

RE: J0125-0179 Lot 71 Magnolia Hills **Trenco** 818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: Project Name: J0125-0179 Lot/Block: Address: City:

Model: Subdivision: State:

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2015/TPI2014 Wind Code: ASCE 7-10 Roof Load: 40.0 psf Design Program: MiTek 20/20 8.4 Wind Speed: 130 mph Floor Load: N/A psf

This package includes 42 individual, dated Truss Design Drawings and 0 Additional Drawings.

| 19 I67090953 D2 7/26/2024 39 I67090973 XH1 7/26/202 | - 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 2 3 4 5 6 7 8 9 10 11 12 13 4 5 6 7 8 9 10 11 12 13 14 5 6 7 8 9 10 11 12 13 14 5 16 7 8 9 10 11 12 13 14 15 16 17 10 11 11 11 11 11 11 11 11 11 11 11 11 | 167090937 167090938 167090940 167090941 167090942 167090943 167090944 167090945 167090946 167090947 167090948 167090949 167090950 167090951 167090952 167090953 | A3 A4 A4-A A5 A6 A7 A8 A9 B1-GE B2 B3 B4 C1-GE C2 C3 D1-GE D2 | 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 | 23 24 25 26 27 28 29 30 31 32 33 34 35 36 37 38 39 | I67090957 I67090958 I67090960 I67090961 I67090962 I67090963 I67090964 I67090965 I67090966 I67090967 I67090968 I67090969 I67090970 I67090971 I67090973 | G3 G5 G6 G7-GE H1 K1 M1 M2 M3 P1 VA1 VA2 VA3 VA4 VA4 VB1 VC1 XH1 | 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 7/26/2024 |
|---|---|---|---|---|--|---|---|---|
|---|---|---|---|---|--|---|---|---|

The truss drawing(s) referenced above have been prepared by

Truss Engineering Co. under my direct supervision

based on the parameters provided by Comtech, Inc - Fayetteville.

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2025

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



Gilbert, Eric



RE: J0125-0179 - Lot 71 Magnolia Hills

Trenco 818 Soundside Rd Edenton, NC 27932

Site Information:

| Projec Lot/Bl Addre | ct Customer: ock: ss: | Project Name: J0 | 125-0179 | Subdivision: |
|---------------------------|---------------------------------|--------------------------|--------------------------------|--------------|
| City, C | County: | | | State: |
| No. 41 42 | Seal# I67090975 I67090976 | Truss Name YH2 ZH1 | Date 7/26/2024 7/26/2024 | |



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)



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GILP.... July 26,2024



This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 11=183, 10=489.

7) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

SEAL 036322 July 26,2024

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818 Soundside Road



2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 16-1-8, Exterior(2) 16-1-8 to 20-6-5, Interior(1) 20-6-5 to 20-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 8=114.



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A MiTek A 818 Soundside Road



 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-15 to 4-6-12, Interior(1) 4-6-12 to 16-1-8, Exterior(2) 16-1-8 to 20-6-5, Interior(1) 20-6-5 to 20-10-4 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Bearing at joint(s) 1 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 7=114.



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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-1-15 to 4-6-12, Interior(1) 4-6-12 to 16-1-8, Exterior(2) 16-1-8 to 20-6-5, Interior(1) 20-6-5 to 21-1-12 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Bearing at joint(s) 1, 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 7=110.



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Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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July 26,2024

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| LOADING | i (psf) | SPACING- | 2-0-0 | CSI. | | DEFL. | in | (loc) | l/defl | L/d | PLATES | GRIP |
|---------|---------|-----------------|--------|-------|------|----------|-------|-------|--------|-----|----------------|----------|
| TCLL | 20.0 | Plate Grip DOL | 1.15 | TC | 0.46 | Vert(LL) | -0.31 | 9-11 | >894 | 360 | MT20 | 244/190 |
| TCDL | 10.0 | Lumber DOL | 1.15 | BC | 0.61 | Vert(CT) | -0.40 | 9-11 | >687 | 240 | | |
| BCLL | 0.0 * | Rep Stress Incr | YES | WB | 0.52 | Horz(CT) | 0.01 | 9 | n/a | n/a | | |
| BCDL | 10.0 | Code IRC2015/TF | 912014 | Matri | k-S | Wind(LL) | 0.05 | 2-14 | >999 | 240 | Weight: 306 lb | FT = 20% |

