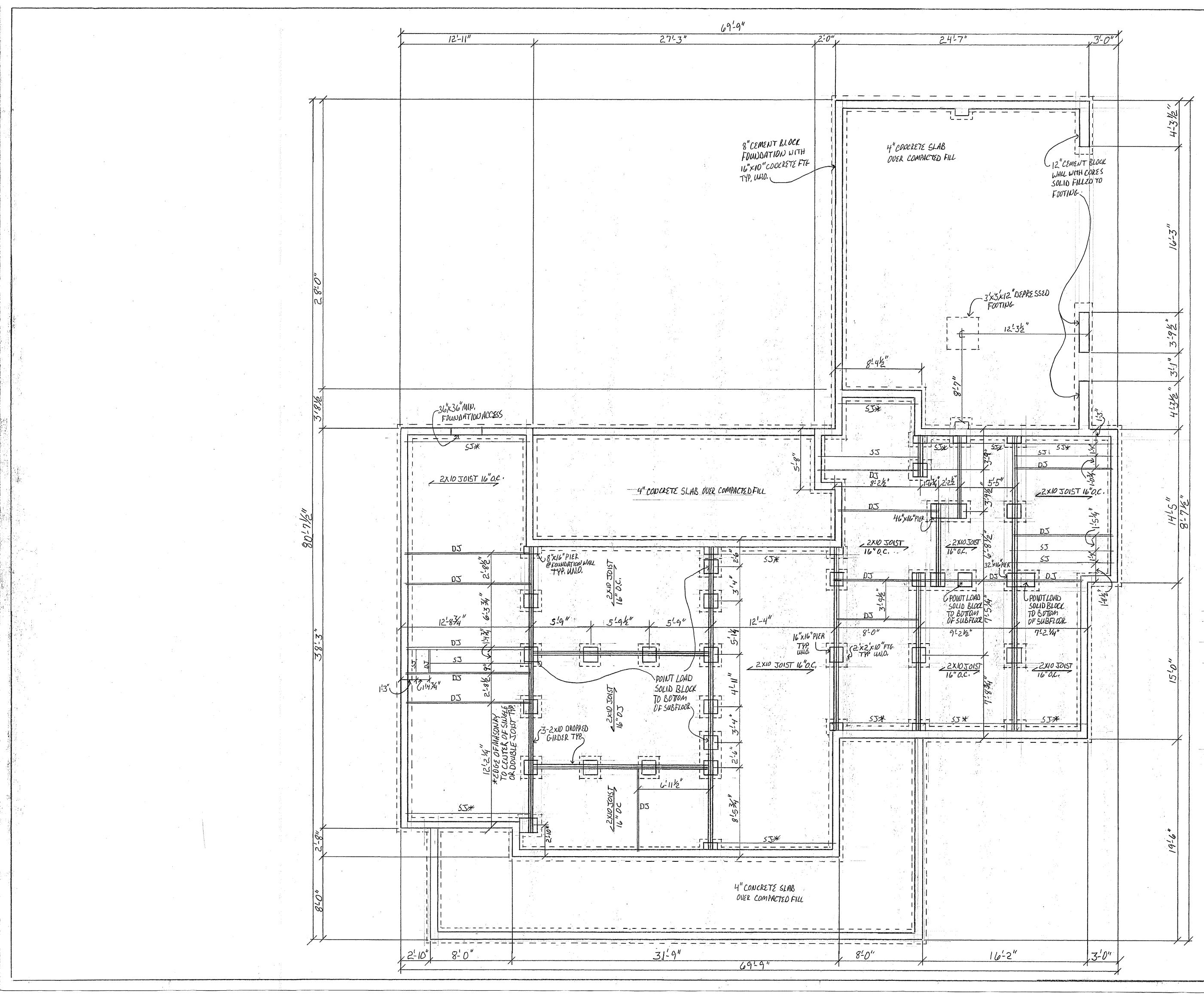


PEYTON ELEVATION DRAWN BY F

PEYTON & AMBERLY HOME ELEVATIONS-LEFT & RIGHT SIDES DRAWN BY FWILLIAM JOHNSON SCALE: 1/4": 1' DATE: JAMUARY 5, 2025 PAGE: 2 DF 7



