be supported on the	bottom edge only.	
6 Top must be laterally braced at end bearings	<i>i.</i>	
7 Bottom must be laterally braced at end bear	ngs.	
	ocation Trib Width Side Deed 0.0	Live 1 Snow 115 Wind 16 Const 125 Commonts
1 Uniform	Far Face 274 PLF	.F 0 PLF 274 PLF 0 PLF 0 PLF A4
		Manufacturer Info

BM4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED Level Image: second s	isDesign	Client: Weaver Homes Project: Address:	Date: 1/24/2025 Input by: Curtis Quick Job Name: The Lauren H Beams Project #:	Page 14 of 1
I SPF 0-3-8 2 SPF 0-3-8 1 SPF 0-3-8 2 SPF 0-3-8 5/8 1/2"	BM4 S-P-F #	² 2.000" X 10.000"	2-Ply - PASSED	
Image: Second				
I SPF 0.3.8 2 SPF 0.3.8 58 1/2" fst 1/2" Multi-Ply Analysis Fasten all plies using 2 rows of SDW22300 at 24" o.c Maximum end distance not to exceed 12". Capacity 93.4 % Load 274.0 PLF Yield Limit per Foot 293.3 PLF Yield Limit per Foot 293.3 ab. Cw 1 Yield Limit per Fastener 293.3 lb. Cw 1 Yield Limit per Foot 11/2" Min. End Distance 6" Load combination D+S Duration Factor		•	▲11/2"	9 1/4"
S8 1/2 1 1 15 Multi-Ply Analysis 58 1/2" 1 15 Fasten all plies using 2 rows of SDW22300 at 24" o.c Maximum end distance not to exceed 12". 1 1 Capacity 93.4 % 1 1 Load 274.0 PLF 1 1 Yield Limit per Foot 293.3 PLF 1 1 Yield Limit per Fastener 293.3 lb. C 1 CM 1 1 1 1 Yield Mode Lookup Edge Distance 1 1/2" 1 Min. End Distance 6" 1 1 1 Load Combination D+S Duration Factor 1.15 1	1 SPF 0-3-8	EI0 4/01	2 SPF 0-3-8	
Multi-Ply Analysis Fasten all plies using 2 rows of SDW22300 at 24" o.c Maximum end distance not to exceed 12". Capacity 93.4 % Load 274.0 PLF Yield Limit per Foot 293.3 PLF Yield Limit per Fastener 293.3 Ib. CM 1 Yield Mode Lookup Edge Distance 1 1/2" Min. End Distance 6" Load Combination D+S Duration Factor 1.15		5'8 1/2"		J
Capacity 93.4 % Load 274.0 PLF Yield Limit per Foot 293.3 PLF Yield Limit per Fastener 293.3 lb. Cm 1 Yield Mode Lookup Edge Distance 1 1/2" Min. End Distance 6" Load Combination D+S Duration Factor 1.15	Multi-Ply Analysis	rows of SDW22300 at 24" o.c. Maxi	mum and distance not to avceed 12"	
Load274.0 PLFYield Limit per Foot293.3 PLFYield Limit per Fastener293.3 lb.Cm1Yield ModeLookupEdge Distance1 1/2"Min. End Distance6"Load CombinationD+SDuration Factor1.15		93.4 %		
Yield Limit per Foot293.3 PLFYield Limit per Fastener293.3 lb.Cm1Cm1Yield ModeLookupEdge Distance1 1/2"Min. End Distance6"Load CombinationD+SDuration Factor1.15	Load	274.0 PLF		
Yield Limit per Fastener293.3 lb.Cm1Yield ModeLookupEdge Distance1 1/2"Min. End Distance6"Load CombinationD+SDuration Factor1.15	Yield Limit per Foot	293.3 PLF		
Cm1Yield ModeLookupEdge Distance1 1/2"Min. End Distance6"Load CombinationD+SDuration Factor1.15	Yield Limit per Fastener	293.3 lb.		
Yield ModeLookupEdge Distance1 1/2"Min. End Distance6"Load CombinationD+SDuration Factor1.15	См	1		
Edge Distance1 1/2"Min. End Distance6"Load CombinationD+SDuration Factor1.15	Yield Mode	Lookup		
Min. End Distance 6" Load Combination D+S Duration Factor 1.15	Edge Distance	1 1/2"		
Load Combination D+S Duration Factor 1.15	Min. End Distance	6" D. C		
	Load Compination	D+S 1 15		
	Duration Factor	1.15		

