

RE: J1224-6956

Lot 27 Magnolia Hills

Trenco

Truss Name

VA4

VA5

VA6

VA7

Date

1/13/2025

1/13/2025

1/13/2025

1/13/2025

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J1224-6956

Lot/Block: Model:
Address: Subdivision:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.6

Wind Code: ASCE 7-10 Wind Speed: 130 mph Roof Load: 40.0 psf Floor Load: N/A psf

This package includes 24 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date	No.	Seal#
1	170686412	A1-GE	1/13/2025	21	170686432
2	170686413	A2	1/13/2025	22	170686433
3	170686414	A2A	1/13/2025	23	170686434
4	170686415	A3-GE	1/13/2025	24	170686435
5	170686416	A4	1/13/2025		
6	170686417	A5-GE	1/13/2025		
7	170686418	B1	1/13/2025		
8	170686419	B2	1/13/2025		
9	170686420	C1-GE	1/13/2025		
10	170686421	C2	1/13/2025		
11	170686422	J1-GE	1/13/2025		
12	170686423	J2	1/13/2025		
13	170686424	J3-GE	1/13/2025		
14	170686425	M1-GE	1/13/2025		
15	170686426	M2	1/13/2025		
16	170686427	P1-GE	1/13/2025		
17	170686428	P2	1/13/2025		
18	170686429	VA1	1/13/2025		
19	170686430	VA2	1/13/2025		
20	170686431	VA3	1/13/2025		

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

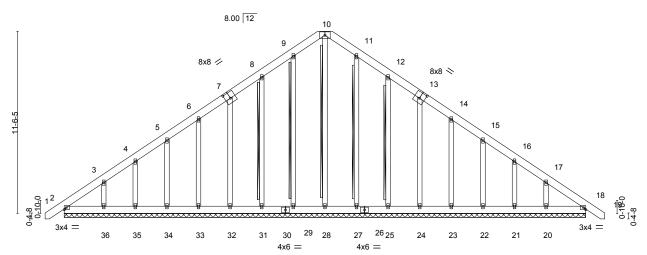


January 13, 2025

Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686412 J1224-6956 A1-GE **GABLE** Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:02 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314,

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 16-6-0 16-6-0

Scale = 1:72.9



5x8 =

Plate Offsets (X,Y)--[7:0-4-0,0-4-8], [13:0-4-0,0-4-8] LOADING (psf) SPACING-CSI **DEFL** in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.05 Vert(LL) -0.00 18 120 244/190 n/r MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.03 Vert(CT) 0.00 18 n/r 120 **BCLL** 0.0 Rep Stress Incr YES WB 0.15 Horz(CT) 0.01 18 n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Weight: 309 lb Matrix-S

33-0-0

LUMBER-TOP CHORD 2x6 SP No.1

BOT CHORD 2x6 SP No.1 **OTHERS** 2x4 SP No.2

BRACING-TOP CHORD **BOT CHORD**

WEBS

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SPF No.2 - 10-28, 9-29, 8-31, 11-27, 12-25

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 33-0-0.

Max Horz 2=-351(LC 10) (lb) -

> Max Uplift All uplift 100 lb or less at joint(s) 2, 18, 29, 32, 33, 34, 35, 27, 24, 23, 22, 21 except 31=-103(LC 12), 36=-144(LC 12), 25=-106(LC 13),

20=-136(LC 13)

All reactions 250 lb or less at joint(s) 2, 18, 28, 29, 31, 32, 33, 34, 35, Max Grav

36, 27, 25, 24, 23, 22, 21, 20

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-363/272, 3-4=-252/217, 8-9=-231/272, 9-10=-264/298, 10-11=-264/298,

11-12=-231/255, 17-18=-281/193

2-36=-176/277, 35-36=-176/277, 34-35=-176/277, 33-34=-176/277, 32-33=-176/277, 31-32=-178/278, 29-31=-178/278, 28-29=-178/278, 27-28=-178/278, 25-27=-178/278,

24-25=-178/278, 23-24=-176/276, 22-23=-176/276, 21-22=-176/276, 20-21=-176/276,

18-20=-176/276

NOTES-

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 29, 32, 33, 34, 35, 27, 24, 23, 22, 21 except (jt=lb) 31=103, 36=144, 25=106, 20=136.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required



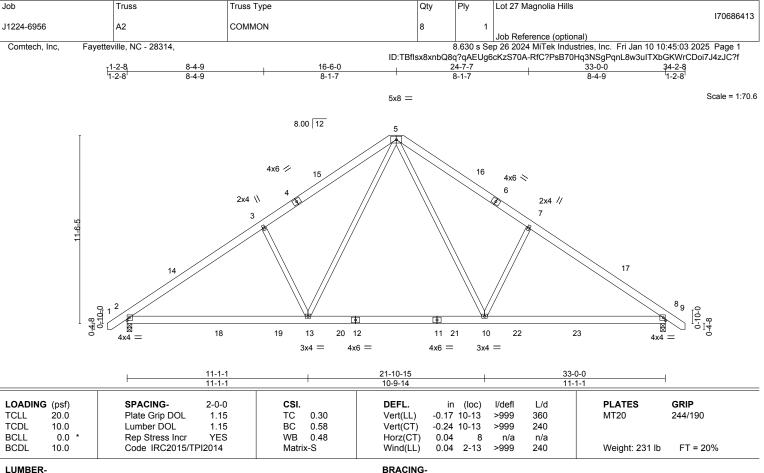
January 13,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 **WEBS** 2x4 SP No.2

REACTIONS.

2=0-3-8, 8=0-3-8 (size) Max Horz 2=281(LC 11)

Max Uplift 2=-85(LC 12), 8=-85(LC 13) Max Grav 2=1606(LC 19), 8=1606(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2167/383, 3-5=-2025/487, 5-7=-2025/487, 7-8=-2167/383 TOP CHORD

BOT CHORD 2-13=-158/1884, 10-13=0/1236, 8-10=-168/1695

WEBS 5-10=-173/1047, 7-10=-526/320, 5-13=-173/1047, 3-13=-526/320

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 34-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.



Structural wood sheathing directly applied or 5-0-12 oc purlins.

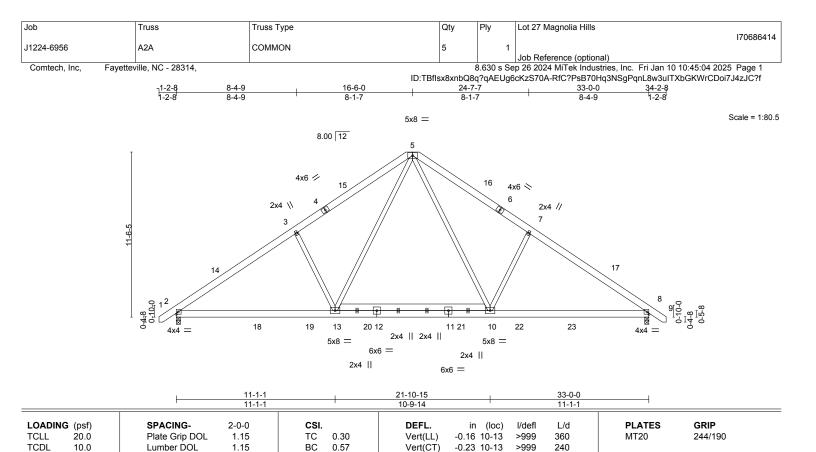
Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Horz(CT)

Wind(LL)

BRACING-

TOP CHORD

BOT CHORD

0.04

0.04 2-13

8

n/a

>999

n/a

240

Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 5-1-0 oc purlins.

Weight: 255 lb

FT = 20%

LUMBER-

REACTIONS.

BCLL

BCDL

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 **WEBS** 2x4 SP No.2

0.0

10.0

2=0-3-8, 8=0-3-8 (size) Max Horz 2=281(LC 11)

Max Uplift 2=-85(LC 12), 8=-85(LC 13) Max Grav 2=1596(LC 19), 8=1596(LC 20)

Rep Stress Incr

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-2148/383, 3-5=-2006/487, 5-7=-2006/487, 7-8=-2148/383 TOP CHORD

BOT CHORD 2-13=-158/1869, 10-13=0/1225, 8-10=-168/1680

WEBS 5-10=-173/1035, 7-10=-526/320, 5-13=-173/1035, 3-13=-526/320

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 16-6-0, Exterior(2) 16-6-0 to 20-10-13, Interior(1) 20-10-13 to 34-0-15 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

WB

Matrix-S

0.48

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

YES

- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 8.





Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686415 J1224-6956 A3-GE **GABLE** Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:04 2025 Page 1 Fayetteville, NC - 28314, Comtech, Inc. ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

8-4-9

5x8 = 2x4 > 8.00 12 16 2x4 || 2x4 || 4x6 🖊 8 4x4 II 9 4x12 // 10 3x10 \\ 2x4 \\ 3x10 2x6 < X 20 21 14 22 13 23 11 12 3x4 = 4x6 = 4x8 =

> 21-10-15 10-9-14

except end verticals.

1 Row at midpt

Structural wood sheathing directly applied or 6-0-0 oc purlins,

9-12

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

2x4 SPF No.2 - 12-16

Rigid ceiling directly applied or 10-0-0 oc bracing.

16-6-0

5-8-8

Scale = 1:69.0

8-1-7

Plate Offsets	s (X,Y)	[2:0-0-0,0-0-3]										
LOADING (p	psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 2	0.0	Plate Grip DOL	1.15	TC	0.29	Vert(LL)	-0.20	12-14	>999	360	MT20	244/190
TCDL 10	0.0	Lumber DOL	1.15	BC	0.56	Vert(CT)	-0.26	12-14	>998	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.56	Horz(CT)	0.01	12	n/a	n/a		
BCDL 10	0.0	Code IRC2015/Ti	PI2014	Matri	x-S	Wind(LL)	0.05	2-14	>999	240	Weight: 203 lb	FT = 20%

TOP CHORD

BOT CHORD

WEBS

LUMBER-BRACING-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 2x4 SP No.2 *Except* **WEBS**

9-12: 2x6 SP No.1, 5-17: 2x8 SP No.1

OTHERS 2x4 SP No.2

> Brace must cover 90% of web length. **JOINTS** 1 Brace at Jt(s): 16, 19

REACTIONS. (size) 2=0-3-8, 12=0-3-8

Max Horz 2=443(LC 12)

Max Uplift 2=-176(LC 12), 12=-246(LC 12) Max Grav 2=1086(LC 19), 12=1157(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1263/227, 3-5=-1120/333, 5-6=-347/247, 12-17=-288/147, 9-17=-292/106

BOT CHORD 2-14=-413/1115, 12-14=-123/409

12-16=-828/297, 14-15=-252/1103, 3-14=-506/400, 5-15=-847/304, 15-16=-765/255, **WEBS**

6-15=-243/385

NOTES-

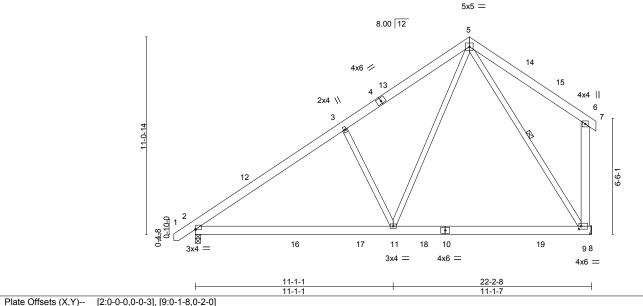
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=176, 12=246,
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



January 13,2025



Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686416 J1224-6956 A4 COMMON 9 Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:04 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 8-4-9 6-11-12 6-10-3



LOADIN	G (psf)	SPACING- 2-0-0	CSI.	DEFL . in (loc) I/defl L/d	PLATES GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC 0.30	Vert(LL) -0.18 9-11 >999 360	MT20 244/190
TCDL	10.0	Lumber DOL 1.15	BC 0.56	Vert(CT) -0.25 9-11 >999 240	
BCLL	0.0 *	Rep Stress Incr YES	WB 0.56	Horz(CT) 0.01 9 n/a n/a	I
BCDL	10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.04 2-11 >999 240	Weight: 176 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

LUMBER-

REACTIONS.

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 **WEBS** 2x4 SP No.2 *Except*

6-9: 2x6 SP No.1

(size) 9=Mechanical, 2=0-3-8

Max Horz 2=260(LC 12)

Max Uplift 9=-71(LC 12), 2=-51(LC 12) Max Grav 9=1119(LC 19), 2=1079(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1269/199, 3-5=-1127/294, 6-9=-309/214 TOP CHORD

BOT CHORD 2-11=-244/1093, 9-11=-58/497

WEBS 3-11=-518/305, 5-9=-829/101, 5-11=-143/1064

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 15-4-5, Exterior(2) 15-4-5 to 19-9-2, Interior(1) 19-9-2 to 22-5-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Refer to girder(s) for truss to truss connections.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9, 2.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

1 Row at midpt

Scale: 3/16"=1

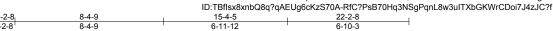
January 13,2025



Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686417 J1224-6956 A5-GE **GABLE**

Comtech, Inc, Fayetteville, NC - 28314, Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:05 2025 Page 1

Scale = 1:64.9



4x6 = 8.00 12 12 13 6x6 / 9 8 14 15 16 3x4 = 21 28 27 26 25 24 23 22 20 19 18 17 4x6 = 11-1-1

Plate Of	Plate Offsets (X,Y) [8:0-3-0,0-4-4], [11:0-3-0,Edge]											
LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	0.00	15	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.01	Vert(CT)	0.00	15	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.00	17	n/a	n/a		
BCDL	10.0	Code IRC2015/TI	PI2014	Matri	x-S						Weight: 227 lb	FT = 20%

LUMBER-BRACING-2x6 SP No.1 TOP CHORD TOP CHORD

BOT CHORD 2x6 SP No.1 **WEBS** 2x6 SP No.1 **BOT CHORD OTHERS** 2x4 SP No.2 **WEBS**

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.2 - 10-21

2x4 SPF No.2 - 9-23, 12-20, 13-19 Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

REACTIONS. All bearings 22-2-8.