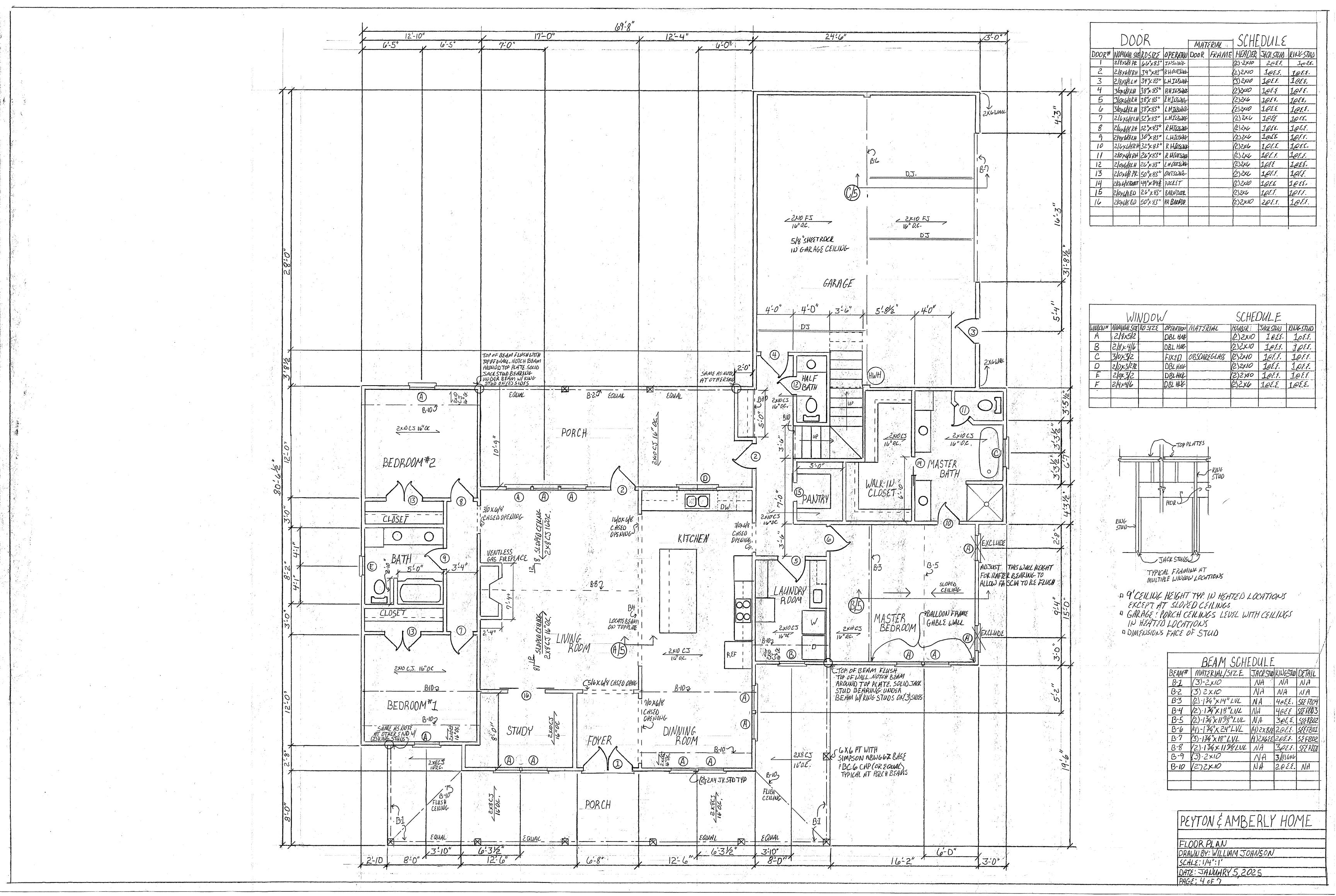
FOUNDATION NOTES

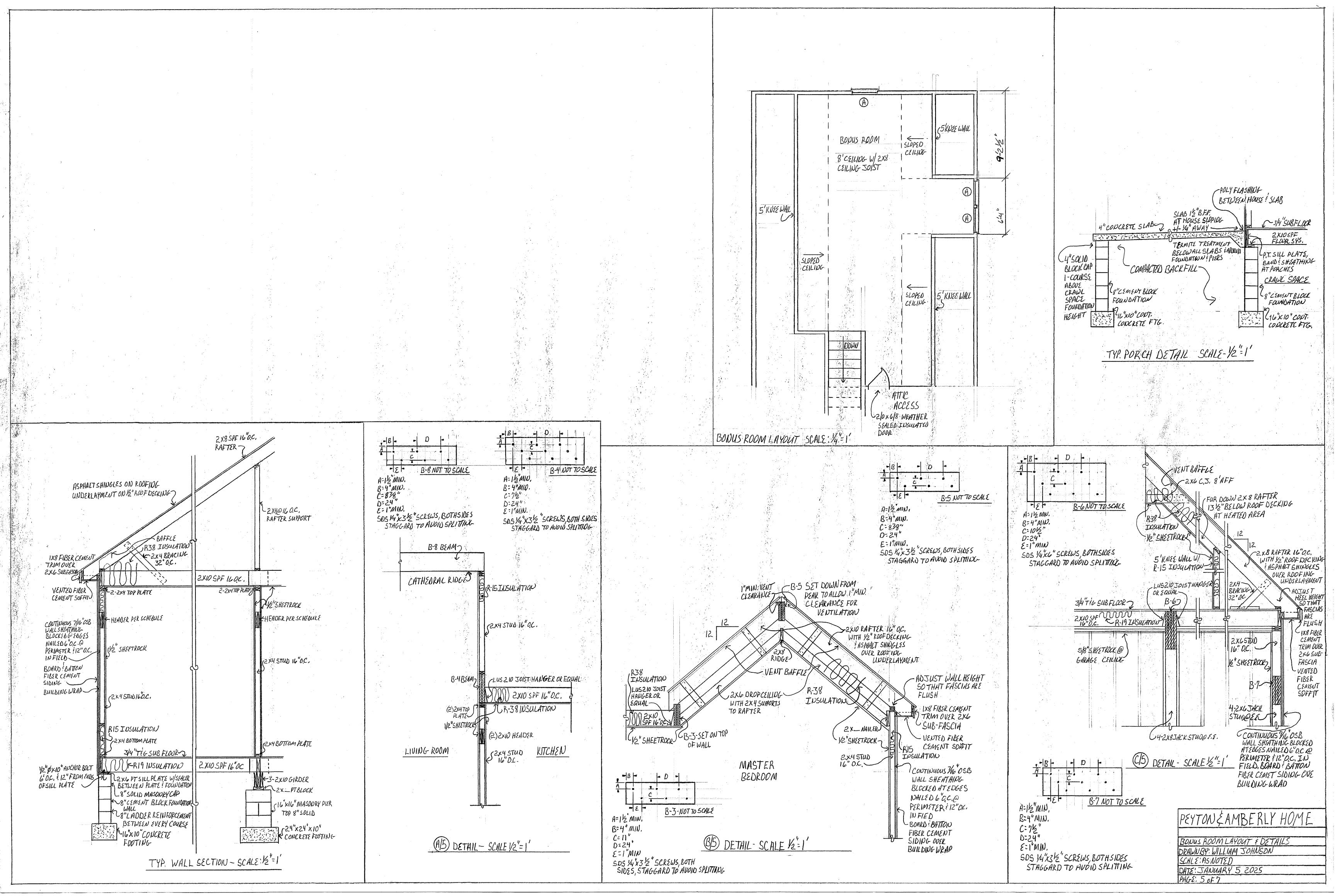
FOUNDATION NOTES FOUNDATION NOTES FOUNDATION PROJECTION BEYOND FOUNDATION PLENS FOR ALL FOUTINGS PROVIDE FOOTING FOR MASONRY STEPS @ FRONT PORCH FOUNDE SOLT UNDER ALL WALLS PARALLEL TO JOIST DOUBLE SOLT UNDER ALL WALLS PARALLEL TO JOIST DOUBLE ALL BANDS PARALLEL TO JOIST PROVIDE SOLID BLOCKING FROM FOUNDATION PLENS TO BOTTOMOF SUBFLOOR PROVIDE V2" O ANCHOR BOLTS @ G'O.C. & WITHIN 12" OF THE END OF MUDSILL PROVIDE V2" O ANCHOR BOLTS @ G'O.C. & WITHIN 12" OF THE END OF MUDSILL PCOMPLY WITH DIMENSION PROVIDED FOR LOCATION OF JOIST TO MISS PLUMBLE DIMENSION ARE FROM OUTSIDE FACE OF MASONRY FOR FOUNDATION \$ TO CENTER OF PLENS GIRDERS' SPECIAL JOIST