| LUMBER- | | BRACING- | | |
|-----------|---------------------------|-----------|------------------------|--|
| TOP CHORD | 2x6 SP No.1 | TOP CHORD | Structural wood s | heathing directly applied or 6-0-0 oc purlins, |
| BOT CHORD | 2x6 SP No.1 | | except end vertication | als, and 2-0-0 oc purlins (10-0-0 max.): 3-4. |
| WEBS | 2x4 SP No.2 *Except* | BOT CHORD | Rigid ceiling direct | tly applied or 10-0-0 oc bracing, Except: |
| | 6-11,7-9,6-9: 2x6 SP No.1 | | 6-0-0 oc bracing: | 12-14. |
| | | WEBS | T-Brace: | 2x4 SPF No.2 - 4-12, 7-9, 6-9 |
| | | | Easten (2X) Tan | d I braces to parrow edge of web with 10d |

REACTIONS. 2=0-3-0, 12=0-4-13, 9=0-3-8 (size) Max Horz 2=313(LC 12) Max Uplift 2=-251(LC 8), 12=-157(LC 12), 9=-80(LC 12) Max Grav 2=443(LC 23), 12=1744(LC 1), 9=1034(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-321/368, 3-4=-509/906, 4-5=-991/35, 5-6=-1045/250

BOT CHORD 2-14=-553/233, 12-14=-327/97, 11-12=-161/863, 9-11=-56/323

- 3-14=-359/341, 3-12=-1212/788, 4-12=-1841/466, 5-11=-515/308, 6-11=-150/1041, WEBS 6-9=-693/146

NOTES-

 Unbalanced roof live loads have been considered for this design.
 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 31-10-8, Exterior(2) 31-10-8 to 36-3-5, Interior(1) 36-3-5 to 36-10-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=251, 12=157.
- 7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 8) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



(0.131"x3") nails, 6in o.c., with 3in minimum end distance.

Brace must cover 90% of web length.

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818 Soundside Road Edenton, NC 27932

July 26,2024



| REACTIONS. | (size) | 2=0-2-15, 14=0-3-8, 10=0-3-8 |
|------------|------------|---|
| | Max Horz | 2=313(LC 12) |
| | Max Uplift | 2=-208(LC 8), 14=-190(LC 12), 10=-74(LC 12) |
| | Max Grav | 2=335(LC 23), 14=1816(LC 1), 10=1062(LC 19) |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-142/305, 3-4=-449/803, 4-5=-807/0, 5-6=-1043/48, 6-7=-1090/248

BOT CHORD 2-16=-384/81, 14-16=-384/81, 13-14=-527/186, 12-13=-115/1010, 10-12=-57/336

WEBS 3-14=-871/686, 4-14=-1340/417, 4-13=-246/1453, 5-13=-743/203, 5-12=-286/16,

6-12=-457/280, 7-12=-150/1097, 7-10=-728/154

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-6 to 3-5-7, Interior(1) 3-5-7 to 31-10-7, Exterior(2) 31-10-7 to 36-3-4, Interior(1) 36-3-4 to 36-10-12 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

6) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 2.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=208, 14=190.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.



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BRACING-

TOP CHORD

BOT CHORD

| REACTIONS. | All bearings 14-3-0. |
|------------|----------------------|
| | |

2x6 SP No.1

2x6 SP No.1

2x4 SP No.2

2x4 SP No.2

(lb) - Max Horz 19=-297(LC 10)

- Max Uplift All uplift 100 lb or less at joint(s) except 19=-159(LC 8), 12=-142(LC 9), 17=-166(LC 12), 18=-244(LC 12), 14=-167(LC 13), 13=-240(LC 13)
- Max Grav All reactions 250 lb or less at joint(s) 17, 14, 13 except 19=266(LC 20), 12=252(LC 19), 16=280(LC 22), 18=255(LC 10), 15=276(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 4-5=-211/286, 7-8=-212/286

NOTES-

LUMBER-

WEBS

OTHERS

TOP CHORD

BOT CHORD

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 2x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 159 lb uplift at joint 19, 142 lb uplift at joint 12, 166 lb uplift at joint 17, 244 lb uplift at joint 18, 167 lb uplift at joint 14 and 240 lb uplift at joint 13.