> Max Horz 2=387(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 17, 2, 21, 23, 24, 25, 26, 27, 18

except 28=-147(LC 12), 19=-104(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 17, 2, 21, 23, 24, 25, 26, 27, 28,

20. 19. 18

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-426/302, 3-4=-308/248

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 17, 2, 21, 23, 24, 25, 26, 27, 18 except (jt=lb) 28=147, 19=104.
- 10) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



January 13,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686418 J1224-6956 **B1** FLAT GIRDER ✓ Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:06 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f 6-9-15 6-9-15 Scale = 1:35.7 4x6 = 3 5 245 91 7 4 9 🖄 15 16 17 18 10 12 14 13 4x6 =5x8 = 4x12 =3x8 M18AHS || 2x4 || 6-9-15 6-9-15 Plate Offsets (X,Y)--[12:0-3-0,0-1-12], [13:0-4-0,0-0-4] **PLATES** LOADING (psf) SPACING-CSI DEFL. in (loc) I/def L/d GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.91 Vert(LL) -0.07 11-12 >999 360 MT20 244/190 TCDL 10.0 Lumber DOL 1.15 ВС 0.55 Vert(CT) -0.12 11-12 >999 240 M18AHS 186/179 **BCLL** 0.0 Rep Stress Incr NO WB 0.80 Horz(CT) 0.03 c n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Wind(LL) 0.06 12-13 >999 240 Weight: 332 lb Matrix-S LUMBER-**BRACING-**TOP CHORD 2x6 SP No.1 2-0-0 oc purlins (5-10-4 max.): 1-7, except end verticals. TOP CHORD **BOT CHORD BOT CHORD** 2x6 SP No.1 Rigid ceiling directly applied or 10-0-0 oc bracing. 2x4 SP No.2 *Except* **WEBS** 2x6 SPF No.2 - 5-9 **WEBS** 2-13,6-9: 2x6 SP No.1

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c.,with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. (size) 13=0-3-8, 9=0-3-8

Max Horz 13=-144(LC 6)

Max Uplift 13=-662(LC 4), 9=-626(LC 5) Max Grav 13=5900(LC 2), 9=5800(LC 2)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-13=-5441/516, 6-9=-1196/127, 2-3=-5389/505, 3-5=-5389/505, 5-6=-265/41

BOT CHORD 12-13=-96/317, 11-12=-494/5444, 9-11=-494/5444

WEBS 2-12=-563/6555, 3-12=-3787/313, 5-11=-41/390, 5-9=-6649/581

NOTES-

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-7-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) All plates are MT20 plates unless otherwise indicated
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 13=662, 9=626.
- 9) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 10) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 69 lb down and 49 lb up at 0-4-4, 1015 lb down and 72 lb up at 2-1-4, 1015 lb down and 72 lb up at 4-1-4, 1015 lb down and 72 lb up at 6-1-4, 1015 lb down and 72 lb up at 10-1-4, 1015 lb down and 72 lb up at 12-1-4, 1015 lb down and 72 lb up at 14-1-4, and 1015 lb down and 72 lb up at 16-1-4, and 1015 lb down and 72 lb up at 18-1-4 on top chord, and 316 lb down and 155 lb up at 12-0-12, 316 lb down and 155 lb up at 16-1-4, and 316 lb down and 155 lb up at 18-1-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
- 11) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

Continued on page 2

LOAD CASE(S) Standard

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Edenton, NC 27932

January 13,2025

SEAL

ORTH

Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686418 J1224-6956 В1 FLAT GIRDER **Z** Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:06 2025 Page 2

Comtech, Inc, Fayetteville, NC - 28314,

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 8-14=-20, 1-2=-60, 2-6=-60, 6-7=-60

Concentrated Loads (lb)

Vert: 2=-65 4=-850(F) 15=-316(B) 16=-316(B) 17=-316(B) 18=-316(B) 19=-850(F) 20=-850(F) 21=-850(F) 22=-850(F) 23=-850(F) 24=-850(F) 25=-850(F)

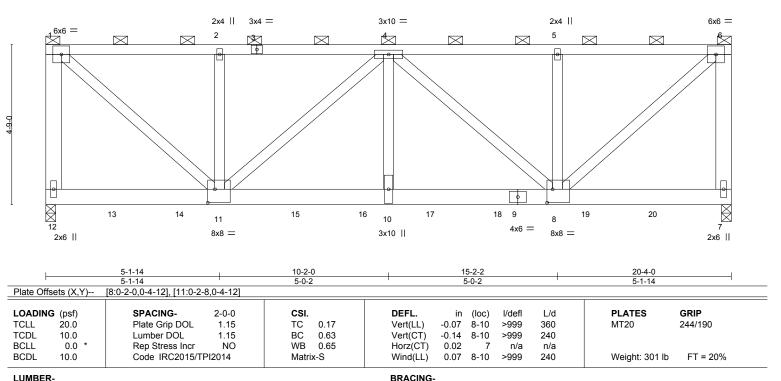
26=-850(F)



818 Soundside Road Edenton, NC 27932

Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686419 J1224-6956 B2 FLAT GIRDER ✓ Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:06 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 5-1-14 5-0-2 5-0-2

Scale = 1:34.2



TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1 2x4 SP No.2 *Except* **WEBS** 1-12,6-7: 2x6 SP No.1

REACTIONS. (size) 12=0-3-8, 7=0-3-8

Max Horz 12=-121(LC 6)

Max Uplift 12=-674(LC 4), 7=-687(LC 5) Max Grav 12=3924(LC 1), 7=3981(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD $1-12 = -3548/658, \ 1-2 = -3902/728, \ 2-4 = -3902/728, \ 4-5 = -4040/725, \ 5-6 = -404$

6-7=-3676/682

BOT CHORD 10-11=-926/5303, 8-10=-926/5303

WEBS 1-11=-921/5122, 2-11=-296/114, 4-11=-1859/312, 4-10=-398/2020, 4-8=-1676/277,

5-8=-295/113, 6-8=-955/5312

1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:

Top chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc, 2x4 - 1 row at 0-9-0 oc. Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.

Webs connected as follows: 2x4 - 1 row at 0-9-0 oc.

- 2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope); end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
- 4) Provide adequate drainage to prevent water ponding.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 12=674 7=687
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 500 lb down and 47 lb up at 2-0-0, 500 lb down and 47 lb up at 4-0-0, 500 lb down and 47 lb up at 16-0-8, 500 lb down and 47 lb up at 18-0-8, 758 lb down and 271 lb up at 5-5-4, 758 lb down and 123 lb up at 7-5-4, 758 lb down and 123 lb up at 9-5-4, 758 lb down and 123 lb up at 11-5-4, and 758 lb down and 123 lb up at 13-5-4, and 758 lb down and 271 lb up at 14-10-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.



2-0-0 oc purlins (6-0-0 max.): 1-6, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

January 13,2025

LOAD CASE(S) Standard

Continued on page 2

MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE

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Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686419 J1224-6956 B2 FLAT GIRDER Z Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:06 2025 Page 2

Comtech, Inc, Fayetteville, NC - 28314,

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-6=-60, 7-12=-20

Concentrated Loads (lb)

Vert: 11=-758(B) 8=-758(B) 13=-441(F) 14=-441(F) 15=-758(B) 16=-758(B) 17=-758(B) 18=-758(B) 19=-441(F) 20=-441(F)



818 Soundside Road Edenton, NC 27932

170686420
25 Page 1
oi7J4zJC?f
16-2-8
1-2-8

Scale = 1:27.9

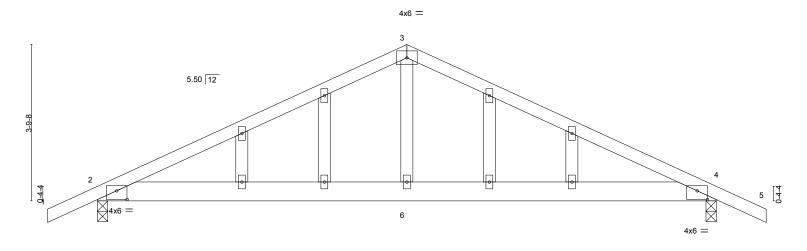


Plate Offsets (X,Y)	7-6-0 [2:0-3-0,0-2-8], [4:0-3-0,0-2-8]		'		7-6-0		'
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	I/defl L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.48 BC 0.26	/	-0.03 4-6 -0.06 4-6	>999 360 >999 240	MT20	244/190
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.10 Matrix-S	- (- /	0.01 4 0.07 4-6	n/a n/a >999 240	Weight: 79 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

15-0-0

Structural wood sheathing directly applied or 5-6-9 oc purlins.

Rigid ceiling directly applied or 8-7-12 oc bracing.

LUMBER-

TOP CHORD 2x4 SP No.1 **BOT CHORD** 2x6 SP No.1 **WEBS** 2x4 SP No.2 **OTHERS** 2x4 SP No.2

REACTIONS.

(size) 2=0-3-0, 4=0-3-0 Max Horz 2=-81(LC 13)

Max Uplift 2=-213(LC 8), 4=-213(LC 9) Max Grav 2=670(LC 1), 4=670(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-899/962, 3-4=-899/962 TOP CHORD BOT CHORD 2-6=-734/728, 4-6=-734/728

WEBS 3-6=-491/377

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7-6-0

- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=213, 4=213.



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gnolia Hills
170686421
ence (optional)
MiTek Industries, Inc. Fri Jan 10 10:45:07 2025 Page 1
RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f
5-0-0 16-2-8
-6-0 1-2-8
-

Scale = 1:27.9

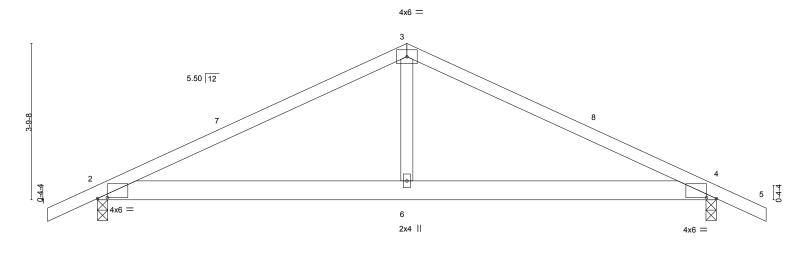


Plate Offsets (X,Y)	[2:0-2-13,0-0-5], [4:0-2-13,0-0-5]		700	
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.48	Vert(LL) -0.03 4-6 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.26	Vert(CT) -0.06 4-6 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.09	Horz(CT) 0.01 4 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL) 0.07 4-6 >999 240	Weight: 68 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

2x4 SP No.1 TOP CHORD **BOT CHORD** 2x6 SP No.1 **WEBS** 2x4 SP No.2

REACTIONS. (size) 2=0-3-0, 4=0-3-0 Max Horz 2=-48(LC 13)

Max Uplift 2=-157(LC 8), 4=-157(LC 9) Max Grav 2=670(LC 1), 4=670(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 2-3=-899/888, 3-4=-899/886

BOT CHORD 2-6=-678/728, 4-6=-678/728

WFBS 3-6=-478/377

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 7-6-0, Exterior(2) 7-6-0 to 11-10-13, Interior(1) 11-10-13 to 16-2-8 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=157, 4=157.



Structural wood sheathing directly applied or 5-8-4 oc purlins.

Rigid ceiling directly applied or 9-0-0 oc bracing.



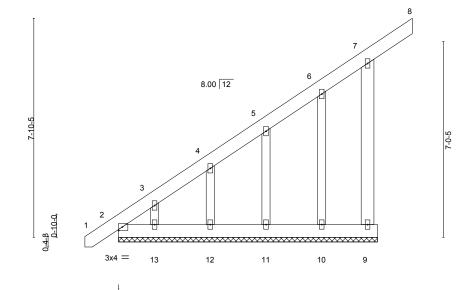
Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
J1224-6956	J1-GE	GABLE	1	1	170686422
31224-0930	J1-GL	GABLE	'	'	Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:08 2025 Page 1

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSqPqnL8w3uITXbGKWrCDoi7J4zJC?f 10-6-8 1-3-0

Scale = 1:41.3



LOADING (psf) TCLL 20.0	SPACING- 2-0-0 Plate Grip DOL 1.15	CSI. TC 0.06	DEFL. Vert(LL) 0.0	in (loc)	l/defl n/r	L/d 120		GRIP 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.01	Vert(CT) 0.0		n/r	120	20	2
BCLL 0.0 * BCDL 10.0	Rep Stress Incr YES Code IRC2015/TPI2014	WB 0.07 Matrix-S	Horz(CT) 0.0	00 9	n/a	n/a	Weight: 86 lb	FT = 20%

LUMBER-BRACING-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

2x6 SP No.1 WEBS **OTHERS** 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 9-3-8.

