PLANS DESIGNED TO THE **2018 NORTH CAROLINA STATE RESIDENTIAL BUILDING CODE**

HEIGHT TO RIDGE: 27'-5'

MEAN ROOF HEIGHT: 19'-9'

Harnett /30/2025

REV1

CLIMATE ZONE ZONE 3A ZONE 4A ZONE 5A 0.35 0.35 0.55 0.55 0.30 0.30 0.35 0.55 0.30 38 or 30ci 38 or 30ci 38 or 30ci BASEMENT WALL R-VALUE * SLAB R-VALUE 10/15 * CRAWL SPACE WALL R-VALUE 5/13 10/15 10/19 "10/13" MEANS R-10 SHEATHING INSULATION OR R-13 CAVITY INSULATION ** INSULATION DEPTH WITH MONOLITHIC SLAB 24" OR FROM INSPECTION GAP TO BOTTOM OF FOOTING; INSULATION DEPTH WITH STEM WALL SUB 24" OR TO BOTTOM OF FOUNDATION WALL DESIGNED FOR WIND SPEED OF 120 MPH. 3 SECOND GUST (93 FASTEST MILE) EXPOSURE "B" datadtrom mus deata tarteny: Statuta and to bris hail paradet s COMPONENT & CLADDING DESIGNED FOR THE FOLLOWING LOADS MEAN ROOF UP TO 30 30°11°TO 35 35°1°TO 40'1 40°1°TO 45' ZONE 1 142, 1-15,0 1149, 1-15,8 115,5 1-6,4 15,9 1-6,8 ZONE 2 14,2 1-18,0 14,9 1-18,9 115,5 1-6,4 15,9 1-6,8 ZONE 2 14,2 1-18,0 14,9 1-18,9 115,5 1-9,6 15,9 20,2 ZONE 3 14,2 1-18,0 14,9 1-18,9 15,5 1-9,6 15,9 20,2

ZONE 4	15.5	-16.0	16.3	-16.8	16.9	-17.4	17.4	-17.9	
ZONE 5	15.5	-20.0	16.3	-21.0	16.9	-21.8	17.4	-22.4	
DESIGNED FOR WIND SPEED OF 130 MPH, 3 SECOND GUST (101 FASTEST MILE) EXPOSURE "B"									
COMPONENT	* & CLA	DDING	DESIG	NED FC	OR THE	FOLLO	WING I	LOADS	
MEAN ROOF	UP T	'O 30'	30'-1"	TO 35'	35'-1"	TO 40'	40'-1"	TO 45'	
ZONE 1	16.7	-18.0	17.5	-18.9	18.2	-19.6	18.7	-20.2	
ZONE 2	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5	
ZONE 3	16.7	-21.0	17.5	-22.1	18.2	-22.9	18.7	-23.5	
ZONE 4	18.2	-19.0	19.1	-20.0	19.8	-20.7	20.4	-21.3	
ZONE 5	18.2	-24.0	19.1	-25.2	19.8	-26.2	20.4	26.9	

GUARD RAIL NOTES

SECTION R312

R312.1 Where required. Guards shall be located along open-sided walking surfaces, including stairs, ramps and landings, that are located more than 30 inches (762 mm) measured vertical to the floor or grade below at any point within 36 inches (914 mm) horizontally to the edge of the open side. Insect screening shall not be considered as a *guard*

R312.2 Height, Required guards at open-sided walking surfaces, including stairs, porches, balconies or landings, shall be not less than 36 inches (914) mm) high measured vertically above the adjacent walking surface, adjacent fixed seating or the line connecting the leading edges of the treads. Exceptions:

1. Guards on the open sides of stairs shall have a height not less than 34 inches (864 mm) measured vertically from a line connecting the leading edges of the treads

2. Where the top of the guard also serves as a handrail on the open sides of 2. Where the top of the guard also serves as a natural of the open sides of stairs, the top of the guard shall not be not less than 34 inches (864 mm) and not more than 38 inches (965 mm) measured vertically from a line connecting the leading edges of the treads.

R312.3 Opening limitations. Required quards shall not have openings from the walking surface to the required guard height which allow passage of a sphere 4 inches (102 mm)in diameter.

Exceptions: 1. The triangular openings at the open side of a stair, formed by the riser, tread and bottom rail of a guard, shall not allow passage of a sphere 6 inches (153 mm) in diameter.

 Guards on the open sides of stairs shall not have openings which allow passage of a sphere 4 3/8 inches (111 mm) in diameter.

ROOF VENTILATION

SECTION R806

STONE FOUNDATION

HEIGHT ALL THE WA

1x10

AROUND THE HOUSE

SQUARE FOOTAGE OF ROOF TO BE VENTED = 2,477 SQ.FT. NET FREE CROSS VENTILATION NEEDED: WITHOUT 50% TO 80% OF VENTING 3'-0" ABOVE EAVE = 16.51 SQ FT. WITH 50% TO 80% OF VENTING 3'-0" ABOVE EAVE; OR WITH CLASS I OR II VAPOR RETARDER ON WARM-IN-WINTER SIDE OF CELLING = 8.26 SO FT

12



RIGHT SIDE ELEVATION

SCALE 1/8" = 1'-0"

2/24/2020

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PAGE 1 OF 7

LEFT SIDE ELEVATION SCALE 1/8" = 1'-0'

XXXXXX





STRUCTURAL NOTES

A construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus all the 2018 North Caroina residential building code, prus an local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans,

Inc. assumes no liability for contractors practices and procedures or safety program. Haynes Home Plans, Inc. takes no responsibility for the contractor's failure to carry out the construction work in accordance with the contract documents. All members shall be framed, anchored, and braced in accordance with good construction practice and the building code.