Structural wood sheathing directly applied or 6-0-0 oc purlins,

Rigid ceiling directly applied or 10-0-0 oc bracing.

except end verticals.

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NOTES-

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2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-2 to 3-3-11, Interior(1) 3-3-11 to 7-1-8, Exterior(2) 7-1-8 to 11-6-5, Interior(1) 11-6-5 to 15-4-2 zone; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 28 lb uplift at joint 9 and 28 lb uplift at joint 7.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent bucking of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road

Vanananan VIIIIIIIIIII SEAL 036322 G mm July 26,2024

| Job | Truss | Truss Type | Qty | Ply | Lot 71 Magnolia Hills | | |
|------------------------|---|-----------------------------------|--------------------------------|-------------------------|--|---|---------------------------------|
| J0125-0179 | B3 | COMMON | 3 | 1 | | | 167090947 |
| | | | | | Job Reference (option | al) | |
| Comtech, Inc, Fayette | wille, NC - 28314, | | ID:d9Okus??o?O | 8.630 s Ju aeo9B6taa | I 12 2024 Millek Industi BuzGAaa-RfC?PsB70H | ies, Inc. Wed Jul 24 14:09:2 la3NSaPanL8w3ulTXbGKW | .8 2024 Page 1 rCDoi7J4zJC?f |
| | | 7-1-8 | | 14-3-0 | | | |
| | | 7-1-8 | | 7-1-8 | | | |
| | | | 5x5 = | | | | Scale = 1:51.4 |
| | | | | | | | |
| | | | 2 | | | | |
| | Ī | | \wedge | | | | |
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| | | | | | | | |
| | | 12.00 12 | | 11 | | | |
| | | 10 | | \mathbf{X} | | | |
| | | | | | | | |
| | 3-15 | | | | 12 | | |
| | 8 | °// | | | | | |
| | | | | | 4x8 \\ | | |
| | 4x8 | " 1 // | | | 3 | | |
| | | | | | | | |
| | 4 | | | | | | |
| | - - - | | | | | | |
| | 1 1 | X | | | | | |
| | | 87 | 6 | | 54 | | |
| | | 3x4 | 4x8 = | | 5,4 [] | | |
| | | 7-1-8 | | 14-3-0 | | | |
| | | | | 110 | | | |
| LOADING (psf) | SPACING- 2-0-0 | CSI. | DEFL. i | n (loc) | l/defl L/d | PLATES GF | 11P |
| TCLL 20.0 TCDI 10.0 | Lumber DOL 1.15 | BC 0.13 | Vert(LL) -0.0 Vert(CT) -0.0 | 1 6-7 2 6-7 | >999 360 | M120 24 | 4/190 |
| BCLL 0.0 * | Rep Stress Incr YES | WB 0.06 | Horz(CT) 0.0 | 5 | n/a n/a | | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-S | Wind(LL) 0.0 | 0 6 | >999 240 | Weight: 116 lb | T = 20% |
| LUMBER- | | I | BRACING- | | | | |
| TOP CHORD 2x6 SP No | b.1 | | TOP CHORD | Structur | al wood sheathing dir | ectly applied or 6-0-0 oc pu | urlins, |
| BOT CHORD 2x6 SP No | p.1 | | | except e | end verticals. | - 40,0,0, | |
| WEBS 2x4 SP No | 0.2 | | BOT CHORD | Rigia ce | elling directly applied c | or 10-0-0 oc bracing. | |
| REACTIONS. (size) | 7=0-3-8, 5=0-3-8 | | | | | | |
| Max Horz | 7=205(LC 9) | | | | | | |
| Max Uplif Max Grav | t 7=-20(LC 13), 5=-20(LC 12) z 7=555(LC 1)_5=555(LC 1) | | | | | | |
| | | | | | | | |
| FORCES. (lb) - Max. Co | mp./Max. Ten All forces 250 | 0 (lb) or less except when shown. | | | | | |
| IOP CHORD 1-2=-51 | 0/103, 2-3=-515/163, 1-7=-48 8/298 | 5/1/4, 3-5=-483/1/5 | | | | | |

WEBS

1) Unbalanced roof live loads have been considered for this design.