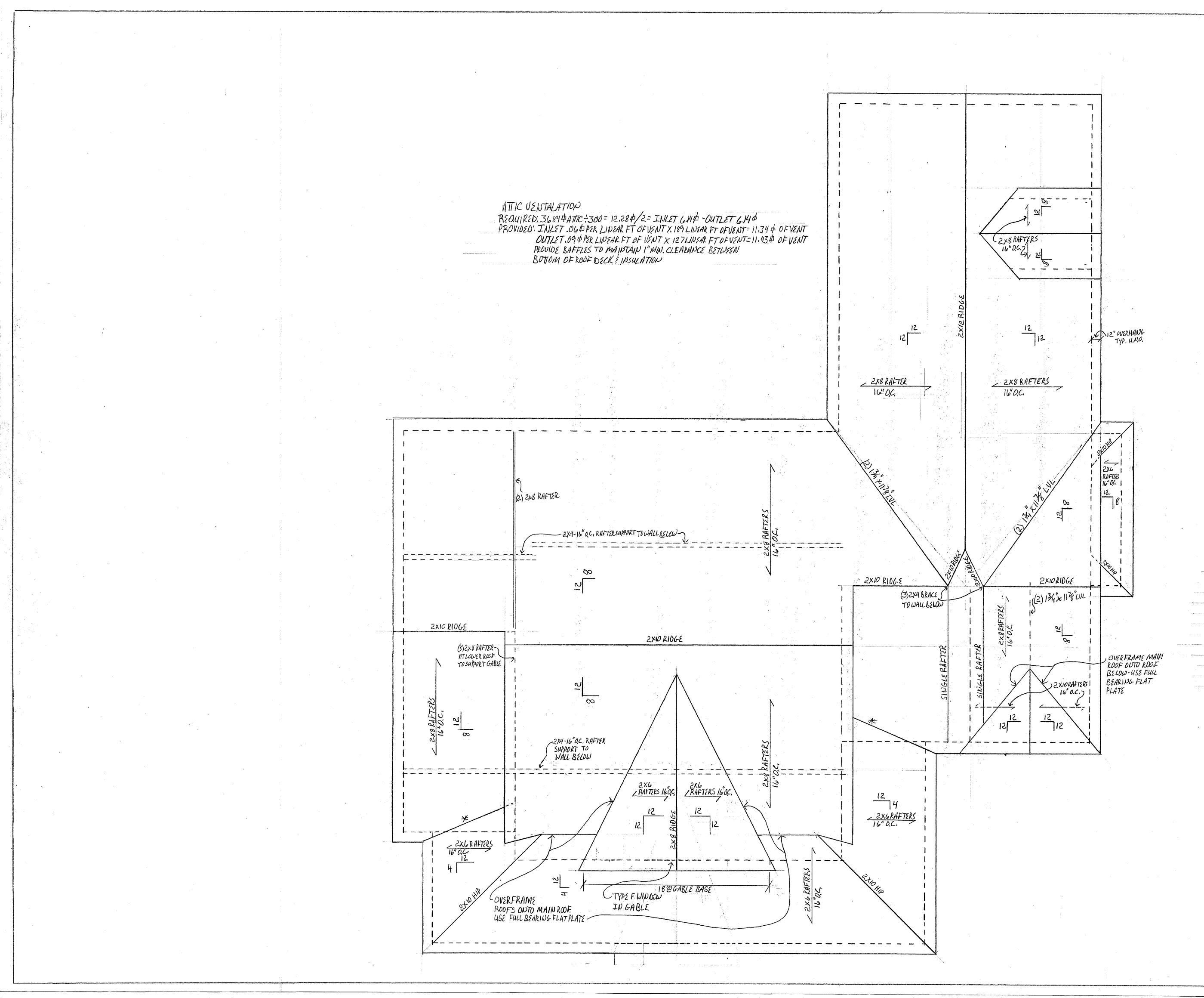
DIMEDSION ARE FROM DUTSIDE FACE OF MASODRY FOR FOUNDATION
TO CENTER OF PIERS, GIRDERS 'SPECIAL JOIST
P.T. MUDSILL ! BLOCKING (JITH APPROVED FOAM SEAL GASKET BETWEEN MASODRY ! FLOOR SYSTEM FRAMINGP.T. BANDOALL PORCH LOCATIONS
DALL PIERS TO BE 944" BELOW TOP OF FOUNDATION
DSEALED CRAWL SPACE DESIG DED BY INSTALLER
DNO FOUNDATION VENTS
D #2 SPF OR SYP FOR ALL FRAMING-MEMBERS
D ST = SWELE TO IST