		-	-
DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10	10	L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200		-
Guardrail in-fil components	50		-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40		L/360
Snow	20		

LADDER FRAMED

noted otherwise.

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise

ENGINEERED WOOD BEAMS :

ENGINEERED WOUD BEARS: Laminated venera lumber (UVL) = F0=2600 PSI, Fv=265 PSI, E=1.9x106 PSI Parallel strand lumber (ISL) = F0=2500 PSI, Fv=290 PSI, E=2.0x106 PSI Laminated strand lumber (ISL) F0=2250 PSI, Fv=400 PSI, E=1.55x106 PSI Instal all connections per manufactures instructions. TRUSS AND I-JOIST MEMBERS: All roof truss and I-joist

layouts shall be prepared in accordance with this document. Trusses and L-joists shall be installed according to the manufacture's specifications. Any change in truss or I-joist layout shall be coordinated with Haynes Homes Plans, Inc. LINTELS: Brick lintels shall be 3 1/2" x 3 1/2" x 1/4" steel angle for up to 6'-0" span. 6" x 4" x 5/16" steel angle with 6" leg vertical for spans up to 9'-0" unless noted otherwise. 3 1/2" x 3 1/2" x 1/4" steel angle with 1/2" bolts at 2-0" on center for spans up to 18'-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 16" on center joist spacing, minimum 5/8" thick for 19.2" on center joist spacing, and minimum 3/4" ROOF SHEATHING: OSB or CDX roof sheathing minimum 3/8" thick for 16" on center rafters and 7/16" for 24" on center rafters. CONCRETE AND SOILS: See foundation notes.



ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Havnes Home Plan, Inc. attention before construction begins. KNEE WALL AND CEILING HEIGHTS. All finished knee wall heights and ceiling heights are shown furred down 10" from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated heel heights, finished knee wall heights, or finished ceiling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins. Any variation due to these conditions not being met is the reasonability of the truss manufacturer. ANCHORAGE. All required anchors for trusses due to uplift or bearing

shall meet the requirements as specified on the truss schematics. BEARING, All trusses shall be designed for bearing on SPF #2 plates or edgers unless noted otherwise.

Plate Heights & Floor Systems. See elevation page(s) for plate heights and floor system thicknesses.



URCHASER MUST VERIFY A

HAYNES HOME PLANS, INC.

SUMES NO LIABILITY FOR

PROCEDURES. PROLEDURES CODES AND CONDITIONS MAY VARY WITH LOCATION. A LOCA DESIGNER, ARCHITECT OR VGINEER SHOULD BE CONSULTI BEFORE CONSTRUCTION.

THESE DRAWING ARE INSTRUMENTS OF SERVICE AN AS SUCH SHALL REMAIN PROPERTY OF THE DESIGNER

Т

Lauren

The

SQUARE FOOTAGE

FRONT PORCH 188 SQ F GARAGE 488 SQ F TOTAL 676 SQ F UNHEATED OPTIONAL

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2/24/2020

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Haynes Home Plans, Inc

HEATED OPTIONAL

UNHEATED

1766 SQ FT 400 SQ FT

148 50 FT 304 50 FT 452 SQ FT

160 SQ FT 108 SQ FT 292 SQ FT

FIRST FLOOR STRUCTURAL

3CG

ROOF TRUSS REQUIREMENTS

TRUSS DESIGN. Trusses to be designed and engineered in accordance with these drawings. Any variation with these drawings must be brought to Haynes Home Plan, Inc. attention before construction begins. KNEE WALL AND CELING HEIGHTS, All finished knee wall heights and celling heights are shown furred down 10° from roof decking for insulation. If for any reason the truss manufacturer fails to meet or exceed designated hele heights, finished knee wall heights, and finished knee wall heights, or finished celling heights shown on these drawings the finished square footage may vary. Any discrepancy must be brought to Haynes Home Plans, Inc. attention, so a suitable solution can be reached before construction begins, Any variation due to these conditions not being met is the reasonability of the truss manufacturer. ANCHORAGE, All required anchors for trusses due to uplift to Planes for

ANCHORAGE. All required anchors for trusses due to uplift or bearing shall meet the requirements as specified on the truss schematics. BEARING. All trusses shall be designed for bearing on SPF #2 plates or

ledgers unless noted otherwise. Plate Heights & Floor Systems. See elevation page(s) for plate heights

and floor system thicknesses.

STRUCTURAL NOTES

All construction shall conform to the latest requirements of the 2018 North Carolina Residential Building Code, plus al local codes and regulations. This document in no way shall be construed to supersede the code. JOB SITE PRACTICES AND SAFETY: Haynes Home Plans, Inc. assumes no liability for contractors practices and procedures or safety program. Haves

Iability for contractors practices and procedures or safety program. Have Home Flans, Inc. Lakes no responsibility for the contractor's failure the orary out the construction work in accordance with the contract documents. All members shall be franded, and hored an accordance with good construction practice and the building code.

DESIGN LOADS	LIVE LOAD	DEAD LOAD	DEFLECTION
USE	(PSF)	(PSF)	(LL)
Attics without storage	10		L/240
Attics with limited storage	20	10	L/360
Attics with fixed stairs	40	10	L/360
Balconies and decks	40	10	L/360
Fire escapes	40	10	L/360
Guardrails and handrails	200	-	-
Guardrail in-fill components	50	-	-
Passenger vehicle garages	50	10	L/360
Rooms other than sleeping	40	10	L/360
Sleeping rooms	30	10	L/360
Stairs	40	-	L/360
Snow	20	-	-

FRAMING LUMBER: All non treated framing lumber shall be SPF #2 (Fb = 875 PSI) or SYP #2 (Fb = 750 PSI) and all treated lumber shall be SYP #2 (Fb = 750 PSI) unless noted other wise.