2-6=0/270

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-1 to 4-7-14, Interior(1) 4-7-14 to 7-1-8, Exterior(2) 7-1-8 to 11-6-5, Interior(1) 11-6-5 to 13-11-15 zone; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 7 and 20 lb uplift at joint 5.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



818 Soundside Road

| Job | Truss | Truss Type | Qty | Ply | Lot 71 Magnolia Hills | | |
|--|--|--|--|--|---|---|--------------------------------------|
| J0125-0179 | B4 | COMMON | 1 | 1 | | | 167090948 |
| Orantzah Iza - Esuatta | NO 20244 | | | 600 a lu | Job Reference (optional |) | 00:00 0004 Dars 4 |
| Comtech, Inc, Fayette | wille, NC - 28314, | 4-9-8 4-9-8 | 8 ID:d9Okus??o?Oq <u>11-11</u> 7-1-8 | .630 s Ju eo9B6tqa - <u>0</u> 3 | I 12 2024 Mi Lek Industrie BuzGAgg-RfC?PsB70Hq | s, Inc. Wed Jul 24 14: 3NSgPqnL8w3ulTXbG | 09:29 2024 Page 1 KWrCDoi7J4zJC?f |
| | | 5x5 | = | | | | Scale = 1:51.4 |
| | 8.8 1.5 | 4x8 // 1 4x8 // 1 6 8 7 8 7 3x4 4x8 | 9 | 10 | 4x8 × 3 54 3x4 | | |
| | | 4-9-8 4-9-8 | 11-11 7-1-{ | -0 3 | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014 | CSI. TC 0.27 BC 0.11 WB 0.13 Matrix-S | DEFL. in Vert(LL) -0.01 Vert(CT) -0.03 Horz(CT) 0.00 Wind(LL) 0.00 | (loc) 5-6 5-6 5 6 | l/defl L/d >999 360 >999 240 n/a n/a >999 240 | PLATES MT20 Weight: 107 lb | GRIP 244/190 FT = 20% |
| LUMBER- TOP CHORD 2x6 SP No BOT CHORD 2x6 SP No WEBS 2x4 SP No 1-7: 2x6 S REACTIONS. (size) Max Horz | 0.1 0.2 *Except* IP No.1 7=0-3-8, 5=0-3-8 17=-186(LC 8) | | BRACING- TOP CHORD BOT CHORD | Structur except e Rigid ce | al wood sheathing direct and verticals. iling directly applied or | xtly applied or 6-0-0 o 10-0-0 oc bracing. | c purlins, |
| Max Uplif Max Grav FORCES. (Ib) - Max. Co | t 7=-45(LC 13), 5=-4(LC 12) 7=459(LC 1), 5=458(LC 1) mp./Max. Ten All forces 25(|) (Ib) or less except when shown. | | | | | |
| | 5/101, 2-3352/113, 1-7=-42 | 1102, 3-3303/141 | | | | | |

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-4-4 to 9-2-5, Interior(1) 9-2-5 to 11-7-15 zone; end vertical right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 7 and 4 lb uplift at joint 5.