- D S.J. = SINGLE JOIST D D.J. = DOUBLE JOIST
- O SEE DETAIL PALE FOR CLARIFICATIONS

,	ć
PEYTON & AMBERLY HOM	E
FOUNDATION PLAN	
SCHLE: V4":11	kaningan na manangan na s
DATE: JANUARY 5, 2025	••••••••••••••••••••••••••••••••••••••
46E: 30F7	<u></u>





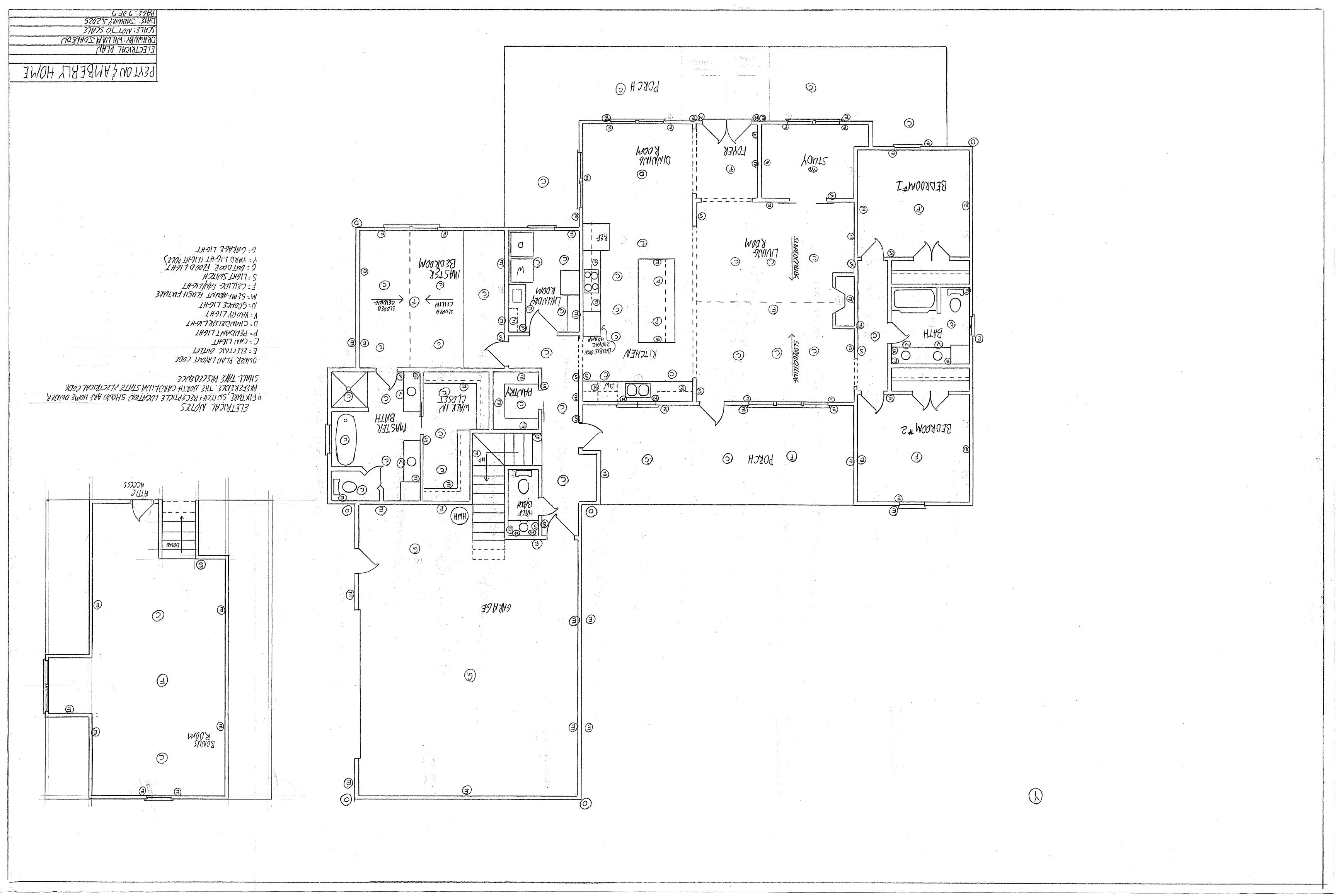


- ROOF NOTES = 2x6 SUBFASCIA AT ALL EAVES & GABLES = GABLE OVER HANG- SHALL BE BUILT LAODER FRAMED WITH 2x6 BLOCKING- NO GREATER THAN 24"O.C., BETWEEN
- 2x6 BLOCKING- NO GREATER THAN 24"O.C., BETWEEN PARALLEL 2X6'S DLET-IN 2X4 DUTRIGGERS SHALL BE USED ON GABLES WITH DUERMANGS GREATER. THAN 12" AND EXTEND NO LESS THAN 32" INSIDE GABLE DRAFTER RIDGE ! SEAT CUTS SHALL BE FULL BEARING-DRAFTER RIDGE SEAL OF DHFERENT PITCH ROOFS TO LINE UP SUBFASCIAS DOMINAL V2"ROOF DECKING- TO BE FASTENED G"O.C. AT EDGES ! 12" OC. IN FIELD WITH 8 PENN! RIDG-SHANL NAILS D PROVIDE CONTINUOUS RIDGE VENTILATION UP TO 2' OF THE END OF RIDGE (GABLES, HIPS OR NALLEYS) D SEE DETAIL FOR RIDGE VENTILATION OVER MASTER BEDROOM!