ENGINEERED WOOD BEAMS :

Enclinetate where fumber (VL) = Fb=2600 PSI, Fv=285 PSI, E=1.9x10⁶ PSI branelist strand lumber (PSI) = Fb=2600 PSI, Fv=209 PSI, E=2.0x10⁶ PSI laminated strand lumber (LSI) = Fb=2200 PSI, Fv=209 PSI, E=2.0x10⁶ PSI Instal all connections per manufacturers instructions. **TRUSS AND** -10SIT **IMEMERS**: All roof truss and Fjoid layouts shall be

IRUSS AND I-JOIST HEMBERS: All root rules and i-joist ayouts shall be prepared in according to the manufacture's specifications. Any change in truss or i-joist layout shall be conditioned with haynes shores Plans, Inc. LINTELS, Brick lintels shall be 31/2" x3 1/2" x 1/4" steel angle for up to 6-0" span. 6" x4" x5/16" steel angle with 6 "lay extincial for spans up to 9-0" unless noted otherwise. 31/2" x3 1/2" x 1/4" steel angle with 1/2" boost at 2-0" on center for spans up to 18-0" unless noted otherwise. FLOOR SHEATHING: OSB or CDX floor sheathing minimum 1/2" thick for 24" on center joist spacing, and minimum 3/4" thick for 24" on center rafters. CORCEFE FLANDE: OSIS 125: See for of sheathing minimum 3/8" thick for 16" on center joist spacing. ROOF SHEATHING: SOBS ICEX mod sheathing minimum 3/8" thick for 16" on center points possible space and the source for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16" on center for sheathing minimum 3/8" thick for 16".

ATTIC ACCESS

SECTION R807

R807.1 Attic access An attic access opening shall be provided to attic areas that exceed 400 square feet (37.16 m2) and have a vertical height of 60 inches (1524 mm) or greater. The net dear opening shall not be less than 20 inches by 30 inches (306 mb y 762 mm) and shall be located in a halway or other readily accessible location. A 30-inch (762 mm) minimum unobstructed headroom in the attic space shall be provided at some point above the access opening. See Section M1305.1.3 for access requirements where mechanical equipment is located in attics.

 Concealed areas not located over the main structure including porches, areas behind knee walls, dormers, bay windows, etc. are not required to have access.
 Pull down stair treads, stringers, handrails, and hardware may protrude into the net clear opening.

EXTERIOR HEADERS

- (2) 2 X 6 WITH 1 JACK STUD EACH END UNLESS NOTED OTHERWISE - KING STUDS EACH END PER TABLE BELOW HEADER SPAN < 3' 3'-4' 4'-8' 8'-12' 12'-16'

KING STUD(S) 1 2 3 5 6 INTERIOR HEADERS

- LOAD BEARING HEADERS (2) 2 X 6 WITH

1 JACK STUD AND 1 KING STUD EACH END UNLESS NOTED OTHERWISE - NON LOAD BEARING HEADERS TO BE LADDER FRAMED















							Beam Legend			
					PlotID	Length	Product	Plies	Net Qty	Fab Type
		All Truss Reactions are I	ess		BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
		than 3,000 lbs. Unless Noted ()+hanwisa		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
A - Deneted Laft En	d of Tours		the wise.		BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
= Denotes Lett End		Nenotes Peaction Greater t	han 3 000 lbs		GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
(Reference Engineered	Truss Drawing)		nun 5,000 ibs.	Truss Placement Plan	GDH (Side Load)	22' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF
Do Not Erect Trusse	s Backwards			SCALE: 3/16" = 1'	BM4	6' 0"	2x10 SPF No.2	2	2	FF
LOAD CHART FOR JACK STUDS (BASED ON TABLES P502.5(1) & (b))	BUILDER	Weaver Homes, Inc.	CITY / CO.	Lillington / Harnett		THIS IS A TRUSS These trusses are de the building design at sheets for each truss	PLACEMENT DIAGRAM ONLY. signed as individual building components to be incorporated into the specification of the building designer. See individual design design identified on the placement drawing. The building design	o		
HEADER A DATA TO COLOR A DATA	JOB NAME	Lot 3A Elbridge Farm	ADDRESS	31 Griffin Rd.		is responsible for tem the overall structure. walls, and columns is regarding bracing, co	porary and permanent bracing of the roof and floor system and The design of the truss support structure including headers, bea the responsibility of the building designer. For general guidance nsult BCSI-B1 and BCSI-B3 provided with the truss delivery pack	for ms, e kage	con	птесн
E END REA (UP 11) (UP 12) (2) PLY FL (2) PLY FL (2) PLY FL (3) PLY FL (3) PLY FL (3) PLY FL	PLAN	Lauren H / 3 Car / SL / Tudor	MODEL	Roof		Bearing reactions le	excome ess than or equal to 3000# are deemed to comply with the equirements. The contractor shall refer to the attached Ta	bles	ROOF	& FLOOR
1700 1 2550 1 3400 1 3400 2 5100 2 6800 2 5100 3 7650 3 10200 3		2/24/20	DATE REV.	06/10/25		(derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greate than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those			Reilly Boar	S & BEAMS
6800 4 10200 4 13600 4 8500 5 12750 5 17000 5 10200 6 15300 6 6 6	QUOTE #	Quote #	DRAWN BY	Curtis Quick	1	specified in the atta etained to design t	ched Tables. A registered design professional shall be he support system for all reactions that exceed 15000#.		Fayettev	ille, N.C. 28309
11900 7 13600 8 15300 9	JOB #	J0125-0034	SALES REP.	Lenny Norris		Signature Curtis Quick			Fax: (910) 864-4444	