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| Job | Truss | Truss Type | Qty | Ply | Lot 71 Magnolia Hills | 167000050 |
|--|--|--|---------------------------|-----------------------|--|--|
| J0125-0179 | C2 | COMMON | 2 | 1 | | 026060101 |
| Comtech, Inc, Faye | tteville, NC - 28314, | | | 8.630 s Ju | Job Reference (option Il 12 2024 MiTek Indust | nal) ries, Inc. Wed Jul 24 14:09:30 2024 Page 1 |
| | | اD:d _ 1-3-0, 5-7-4 | 90kus??o?00 11-2-8 | qeo9B6tqa | ାBuzGAgg-RfC?PsB70I 12-5-8 | Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f |
| | | 1-3-0 5-7-4 | 5-7-4 | 1 | 1-3-0 | |
| | | 5x5 5 | = | | | Scale = 1:65.8 |
| | | | | | | |
| | | 3 | | | | |
| | | 12.00 12 | \backslash | | | |
| | | 10 | 11 | | | |
| | | | | 4.0 | | |
| | | 4x8 1/ 9 | | 12 | × | |
| | | | | | 4 | |
| | | | | | | |
| | | | | | | |
| | | | | | 5-5-16 | |
| | | | | | | |
| | | | / | | | |
| | | ⁸ 3×4 II 7 | | 3x4 [⊠] 6 | 1 | |
| | | 3x10 | = | | | |
| | | <u>5-7-4</u> 5-7-4 | <u>11-2-8</u> 5-7-4 | | I | |
| LOADING (psf) | SPACING- 2-0-(|) CSL DEF | iL. ir |) (loc) | l/defl l/d | PLATES GRIP |
| TCLL 20.0 | Plate Grip DOL 1.15 | 5 TC 0.14 Vert | (LL) -0.00 | 7-8 | >999 360 | MT20 244/190 |
| BCLL 0.0 * | Rep Stress Incr YES | S WB 0.12 Horz | (CT) -0.01 z(CT) -0.00 | 7-8 6 | >999 240 n/a n/a | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-S Wine | d(LL) 0.00 | 7-8 | >999 240 | Weight: 131 lb FT = 20% |
| LUMBER- | No 1 | BRA TOP | CING- | Structur | ral wood sheathing di | rectly applied or 6-0-0 oc purlins |
| BOT CHORD 2x6 SP | No.1 | | | except | end verticals. | |
| 2-8,4-6: | No.2 ^Except^ 2x6 SP No.1 | WEE BOI | S | Rigid ce 1 Row a | at midpt 3 | or 10-0-0 oc bracing. |
| REACTIONS. (size) | 8=0-3-8, 6=0-3-8 | | | | | |
| Max Ho | rz 8=-342(LC 10) | | | | | |
| Max Op Max Gra | av 8=526(LC 20), 6=526(LC 19) | 9) | | | | |
| FORCES. (lb) - Max. C | Comp./Max. Ten All forces 25 | 0 (lb) or less except when shown. | | | | |
| TOP CHORD 2-3=-3 BOT CHORD 7-8=-3 | 00/221, 3-4=-300/221, 2-8=-47 35/369 | 8/293, 4-6=-478/293 | | | | |
| WEBS 4-7=-1 | 26/250 | | | | | |
| NOTES- | | | | | | |
| Unbalanced roof live Wind: ASCE 7-10: Vu | loads have been considered fo It=130mph Vasd=103mph: TC | r this design. DL=6.0psf: BCDL=6.0psf: h=15ft: Cat. II: Exc | C: Enclosed | : MWFR | S (envelope) | |
| and C-C Exterior(2) - | 1-1-2 to 3-3-11, Interior(1) 3-3- | 11 to 5-7-12, Exterior(2) 5-7-12 to 10-0-9, Int | erior(1) 10-0- | 9 to 12-4- | -10 zone; end | |
| 3) This truss has been d | esigned for a 10.0 psf bottom of | chord live load nonconcurrent with any other | live loads. | plate gri | p DOL-1.00 | |
| 4) * This truss has been will fit between the bo | designed for a live load of 30.0 ttom chord and any other mem | opsf on the bottom chord in all areas where a bers. | rectangle 3- | 6-0 tall by | / 2-0-0 wide | |
| Provide mechanical c ioint 6 | onnection (by others) of truss t | o bearing plate capable of withstanding 39 lb | o uplift at joint | 8 and 39 | lb uplift at | |
| joint of | | | | | | BTH CARO |
| | | | | | L | with the second |
| | | | | | | E Star I and the start of the |
| | | | | | | SEAL |
| | | | | | | 036322 |
| | | | | | | 5 N |
| | | | | | | MGINEEP & |
| | | | | | | CA GILBE |
| | | | | | | |