Max Horz 2=353(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 2, 10, 11, 12 except 9=-140(LC 12), 13=-139(LC 12)

Max Grav All reactions 250 lb or less at joint(s) 9, 2, 10, 11, 12, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-469/347, 3-4=-364/265, 4-5=-280/197, 7-9=-208/253

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 10, 11, 12 except (jt=lb) 9=140, 13=139.



January 13,2025



Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686423 J1224-6956 J2 JACK-OPEN Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:08 2025 Page 1

Comtech, Inc, Fayetteville, NC - 28314,

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

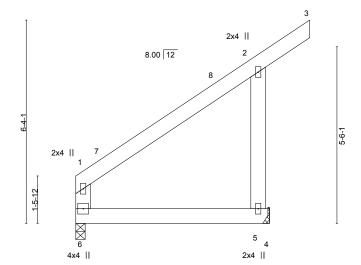
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

6-0-8 1-3-0

Scale = 1:35.9



6-0-8
6-0-8

LOADIN	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.19	Vert(LL)	-0.01	5-6	>999	360	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	-0.02	5-6	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TP	12014	Matri	x-S	Wind(LL)	0.01	5-6	>999	240	Weight: 47 lb	FT = 20%

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1

WEBS 2x6 SP No.1

REACTIONS. 6=0-3-8, 5=Mechanical (size) Max Horz 6=146(LC 12) Max Uplift 5=-135(LC 12)

Max Grav 6=204(LC 1), 5=359(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS 2-5=-347/349

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-2-12 to 4-7-9, Interior(1) 4-7-9 to 7-3-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 5=135.



January 13,2025

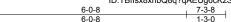
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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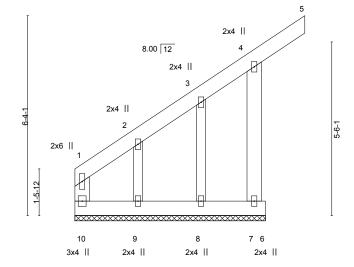


Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills	٦
14004 0050	12.05	CARLE			170686424	
J1224-6956	J3-GE	GABLE	11	1	Lib B. C C. C D	
					Job Reference (optional)	

8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:08 2025 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



Scale = 1:36.5



LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	0.00	5	n/r	120	MT20	244/190
TCDL	10.0	Lumber DOL	1.15	BC	0.09	Vert(CT)	0.00	4	n/r	120		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(CT)	0.00		n/a	n/a		
BCDL	10.0	Code IRC2015/TP	PI2014	Matri	x-P						Weight: 55 lb	FT = 20%

BOT CHORD

LUMBER-BRACING-

TOP CHORD 2x6 SP No.1 BOT CHORD 2x6 SP No.1 2x6 SP No.1 WEBS

OTHERS 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 6-0-8. Max Horz 10=213(LC 12) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 10, 6, 8 except 9=-295(LC 12), 7=-156(LC 12) Max Grav All reactions 250 lb or less at joint(s) 6, 9, 8, 7 except 10=278(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

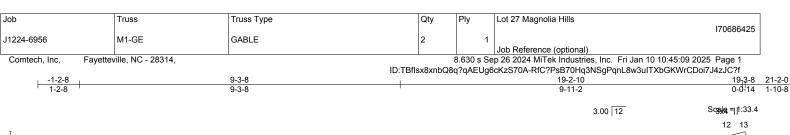
TOP CHORD 1-2=-343/240 WEBS 4-7=-222/278

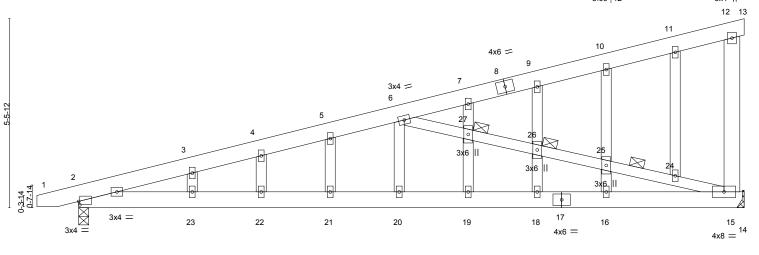
NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 2-0-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 6, 8 except (jt=lb) 9=295, 7=156.









L	7-1-6	9-3-8		19-3-	-8
l l	7-1-6	2-2-2	1	10-0-	-0
Plate Offsets (X,Y)	[2:0-0-6,0-1-3]				
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc) I/defl L/d	PLATES GRIP
TCLL 20.0	Plate Grip DOL 1.15	TC 0.21	Vert(LL)	-0.08 18 >999 360	MT20 244/190
TCDL 10.0	Lumber DOL 1.15	BC 0.36	Vert(CT)	-0.16 18-19 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.25	Horz(CT)	0.03 15 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-S	Wind(LL)	0.09 18 >999 240	Weight: 145 lb FT = 20%

BRACING-

TOP CHORD

BOT CHORD

WEBS

JOINTS

LUMBER-

2x6 SP No.1 TOP CHORD BOT CHORD 2x6 SP No.1 **WEBS**

2x4 SP No.2 *Except* 12-15: 2x6 SP No.1

OTHERS 2x4 SP No.2

REACTIONS. (size) 15=Mechanical, 2=0-3-8

Max Horz 2=236(LC 8)

Max Uplift 15=-251(LC 12), 2=-238(LC 8) Max Grav 15=778(LC 1), 2=816(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1744/443, 3-4=-1682/466, 4-5=-1666/484, 5-6=-1642/503

BOT CHORD 2-23=-642/1617, 22-23=-642/1617, 21-22=-642/1617, 20-21=-642/1617, 19-20=-642/1617,

18-19=-642/1617, 16-18=-642/1617, 15-16=-642/1617

WEBS 6-20=-106/362, 6-27=-1611/641, 26-27=-1576/627, 25-26=-1580/630, 24-25=-1587/631,

15-24=-1621/646

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 15=251, 2=238



Structural wood sheathing directly applied or 5-9-12 oc purlins,

15-26

Rigid ceiling directly applied or 9-8-5 oc bracing.

except end verticals.

1 Brace at Jt(s): 26, 27

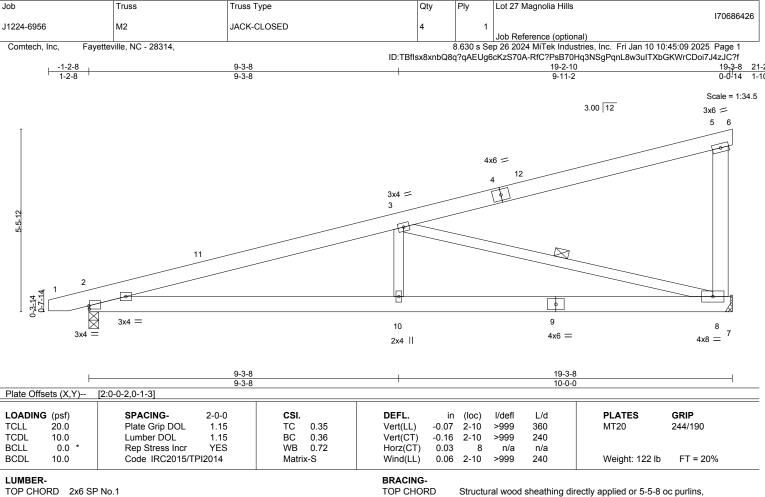
1 Row at midpt

January 13,2025

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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BOT CHORD

WEBS

except end verticals.

1 Row at midpt

Rigid ceiling directly applied or 10-0-0 oc bracing.

3-8

REACTIONS.

TOP CHORD BOT CHORD 2x6 SP No.1 **WEBS** 2x4 SP No.2 *Except*

5-8: 2x6 SP No.1

Max Horz 2=165(LC 8) Max Uplift 8=-103(LC 12), 2=-98(LC 8) Max Grav 8=778(LC 1), 2=816(LC 1)

(size) 8=Mechanical, 2=0-3-8

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1758/239, 5-8=-268/189 **BOT CHORD** 2-10=-383/1636, 8-10=-383/1636 **WEBS** 3-10=0/418, 3-8=-1601/364

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-10-15 to 3-5-14, Interior(1) 3-5-14 to 19-3-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=103.



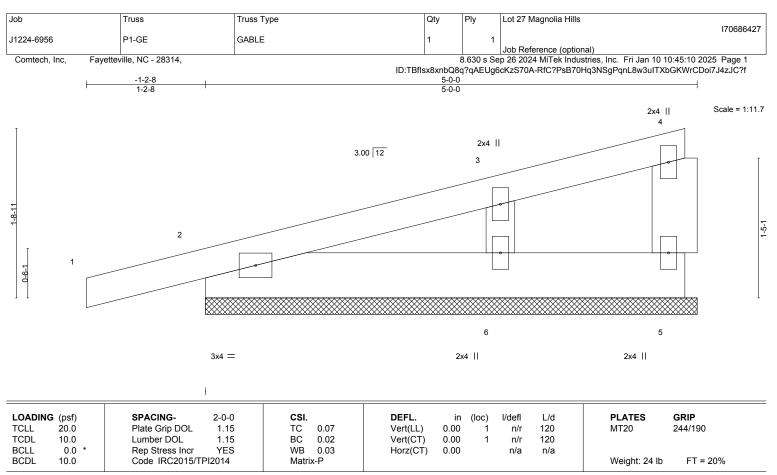


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building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





TOP CHORD

BOT CHORD

BRACING-LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1

WEBS 2x6 SP No.1 **OTHERS** 2x4 SP No.2

(size) 5=5-0-0, 2=5-0-0, 6=5-0-0

Max Horz 2=73(LC 8)

Max Uplift 5=-17(LC 8), 2=-92(LC 8), 6=-71(LC 12) Max Grav 5=40(LC 1), 2=189(LC 1), 6=225(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

REACTIONS.

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) Gable requires continuous bottom chord bearing.
- 4) Gable studs spaced at 2-0-0 oc.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2, 6.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



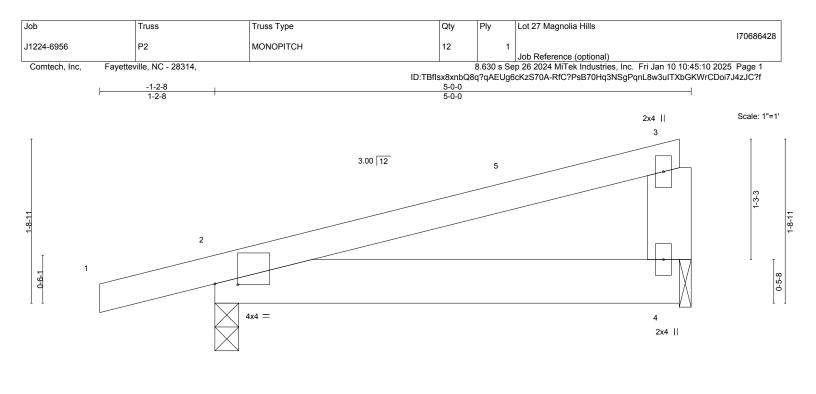


Plate Offsets (X,Y) [2	2:0-2-14,0-0-2]			
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d PLATES GRIP	
TCLL 20.0	Plate Grip DOL 1.15	TC 0.26	Vert(LL) -0.01 2-4 >999 360 MT20 244/190	
TCDL 10.0	Lumber DOL 1.15	BC 0.08	Vert(CT) -0.01 2-4 >999 240	
BCLL 0.0 *	Rep Stress Incr YES	WB 0.00	Horz(CT) 0.00 n/a n/a	
BCDL 10.0	Code IRC2015/TPI2014	Matrix-P	Wind(LL) 0.01 2-4 >999 240 Weight: 23 lb FT = 20%	

BRACING-

TOP CHORD

BOT CHORD

LUMBER-

WEBS

TOP CHORD 2x4 SP No.1 BOT CHORD 2x6 SP No.1

2x6 SP No.1 REACTIONS. (size) 2=0-3-0, 4=0-1-8 Max Horz 2=51(LC 8)

Max Uplift 2=-121(LC 8), 4=-71(LC 8) Max Grav 2=277(LC 1), 4=174(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 4-9-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 5) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4 except (jt=lb) 2 = 121.