							Beam Legend			
					PlotID	Length	Product	Plies	Net Qty	Fab Type
		All Truss Reactions are L	ess		BM1	8' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
		than 3 000 lbs. Unless Noted C)therwise		BM3	7' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
A - Donator Loft End of Tru	200				BM2	6' 0"	1-3/4"x 9-1/4" LVL Kerto-S	2	2	FF
- Denotes Lettend of The		Denotes Peaction Greater t	han 3 000 lbs		GDH-1	14' 0"	1-3/4"x 11-7/8" LVL Kerto-S	2	2	FF
(Reference Engineered Truss L	Drawing)		Truss Placement Plan		GDH (Side Load)	22' 0"	1-3/4"x 18" LVL Kerto-S	3	3	FF
Do Not Erect Trusses Backw	ƙwards			SCALE: 3/16" = 1'	BM4	6' 0"	2x10 SPF No.2	2	2	FF
LOAD CHART FOR JACK STUDS (BASED ON TABLES REQ2.5(1) & (b)) NUMBER OF JACK STUDS EQUIPED & EA END OF HEADER/GIRDER TO SO THE STUDS REQUIPED & EA END OF HEADER/GIRDER TO SO THE STUDS REQUIPED & TO SO THE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A STUDE AND A STUDE AND A STUDE TO SO THE STUDE AND A	LDER NAME	Weaver Homes, Inc. Lot 3A Elbridge Farm	CITY / CO. ADDRESS	Lillington / Harnett 31 Griffin Rd.		THIS IS A TRUSS These trusses are de the building design at sheets for each truss is responsible for tem the overall structure. walls, and columns is regarding bracing, co or online @ sbcindust	PLACEMENT DIAGRAM ONLY. signed as individual building components to be incorporated int the specification of the building designer. See individual design design identified on the placement drawing. The building design porary and permanent bracing of the roof and floor system and The design of the truss support structure including headers, bea the responsibility of the building designer. For general guidance sult BCSI-B1 and BCSI-B3 provided with the truss delivery pact ry.com	o ler for ims, e kage	con	птесн
	V	Lauren H / 3 Car / SL / Tudor	MODEL	Roof		Bearing reactions le	ess than or equal to 3000# are deemed to comply with the) hles	ROOF	& FLOOR
1700 1 2550 1 3400 1 3400 2 5100 2 6800 2 5100 3 7650 3 10200 3	2550 1 3400 1 5100 2 6800 2 7650 3 10200 3		DATE REV.	06/10/25		c) (derived from the prescriptive Code requirements) to determine the minimum foundation size and number of wood studs required to support reactions greater than 3000# but not greater than 15000#. A registered design professional shall be retained to design the support system for any reaction that exceeds those			TRUSSES & BEAMS	
6800 4 10200 4 13600 4 8500 5 12750 5 17000 5 10200 6 15300 6 11900 7	DTE #	Quote #	DRAWN BY	Curtis Quick		specified in the atta retained to design t Signature	ched Tables. A registered design professional shall be he support system for all reactions that exceed 15000#. Curfis Quick		Fayettev Phone: (ille, N.C. 28309 910) 864-8787
13600 8 15300 9 JOB 7	#	J0125-0034	SALES REP.	Lenny Norris		o.g	Curtis Quick		Fax: (91	10) 864-4444



isDesign	Client: Project: Address:	Weaver Homes		Date: Input by: Job Name Project #:	6/11/2025 Curtis Quick e: The Lauren H Beams	Page 2 of 1
GDH (Side Load)	Kerto-S LVL	1.750" X 18.00	00" 3-Ply -	PASSED	Level: Level	
		• •	· ·	•		· [7] [7]
1 SPF End Grain 0-3-8	•	• •	•	•	• •	
		18'1	0"			f5 1/4"
		18'1	10"			1
Multi-Ply Analysis						
Fasten all plies using 3	rows of SDW22500 a	t 24" o.c Maximum	end distance r	ot to exceed	12".	
Load Viald Limit per Feet	0.0 PLF					
Yield Limit per Foot Yield Limit per Fastener	255.0 lb.					
См Yield Mode	1 Lookup					
Edge Distance Min. End Distance	1 1/2" 6"					
Load Combination	1.00					
	1.00					
Notos	chemicale	6 E.	or flat roofs provide proper	drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsit structural adequacy of this component b	ble only of the Handling & Install	ation p	onding		Metsä Wood 301 Merritt 7 Building, 2nd Floor	1
design criteria and loadings shown. responsibility of the customer and/or the ensure the component suitability of	the intended	turer's product information on requirements, multi-ply			Norwalk, CT 06851 (800) 622-5850	
application, and to verify the dimensions an Lumber	nd loads. approvals 3. Damaged Beams mus	it not be used			www.metsawood.com/us	
 Dry service conditions, unless noted oth LVL not to be treated with fire retardant 	4. Design assumes top e 5. Provide lateral suppor lateral displacement a	ege is laterally restrained ort at bearing points to avoid nd rotation T	his design is valid until	6/28/2026		