PEYTON & AMBERLY HOME

ROOF PLAN DRAINBY: WILLIAM JOHNSON SCALE: 1/44:11

DATE: JADUARY 5,2025 PAGE: 60F7



23											
Boise Cascade	Double	1-3/4" x 1	4" VERS	A-LAM®	LVL	2.1E 3	100 SI	Р		PA	SSED
ENGINEERED WOOD PRODUCTS			FB04 (Flu	ush Beam	R-3						
BC CALC® Member Re Build 8892	eport			an No cant		-			Octobe	er 1, 2024	4 01:27:58
Job name:				File name							
Address:				Descriptio							
				Specifier:							
City, State, Zip: Customer:				Designer:	W	illiam Jo	hnson				
	SR-1040			Company				Co., Inc	;		\sim
	011-10+0			e en ipen ij							
					, , , , , ,		• • • •	• •	• • • •	+ + + +	
8									1		×
<u>}</u>			15.	-04-00							
B1					- 45 04 0						B2
D			orizontal Pro	auct Length	= 15-04-0	0					
Reaction Summa Bearing	ry (Down / Upliπ)	(IDS) Dead	S	now	1	Nind		Roo	f Live		
B1, 3-1/2"	1840 / 0	569 / 0									
B2, 3-1/2"	1840 / 0	569 / 0									
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0 Self-Weight	Unf. Lin. (I		00-00-00	15-04-00	Тор		14				00-00-00
1	Unf. Area	lb/ft²) L	00-00-00	15-04-00	Back	40	10				06-00-00
Controls Summa	Y Value	% Allow	able D	Juration	Case	Loca	tion				
Pos. Moment	8690 ft-lbs	29.9%	1	00%	1	07-0	8-00				
End Shear	1951 lbs	21.0%	1	00%	1	01-0	5-08				
Total Load Deflection	L/792 (0.225")	60.6%	n	n\a	1	07-0	8-00				
Live Load Deflection	L/1037 (0.172")	46.3%	n	n\a	2	07-0	8-00				
Max Defl.	0.225"	22.5%	n	n\a	1	07-0	00-8				
Span / Depth	12.8										
Pearing Supports	D' (1140)	Value	% Allow	% Allow	Matori	al					
Bearing Supports	Dim. (LxW) 3-1/2" x 3-1/2"	2409 lbs	Support n\a	Member 26.2%	Unspe	ecified					
B1 Wall/Plate B2 Wall/Plate	3-1/2" x 3-1/2"	2409 lbs	n\a	26.2%		ecified					
DZ Waii/Flate	5-112 × 5-112	2400 100	ma	2012/0	eep.						
Notes											
Design meets User sp	ecified (L/480) Total lo	ad deflection	criteria.								
Design meets User sp	ecified (L/480) Live loa	ad deflection	criteria.								
Design meets arbitrary											
Design based on Dry											
BC CALC® analysis is											
Calculations assume r											
Connection Diag	ram: Full Length	of Membe	r								
D -	d										
a											
	-										
T C											
• 1											

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Boise Cascade'	Double 1-3/4" x 14" VERSA-LAM® LVL	2.1E 3100 SF	PAS	SED
ENGINEERED WOOD PRODUCTS	FB04 (Flush Beam)			
BC CALC® Member Report	Dry 1 span No cant.		October 1, 2024 0)1:27:5
Build 8892				
Job name:	File name:			
Address:	Description:			
City, State, Zip:	Specifier:			
Customer:	Designer:	William Johnson		
Code reports: ESR-1040	Company:	Lariat Construction	Co., Inc	

c = 11" a minimum = 1-1/2" d = 24" b minimum = 4"

e minimum = 1"

Calculated Side Load = 300.0 lb/ft Install screws from both sides, staggering screws by half of the spacing to avoid splitting. Connectors are: SDS 1/4 x 3-1/2

Disclosure

Use of the Boise Cascade Software is subject to the terms of the End User License Agreement (EULA). Completeness and accuracy of input must be reviewed and verified by a qualified engineer or other appropriate expert to assure its adequacy, prior to anyone relying on such output as evidence of suitability for a particular application. The output here is based on building code-accepted design properties and analysis methods. Installation of Boise Cascade engineered wood products must be in accordance with current Installation Guide and applicable building codes. To obtain Installation Guide or ask questions, please call (800)232-0788 before installation.

6											
o4											
	Double 1-	3/4" x 18	" VERS/	A-LAM®	LVL 2	.1E 31	00 SF	2		PA	SSED
Boise Cascade	Double .		FB03 (Flu								
			Dry 1 spar	n I No cant.	U				October	1, 2024	01:08:48
BC CALC® Member Report			Dijiopa								
Build 8892 Job name:				File name:							
Address:				Description	n:						
City, State, Zip:				Specifier:		- 2					
Customer:				Designer:		liam Joh					
Code reports: ESR-1	040			Company:	Lar	riat Cons	struction	Co., Inc		and the owner the second side	
										1 1	<u></u>
\downarrow \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow	+ + + +	• • •		1 + + +	* *		· · · ·	• •	<u> </u>	÷ •	* * • •
\downarrow \downarrow \downarrow \downarrow \downarrow \downarrow \downarrow	* * * *	• • •	* * *	0 + + +	* *	• •					
											\otimes
\boxtimes											
			17-	06-00							B2
B1		Total Ho	rizontal Proc	duct Length	= 17-06-0	0					
Reaction Summary (Down / Uplift)										
Bearing Liv	ve	Dead	Sr	now	V	Vind		Root	Live		
B1, 3-1/2" 27	13/0	1516/0									
B2, 3-1/2" 27	13/0	1516/0									
						Live	Dead	Snow	Wind	Roof	Tributary
Load Summary						Live	Deau	Onow		Live	,
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	00-00-00
0 Self-Weight	Unf. Lin. (Ib	/ft) L	00-00-00	17-06-00	Тор		18				15-06-00
1	Unf. Area (I	b/ft²) L	00-00-00	17-06-00	Back	20	10				10-00-00
				Vuration	Case	Loca	tion				
Controls Summary	Value	% Allow: 37.6%		Ouration	1	the local division in	9-00				
Pos. Moment	17543 ft-lbs 3363 lbs	28.1%		00%	1		9-08				
End Shear Total Load Deflection	L/712 (0.287")	67.4%		n\a	1	08-0	9-00				
Live Load Deflection	L/1110 (0.184")	43.3%	r	n\a	2	08-0	9-00				
Max Defl.	0.287"	28.7%	r	n\a	1	08-0	9-00				
Span / Depth	11.4										
opun, opun											
			% Allow	% Allow							
strength with the product of the pro	Dim. (LxW)	Value	Support	Member	Mater	ecified					
	3-1/2" x 3-1/2"	4228 lbs	n\a	46.0% 46.0%	-	ecified					
B2 Wall/Plate	3-1/2" x 3-1/2"	4228 lbs	n\a	40.070	Ulisp	comed					
Notes		I I Gention	aritaria								
Design meets User specifi	ed (L/480) Total lo	ad deflection	criteria.								
Design meets User specifi Design meets arbitrary (1"	ed (L/480) Live los	ad deflection	n criteria								
Design meets arbitrary (1 Design based on Dry Serv	ico Condition	Jau denectio	n ontona.								
BC CALC® analysis is ba											
Calculations assume unbr	aced length of Top	: 00-00-00, E	Bottom: 00-0	00-00.							
Calculations assume uner	acca longer er rep										
Connection Diagram		of Membe	r								
Connection Diagram	i. i un Lengui	or membe									
b -	d										
	•										
А с											
• •	• •										
•	•	-									
- B											