Structural wood sheathing directly applied or 5-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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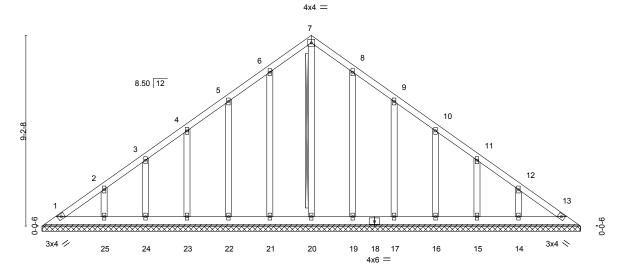


Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686429 J1224-6956 VA1 **GABLE** Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:11 2025 Page 1

Comtech, Inc, Fayetteville, NC - 28314,

Scale = 1:55.6

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 12-11-15 13-0-0



25-11-15 Plate Offsets (X,Y)--[8:0-0-0,0-0-0], [9:0-0-0,0-0-0], [10:0-0-0,0-0-0], [11:0-0-0,0-0-0], [12:0-0-0,0-0-0]LOADING (psf) SPACING-DEFL. in (loc) I/defl L/d **PLATES** GRIP TCLL 20.0 Plate Grip DOL 1.15 TC 0.05 Vert(LL) 999 244/190 n/a n/a MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.03 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.14 0.01 13 Horz(CT) n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Weight: 181 lb Matrix-S

25-11-15

LUMBER-TOP CHORD 2x4 SP No.1

2x6 SP No.1 **BOT CHORD OTHERS** 2x4 SP No.2 BRACING-

TOP CHORD BOT CHORD **WEBS**

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

2x4 SPF No.2 - 7-20

Fasten (2X) T and I braces to narrow edge of web with 10d (0.131"x3") nails, 6in o.c., with 3in minimum end distance. Brace must cover 90% of web length.

REACTIONS. All bearings 25-11-15.

Max Horz 1=-264(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1, 13, 21, 22, 23, 24, 19, 17, 16, 15 except 25=-126(LC 12),

14=-124(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 13, 20, 21, 22, 23, 24, 25, 19, 17, 16, 15, 14

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 1-2=-286/197

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 13, 21, 22, 23, 24, 19, 17, 16, 15 except (jt=lb) 25=126, 14=124.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.





Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686430 J1224-6956 VA2 VALLEY Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:11 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 22-2-12 11-1-6 11-1-6 Scale = 1:47.2 4x4 = 8.50 12 15 5 16 3x4 // 3x4 × 13 12 11 9 4x6 = 22-2-12 0-0-8 LOADING (psf) SPACING-2-0-0 CSI. DEFL. I/defl L/d **PLATES GRIP** (loc) 20.0 Plate Grip DOL 1.15 Vert(LL) 999 244/190 **TCLL** TC 0.15 n/a n/a MT20 TCDL 10.0 Lumber DOL 1.15 ВС 0.08 Vert(CT) n/a 999 n/a **BCLL** 0.0 Rep Stress Incr YES WB 0.15 Horz(CT) 0.00 n/a n/a

BRACING-

TOP CHORD

BOT CHORD

LUMBER-TOP CHORD

BCDL

2x4 SP No 1

BOT CHORD 2x6 SP No.1 **OTHERS** 2x4 SP No.2

10.0

REACTIONS. All bearings 22-1-11. Max Horz 1=-179(LC 8)

Max Uplift All uplift 100 lb or less at joint(s) 1, 7, 13, 8 except 12=-113(LC 12), 9=-113(LC 13)

All reactions 250 lb or less at joint(s) 1, 7 except 11=434(LC 22), 12=462(LC 19), 13=293(LC 19), Max Grav

Matrix-S

9=462(LC 20), 8=292(LC 20)

Code IRC2015/TPI2014

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

3-12=-326/218, 2-13=-263/189, 5-9=-326/218, 6-8=-263/189 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-8-7 to 5-1-3, Interior(1) 5-1-3 to 11-1-6, Exterior(2) 11-1-6 to 15-6-3, Interior(1) 15-6-3 to 21-6-5 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) All plates are 2x4 MT20 unless otherwise indicated.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 7, 13, 8 except (jt=lb) 12=113, 9=113.



Weight: 118 lb

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

FT = 20%



Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686431 VALLEY J1224-6956 VA3 Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:12 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 9-2-13 9-2-12 Scale = 1:41.6 4x4 = 3 8.50 12 2x4 || 2x4 || 10 2 4 11 3x4 // 3x4 × 8 13 6 9 12 $3x4 = 2x4 \parallel$ 2x4 || 2x4 || 18-5-9 0-0-8 18-5-1

Plate Off	sets (X,Y)	[4:0-0-0,0-0-0]									
LOADIN	G (psf)	SPACING- 2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.0	Plate Grip DOL 1.15	TC	0.22	Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL 1.15	BC	0.17	Vert(CT)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr YES	WB	0.10	Horz(CT)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2015/TPI2014	Matrix	:-S						Weight: 77 lb	FT = 20%

LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2 **BRACING-**

TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 18-4-8.

(lb) -Max Horz 1=149(LC 11)

Max Uplift All uplift 100 lb or less at joint(s) 1 except 9=-136(LC 12), 6=-136(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 8=408(LC 22), 9=516(LC 19), 6=516(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-9=-388/249, 4-6=-388/249 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-9 to 4-10-6, Interior(1) 4-10-6 to 9-2-13, Exterior(2) 9-2-13 to 13-7-9, Interior(1) 13-7-9 to 18-0-0 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=136, 6=136





Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686432 J1224-6956 VA4 VALLEY Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:12 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f 7-4-3 7-4-3 Scale = 1:31.3 4x4 = 8.50 12 11 10 2x4 || 2x4 || 12 3x4 / 8 7 6 2x4 || 2x4 || 2x4 || 14-7-14 Plate Offsets (X,Y)--[4:0-0-0,0-0-0] SPACING-DEFL. **PLATES GRIP** LOADING (psf) CSI. in (loc) I/defl L/d 20.Ó TCLL Plate Grip DOL 1.15 TC 0.14 Vert(LL) 999 MT20 244/190 n/a n/a TCDL 10.0 Lumber DOL 1.15 ВС 0.08 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.07 Horz(CT) 0.00 5 n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Matrix-S Weight: 59 lb BRACING-LUMBER-

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **OTHERS** 2x4 SP No.2 TOP CHORD **BOT CHORD** Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 14-7-5.

Max Horz 1=-117(LC 8) (lb) -

Max Uplift All uplift 100 lb or less at joint(s) 1 except 8=-108(LC 12), 6=-107(LC 13)

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=252(LC 1), 8=351(LC 19), 6=351(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-8=-305/211, 4-6=-305/211 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-9 to 4-10-6, Interior(1) 4-10-6 to 7-4-3, Exterior(2) 7-4-3 to 11-9-0, Interior(1) 11-9-0 to 14-2-13 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=108, 6=107,

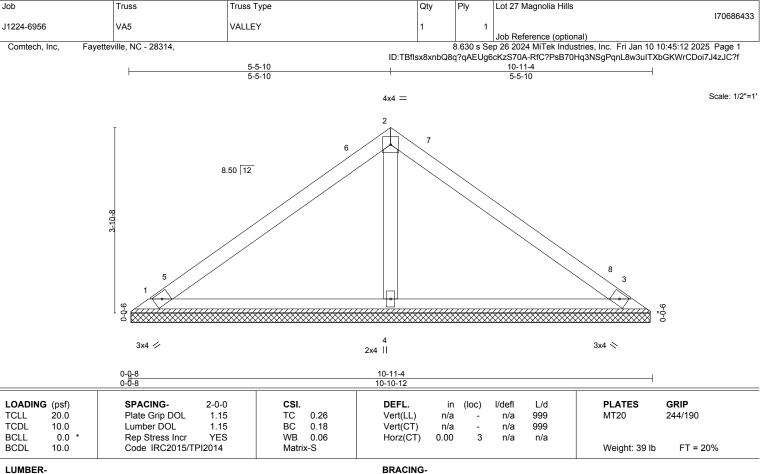


WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)





TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1

OTHERS 2x4 SP No.2

> 1=10-10-3, 3=10-10-3, 4=10-10-3 (size) Max Horz 1=-85(LC 8)

Max Uplift 1=-25(LC 12), 3=-33(LC 13)

Max Grav 1=202(LC 1), 3=203(LC 1), 4=395(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-9 to 4-10-6, Interior(1) 4-10-6 to 5-5-10, Exterior(2) 5-5-10 to 9-10-7, Interior(1) 9-10-7 to 10-5-11 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

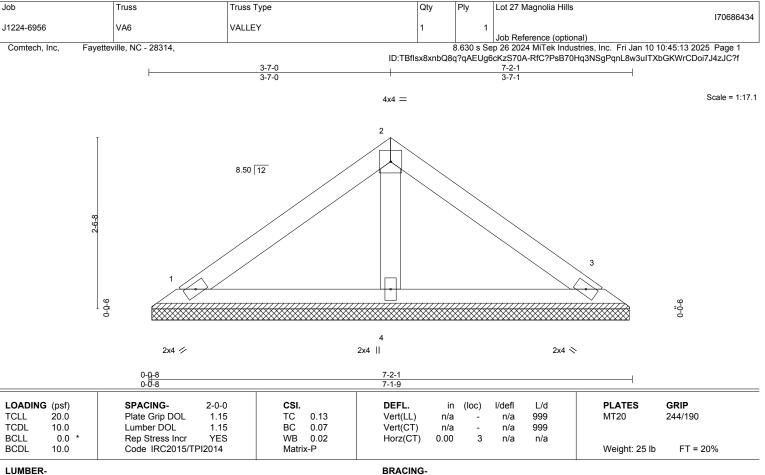
Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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TOP CHORD

BOT CHORD

LUMBER-

REACTIONS.

TOP CHORD 2x4 SP No.1 2x4 SP No.1

BOT CHORD **OTHERS** 2x4 SP No.2

1=7-1-0, 3=7-1-0, 4=7-1-0 (size) Max Horz 1=-53(LC 10)

Max Uplift 1=-22(LC 12), 3=-27(LC 13) Max Grav 1=138(LC 1), 3=138(LC 1), 4=223(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 6) Non Standard bearing condition. Review required.



Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

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Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 170686435 J1224-6956 VA7 VALLEY Job Reference (optional) 8.630 s Sep 26 2024 MiTek Industries, Inc. Fri Jan 10 10:45:13 2025 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 3-4-14 1-8-7 1-8-7 Scale = 1:8.8 3x4 = 2 8.50 12 3 9-0-0 9-0-0 2x4 // 2x4 × 3-4-6 Plate Offsets (X,Y)--[2:0-2-0,Edge] SPACING-DEFL. **PLATES GRIP** LOADING (psf) 2-0-0 CSI. in (loc) I/defl L/d 20.Ó 244/190 TCLL Plate Grip DOL 1.15 TC 0.02 Vert(LL) 999 MT20 n/a n/a TCDL 10.0 Lumber DOL 1.15 ВС 0.05 Vert(CT) n/a n/a 999 BCLL 0.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 3 n/a n/a Code IRC2015/TPI2014 FT = 20% **BCDL** 10.0 Matrix-P Weight: 10 lb LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 **TOP CHORD BOT CHORD** Structural wood sheathing directly applied or 3-4-14 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=3-3-13, 3=3-3-13

Max Horz 1=-21(LC 10) Max Uplift 1=-5(LC 12), 3=-5(LC 13)

Max Grav 1=99(LC 1), 3=99(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.



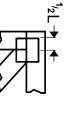
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

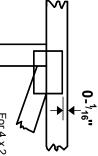


Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths Center plate on joint unless x, y



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

₹

connector plates required direction of slots in This symbol indicates the

* Plate location details available in MiTek software or upon request

PLATE SIZE

4 × 4

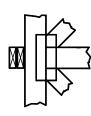
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing if indicated by text in the bracing section of the ndicated by symbol shown and/or

BEARING



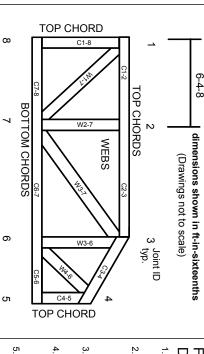
Min size shown is for crushing only number/letter where bearings occur reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

Industry Standards:

DSB-22:

ANSI/TPI1: National Design Specification for Metal Installing, Restraining & Bracing of Metal Guide to Good Practice for Handling, Building Component Safety Information, Design Standard for Bracing. Plate Connected Wood Trusses Plate Connected Wood Truss Construction.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

established by others section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5 Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- œ Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated

	1	01-02-00 ET1	20-03-00	20-03-00		<u></u>	Joint 18 69.7 lbs.	Joint 19 160.8 lbs.	Joint 20 142.8 lbs.	Joint 21 147.7 lbs.	Joint 22 146.4 lbs.
		LII	20-03-00	20-03-00		<u> </u>	U3.1 IDS.	100.0 ibs.	142.0 105.	147.7 105.	140.4 105.
	1	01-02-00 ET2	13-00-08	13-00-08		<u></u>	Joint 12 39.7 lbs.	Joint 13 124.3 lbs.	Joint 14 151.6 lbs.	Joint 15 145.4 lbs.	Joint 16 147.0 lbs.
			10 00 00	10 00 00			00.7 1201	12 110 1201	10110100	. 10.1 120.	
	1	01-02-00 ET3	21-03-08	21-03-08		<u> </u>	Joint 19 53.4 lbs.	Joint 20 140.6 lbs.	Joint 21 148.4 lbs.	Joint 22 146.2 lbs.	Joint 23 146.8 lbs.
			1	1			I				
	1	01-02-00 F1	32-11-00	32-11-00		<u> </u>	Joint 24 49.7 lbs.	Joint 30 2863.7 lbs.	Joint 41 1576.3 lbs.		
1							-246.9 lbs.	2402.5 lbs.	1219.9 lbs.		
	1	01-02-00			<u> =</u>	-	Joint 22	Joint 27	Joint 37		
	5	F2	32-11-00	32-11-00		<u></u>	621.5 lbs. 15.9 lbs.	2149.8 lbs. 1163.6 lbs.	953.1 lbs. 232.4 lbs.		
vw vw	13	01-02-00 F3	20-03-00	20-03-00		<u> </u>	Joint 14 874.4 lbs.	Joint 24 874.4 lbs.			
							434.5 lbs.	434.5 lbs.			
	1	01-02-00			<u></u> =	_ _	Joint 15	Joint 18	Joint 25		
	11	F4	21-03-08	21-03-08	<u> </u>	Щ	266.5 lbs. 12.7 lbs.	1250.8 lbs. 558.9 lbs.	837.2 lbs. 226.6 lbs.		
		01-02-00	00.00.00	00.00.00	<u>-</u> -	TI	Joint 9	Joint 14	Joint 15	Joint 16	
N PT A 30	1	F5-GR	08-03-08	08-03-08			793.3 lbs. 645.7 lbs.	1114.5 lbs. 953.1 lbs.	99.4 lbs.	255.8 lbs. 236.0 lbs.	