		Client:	Weaver Homes		Date:	6/11/2025		Page 4 of 14
		Project:			Input by:	Curtis Quick		
isDes	sign	Address:			Job Nam	e: The Lauren H Beams		
	•				Project #			
		4 750			DACOED	Level: Level		
GDH-1 K	erto-5 LVL	. 1.750	X 11.8/5	2-Piy ·	- PASSED	2010 2010.		
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1 SPF End Gra	ain 0-3-8					2 SPF End Grain 0-3-8		1 1
			10'				1	3 1/2"
/ <u>/</u>			10'				ł	
			10				I	
	•••							
Multi-Ply Analys	5IS							
Fasten all plies us	sing 2 rows of S	DW22338 at 2	24" o.c Maximur	m end distand	ce not to exceed	d 12".		
Capacity)						
Load	0.0 P	, I F						
Yield Limit per Foot	255 (PIF						
Yield Limit per Fastene	r 255.0) lb						
CM	1							
Yield Mode	Look	au						
Edge Distance	1 1/2	.'						
Min. End Distance	6"							
Load Combination								
Duration Factor	1.00							
							-	
Notes		chemicals	6.	For flat roofs provide p	proper drainage to prevent	Manufacturer Info		
Calculated Structured Designs is	s responsible only of the Ha	andling & Installatio	on	ponding	U	Metsä Wood		
structural adequacy of this con design criteria and loadings	mponent based on the 1. Is shown. It is the 2	LVL beams must not be cu Refer to manufacture	ut or drilled r's product information			301 Merritt 7 Building, 2nd Floor		
responsibility of the customer a ensure the component suital	and/or the contractor to bility of the intended	regarding installation	requirements, multi-ply			(800) 622-5850		
application, and to verify the dim	ensions and loads.	approvals	the used			www.metsawood.com/us		
Lumber	3. 4.	Damaged Beams must no Design assumes top edge	t be used is laterally restrained					
 Dry service conditions, unles LVL not to be treated with fit 	re retardant or corrosive 5.	Provide lateral support a lateral displacement and r	at bearing points to avoid otation	This design is velic	1 until 6/28/2026			
1				THIS UCSIGHTS VAIIO	a untili U/20/2020	1	1	





Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	Handling & Installation 1. UVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-piy fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	



Multi-Ply Analysis

Fasten all plies using 2 rows of SDW22338 at 24" o.c.. except for regions covered by concentrated load fastening. Maximum

end distance not to exceed	12".
----------------------------	------

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	255.0 PLF	
Yield Limit per Fastener	255.0 lb.	
См	1	
Yield Mode	Lookup	
Edge Distance	1 1/2"	
Min. End Distance	6"	
Load Combination		
Duration Factor	1.00	

Concentrated Load

Fasten at concentrated side load at 2-0-0 with a

minimum of (4) – SDW22338 in the pattern shown.

All fasteners shall be installed with the head on the side of the applied load

side of the applied load.	
Capacity	77.2 %
Load	906.0lb.
Total Yield Limit	1173.0 lb.
Cg	1.0000
См	1
Yield Limit per Fastener	293.3 lb.
Yield Mode	Lookup
Load Combination	D+S
Duration Factor	1.15

Min/Max fastener distances for Concentrated Side Loads

This design is valid until 6/28/2026



Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Notes

Lumber

chemicals

3

5

Handling & Installation



Multi-Ply Analysis

Concentrated Load

Fasten at concentrated side load at 6-10-8 with a minimum of (4) – SDW22338 in the pattern shown. All fasteners shall be installed with the head on the side of the applied load.

Capacity	77.2 %	
Load	906.0lb.	
Total Yield Limit	1173.0 lb.	
Cg	1.0000	
См	1	
Yield Limit per Fastener	293.3 lb.	
Yield Mode	Lookup	
Load Combination	D+S	
Duration Factor	1.15	

Min/Max fastener distances for Concentrated Side Loads



Notes	chemicals	6. For flat roofs provide proper drainage to prevent	Manufacturer Info	
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive	Handling & Installation 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-phy fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation	ponding This design is valid until 6/28/2026	Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	

		C	lient:	Weaver Homes	5			Date:	6/11/20	25				Page 9 of 14
	Design	Р	roject:					Input by:	Curtis (Quick				
	Design	A	ddress:					Job Name	e: The La	uren H B	eams			
		/1 4	7501	V 0 05	<u></u>				Level: Lev	el				
BM2	Kerto-S L	VL 1	.750	X 9.25	0. 2	-Piy -	PAS	SED	LOVOI. LOV	01				
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													IXIX	0.1/
	a sitter			4	Fin France		-							9 1/-
													<u> </u>	
1 SPF	End Grain 0-3-8		<u> </u>		2 SPF E	nd Grain 0-3	3-8	,						
,			5'4"					,					I	3 1/2"
11			5'4"				1							
March	f a						P -	41	DATT	NICO !!				
Type:	Girder		Applicat	ion: Flo	or		Bro	Direction			Dead	Snow	Wind	Conet
Plies:	2		Design	Method: AS	D		1	Vertical	LIV	0	1942	1923	0	001130
Moisture Con	dition: Dry		Building	Code: IBC	C 2012		2	Vertical		0	1942	1923	0	0
Deflection LL:	360		Load Sh	aring: No										
Deflection TL:	: 240		Deck:	No	t Checked									
Importance:	Normai - II Temp <= 100°	· E												
remperature.	Temp <= 100	1					Bear	ings						
							Bea	ring Lengtl	h Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 - 5	SPF 3.500"	Vert	38%	1942 / 1923	3865	L	D+S
	•						End	'n						
Analysis Re	esults	1 ti A		O an a site :	Quest	0	2-5	SPF 3.500"	Vert	38%	1942 / 1923	3865	L	D+S
Analysis	Actual	Location A				Case	End							
Unbraced	4305 ft-lb	2'8" 1 [.]	1811 ft-lb	0.365 (36%)	D+S	L	Grai	n						
Shear	2330 lb	4'3 1/4" 7	943 lb	0.293 (29%)	D+S	L								
LL Defl inch	0.027 (L/2130)	2'8" 0	.162 (L/360	0.169 (17%)	S	L								
TL Defl inch	0.055 (L/1060)	2'8" 0	.244 (L/240) 0.226 (23%)	D+S	L								
Design Not	tes													
1 Provide su	pport to prevent later	al movement	and rotatio	n at the end bea	arings. Late	ral support	7							
2 Fasten all	plies using 2 rows of	SDW22338 a	t 24" o.c. N	laximum end di	stance not t	o exceed								
12".														
4 Simpson fa	st page of calculation	s for fastener	s required to of the mer	or specified loa	as. ues where r	oublished.								
5 Girders are	e designed to be supp	ported on the	bottom edg	e only.										
6 Top loads r	must be supported ec	ually by all pl	lies.											
8 Bottom mu	st be laterally braced at	at end bearings.	ngs.											
9 Lateral sler	nderness ratio based	on single ply	width.											
ID	Load Type	L	ocation	Trib Width	Side -	Dead 0.9	L	Live 1 Sno	ow 1.15	Wind ?	1.6 Const. 1	.25 Col	mments	
1	Uniform				юр	/21 PLF		UPLF 7	/21 PLF	0 P	'L⊢ OF	r∟⊦ A3		
	Self Weight					7 PLF								
Notes		chemicals	5		6. For fla	at roofs provide	proper drain	age to prevent	Manufactu	irer Info				
Calculated Structured structural adequacy	Designs is responsible only of of this component based on	the Handling	& Installation	on It or drilled	pondin	ig			Metsä Woo 301 Merritt	od 7 Building	1 2nd Floor	7		
design criteria and responsibility of the	d loadings shown. It is customer and/or the contracto	the 2. Refer to r to regarding	o manufacture installation	r's product informa requirements, mult	ition i-ply				Norwalk, C	T 06851	j, zna i 1001			
ensure the compor application, and to ve	nent suitability of the inten rify the dimensions and loads.	approvals	details, beam :	strength values, and o	ode				(800) 622- www.metsa	awood.cor	n/us			
1. Dry service condit	tions, unless noted otherwise	 Jamageo 4. Design as 5. Provide I 	ssumes top edge ateral support a	is laterally restrained at bearing points to a	void									
LVL not to be treat	ated with fire retardant or corro	sive lateral dis	placement and r	otation	This	design is vali	d until 6/28	8/2026						