H

Boise Cascade		Double 1-3/4" x 18" VERSA-LAM® LVL	2.1E 3100 SP	PASSED
BC CALC® Membe		FB03 (Flush Beam) Dry 1 span No cant.		October 1, 2024 01:08:4
Build 8892 Job name: Address: City, State, Zip: Customer: Code reports:	ESR-1040	Poolgholt	William Johnson Lariat Construction	Co., Inc

a minimum = 1-1/2" c = 7-1/2" b minimum = 4" d = 24"

e minimum = 1"

Calculated Side Load = 300.0 lb/ft Install screws from both sides, staggering screws by half of the spacing to avoid splitting. Connectors are: SDS $1/4 \times 3-1/2$

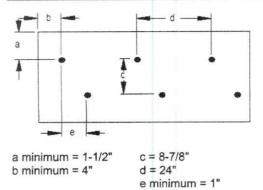
Disclosure

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r											
Boise Cascade'	Double 1-		2				3100	SP		P	ASSED
BC CALCO Momber Bon	ort	R		f Flush Be an No can		_			Octobe	er 1 202	4 01:20:57
BC CALC® Member Rep Build 8892	on		Diy [i sp	an i no can					000000	1,202	4 01.20.07
Job name:				File name	9:						
Address:				Descriptio							
City, State, Zip:				Specifier							
Customer:				Designer		lliam Jo					
Code reports: ESF	R-1040	and the second second second		Company	/: La	riat Con	struction	n Co., Inc	;		
			4	0							
		<u> </u>	- I I I		1 1 1		1 1	1 1	1 1	1 1	TI
		<u> </u>		0 1 1		÷			• •	+ +	¥ ¥
											- F - Barris - Constant - Constant - Barris
\boxtimes											
B1			15	5-04-00							B2
ы		Total Ho	orizontal Pro	duct Length	= 15-04-0	0					22
Reaction Summary	(Down / Uplift) _{Live}	Dead	S	now	v	Vind		Root	fLive		
B1, 3-1/2"		552 / 0		20/0							
B2, 3-1/2"		552 / 0	9	20 / 0							
F21 - 121 - 121										-	
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
0 Self-Weight	Unf. Lin. (II		00-00-00		Тор		12				00-00-00
1	Unf. Area	(lb/ft²) L	00-00-00	15-04-00	Front		10	20			06-00-00
Controls Summary	Value	% Allow	able I	Duration	Case	Loca	tion				
Pos. Moment	5311 ft-lbs	21.7%		115%	4	The second se	8-00				
End Shear	1226 lbs	13.5%		115%	4	01-0	3-06				
Total Load Deflection	L/810 (0.22")	59.2%	r	n\a	4	07-0	8-00				
Live Load Deflection	L/1297 (0.138")	37.0%	r	n\a	5	07-0					
Max Defl.	0.22"	22.0%	r	n\a	4	07-0	8-00				
Span / Depth	15.0										
			% Allow	% Allow							
Bearing Supports	Dim. (LxW)	Value	Support	Member	Materia	al					
B1 Wall/Plate	3-1/2" x 3-1/2"	1472 lbs	n\a	16.0%	Unspe						
B2 Wall/Plate	3-1/2" x 3-1/2"	1472 lbs	n\a	16.0%	Unspe	cified					
Cautions											
For roof members with sk	ope (1/4)/12 or less	final design r	nust ensure	that pondir	ng instabil	ity will r	not				
occur.		5			9	0.7 4 0 (100007 20					
For roof members with sl	ope (1/2)/12 or less	final design r	nust accour	nt for Rain-c	on-Snow s	urcharg	je				
load.											
Notos											
Notes	find (1 (490) Total In	ad doflaction	oritoria								
Design meets User speci Design meets User speci	terre and the second										
Design meets arbitrary (1	5 S										
Design based on Dry Ser			ontona.								
BC CALC® analysis is ba											
Calculations assume mer											

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Boise Cascade"	Double 1-3/4" x 11-7/8" VERSA-LAM® L	VL 2.1E 3100	SP PASSE
	RB01 (Roof Flush Beam)		
BC CALC® Member Report	Dry 1 span No cant.		October 1, 2024 01:20
Build 8892			
Job name:	File name:		
Address:	Description:		
City, State, Zip:	Specifier:		
Customer:	Designer:	William Johnson	
Code reports: ESR-104	0 Company:	Lariat Construction	Co., Inc