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July 26,2024



| Job | Truss | Truss Type | Qty | Ply | Lot 71 Magnolia Hills | | |
|--|---|---|--|---|--|---|------------------------------------|
| J0125-0179 | C3 | COMMON | 4 | 1 | | | 167090951 |
| | | | | | Job Reference (option | al) | |
| Comtech, Inc, Fayettev | ville, NC - 28314, | | 8 ID:d9Okus22o2Oa | .630 s Jul | 12 2024 MiTek Industr | ies, Inc. Wed Jul 24 14: a3NSaPapl 8w3ulTXb6 | :09:30 2024 Page 1 |
| | | 5-7-4 | 11-2-8 | SUSDUIGAL | | | 51(110001/34230?1 |
| | | 5-7-4 | 5-7-4 | 1 | I | | |
| | | | 5x5 = | | | | Scale = 1:65.8 |
| | | | | | | | |
| | | 4x8 2 1 4x8 2 1 4x8 2 1 4x8 2 1 4x8 2 1 4x8 2 1 4x8 2 1 1 1 1 1 1 2 1 2 1 2 1 2 1 2 1 2 1 2 | 2 9 Ø | 4x8 | 10 N <u>5-5-5</u> | | |
| | | | | 3x4 ₫ | I I | | |
| | | ⁶ 3x4 | 5 | 4 | | | |
| | | 5-7-4 | 3x10 — 11-2-8 | | I | | |
| | | 5-7-4 | 5-7-4 | | 1 | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 TCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IBC2015/TPI2014 | CSI. TC 0.13 BC 0.08 WB 0.05 Matrix-S | DEFL. in Vert(LL) -0.00 Vert(CT) -0.01 Horz(CT) 0.00 Wind(LL) 0.00 | (loc) 5-6 5-6 4 5 | l/defl L/d >999 360 >999 240 n/a n/a >999 240 | PLATES MT20 Weight: 124 lb | GRIP 244/190 FT = 20% |
| LUMBER- | | | BRACING- | - | | | |
| TOP CHORD 2x6 SP No. BOT CHORD 2x6 SP No. WEBS 2x4 SP No. 1-6,3-4: 2x0 | .1 .1 .2 *Except* 6 SP No.1 | | TOP CHORD BOT CHORD WEBS | Structura except e Rigid cei 1 Row a | al wood sheathing dire and verticals. iling directly applied o t midpt 2- | ectly applied or 6-0-0 c r 10-0-0 oc bracing. 5 | oc purlins, |
| REACTIONS. (size) Max Horz Max Uplift Max Grav | 6=0-3-8, 4=0-3-8 6=129(LC 9) 6=-45(LC 13), 4=-45(LC 12) 6=433(LC 20), 4=433(LC 19 |) | | | | | |
| FORCES. (lb) - Max. Con TOP CHORD 1-2=-284, | np./Max. Ten All forces 250 /158, 2-3=-284/158, 1-6=-386 |) (lb) or less except when shown. 5/145, 3-4=-386/145 | | | | | |

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-3-4 to 4-8-1, Interior(1) 4-8-1 to 5-7-12, Exterior(2) 5-7-12 to 10-0-9, Interior(1) 10-0-9 to 11-0-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide

will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 45 lb uplift at joint 6 and 45 lb uplift at joint 4.



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4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Pageia et initial 2, 5 area in the target area area in the target area in the target area in the target area area in the target area area.

5) Bearing at joint(s) 7, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 5.



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LUMBER-

 TOP CHORD
 2x6 SP No.1

 BOT CHORD
 2x6 SP No.1

 WEBS
 2x4 SP No.2

 SLIDER
 Left 2x8 SP No.1 5-10-11

 BRACING

 TOP CHORD
 Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.

 BOT CHORD
 Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 2=0-3-8, 7=0-3-8 Max Horz 2=223(LC 9) Max Uplift 2=-26(LC 13), 7=-33(LC 12) Max Grav 2=662(LC 1), 7=598(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-4=-591/181, 4-5=-533/206, 5-7=-537/211

BOT CHORD 2-8=-21/309

WEBS 4-8=0/296, 5-8=-54/287

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-11, Interior(1) 3-3-11 to 7-11-0, Exterior(2) 7-11-0 to 12-3-13, Interior(1) 12-3-13 to 14-10-8 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 7.



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- BOT CHORD 2-14=0/1134, 12-14=0/1134, 10-12=0/1134
- WEBS 9-12=0/1238, 3-14=0/1238, 4-8=-1349/228

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-12, 3-14

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

7) Attic room checked for L/360 deflection.

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NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-14 to 3-4-15, Interior(1) 3-4-15 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 22-11-14 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-12, 3-14

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 12-14

7) Attic room checked for L/360 deflection.