isDesign	Client: Project Addres	Weaver Homes		Date: Input by: Job Nam Project #	6/11/2025 Curtis Quick e: The Lauren H Beams	Page 10 of 7
BM2 Kerto-S	LVL 1.7	50" X 9.250"	2-Ply -	PASSED	Level: Level	
•	•		•	<1 1/2"		9 1/
1 SPF End Grain 0-3-8	-	2	SPF End Grain 0-3	<u> </u>		
 	5' 5'	4" 4"				1 1/2"
Multi-Ply Analysis						
Fasten all plies using 2 r Capacity Load	ows of SDW22338 0.0 % 0.0 PLF	at 24" o.c Maxim	um end distan	ce not to exceed	d 12".	
Yield Limit per Foot Yield Limit per Fastener	255.0 PLF 255.0 lb.					
См Yield Mode	1 Lookup					
Edge Distance Min. End Distance	1 1/2" 6"					
Load Combination Duration Factor	1.00					
Notes	chemicals	tallation	6. For flat roofs provide ponding	proper drainage to prevent	Manufacturer Info	_
 carculated structured Designs is responsible structural adequacy of this component bas design criteria and loadings shown. responsibility of the customer and/or the customer and/or	ed on the It is the ontractor to It is che is the 1. LVL beams must is 2. Refer to man providenti in the contractor to contractor to	not be cut or drilled ufacturer's product information			301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851	
ensure the component suitability of the application, and to verify the dimensions and Lumber	e intended fastening details, oads. approvals 3. Damaged Beams	beam strength values, and code must not be used			(800) 622-5850 www.metsawood.com/us	
 Dry service conditions, unless noted other LVL not to be treated with fire retardant of 	wise 4. Design assumes 5. Provide lateral s lateral displacement	top edge is laterally restrained upport at bearing points to avoid ent and rotation	This design is vali	d until 6/28/2026		