Joint 5

185.6 lbs.

125.9 lbs.

Joint 8

185.6 lbs.

125.9 lbs.

01-02-00

F6

03-07-08

03-07-08

SOLD Raeford, NC 28376 T O (910) 988-8172 SHIP

60 miles round trip

Precision Custom Homes and 165 Mahogany Ct. Cameron, NC

SPECIAL INSTRUCTIONS:

Modified from Lot 24 Magnolia Hills (15'x15') CP

PLAN SEAL DATE: 1/13/25 DATE BY

BUILDING DEPARTMENT OVERHANG INFO Floor Order

HEEL HEIGHT 00-06-08 **REQ. LAYOUTS REQ. ENGINEERING** QUOTE END CUT RETURN LAYOUT NB 01/14/25 **CUTTING** NB 01/14/25 PLUMB **GABLE STUDS** 24 IN. OC JOBSITE JOBSITE

FLOOR TRUSSES

LOADING INFORMATION TCLL-TCDL-BCLL-BCDL STRESS INCR. 40.0,10.0,0.0,5.0 1.00

FLOOR TRUSS SPACING: 24.0 IN. O.C. (TYP.)

DEPTH FLOOR QTY BASE O/A **INT BEARING END TYPE REACTIONS PROFILE** PLY ID **SPAN SPAN** LEFT RIGHT SIZE LOCATION

01-02-00 F7-GR

08-03-08

Joint 9 630.1 lbs. 506.4 lbs. Joint 14 1198.7 lbs. 1043.6 lbs. Joint 15 158.6 lbs. 0.7 lbs.

Joint 16 279.9 lbs. 248.5 lbs.

ITFMS

T O

/ / L/V/	J			_	
QTY	ITEM TYPE	SIZE	LENGTH FT-IN-16	PART NUMBER	NOTES
4	BlueLinx (F)	LVL, Metsa(F) 2.0,	07-00-00		BM2 & BM3
2	BlueLinx (F)	LVL, Metsa(F) 2.0,	06-00-00		BM4
4	BlueLinx (F)	LVL, Metsa(F) 2.0,	16-00-00		BM5
3	BlueLinx (F)	LVL, Metsa(F) 2.0, 14"	21-00-00		BM1
2	BlueLinx (F)	LVL, Metsa(F) 2.0, 16"	21-00-00		GDH
9	Hangers, USP	MSH422			SIMPSON (THA422)
				•	



RE: J1224-6957

Lot 27 Magnolia Hills

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J1224-6957

Lot/Block: Model:
Address: Subdivision:
City: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Design Program: MiTek 20/20 8.4

Wind Code: N/A Wind Speed: N/A mph Roof Load: N/A psf Floor Load: 55.0 psf

This package includes 10 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	168605140	ET1	10/3/2024
2	168605141	ET2	10/3/2024
3	168605142	ET3	10/3/2024
4	168605143	F1	10/3/2024
5	168605144	F2	10/3/2024
6	168605145	F3	10/3/2024
7	168605146	F4	10/3/2024
8	168605147	F5-GR	10/3/2024
9	168605148	F6	10/3/2024
10	168605149	F7-GR	10/3/2024

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



October 03, 2024

Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
J1224-6957	ET1	GABLE	1	1	168605140
31224-0937		GABLE	'	'	Job Reference (optional)

0-1-8

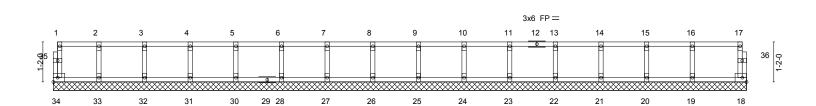
3x4 =

8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:32 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-11-8

Scale = 1:33.7

3x4 =



	-8-0 4-0-0 5-4-0 6-8-0 -4-0 1-4-0 1-4-0 1-4-0	8-0-0 9-4-0 1-4-0 1-4-0	10-8-0 12-0-0 1-4-0 1-4-0		14-8-0 1-4-0 1-4-0	17-4-0 1-4-0 1-4-0 18-8-0 1-4-0	20-3-0
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.09 BC 0.01 WB 0.03 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) I/ n/a - n/a - 0.00 18	/defl L/d n/a 999 n/a 999 n/a n/a	PLATES MT20 Weight: 84 lb	GRIP 244/190 FT = 20%F, 11%E

LUMBER-BRACING-

3x6 FP =

TOP CHORD 2x4 SP No.1(flat) 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.3(flat) **WEBS**

OTHERS 2x4 SP No.3(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 20-3-0.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 34, 18, 33, 32, 31, 30, 28, 27, 26, 25, 24, 23, 22, 21, 20, 19

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.

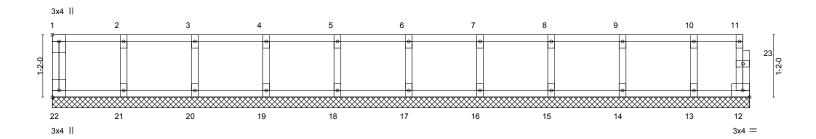


Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
J1224-6957	ET2	GABLE	1	1	168605141
31224-0937	E12	GABLE		'	Job Reference (optional)

8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:32 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0₁1₇8

Scale = 1:21.5



<u> </u>	1-4-0	2-8-0	4-0-0	5-4-0	_	6-8-0	8-0-0			-4-0	10-8-0	12-0-0	13-0-8
Dioto Offic	1-4-0 sets (X,Y)	1-4-0 '	1-4-0	1-4-0		1-4-0	1-4-0) .	1-	-4-0	1-4-0	1-4-0	1-0-8
Flate Oils	sels (A, T)	[1:Edge,0-1-8], [22:Edge	,0-1-0]										
LOADING	(psf)	SPACING-	2-0-0	CSI.			DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.06		Vert(LL)	n/a	-	n/a	999	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.01		Vert(CT)	n/a	-	n/a	999		
BCLL	0.0	Rep Stress Incr	YES	WB	0.03		Horz(CT)	0.00	12	n/a	n/a		
BCDL	5.0	Code IRC2015/TI	PI2014	Matri	x-R							Weight: 56 lb	FT = 20%F, 11%E

LUMBER-**BRACING-**TOP CHORD

2x4 SP No.1(flat) TOP CHORD BOT CHORD 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat) **OTHERS** 2x4 SP No.3(flat)

except end verticals. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

Structural wood sheathing directly applied or 6-0-0 oc purlins,

REACTIONS. All bearings 13-0-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 22, 12, 21, 20, 19, 18, 17, 16, 15, 14, 13

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.





Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
J1224-6957	ET3	GABLE	1	1	168605142
31224-0937	[213	GABLE	'	'	Job Reference (optional)

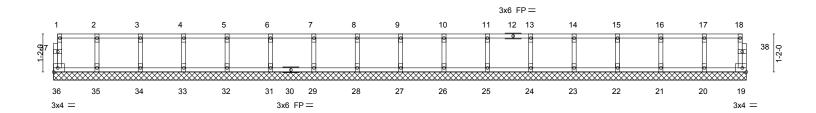
0-<u>1</u>-8

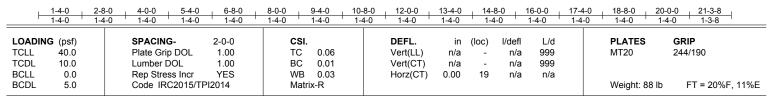
8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:33 2024 Page 1

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-1-8

Scale = 1:35.4





LUMBER-BRACING-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) 2x4 SP No.3(flat) WEBS **OTHERS** 2x4 SP No.3(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 21-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 36, 19, 35, 34, 33, 32, 31, 29, 28, 27, 26, 25, 24, 23, 22, 21, 20

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) All plates are 1.5x3 MT20 unless otherwise indicated.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable requires continuous bottom chord bearing.
- 4) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 5) Gable studs spaced at 1-4-0 oc.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 168605143 **FLOOR** J1224-6957 F1 Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:34 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-1-8 HI 1-3-0

2-4-4

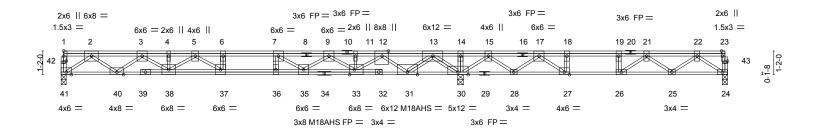
2-3-12

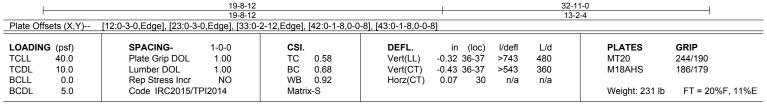
Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

0-1-8 Scale = 1:56.9





TOP CHORD

BOT CHORD

LUMBER-**BRACING-**

TOP CHORD 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP 2400F 2.0E(flat)

BOT CHORD 2x4 SP No.3(flat) **WEBS**

(size) 41=0-3-0, 30=0-3-8, 24=0-3-0

Max Uplift 24=-247(LC 3)

Max Grav 41=1576(LC 3), 30=2864(LC 1), 24=50(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-3504/0, 3-4=-6223/0, 4-5=-6223/0, 5-6=-7661/0, 6-7=-7661/0, 7-9=-6965/0,

9-11=-4970/0, 11-12=-4970/0, 12-13=-746/0, 13-14=0/4779, 14-15=0/4779, 15-17=0/3457, 17-18=0/1956, 18-19=0/1956, 19-21=0/1956, 21-22=0/669

BOT CHORD 40-41=0/2080, 38-40=0/5030, 37-38=0/7087, 36-37=0/7661, 35-36=0/7661, 33-35=0/6235,

31-33=0/3400, 30-31=-1894/0, 28-30=-3921/0, 27-28=-2958/0, 26-27=-1956/0,

25-26=-1032/0, 24-25=-366/28

WEBS 2-41=-2547/0, 2-40=0/1809, 3-40=-1939/0, 3-38=0/1454, 13-30=-3542/0, 13-31=0/3079,

12-31=-3335/0, 12-33=0/1934, 9-33=-1560/0, 9-35=0/959, 5-38=-1054/0, 5-37=0/835, 6-37=-369/0, 7-35=-995/0, 15-30=-1141/0, 15-28=0/870, 17-28=-919/0, 22-24=-31/451 22-25=-384/0, 21-25=0/461, 21-26=-1169/0, 19-26=0/606, 17-27=0/1513, 18-27=-780/0

NOTES-

REACTIONS.

TOP CHORD

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x6 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 247 lb uplift at joint 24.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- CAUTION, Do not erect truss backwards.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 783 lb down at 15-9-12 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 24-41=-5, 1-12=-170, 12-23=-50

Concentrated Loads (lb) Vert: 12=-743(F)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall

building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
					168605144
J1224-6957	F2	Floor	5	1	
					Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

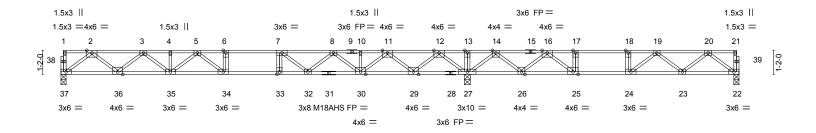
8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:34 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

0-1-8 HI 1-3-0

2-4-4

2-3-12

0-1-8 Scale = 1:55.9



14-7-4 14-7-4			19-8-12 5-1-8	+	32-11-0 13-2-4	
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.70 BC 0.65 WB 0.70 Matrix-S	Vert(CT) -0.	in (loc) l/defl L/d .27 34 >858 480 .38 34 >627 360 .05 22 n/a n/a	MT20 2	GRIP 244/190 186/179 FT = 20%F, 11%E

LUMBER-BRACING-

TOP CHORD 2x4 SP 2400F 2.0E(flat) 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP No.3(flat) WEBS

TOP CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins,

except end verticals.

BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. 37=0-3-0, 27=0-3-8, 22=0-3-0

Max Grav 37=953(LC 10), 27=2150(LC 1), 22=621(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-2014/0, 3-4=-3356/0, 4-5=-3356/0, 5-6=-3950/0, 6-7=-3950/0, 7-8=-3454/0,

8-10=-2300/0, 10-11=-2300/0, 11-12=-379/347, 12-13=0/2735, 13-14=0/2735, 14-16=-486/1427, 16-17=-1633/515, 17-18=-1633/515, 18-19=-1633/515, 19-20=-1198/26

36-37=0/1196, 35-36=0/2803, 34-35=0/3743, 33-34=0/3950, 32-33=0/3950, 30-32=0/3014,

29-30=-29/1454, 27-29=-1220/0, 26-27=-1763/0, 25-26=-1049/1109, 24-25=-515/1633,

23-24=-155/1563 22-23=0/765

WEBS 2-37=-1497/0, 2-36=0/1065, 3-36=-1027/0, 3-35=0/705, 12-27=-1900/0, 12-29=0/1468,

11-29=-1445/0, 11-30=0/1128, 8-30=-947/0, 8-32=0/684, 5-35=-495/0, 5-34=-175/543,

7-32=-902/0, 14-27=-1409/0, 14-26=0/960, 16-26=-1036/0, 20-22=-957/0, 20-23=-38/563, 19-23=-475/168, 19-24=-452/87, 16-25=0/1148, 17-25=-491/0

NOTES-

BOT CHORD

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) All plates are 3x4 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 6) CAUTION, Do not erect truss backwards.



October 3,2024

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
11004 6057		FLOOR	12	1	168605145
J1224-6957	1-3	FLOOR	13	1	Job Reference (optional)

Comtech, Inc, Fayetteville, NC - 28314,

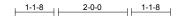
8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:35 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Structural wood sheathing directly applied or 6-0-0 oc purlins,

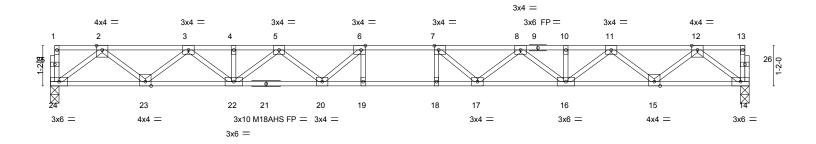
Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.





0-1-8 Scale = 1:33.4



				20-3-0					
Plate Offsets (X	Y) [6:0-1-8,Edge], [7:0-1-8	,Edge]							
LOADING (psf	SPACING-	1-7-3	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 40.0	Plate Grip DOL	1.00	TC 0.27	Vert(LL)	-0.30 18-19	>801	480	MT20	244/190
TCDL 10.0	Lumber DOL	1.00	BC 0.55	Vert(CT)	-0.41 18-19	>581	360	M18AHS	186/179
BCLL 0.0	Rep Stress Incr	YES	WB 0.49	Horz(CT)	0.07 14	n/a	n/a		
BCDL 5.0	Code IRC2015/7	TPI2014	Matrix-S					Weight: 102 lb	FT = 20%F, 11%E

BRACING-

TOP CHORD

BOT CHORD

20-3-0

LUMBER-

TOP CHORD 2x4 SP 2400F 2.0E(flat) BOT CHORD 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 24=0-3-0, 14=0-3-0 Max Grav 24=874(LC 1), 14=874(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2-3=-1894/0, 3-4=-3231/0, 4-5=-3231/0, 5-6=-3966/0, 6-7=-4188/0, 7-8=-3966/0, TOP CHORD

8-10=-3231/0, 10-11=-3231/0, 11-12=-1894/0

BOT CHORD $23-24=0/1104,\ 22-23=0/2656,\ 20-22=0/3714,\ 19-20=0/4188,\ 18-19=0/4188,\ 17-18=0/4188,$

16-17=0/3714, 15-16=0/2656, 14-15=0/1104

2-24=-1383/0, 2-23=0/1028, 3-23=-993/0, 3-22=0/734, 12-14=-1383/0, 12-15=0/1028, WFBS

 $11-15 = -993/0,\ 11-16 = 0/734,\ 5-22 = -617/0,\ 5-20 = 0/453,\ 8-16 = -617/0,\ 8-17 = 0/453,$

7-17=-555/72, 6-20=-555/72

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- All plates are MT20 plates unless otherwise indicated.
 All plates are 1.5x3 MT20 unless otherwise indicated.
- 4) Plates checked for a plus or minus 1 degree rotation about its center.
- 5) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.





Job	Truss	Truss Type	Qty	Ply	Lot 27 Magnolia Hills
14004 6057		Floor	11	1	168605146
J1224-6957	F4	Floor	11	1	Job Reference (optional)

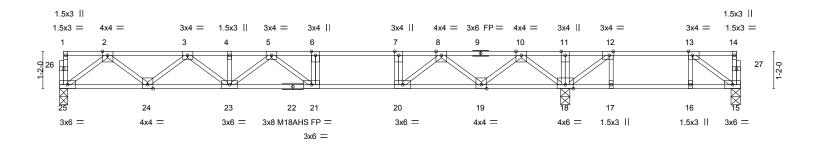
Comtech, Inc. Fayetteville, NC - 28314,

8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:35 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f



2-4-4 1-2-8

2-4-4



-		15-9-12 15-9-12		+	21-3-8 5-5-12	
Plate Offsets (X,Y)	[12:0-1-8,Edge], [13:0-1-8,Edge]					
LOADING (psf) TCLL 40.0 TCDL 10.0 BCLL 0.0 BCDL 5.0	SPACING- 2-0-0 Plate Grip DOL 1.00 Lumber DOL 1.00 Rep Stress Incr YES Code IRC2015/TPI2014	CSI. TC 0.67 BC 0.62 WB 0.49 Matrix-S	DEFL. in (loc) l/der Vert(LL) -0.26 21-23 >72 Vert(CT) -0.35 21-23 >53 Horz(CT) 0.04 15 n/	4 480 5 360	PLATES MT20 M18AHS Weight: 108 lb	GRIP 244/190 186/179 FT = 20%F, 11%E

TOP CHORD

BOT CHORD

LUMBER-**BRACING-**

TOP CHORD 2x4 SP 2400F 2.0E(flat) **BOT CHORD** 2x4 SP 2400F 2.0E(flat)

WEBS 2x4 SP No.3(flat)

REACTIONS. (size) 25=0-3-0, 18=0-3-8, 15=0-3-8

Max Grav 25=837(LC 10), 18=1251(LC 9), 15=267(LC 4)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1720/0, 3-4=-2792/0, 4-5=-2792/0, 5-6=-2928/0, 6-7=-2928/0, 7-8=-2928/0,

8-10=-1500/0, 10-11=0/406, 11-12=0/406, 12-13=-283/78

BOT CHORD $24 - 25 = 0/1041,\ 23 - 24 = 0/2381,\ 21 - 23 = 0/3024,\ 20 - 21 = 0/2928,\ 19 - 20 = 0/2225,\ 18 - 19 = 0/810,\ 19 - 20 = 0/2225,\ 18 - 19 = 0/222$

17-18=-78/283, 16-17=-78/283, 15-16=-78/283

2-25=-1303/0, 2-24=0/883, 3-24=-861/0, 3-23=0/525, 5-23=-296/0, 5-21=-310/289, WFBS

7-20=-456/0, 10-18=-1382/0, 10-19=0/910, 8-19=-962/0, 12-18=-670/0, 13-15=-346/99,

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) All plates are MT20 plates unless otherwise indicated.
- 3) Plates checked for a plus or minus 1 degree rotation about its center.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 6-0-0 oc bracing.

except end verticals.

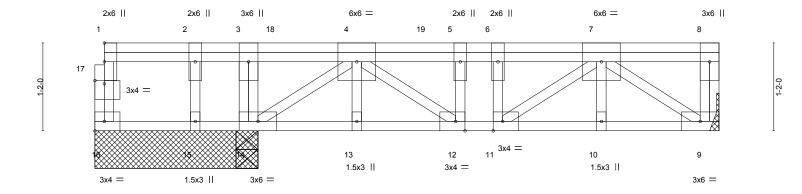
Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 168605147 J1224-6957 F5-GR **GABLE** Job Reference (optional)

Fayetteville, NC - 28314, Comtech, Inc.

8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:36 2024 Page 1 ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



Scale = 1:15.3



		1-4-0	2-0-6 Z _[2	-p 3-0-6		4-3-0				0-3-0		
	1	1-4-0	0-8-8 0 ^l -1	-8 1-4-8	ı	0-10-8				3-10-8		ı
Plate Offs	sets (X,Y)	[11:0-1-8,Edge], [12:0	0-1-8,Edge],	[17:0-1-8,0-0-8]								
LOADING	(psf)	SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DO	L 1.00	TC	0.16	Vert(LL)	-0.01	12	>999	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.25	Vert(CT)	-0.02	12	>999	360		
BCLL	0.0	Rep Stress Inc	r NO	WB	0.27	Horz(CT)	0.01	9	n/a	n/a		
BCDL	5.0	Code IRC201	5/TPI2014	Matr	ix-S						Weight: 58 lb	FT = 20%F, 11%E

LUMBER-**BRACING-**

TOP CHORD 2x4 SP No.1(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, BOT CHORD 2x4 SP No.1(flat) except end verticals. **WEBS** 2x4 SP No.3(flat) **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 2-2-0 except (jt=length) 9=Mechanical.

Max Uplift All uplift 100 lb or less at joint(s) 15 (lb) -

Max Grav All reactions 250 lb or less at joint(s) 15 except 16=256(LC 4), 9=793(LC 4), 14=1115(LC 4), 14=1106(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 4-5=-1242/0, 5-6=-1242/0, 6-7=-1242/0

2x4 SP No.3(flat)

BOT CHORD 13-14=0/862, 12-13=0/862, 11-12=0/1242, 10-11=0/940, 9-10=0/940

WEBS 3-14=-485/0, 4-14=-1024/0, 7-9=-1137/0, 7-11=0/371, 4-12=0/471, 5-12=-264/0

NOTES-

OTHERS

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable studs spaced at 1-4-0 oc.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15.
- 6) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 7) CAUTION, Do not erect truss backwards.
- 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 173 lb down at 0-2-4, 141 lb down at 2-5-4, and 166 lb down at 4-5-4, and 166 lb down at 6-5-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00 Uniform Loads (plf)

Vert: 9-16=-10, 1-8=-220

Concentrated Loads (lb) Vert: 1=-123(F) 7=-86(F) 18=-86(F) 19=-86(F)



October 3,2024



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 168605148 J1224-6957 F6 **FLOOR** Job Reference (optional)
8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:36 2024 Page 1 Comtech, Inc, Fayetteville, NC - 28314, ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f 0-8-0 1-6-8 1 3x4 || 4 3x4 II 3x4 = Scale = 1:8.6 3x6 =1.5x3 II 1.5x3 II 5 8 3x6 = Plate Offsets (X,Y)--[1:Edge,0-1-8], [2:0-1-8,Edge], [3:0-1-8,Edge] SPACING-DEFL. **PLATES GRIP** LOADING (psf) CSI. in (loc) I/defl L/d 40.Ó 0.08 244/190 TCLL Plate Grip DOL 1.00 TC Vert(LL) -0.00 >999 480 MT20 **TCDL** 10.0 Lumber DOL 1.00 ВС 0.04 Vert(CT) -0.00 >999 360 BCLL 0.0 Rep Stress Incr YES WB 0.05 Horz(CT) 0.00 5 n/a n/a Code IRC2015/TPI2014 FT = 20%F. 11%E **BCDL** 5.0 Matrix-S Weight: 22 lb **BRACING-**

TOP CHORD

BOT CHORD

LUMBER-

TOP CHORD 2x4 SP No.1(flat) **BOT CHORD** 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat)

REACTIONS. (size) 5=Mechanical, 8=Mechanical Max Grav 5=186(LC 1), 8=186(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Refer to girder(s) for truss to truss connections.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.



Structural wood sheathing directly applied or 3-7-8 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.



building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

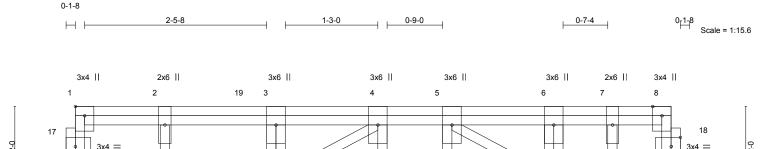


Job Truss Truss Type Qty Ply Lot 27 Magnolia Hills 168605149 J1224-6957 F7-GR **GABLE** Job Reference (optional)
8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 1 12:24:36 2024 Page 1

Fayetteville, NC - 28314, Comtech, Inc,

6x6 =

ID:TBflsx8xnbQ8q?qAEUg6cKzS70A-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f



	<u> </u>	1-4-0	2-10-0	2-11-8		6-5-				7-4-8 8-3-8	
		1-4-0	1-6-0	0-1-8		3-6-)		<u> </u>	0-11-0 ' 0-11-	<u>0 </u>
Plate Offs	sets (X,Y)	[1:Edge,0-1-8], [17:0-1-8	8,0-1-8], [18:0-	1-8,0-1-8]							
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL	40.0	Plate Grip DOL	1.00	TC	0.54	Vert(LL)	-0.08 11-12	>797	480	MT20	244/190
TCDL	10.0	Lumber DOL	1.00	BC	0.67	Vert(CT)	-0.11 11-12	>570	360		
BCLL	0.0	Rep Stress Incr	NO	WB	0.21	Horz(CT)	0.00 9	n/a	n/a		
BCDL	5.0	Code IRC2015/T	TPI2014	Matrix	K-S					Weight: 65 lb	FT = 20%F, 11%E

13

2x6 ||

12

2x6 ||

LUMBER-BRACING-

TOP CHORD 2x4 SP No.1(flat) BOT CHORD 2x4 SP No.1(flat) **WEBS** 2x4 SP No.3(flat)

OTHERS 2x4 SP No.3(flat) TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins,

11

6x6 =

10

2x6 ||

6x6 =

except end verticals.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

All bearings 2-11-8 except (jt=length) 9=0-3-8.