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TRENCO A MiTek Affiliate

818 Soundside Road

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-14 to 3-4-15, Interior(1) 3-4-15 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 21-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0ps on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Ceiling dead load (10.0 psf) on member(s). 3-4, 8-9, 4-8; Wall dead load (5.0psf) on member(s).9-11, 3-13

6) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 11-13

7) Attic room checked for L/360 deflection.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcaccomponents.com)

July 26,2024

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818 Soundside Road

| LUMBER- | | BRACING- | |
|-----------|-------------------------|-----------|---|
| TOP CHORD | 2x6 SP No.1 | TOP CHORD | Structural wood sheathing directly applied or 6-0-0 oc purlins, |
| BOT CHORD | 2x6 SP No.1 | | except end verticals. |
| WEBS | 2x4 SP No.2 *Except* | BOT CHORD | Rigid ceiling directly applied or 10-0-0 oc bracing. Except: |
| | 10-12: 2x6 SP No.1 | | 6-0-0 oc bracing: 9-13 |
| SLIDER | Left 2x8 SP No.1 4-5-13 | | 8-11-0 oc bracing: 8-14 |
| | | WEBS | 1 Row at midpt 4-15 |
| | | JOINTS | 1 Brace at Jt(s): 14 |

| REACTIONS. | (size) | 2=0-3-8, 13=0-3-8, 12=Mechanical |
|------------|------------|---|
| | Max Horz | 2=302(LC 9) |
| | Max Uplift | 2=-93(LC 13), 12=-144(LC 13) |
| | Max Grav | 2=663(LC 20), 13=1034(LC 19), 12=234(LC 20) |

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-4=-678/235, 4-5=-741/76, 5-7=-282/38, 7-8=-332/64, 8-9=-541/273, 9-10=-182/251
- BOT CHORD 2-16=-113/444, 15-16=-113/444, 14-15=-112/598, 8-14=-172/377, 9-13=-970/31
- WEBS 4-15=-671/177, 4-14=-112/547

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-1-1 to 3-3-11, Interior(1) 3-3-11 to 11-0-0, Exterior(2) 11-0-0 to 15-4-13, Interior(1) 15-4-13 to 21-4-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 12=144.