		(lient [.]	Weaver Ho	mes			Da	te [.]	6/11/2	025				Page 11 of 1
-		F	Proiect:					Inp	ut by:	Curtis	Quick				. ago o
1 i	sDesign	ļ	Address:					Job	o Name	: The L	auren H E	leams			
								Pro	piect #:						
DM2	Karta C I		750"	VOO	E0" 4			000		_evel: Le	vel				
DIVI 3	Kerto-5 L	VL 1	1.750	X 9.2	50 4	2-Piy -	PA	3 35	ש:						
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	No Printe		100		alt in the					10				IAI/	9 1/
•			•			•			•					L V	
	F End Grain 0-3-8					2 5	SPF En	d Grain	∟ 0-3-8					I	
<i> </i>				6'3"						\rightarrow				/	3 1/2"
/				6'3"						\rightarrow					
										I					
Member I	nformation						Rea	ction				h (Unlift)			
	Header		Applicati	on:	Floor		Bra	Direc	ction	Li	ve	Dead	Snow	Wind	Const
Plies:	2		Design N	/lethod:	ASD		1	Vertic	cal		0	2391	2369	0	0
Moisture Co	ndition: Dry		Building	Code:	IBC 2012		2	Vertic	cal		0	2391	2369	0	0
Deflection LI	L: 360		Load Sh	aring:	No			vorue	Jui		0	2001	2000	Ũ	0
Deflection TI	L: 240		Header	Supports	No										
Importance:	Normal - II		Glass:												
Temperature	e: Temp <= 100	D°F	Deck:		Not Checked	ł									
							Bea	rings							
							Be	aring	Length	Dir.	Cap.	React D/L lb	Total	Ld. Case	Ld. Comb.
							1 -	SPF	3.500"	Vert	46%	2391 / 2369	4760	L	D+S
Analysia D							_ En Gra	d ain							
Analysis K	Actual	Lesstian (Allowed	Canaaita	(Camela		2-	SPF	3.500"	Vert	46%	2391 / 2369	4760	L	D+S
Analysis	Actual	Location A		Capacity		Case	En	d							
	6387 TT-ID	311/2	14423 π-ID	0.443 (44	1%) D+S	L .	Gra	ain							
Unbraced	6387 TT-ID	311/2	10779 π-ID	0.593 (55	9%) D+S	L .									
Snear	3147 ID	1.3/4. /	943 ID	0.396 (40	1%) D+S	L .									
LL Defi inci	n 0.053 (L/1314)	311/2" ().193 (L/360) 0.274 (27	(%) S	L									
IL Defl incl	h 0.106 (L/654)	3'1 1/2" ().290 (L/240) 0.367 (37	′%) D+S	L	-								
Design No	otes	and movement	and rotation	at the end	boarings I a	toral support	4								
may also	be required at the inf	terior bearings	by the build	ing code.	bearings. La	lerai support									
2 Fasten all	I plies using 2 rows o	f SDW22338 a	at 24" o.c. M	aximum en	d distance no	t to exceed									
12°. 3 Refer to la	ast page of calculatio	ons for fastene	rs required f	or specified	loads										
4 Simpson	fasteners applied from	m a single side	e of the men	ber use tip	values where	published.									
5 Girders a	re designed to be su	pported on the	bottom edg	e only.											
6 Top loads	s must be supported e	equally by all p	olies.												
8 Bottom m	be laterally braced a	d at end bearings	nas												
9 Lateral sle	enderness ratio base	d on single ply	/ width.												
ID	Load Type	L	ocation	Frib Width	Side	Dead 0.9		Live 1	Snov	w 1.15	Wind	1.6 Const. 1	.25 Com	nments	
1	Uniform				Тор	758 PLF		0 PLF	7	58 PLF	0 6	PLF 01	PLF A2		
	Self Weight					7 PLF									
	5														
Notes		chemica	ls		6. For	flat roofs provide	proper dra	inage to p	prevent	Manufac	turer Info		_		
Calculated Structure structural adequace	ed Designs is responsible only y of this component based of	of the Handling	g & Installation ms must not be cu	n t or drilled	pono	y				Metsä Werr	ood itt 7 Buildin	g, 2nd Floor			
design criteria a responsibility of the	and loadings shown. It is e customer and/or the contract	s the 2. Refer ctor to regardin	to manufacturer g installation	's product in requirements,	formation multi-ply					Norwalk,	CT 06851	•••••			
application, and to v	verify the dimensions and loads	approval	g details, beam s ls	trength values, a	and code					www.met	5650 sawood.co	m/us			
Lumber 1. Dry service con	ditions, unless noted otherwise	 Jamage Design a Provide 	assumes top edge	is laterally restrait	ned to avoid										
2. LVL not to be tr	reated with fire retardant or cor	rosive 5. Provide lateral di	splacement and ro	tation	Thi	s design is valio	d until 6/	28/2026							

	isDesign	Client: Weaver Homes Project: Address:		Date: 6/11/2025 Input by: Curtis Quick Job Name: The Lauren H Beams	Page 12 of 1
BM3	Kerto-S LV	L 1.750" X 9.250	" 2-Ply - PASS	SED Level: Level	
				I	
	•	•	•	•	
	•	•	•	•	9 1/2
	SPF End Grain 0-3-8		2 SPF End G	rain 0-3-8	
		6'3"			3 1/2"
/		6'3"		ł	
	h. Analusia				
Fasten al	l y Analysis Il plies using 2 rows c	of SDW22338 at 24" o.c Maxi	mum end distance not to	exceed 12".	
Capacity	0	.0 % 0 PI F			
Yield Limit p	per Foot 2	55.0 PLF			
Yield Limit p Cm	Der Fastener 2	55.U ID.			
Yield Mode Edge Distar	nce 1	ookup 1/2"			
Min. End Di	istance 6	"			
Duration Fa	ictor 1	.00			
Notes		chemicals	6. For flat roofs provide proper drainage	to prevent Manufacturer Info	
Calculated Stru structural adeo	uctured Designs is responsible only of the quacy of this component based on the	 Handling & Installation 1. LVL beams must not be cut or drilled 	ponding	Metsä Wood 301 Merritt 7 Building, 2nd Floor	
design criteria responsibility o ensure the c	a and loadings shown. It is the of the customer and/or the contractor to component suitability of the intended	2. Refer to manufacturer's product informatic regarding installation requirements, multi-p fastening details beam strength values and con-	n ly e	Norwalk, CT 06851 (800) 622-5850	
application, and Lumber	d to verify the dimensions and loads.	approvals 3. Damaged Beams must not be used 4. Decim assume to odo in Interally restained	-	www.metsawood.com/us	
1. Dry service 2. LVL not to I	conditions, unless noted otherwise be treated with fire retardant or corrosive	 Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation 	^{id} This design is valid until 6/28/2	026	