This design is valid until 6/28/2026

Manufacturer Info





							Beam Legend						
					PlotID	Length	Product	Plies	Net Qty	Fab Type			
					BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF			
	All Truss Reactions are Le				BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF			
		Otherwise.		BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF				
= Denotes Left Er	nd of Iruss				GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF			
(Reference Engineered	l Truss Drawing) () Denotes Reaction Greater	than 3,000 lbs.	Truss Placement Plan	GDH (Side Load)	22' 0"	1-3/4"x 18" LVL Kerto-S	3	3 FF				
Do Not Erect Truss	es Backwards			SCALE: 3/16" = 1'	BM4	6' 0"	2x10 SPF No.2	2	2	FF			
LOAD CHART FOR JACK STUDS (BASED ON TABLES R502.5(1) & (b))	BUILDER	Weaver Homes, Inc.	CITY / CO.	Lillington / Harnett		THIS IS A TRUSS These trusses are det the building design at sheets for each truss	PLACEMENT DIAGRAM ONLY. signed as individual building components to be incorporated into the specification of the building designer. See individual design design identified on the placement drawing. The building designer						
	JOB NAME	Lot 3A Elbridge Farm	ADDRESS 90 Larime Lane		is responsible for temporary and permanent bracing of the roof and floor system and for the overall structure. The design of the truss support structure including headers, beams, walls, and columns is the responsibility of the building designer. For general guidance regarding bracing, consult BCSI-B1 and BCSI-B3 provided with the truss delivery package			com	гесн				
на						or online @ sbcindust	ry.com						

END REAC (UP TC (UP TC (2) PLY HE	END REACT (UP TC REQ'D STUI (3) PLY HE	END REAC (UP TC (UP TC (4) PLY HE	PLAN	Lauren H / 3 Car / SL / Tudor	MODEL	Roof	or online @ sbcindustry.com Bearing reactions less than or equal to 3000# are deemed to comply with the prescriptive Code requirements. The contractor shall refer to the attached Tables	ROOF & FLOOR
1700 1 3400 2 5100 3	2550 1 5100 2 7650 3	3400 1 6800 2 10200 3	SEAL DATE	2/24/20	DATE REV.	01/08/25	(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those	TRUSSES & BEAMS
6800 4 8500 5 10200 6	10200 4 12750 5 15300 6	13600 4 17000 5	QUOTE #	Quote #	DRAWN BY	Curtis Quick	specified in the attached Tables. A registered design professional shall be retained to design the support system for all reactions that exceed 15000#.	Fayetteville, N.C. 28309 Phone: (910) 864-8787
13600 7 13600 8 15300 9			JOB #	J0125-0034	SALES REP.	Lenny Norris	Curtis Quick	Fax: (910) 864-4444





Curtis Quick

Curtis Quick

Signature

						Beam Legend					
					PlotID	Length	Product	Plies	Net Qty	Fab Type	
					BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
		All Truss Reactions are	Less		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
		than 3,000 lbs. Unless Noted	Otherwise.		BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF	
= Denotes Left Er	nd of Iruss				GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF	
(Reference Engineered	Truss Drawing) Denotes Reaction Greater	than 3,000 lbs.	Truss Placement Plan	GDH (Side Load)	22' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF	
Do Not Erect Truss	es Backwards			SCALE: 3/16" = 1'	BM4	6' 0"	2x10 SPF No.2	2	2	FF	
LOAD CHART FOR JACK STUDS (BASED ON TABLES B502.5(1) & (b))	BUILDER	Weaver Homes, Inc.	CITY / CO.	Lillington / Harnett		THIS IS A TRUSS These trusses are det the building design at sheets for each truss	PLACEMENT DIAGRAM ONLY. signed as individual building components to be incorporated into the specification of the building designer. See individual design design identified on the placement drawing. The building designer				
	JOB NAME	Lot 3A Elbridge Farm	ADDRESS	90 Larime Lane		the overall structure. walls, and columns is regarding bracing, col	prorary and permanent bracing of the roof and floor system and for The design of the truss support structure including headers, beams, the responsibility of the building designer. For general guidance sult BCSI-B1 and BCSI-B3 provided with the truss delivery package		com	гесн	
END REAL (UP 1 (2) PLY H (2) PLY H (2) PLY H (2) PLY H (2) PLY H (2) PLY H (3) PLY H (3) PLY H (4) PLY H	PLAN	Lauren H / 3 Car / SL / Tudor	MODEL	Roof		Bearing reactions le prescriptive Code re	ry.com ess than or equal to 3000# are deemed to comply with the equirements. The contractor shall refer to the attached Table	s	ROOF & FLOO		
1/00 1 2550 1 3400 1 3400 2 5100 2 6800 2 5100 3 7650 3 10200 3	SEAL DATE	2/24/20	DATE REV.	01/08/25		(derived from the p foundation size and than 3000# but not g be retained to design	rescriptive Code requirements) to determine the minimum number of wood studs required to support reactions greate greater than 15000#. A registered design professional shall in the support system for any reaction that exceeds those		RUSSES	& BEAMS	
6800 4 10200 4 13600 4 8500 5 12750 5 17000 5	4 10200 4 5 12750 5 17000 5 QUOTE # Quote # DRAWN BY Curtis Quick		Curtis Quick		specified in the atta retained to design the	ched Tables. A registered design professional shall be he support system for all reactions that exceed 15000#.		Fayetteville,	N.C. 28309		

Curtis Quick

Lenny Norris

DRAWN BY

SALES REP.

15300 6

QUOTE #

JOB #

Quote #

J0125-0034

Reilly Road Industrial Park Fayetteville, N.C. 28309 Phone: (910) 864-8787 Fax: (910) 864-4444