(lb) - Max Grav All reactions 250 lb or less at joint(s) 15 except 16=280(LC 4), 14=1199(LC 1), 14=1199(LC 1), 14=1199(LC 1), 9=630(LC 4)

6x6 =

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

2x6 ||

TOP CHORD 8-9=-410/0, 4-5=-778/0

BOT CHORD 13-14=0/778, 12-13=0/778, 11-12=0/778

WFBS 4-14=-905/0, 5-11=-892/0, 2-15=-257/0, 3-14=-550/0

NOTES-

- 1) Unbalanced floor live loads have been considered for this design.
- 2) Plates checked for a plus or minus 1 degree rotation about its center.
- 3) Gable studs spaced at 1-4-0 oc.
- 4) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-10d (0.131" X 3") nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
- 5) CAUTION, Do not erect truss backwards.
- 6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 173 lb down at 0-2-4, 141 lb down at 2-5-4, and 166 lb down at 4-5-4, and 166 lb down at 6-5-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
- 7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Floor Live (balanced): Lumber Increase=1.00, Plate Increase=1.00

Uniform Loads (plf)

Vert: 9-16=-10, 1-8=-220

Concentrated Loads (lb)

Vert: 1=-123(B) 4=-86(B) 6=-86(B) 19=-86(B)



October 3,2024

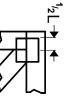
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPII Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

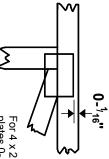


Symbols

PLATE LOCATION AND ORIENTATION



offsets are indicated and fully embed teeth Apply plates to both sides of truss Dimensions are in ft-in-sixteenths Center plate on joint unless x, y



edge of truss. plates 0- 1/16" from outside For 4 x 2 orientation, locate

connector plates required direction of slots in This symbol indicates the

* Plate location details available in MiTek software or upon request

PLATE SIZE

4 × 4

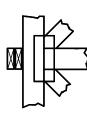
to slots. Second dimension is the length parallel to slots. width measured perpendicular The first dimension is the plate

LATERAL BRACING LOCATION



output. Use T or I bracing if indicated by text in the bracing section of the ndicated by symbol shown and/or

BEARING



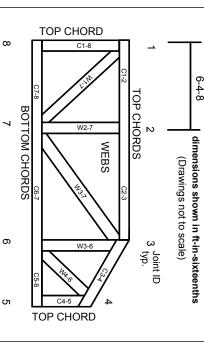
Min size shown is for crushing only number/letter where bearings occur reaction section indicates joint (supports) occur. Icons vary but Indicates location where bearings

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Installing, Restraining & Bracing of Metal Guide to Good Practice for Handling, Building Component Safety Information, Design Standard for Bracing. Plate Connected Wood Trusses Plate Connected Wood Truss Construction.

DSB-22:

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

ICC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

truss unless otherwise shown Trusses are designed for wind loads in the plane of the

established by others section 6.3 These truss designs rely on lumber values Lumber design values are in accordance with ANSI/TPI 1

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MiTek



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

General Safety Notes

Damage or Personal Injury Failure to Follow Could Cause Property

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI
- Ņ Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- ယ Never exceed the design loading shown and never stack materials on inadequately braced trusses
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other

5

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.

œ

- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the camber for dead load deflection responsibility of truss fabricator. General practice is to
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted
- 15. Connections not shown are the responsibility of others
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable project engineer before use environmental, health or performance risks. Consult with
- 19. Review all portions of this design (front, back, words is not sufficient. and pictures) before use. Reviewing pictures alone
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated



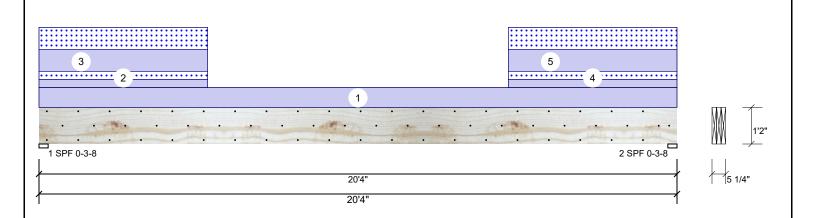
Date: 1/14/2025

Input by: Neal Baggett Job Name: LOT 27 MAGNOLIA HILLS Page 1 of 13

Project #:

1.750" X 14.000" 3-Ply - PASSED **Kerto-S LVL** BM₁

Level: Level



Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Direction Live Snow Wind Const Type: Dead Plies: 3 Design Method: ASD Vertical 0 2362 976 0 0 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 0 2362 976 0 0 Deflection LL: 480 Load Sharing: Yes Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temperature: Temp <= 100°F **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case Ld. Comb. 1-SPF 3.500" D+S Vert 43% 2362 / 976 3337 L 2 - SPF 3.500" Vert 43% 2362 / 976 3337 L D+S

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9135 ft-lb	10'2"	37907 ft-lb	0.241 (24%)	D	Uniform
Unbraced	11538 ft-lb	10'2"	11541 ft-lb	1.000 (100%)	D+S	L
Shear	2628 lb	18'10 1/2"	18032 lb	0.146 (15%)	D+S	L
LL Defl inch	0.085 (L/2795)	10'2 1/16"	0.497 (L/480)	0.172 (17%)	S	L
TL Defl inch	0.381 (L/627)	10'2 1/16"	0.662 (L/360)	0.574 (57%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6". Nail from both sides.
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 15'1 11/16" o.c.
- 7 Bottom must be laterally braced at end bearings.

O Lateral Sieric	ierriess ratio paseu t	on single ply widin.								
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Part. Uniform	0-0-0 to 5-4-8		Тор	49 PLF	0 PLF	49 PLF	0 PLF	0 PLF	J2
3	Tie-In	0-0-0 to 5-4-8	6-7-8	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	STICK FRAMING
4	Part. Uniform	14-11-8 to 20-4-0		Тор	49 PLF	0 PLF	49 PLF	0 PLF	0 PLF	J2
_										

Continued on page 2...

Notes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Version 23.40.705 Powered by iStruct™ Dataset: 24051401.1529

- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code
- approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us

|--|

This design is valid until 6/28/2026



6. For flat roofs provide proper drainage to prevent ponding





1/14/2025 Input by:

Neal Baggett Job Name: LOT 27 MAGNOLIA HILLS Page 3 of 13

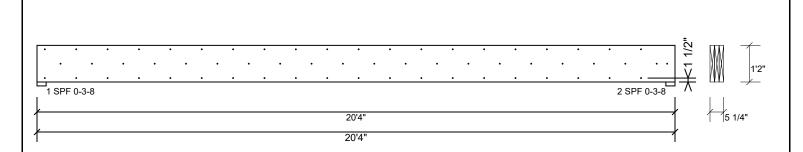
Project #:

Kerto-S LVL BM₁

1.750" X 14.000"

3-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Nail from both sides. Maximum end distance not to exceed

Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1 00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- For flat roofs provide proper drainage to prevent ponding

Manufacturer Info loor www.metsawood.com/us

М	etsä Wood
30	1 Merritt 7 Building, 2nd F
N	orwalk, CT 06851
/8	00) 622-5850

This design is valid until 6/28/2026





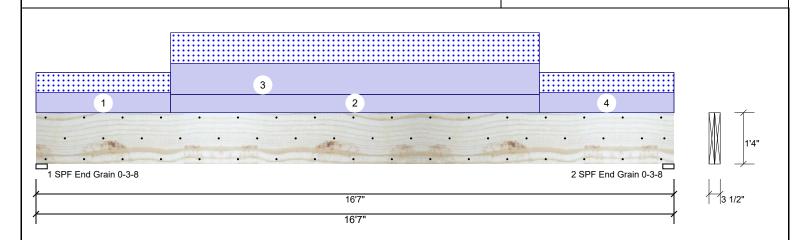
1/14/2025

Input by: Neal Baggett Job Name: LOT 27 MAGNOLIA HILLS Page 4 of 13

Project #:

1.750" X 16.000" **Kerto-S LVL** 2-Ply - PASSED **GDH**

Level: Level



Member Inform	nation		
Type:	Header	Application:	Floor
Plies:	2	Design Method:	ASD
Moisture Condition:	Dry	Building Code:	IBC 2012
Deflection LL:	480	Load Sharing:	No
Deflection TL:	360	Header Supports	No
Importance:	Normal - II	Glass:	
Temperature:	Temp <= 100°F	Deck:	Not Checked

Reactions	UNPATTERNED lb (Uplift)	

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	2119	1441	0	0
2	Vertical	0	2119	1441	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	16195 ft-lb	8'3 1/2"	39750 ft-lb	0.407 (41%)	D+S	L
Unbraced	16195 ft-lb	8'3 1/2"	39750 ft-lb	0.407 (41%)	D+S	L
Shear	3126 lb	14'11 1/2"	13739 lb	0.228 (23%)	D+S	L
LL Defl inch	0.135 (L/1439)	8'3 9/16"	0.404 (L/480)	0.333 (33%)	S	L
TL Defl inch	0.347 (L/559)	8'3 9/16"	0.538 (L/360)	0.644 (64%)	D+S	L

Bearings

Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	35%	2119 / 1441	3561	L	D+S
2 - SPF End Grain	3.500"	Vert	35%	2119 / 1441	3561	L	D+S

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be continuously laterally braced.
- 7 Bottom must be laterally braced at bearings.
- 8 Lateral slenderness ratio based on single ply width.

o Eater ar elemantineer ratio based on enigle pry matri											
	ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
	1	Tie-In	0-0-0 to 3-6-0	6-7-8	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	STICK FRAMING
	2	Part. Uniform	3-6-0 to 13-1-0		Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	DORMER WALL
	3	Part. Uniform	3-6-0 to 13-1-0		Тор	204 PLF	0 PLF	204 PLF	0 PLF	0 PLF	M2
	4	Tie-In	13-1-0 to 16-7-0	6-7-8	Тор	20 PSF	0 PSF	20 PSF	0 PSF	0 PSF	STICK FRAMING
		Self Weight				12 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

1. Druggeries conditions unless noted athenuise	Damaged Beams must not be used Design assumes top edge is laterally restrained Provide lateral support at bearing points to avoid lateral displacement and rotation
Version 23.40.705 Powered by iStruct™ Datas	et: 24051401.1529

isDesign

Client: Project: Address:

1/14/2025 Input by:

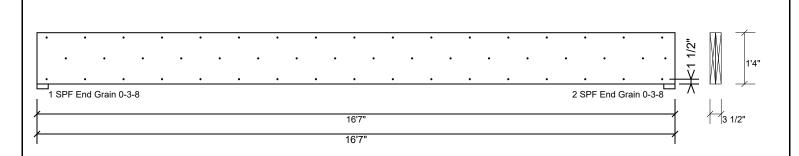
Neal Baggett Job Name: LOT 27 MAGNOLIA HILLS Page 5 of 13

Project #:

GDH Kerto-S LVL 1.750" X 16.000"

2-Ply - PASSED

Level: Level



Multi-Ply Analysis

Fasten all plies using 3 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	•	•
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	245.6 PLF	
Yield Limit per Fastener	81.9 lb.	
См	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850 www.metsawood.com/us

Manufacturer Info

Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



Date: 1/14/2025

Input by: Neal Baggett

Job Name: LOT 27 MAGNOLIA HILLS

Project #:

1.750" X 9.250" 2-Ply - PASSED Kerto-S LVL BM₂

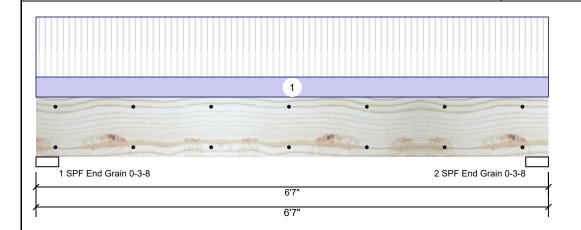
> Application: Design Method:

Building Code:

Load Sharing:

Deck:

Level: Level

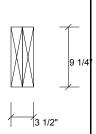


ASD

No

IBC 2012

Not Checked



Page 6 of 13

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal - II
Tomporatura	Tomp <= 1

Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	2670	912	0	0	0
2	Vertical	2670	912	0	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	5103 ft-lb	3'3 1/2"	12542 ft-lb	0.407 (41%)	D+L	L
Unbraced	5103 ft-lb	3'3 1/2"	9934 ft-lb	0.514 (51%)	D+L	L
Shear	2431 lb	1' 3/4"	6907 lb	0.352 (35%)	D+L	L
LL Defl inch	0.069 (L/1063)	3'3 1/2"	0.153 (L/480)	0.452 (45%)	L	L
TL Defl inch	0.093 (L/792)	3'3 1/2"	0.204 (L/360)	0.455 (45%)	D+L	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

	Bearings	S						
	Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
	1 - SPF End Grain	3.500"	Vert	35%	912 / 2670	3582	L	D+L
ı	2 - SPF End	3.500"	Vert	35%	912 / 2670	3582	L	D+L