			Client:	Weaver Homes	;		Dat	te:	6/11/2025	5				Page 13 of 14
			Project:				Inp	ut by:	Curtis Qu	lick				Ū
l i	sDesign		Address:				Job	Name	: The Laur	en H Bea	ams			
							Pro	oject #:						
	SDE# 2	2.0		40 000"	2 01/		COED	,	_evel: Level					
DIVI4	Э-Р-Г # 2	2.0	UU X	10.000	2-Piy	- PA	199ED							
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	and the second second			1	Flante								IAIA	9 1/4"
-					•		•						V ∖	
]						
1 SPF	= 0-3-8					2 SF	PF 0-3-8	l.						
			5'8 1/	/2"				1						3"
/ <u>/</u>			5'8 1	/2"				ł						
			001	-				•						
ļ														
Member Ir	nformation						Reactions	s UNF	PATTERN	ED lb	(Uplift)			
Туре:	Girder		Applicat	ion: Flo	or		Brg Direc	ction	Live	[Dead	Snow	Wind	Const
Plies:	2		Design	Method: AS	D		1 Vertic	al	0		756	756	0	0
Moisture Cor	ndition: Dry		Building	Code: IBC	2012		2 Vertic	cal	0		756	756	0	0
Deflection LL	.: 360		Load St	naring: No										
Deflection TL	.: 240		Deck:	No	t Checked									
Importance:	Normal - II													
Temperature	: Temp <= 10	0°F				-	D							
							Bearings							
							Bearing I	Length	ı Dir.	Cap. F	leact D/L lb	o Total	Ld. Case	Ld. Comb.
							1 - SPF 🗧	3.500"	Vert	34%	756 / 756	5 1513	L	D+S
Analysia D							2 - SPF 🗧	3.500"	Vert	34%	756 / 756	5 1513	L	D+S
	Astual	1 4	A 11 I	O it -	O									
Analysis	Actual	Location	Allowed	Capacity	Comb. Ca	ase								
Moment	1826 π-ΙΒ	2'10 1/4"	3946 π-ID	0.463 (46%)	D+S L									
Unbraced	1826 π-Ib	2'10 1/4"	3629 π-Ib	0.503 (50%)	D+S L									
Shear	1358 ID	1' 3/4"	2872 lb	0.473 (47%)	D+S L									
LL Defl inch	n 0.016 (L/3853)	2'10 1/4"	0.175 (L/360)) 0.093 (9%)	S L									
TL Defl inch	n 0.033 (L/1926)	2'10 1/4"	0.262 (L/240	0) 0.125 (12%)	D+S L									
Design No	otes													
1 Provide si	upport to prevent late	eral moveme	nt and rotatio	n at the end bea	arings. Lateral su	ipport								
2 Easten all	be required at the in	terior bearing	at 24" o.c. M	ling code. Iavimum end die	tance not to evo	bee								
12".	plies using 2 rows c	000022300) at 24 0.0. IV		stance not to exc	,eeu								
3 Refer to la	ast page of calculation	ons for fasten	ers required	for specified loa	ds.									
4 Simpson f	fasteners applied fro	m a single si	de of the mer	nber use tip valı	ies where publis	hed.								
6 Top must	be laterally braced a	t end bearing	ie bolloni edų is.	je only.										
7 Bottom m	ust be laterally brace	ed at end bea	arings.											
8 Lateral sle	enderness ratio base	ed on single p	oly width.											
ID	Load Type		Location	Trib Width	Side Dea	ad 0.9	Live 1	Sno	w 1.15	Wind 1.	6 Const.	1.25 Co	mments	
1	Uniform			F	Far Face 26	65 PLF	0 PLF	2	65 PLF	0 PL	F 0	PLF A4		
									Manufacture	er Info				
								F						
					This dooin	n je volid ·	Intil 6/28/2020							
					i nis design	i is valid t	anui 0/28/2026							

BM4 S-P-F #2 2.000" X 10.000" 2-Ply - PASSED Level: Level Level: Level Image: Comparison of the second seco		Pa	6/11/2025 Curtis Quick The Lauren H Beams	te: out by: o Name: oject #:		Weaver Homes	Client: Project: Address:	isDesign	1
Image: Second			evel: Level	Le	2-Ply - PASSE	10.000"	2.000" X	S-P-F #2	BM4
1 SPF 0-3-8 2 SPF 0-3-8 5'8 1/2" 5'8 1/2" Multi-Ply Analysis Fasten all plies using 2 rows of SDW22300 at 24" o.c Maximum end distance not to exceed 12". Capacity 90.4 % Load 265.0 PLF Yield Limit per Foot 293.3 PLF Yield Limit per Fastener 293.3 lb.	9 1/4'		× 12	1 10"	• •		•	•	
Multi-Ply Analysis Fasten all plies using 2 rows of SDW22300 at 24" o.c Maximum end distance not to exceed 12". Capacity 90.4 % Load 265.0 PLF Yield Limit per Foot 293.3 PLF Yield Limit per Fastener 293.3 lb.		3"		' 	2 SPF 0-3-8	/2"	5'8 1 5'8 1	SPF 0-3-8	
Capacity90.4 %Load265.0 PLFYield Limit per Foot293.3 PLFYield Limit per Fastener293.3 lb.			12".	xceed 1	m end distance not to	24" o.c Maxin	of SDW22300 at	l y Analysis Il plies using 2 rows	Multi-Pl Fasten al
Load265.0 PLFYield Limit per Foot293.3 PLFYield Limit per Fastener293.3 lb.							90.4 %	• -	Capacity
Yield Limit per Foot 293.3 PLF Yield Limit per Fastener 293.3 lb.							265.0 PLF		Load
Yield Limit per Fastener 293.3 lb.							293.3 PLF	per Foot	Yield Limit p
							293.3 ID. 1	ber Fasterier	тівій сітіі р См
Yield Mode Lookup							Lookup		Yield Mode
Edge Distance 1 1/2"							1 1/2"	nce	Edge Distan
Min. End Distance 6"							6"	istance	Min. End Dis
Load Combination D+S							D+S	bination	Load Combi
Duration Factor 1.15							1.15	actor	Duration Fa

	Manufacturer Info	
This design is valid until 6/28/2026		