Calculated Side Load = 180.0 lb/ft Install screws from both sides, staggering screws by half of the spacing to avoid splitting. Connectors are: SDS $1/4 \times 3-1/2$

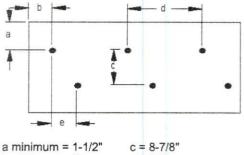
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-														
Boise C	Cascade'		ouble 1	-3/4"	x 11	-7/8" VE	RSA-LA	MRIV	1 1 95	2650	CD			10020
	ED WOOD PRODUCTS			0/4 /						2030	SF		P	ASSED
BC CALC	® Member Re	anort			P		of Flush Be pan No car		8			• • • •		
Build 8892		spon				DIYIIS	pan No car	n. —				Octob	er 1, 202	24 00:12:34
Job name							File nam	0.						
Address:	2						Descript							
City, State	, Zip:						Specifier							
Customer							Designe		/illiam Jo	hnson				
Code repo	orts: E	SR-1040					Compan				n Co., In	c		
							1							
						~	12 0							
• •	↓ ↓ ↓	+ +	• • •	• •	•	Ļ.	, 1 🗸 🗼	÷ ÷	• •	¥ ¥	+ +	• •	+ +	1.1
* *	* * *	* *	• • •	• •	+	+ + +	. 0	+ +	+ +	+ +	• •	• •	+ +	• •
\bowtie		_												
B1				-			8-00-00							B2
Reaction	n Summar	v (Down	/ Uplift		otal H	orizontal Pro	oduct Length	1 = 18-00-	00					
Bearing		Live		Dead	_	s	Snow		Wind		Roo	f Live		
B1, 3-1/2"				1188	-									
B2, 3-1/2"				1188	/ 0									
Load Su	Immarv								Live	Dead	Snow	Wind	Roof	Tributary
Tag Descr			Load Type		Ref.	Start	End	Loc.	100%	90%	115%		Live	y
THE REAL PROPERTY AND ADDRESS OF THE PARTY OF	Veight		Unf. Lin. (I	b/ft)	L	00-00-00	the second data and the second second data and the second data and	Тор	100%	12	115%	160%	125%	00-00-00
1		I	Unf. Area	(Ib/ft²)	L	00-00-00	18-00-00	Front		10				12-00-00
Controls	s Summary	Value		%	Allow	able I	Duration	Case	Loca	tion				
Pos. Mome		5079	ft-Ibs	31	.0%		90%	0	09-0	the second s				
End Shear		1019	bs	14	.3%	ş	90%	0	01-03	3-06				
Total Load			(0.403")	34	.5%	r	n\a	2	09-0	0-00				
Live Load I	Deflection		(0.068")	n\a		r	n\a	5	09-00	0-00				
Max Defl.		0.403	,	40	.3%	r	n\a	2	09-00	0-00				
Span / Dep	oth	17.7												
						% Allow	% Allow							
	Supports	Dim. (Lx		Value		Support	Member	Materia	STREET, STREET					
	Wall/Plate Wall/Plate	3-1/2" x 3-1/2" x		1459 II 1459 II		n\a n\a	15.9%	Unspe						
		0-112 X	0-1/2	1409 1	05	II\a	15.9%	Unspe	cified					
Cautions														
Controlling	Bearing value	e was gene	erated by I	oad cas	e 1, u	sing Man L	.oad.							
Controlling	Bearing value	e was gene	erated by I	oad cas	e 3, u	sing Man L	.oad.							
occur.	embers with s	lope (1/4)/	12 or less	final des	sign n	nust ensure	that pondin	g instabil	ity will no	ot				
For roof me	mbers with s	lope (1/2)/	12 or less	final des	sign m	nust accour	t for Rain-o	n-Snow s	urcharge	9				
load.					U				aronarge					
Notes														
	ets Code mini	mum (1/18	0) Total Io	ad defle	ction	criteria								
Design mee	ets Code mini	mum (L/24	0) Live loa	ad deflec	ction of	criteria.								
Design mee	ets arbitrary (*	1") Maximu	m Total lo	ad defle	ction	criteria.								
Design base	ed on Dry Sei	rvice Cond	ition.											
Calculations	analysis is ba assume me	ased on IB	C 2009.											
	assume me		y braced.											

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Boise Cascade"	Double 1-3/4" x 11-7/8" VERSA-LAM® LV	/L 1.8E 2650	SP	PASSED
	RB01 (Roof Flush Beam)			
BC CALC® Member Report	Dry 1 span No cant.		October 1	, 2024 00:12:34
Build 8892				
Job name:	File name:			
Address:	Description:			
City, State, Zip:	Specifier:			
Customer:	Designer: V	Villiam Johnson		
Code reports: ESR-10	40 Company: L	ariat Construction	Co., Inc	



b minimum = 4" d = 24" e minimum = 1"

Calculated Side Load = 110.0 lb/ft Install screws from both sides, staggering screws by half of the spacing to avoid splitting. Connectors are: SDS $1/4 \times 3-1/2$