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Ton	270 PI F	811 PI F	0 PI F	0 PLF	0 PI F	F2

1 Self Weight 7 PLF

П

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

LVL beams must not be cut or drilled
Refer to manufacturer's product information
regarding installation requirements, multi-ply
fastening details, beam strength values, and code

approvals

Damaged Beams must not be used Design assumes top edge is laterally restrained
Provide lateral support at bearing points to avoid
lateral displacement and rotation 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Grain

·	
Version 23.40.705 Powered by iStruct™ Dataset: 24051401.1529	

isDesign

Client: Project: Address: Date: 1/14/2025

Input by: Neal Baggett

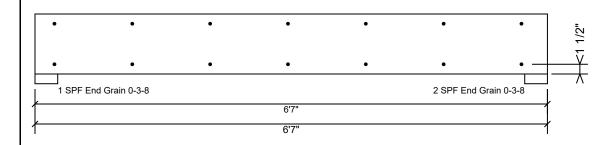
Job Name: LOT 27 MAGNOLIA HILLS

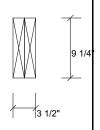
Project #:

Kerto-S LVL BM2

1.750" X 9.250" 2-Ply - PASSED

Level: Level





Page 7 of 13

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

(800) 622-5850 www.metsawood.com/us

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851



Date: 1/14/2025

Input by: Neal Baggett Job Name: LOT 27 MAGNOLIA HILLS

Project #:

1.750" X 9.250" 2-Ply - PASSED **Kerto-S LVL** BM₃

Application:

Design Method:

Building Code:

Load Sharing:

Deck:

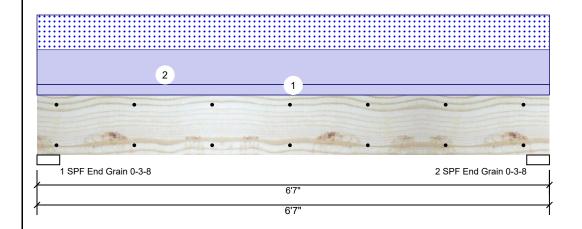
ASD

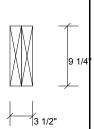
No

IBC 2012

Not Checked

Level: Level





Page 8 of 13

Member Information

Type: Plies: 2 Moisture Condition: Dry Deflection LL: 480 Deflection TL: 360 Importance:

Normal - II Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	0	1732	1313	0	0
2	Vertical	0	1732	1313	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4339 ft-lb	3'3 1/2"	14423 ft-lb	0.301 (30%)	D+S	L
Unbraced	4339 ft-lb	3'3 1/2"	10451 ft-lb	0.415 (42%)	D+S	L
Shear	2068 lb	1' 3/4"	7943 lb	0.260 (26%)	D+S	L
LL Defl inch	0.034 (L/2160)	3'3 1/2"	0.153 (L/480)	0.222 (22%)	S	L
TL Defl inch	0.079 (L/932)	3'3 1/2"	0.204 (L/360)	0.386 (39%)	D+S	L

Bearings

Bearing	Length	Dir.	Cap. F	React D/L lb	Total	Ld. Case	Ld. Comb.
1 - SPF End Grain	3.500"	Vert	30%	1732 / 1313	3045	L	D+S
2 - SPF End Grain	3.500"	Vert	30%	1732 / 1313	3045	L	D+S

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL
2	Uniform			Тор	399 PLF	0 PLF	399 PLF	0 PLF	0 PLF	A2
	Self Weight				7 PLF					

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Lumber

- noted otherwise e retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

1.	Dry	service conditions, unle	ess
2	11//	not to be treated with	fire

isDesign

Client: Project: Address: Date: Input by:

1/14/2025 Neal Baggett

Job Name: LOT 27 MAGNOLIA HILLS

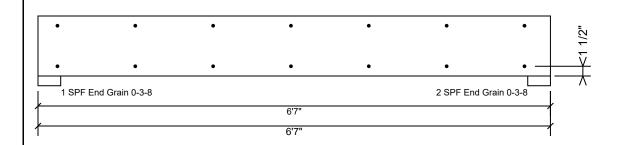
Project #:

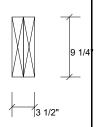
Kerto-S LVL BM₃

1.750" X 9.250"

2-Ply - PASSED

Level: Level





Page 9 of 13

Multi-Ply Analysis

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	,	
Capacity	0.0 %	
Load	0.0 PLF	
Yield Limit per Foot	163.7 PLF	
Yield Limit per Fastener	81.9 lb.	
CM	1	
Yield Mode	IV	
Edge Distance	1 1/2"	
Min. End Distance	3"	
Load Combination		
Duration Factor	1.00	

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026



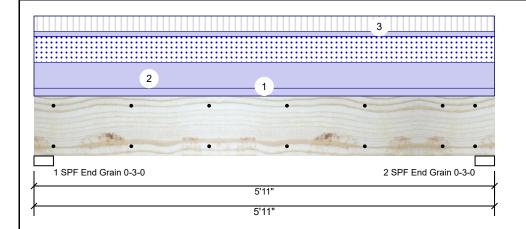
Date: 1/14/2025 Input by: Neal Baggett

Job Name: LOT 27 MAGNOLIA HILLS

Project #:

Kerto-S LVL 2-Ply - PASSED 1.750" X 9.250" BM4

Level: Level



Application: Design Method:

Building Code:

Load Sharing:

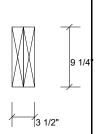
Deck:

ASD

No

IBC 2012

Not Checked



Page 10 of 13

Member Information

Type:	Girder
Plies:	2
Moisture Condition:	Dry
Deflection LL:	480
Deflection TL:	360
Importance:	Normal -
_	_

Ш Temperature: Temp <= 100°F

Reactions UNPATTERNED Ib (Uplift)

Grain

Brg	Direction	Live	Dead	Snow	Wind	Const
1	Vertical	695	1787	1180	0	0
2	Vertical	695	1787	1180	0	0

Analysis Results

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	4145 ft-lb	2'11 1/2"	14423 ft-lb	0.287 (29%)	D+0.75(L+S)	L
Unbraced	4145 ft-lb	2'11 1/2"	11027 ft-lb	0.376 (38%)	D+0.75(L+S)	L
Shear	2097 lb	4'10 3/4"	7943 lb	0.264 (26%)	D+0.75(L+S)	L
LL Defl inch	0.028 (L/2346)	2'11 1/2"	0.139 (L/480)	0.205 (20%)	0.75(L+S)	L
TL Defl inch	0.064 (L/1033)	2'11 1/2"	0.185 (L/360)	0.348 (35%)	D+0.75(L+S)	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at end bearings.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

l	Bearings										
I	Bearing	Length	Dir.	Сар.	React D/L lb	Total	Ld. Case	Ld. Comb.			
	1 - SPF End Grain	3.000"	Vert	36%	1787 / 1407	3194	L	D+0.75(L+S)			
ı	2 - SPF End	3.000"	Vert	36%	1787 / 1407	3194	L	D+0.75(L+S)			

ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	120 PLF	0 PLF	0 PLF	0 PLF	0 PLF	WALL	
2	Uniform			Тор	399 PLF	0 PLF	399 PLF	0 PLF	0 PLF	A2	
3	Uniform			Тор	78 PLF	235 PLF	0 PLF	0 PLF	0 PLF	F2	
	Self Weight				7 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

- LVL beams must not be cut or drilled
 Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 2 Damaged Beams must not be used

- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Client: Date: 1/14/2025 Page 11 of 13 Project: Input by: Neal Baggett isDesign Address: Job Name: LOT 27 MAGNOLIA HILLS Project #: **Kerto-S LVL** 1.750" X 9.250" 2-Ply - PASSED Level: Level BM4 1 SPF End Grain 0-3-0 2 SPF End Grain 0-3-0 5'11" 5'11' Multi-Ply Analysis Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

	• • • • • • • • • • • • • • • • • • • •
Capacity	0.0 %
Load	0.0 PLF
Yield Limit per Foot	163.7 PLF
Yield Limit per Fastener	81.9 lb.
CM	1
Yield Mode	IV
Edge Distance	1 1/2"
Min. End Distance	3"
Load Combination	
Duration Factor	1.00

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

- Handling & Installation
- Handling & Installation

 1. UVI beams must not be cut or drilled

 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 3. Damaged Beams must not be used

 4. Design assumes top edge is laterally restrained

 5. Provide lateral support at bearing points to avoid lateral displacement and rotation

- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer info	
Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us	



Date: 1/14/2025

Input by: Neal Baggett Job Name: LOT 27 MAGNOLIA HILLS Page 12 of 13

Wind

1355 / 1285

2640 L

0

0

Const

Ld. Comb.

D+S

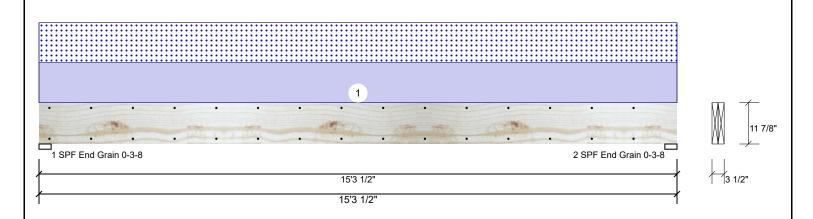
D+S

0

0

1.750" X 11.875" 2-Ply - PASSED Kerto-S LVL BM₅

evel: Level



Grain 2 - SPF 3.500"

End Grain Vert

Member Information Reactions UNPATTERNED Ib (Uplift) Application: Brg Direction Live Type: Dead Snow Plies: 2 Design Method: ASD Vertical 0 1355 1285 1 Moisture Condition: Dry **Building Code:** IBC 2012 2 Vertical 0 1355 1285 Deflection LL: 480 Load Sharing: No Deflection TL: 360 Deck: Not Checked Importance: Normal - II Temp <= 100°F Temperature: **Bearings** Bearing Length Dir. Cap. React D/L lb Total Ld. Case 1-SPF 3.500" 1355 / 1285 2640 L Vert End

Analysis	Results
Analysis	Actua

Analysis	Actual	Location	Allowed	Capacity	Comb.	Case
Moment	9495 ft-lb	7'7 3/4"	22897 ft-lb	0.415 (41%)	D+S	L
Unbraced	9495 ft-lb	7'7 3/4"	9500 ft-lb	0.999 (100%)	D+S	L
Shear	2206 lb	14' 1/8"	10197 lb	0.216 (22%)	D+S	L
LL Defl inch	0.200 (L/889)	7'7 13/16"	0.371 (L/480)	0.540 (54%)	S	L
TL Defl inch	0.411 (L/433)	7'7 13/16"	0.494 (L/360)	0.832 (83%)	D+S	L

Design Notes

- 1 Provide support to prevent lateral movement and rotation at the end bearings. Lateral support may also be required at the interior bearings by the building code.
- 2 Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c. Maximum end distance not to exceed 6".
- 3 Refer to last page of calculations for fasteners required for specified loads.
- 4 Girders are designed to be supported on the bottom edge only.
- 5 Top loads must be supported equally by all plies.
- 6 Top must be laterally braced at a maximum of 9'9 7/8" o.c.
- 7 Bottom must be laterally braced at end bearings.
- 8 Lateral slenderness ratio based on single ply width.

		F-)									
ID	Load Type	Location	Trib Width	Side	Dead 0.9	Live 1	Snow 1.15	Wind 1.6	Const. 1.25	Comments	
1	Uniform			Тор	168 PLF	0 PLF	168 PLF	0 PLF	0 PLF	C2	
	Self Weight				9 PLF						

Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

- Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive
- Handling & Installation
- LVL beams must not be cut or drilled Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

 Damaged Beams must not be used
- Design assumes top edge is laterally restrained
 Provide lateral support at bearing points to avoid
 lateral displacement and rotation
- 6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850

www.metsawood.com/us

Client: 1/14/2025 Page 13 of 13 Project: Input by: Neal Baggett isDesign Address: Job Name: LOT 27 MAGNOLIA HILLS Project #: 1.750" X 11.875" 2-Ply - PASSED Level: Level **Kerto-S LVL** BM5

Multi-Ply Analysis

1 SPF End Grain 0-3-8

Fasten all plies using 2 rows of 10d Box nails (.128x3") at 12" o.c.. Maximum end distance not to exceed 6".

15'3 1/2" 15'3 1/2"

rasterran pries asing E	disterr dir pries distrig E rous or roa Box rians (: 120x3) t					
Capacity	0.0 %					
Load	0.0 PLF					
Yield Limit per Foot	163.7 PLF					
Yield Limit per Fastener	81.9 lb.					
CM	1					
Yield Mode	IV					
Edge Distance	1 1/2"					
Min. End Distance	3"					
Load Combination						
Duration Factor	1.00					

Notes

NOtes
Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads.

Dry service conditions, unless noted otherwise
 LVL not to be treated with fire retardant or corrosive

Handling & Installation

Handling & Installation

1. UVI beams must not be cut or drilled

2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals

3. Damaged Beams must not be used

4. Design assumes top edge is laterally restrained

5. Provide lateral support at bearing points to avoid lateral displacement and rotation

6. For flat roofs provide proper drainage to prevent ponding

This design is valid until 6/28/2026

Manufacturer Info Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851

(800) 622-5850 www.metsawood.com/us

2 SPF End Grain 0-3-8