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BC CALC® Member Rep Build 8892			FB02 (0	SA-LAM® Drop Beam) <u>B7</u>				Octob	L	4 00:49:00
Job name: Address:				File name							
City, State, Zip:				Description Specifier:							
Customer:				Designer		illiam Jo	hnson				
Code reports: ES	R-1040		and the base of the second second	Company				n Co., In	C		
+ + + + +	• • • • •	• • •	· · · ·	2			• •	• •	• •	+ +	•
	* * * * *	* * *	* * *	0 + +	• • •		* *	* *	* *	* *	* *
×											
B1		Total Ho		7-00-00 oduct Length	= 17-00-0	0					B2
Reaction Summary	/ (Down / Uplift) Live			Snow		Vind		Poo	f Live		
B1, 3-1/2"	2097 / 0	757 / 0				VIIIG		of the local division of the local divisiono	7/0		
82, 3-1/2"	2097 / 0	757 / 0						209	7/0		
Load Summary						Live	Dead	Snow	Wind	Roof Live	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	125%	
2 Self-Weight	Unf. Lin. (I Unf. Area		00-00-00 00-00-00		Тор Тор	40	27 10			40	00-00-00 06-02-00
Controls Summary	Value	% Allow	able	Duration	Case	Loca	tion				
Pos. Moment	15700 ft-lbs	84.2%		125%	3	08-0	Station of the local division of the local d				
End Shear	3079 lbs	13.7%		125%	3	01-0	S. S. S.				
Total Load Deflection	L/1221 (0.163") L/1515 (0.131")	39.3% 31.7%		n\a	3	08-0					
Max Defl.	0.163"	16.3%		n\a n\a	6 3	08-0 08-0					
Span / Depth	11.0	10.578		ina	5	00-0	0-00				
Bearing Supports	Dim. (LxW)	Value	% Allow	% Allow							
31 Wall/Plate	3-1/2" x 5-1/4"	3902 lbs	Support n\a	Member 28.3%	Materia Unspe	and the second second					
2 Wall/Plate	3-1/2" x 5-1/4"	3902 lbs	n\a	28.3%	Unspe						
lotes											
Design meets User spec Design meets User spec											
Design meets arbitrary (1											
Design based on Dry Ser											
3C CALC® analysis is ba											
Calculations assume me	mber is braced at er	nds. See engi	neering rep	ort for the ur	braced le	ength.					
Connection Diagra	m: Full Length	of Member									
	d										
	•										
c c											
	-• •										

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Boise Cascade	Triple 1-3/4" x 18" VERSA-LAM® LVL	2.1E 3100 SP	PASSED
	FB02 (Drop Beam)		
BC CALC® Member Report	Dry 1 span No cant.		October 1, 2024 00:49:00
Build 8892			
Job name:	File name:		
Address:	Description:		
City, State, Zip:	Specifier:		
Customer:	Designer:	William Johnson	
Code reports: ESR-1040	Company:	Lariat Construction	Co., Inc

a minimum = 1-1/2" c = 7-1/2" b minimum = 4" d = 24"

a = 24"e minimum = 1"

Calculated Side Load = 0.0 lb/ft Install screws from both sides, staggering screws by half of the spacing to avoid splitting. Connectors are: SDS 1/4 x 3-1/2

Disclosure

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	Quadrup	le 1-3/4" x		RSA-LAI		_ 2.1E	3100	SP		P	ASSED
BC CALC® Member Re Build 8892	port			an No can	ť. —	•		Se	ptembe	r 20, 202	4 20:58:09
Job name:				File name							
Address:				Descripti							
City, State, Zip:				Specifier							
Customer:				Designer	: W	illiam Jo	hnson				
Code reports: ES	SR-1040			Company	r: La	riat Cor	structio	n Co., Ind	C		
· · · · ·			* * *	2		•					
* * * * *	* * * * *	* * *	* * *	0	• • •	•	• •	• •	• •	• •	• •
				eneritaria de la constanada da de de da							(2×1)
			20	-00-00							1
B1											B2
			prizontal Pro	duct Length	= 32-00-0	0					
Reaction Summar											
Bearing	Live	Dead	S	now	V	Vind		Roo	f Live		
B1, 3-1/2"	7840/0	2738 / 0									
B2, 3-1/2"	7840 / 0	2738 / 0									
Load Summary						Live	Dead	Snow	Wind	Roof	Tributary
Tag Description	Load Type	Ref.	Start	End	Loc.	100%	90%	115%	160%	Live 125%	
0 Self-Weight	Unf. Lin. (i		00-00-00	32-00-00	Тор		49				00-00-00
2	Unf. Area	(lb/ft²) L	00-00-00	32-00-00	Back	40	10				12-03-00
Controls Summary		% Allow	the second se	Juration	Case	Loca	tion				
Pos. Moment	82220 ft-lbs	51.2%	1	00%	1	16-0	0-00				
End Shear	9063 lbs	28.4%	1	00%	1	02-0	3-08				
Total Load Deflection	L/410 (0.923")	58.5%	п	la	1	16-0	0-00				
Live Load Deflection	L/553 (0.684")	86.8%	n	la	2	16-0	0-00				
Max Defl.	0.923"	92.3%	n	la	1	16-0	0-00				
Span / Depth	15.8										
Bearing Supports	51 / 140		% Allow	% Allow							
B1 Wall/Plate	Dim. (LxW)	Value	Support	Member	Materia						
	3-1/2" x 7"	10578 lbs	n\a	57.6%	Unspe						
B2 Wall/Plate	3-1/2" x 7"	10578 lbs	n\a	57.6%	Unspe	cified					
Notes											
Design meets Code min											
Design meets User spec											
Design meets arbitrary (oad deflection	riteria.								
Design based on Dry Se											
BC CALC® analysis is b											
To minimize rotation, 7" braced.	wide beams should	be top-loaded	or loaded f	rom each si	de and pi	roperly					
	mbor is fully braced										
Calculations assume me	ember is fully braced										
0 (1 5)	-										
Connection Diagra	m: Full Length	of Member									
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-	Engineer to wood PRODUCTS		Quadruple 1-3/4" x 24" VERSA-LAM® LVL 2.1E 3100 SP		PASSED
			FB01 (Drop Beam)		·
	BC CALC® Member Build 8892	Report	Dry 1 span No cant.	September 20,	2024 20:58:09
	Job name:		File name:		
	Address:		Description:		
	City, State, Zip:		Specifier:		
	Customer:		Designer: William Johnson		
01	Code reports:	ESR-104	0 Company: Lariat Construction Co.	, Inc	

a minimum = 1-1/2" b minimum = 4"

c = 10-1/2" d = 24" e minimum = 1"

Calculated Side Load = 306.3 lb/ft Install screws from both sides, staggering screws by half of the spacing to avoid splitting. Connectors are: SDS 1/4 x 6

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