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| Job | Truss | Truss Type | Qty | Ply | Lot 71 Magnolia Hills | | 167090960 |
|--|--|--|--|---------------|---------------------------|---|----------------|
| J0125-0179 | G7-GE | ROOF SPECIAL | 1 | 1 | lah Dafaranan (antiar | -1) | |
| Comtech, Inc, Faye | etteville, NC - 28314, | | | 3.630 s Ju | I 12 2024 MiTek Industr | iai) ries, Inc. Wed Jul 24 14:09:34 20 | 024 Page 1 |
| | | | ID:d9Okus??o?Oq | eo9B6tqa | BuzGAgg-RfC?PsB70H | lq3NSgPqnL8w3uITXbGKWrCD | oi7J4zJC?f |
| | - <u>1</u> 1: | -2-8 5-7-8 9-3-8 2-8 5-7-8 3-8-0 | 11-0-0 12-8-8 15-6 | -8 | 21-8-8 | | |
| | | | 0-3-0 | - | | | Scale = 1:77.0 |
| | | | 4x0 — | | | | |
| | | 12.50 12 | 7 | | | | |
| | I | | Å. | | | | |
| | | 4x6 1/ | | | | | |
| | | 6 | | | | | |
| | | 5 | 15 | 8 4) | ĸ6 | | |
| | | 3x10 // 4x4 | z 14 5x5 | Ť | | Ī | |
| | 10 | 4 | 6×6 = | | X | | |
| | 2-11- | 5x5 1/ | 18 20 | - 19 | $\langle \rangle$ | | |
| | 5x5 | // 🔪 - | 2x6 = | 2x6 | =/// | φ | |
| | | 3 | 2x6 2x4 | | 4x6 \\ | 8-11 | |
| | | | | | 10 | | |
| | -1 4 | 2 | 3-7-0 | | | ĪN | |
| | | | <u>\</u> | | | 1-9-1 | |
| | - <u>G</u> - C | 3x10 17 21 | | × 2 | 2 23 24 12 | 11 | |
| | | 0.4 II | 13-0-8 41 0 3-012-8-8 | 15-6-8- | 12 5-0 — | | |
| | | 5-7-8 2x4 11 9-3-8 5-7-8 3-8-0 | <u>11-0-0 12-0-8 14-10-</u> 1-8-8 0-9-8 0-4-0 | 0-8-8 | 21-8-8 5x0 — 6-2-0 | | |
| Plate Offsets (X,Y) | 2:Edge,0-0-0], [7:Edge,0-4-13], | [8:0-1-7,0-1-2], [10:0-1-12,0-2-0] | 0-3-0 0-8-0 1-9-8 | | | | |
| | SPACING 2.0.0 | 190 | DEEI in | (loc) | l/dofl l/d | | |
| TCLL 20.0 | Plate Grip DOL 1.15 | TC 0.45 | Vert(LL) -0.08 | (ioc) 5-15 | >999 360 | MT20 244/19 | 90 |
| TCDL 10.0 BCU 0.0 * | Lumber DOL 1.15 Rep Stress Incr NO | BC 0.64 WB 0.22 | Vert(CT) -0.19 Horz(CT) 0.22 | 5-15 12 | >938 240 n/a n/a | | |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-S | Wind(LL) 0.11 | 5-15 | >999 240 | Weight: 241 lb FT = | 20% |
| LUMBER- | | | BRACING- | | | | |
| TOP CHORD 2x6 SP | No.1 | | TOP CHORD | Structur | ral wood sheathing dir | ectly applied or 6-0-0 oc purlin | S, |
| WEBS 2x4 SP | No.2 *Except* | | BOT CHORD | Rigid ce | eiling directly applied c | or 10-0-0 oc bracing, Except: | |
| 10-12,1 | 8-19: 2x6 SP No.1 | | WEBS | 6-0-0 or | c bracing: 13-19. | -16 | |
| | 01 10.1 4 10 | | JOINTS | 1 Brace | at Jt(s): 15, 20 | | |
| REACTIONS. (size Max Ho |) 2=0-3-8, 13=0-3-8, 12=Mech prz 2=374(I C 5) | anical | | | | | |
| Max Up | blift 2=-180(LC 9), 13=-144(LC 8 |), 12=-274(LC 9) | | | | | |
| Max Gr | av 2=701(LC 34), 13=1308(LC | 33), 12=685(LC 20) | | | | | |
| FORCES. (lb) - Max. (| Comp./Max. Ten All forces 250 | (lb) or less except when shown. | 0 40 405/054 | | | | |
| 10P CHORD 2-4=-7 10-12 | 2=-187/259 | 3/52, 7-8=-336/105, 8-9=-670/248, | 9-10=-185/254, | | | | |
| BOT CHORD 2-17= | -165/510, 16-17=-165/509, 16-1 | 8=-178/622, 15-18=-163/612, 5-15 | =-276/115, | | | | |
| WEBS 10-13 | =-327/106, 4-16=-677/265, 4-15 | =-127/596, 18-20=-49/267, 19-20= | -49/267 | | | | |
| NOTES. | | | | | | | |
| 1) Unbalanced roof live | loads have been considered for | this design. | | | | | |
| 2) Wind: ASCE 7-10; Vi gable end zone: Lum | ult=130mph Vasd=103mph; TCE ber DOI =1 60 plate grip DOI =1 | 0L=6.0psf; BCDL=6.0psf; h=15ft; C 60 | at. II; Exp C; Enclosed | ; MWFRS | S (envelope) | | |
| 3) This truss has been of | designed for a 10.0 psf bottom c | nord live load nonconcurrent with a | any other live loads. | | | MATTER | 1 |
| This truss has been will fit between the boots | I designed for a live load of 30.0 ottom chord and any other memile | osf on the bottom chord in all areas pers, with BCDL = 10.0psf. | s where a rectangle 3-6 | 6-0 tall by | 2-0-0 wide | TH CARO | 111 Acres |
| 5) Refer to girder(s) for | truss to truss connections. | | | | | ESSION | 5/1-1- |
| 2=180, 13=144, 12=2 | connection (by others) of truss to 274. | bearing plate capable of withstand | ung 100 id uplift at join | u(s) exce | pr (jt=ib) | | |
| 7) Hanger(s) or other co | onnection device(s) shall be prov | ided sufficient to support concentra | ated load(s) 312 lb dow | vn and 37 | 7 lb up at | | 15 3 |
| such connection dev | ice(s) is the responsibility of othe | ∠⊶o io uowir anu o7 io up at 20-8- ≊rs. | | ie uesign | | SEAL | 1 |
| 8) In the LOAD CASE(S | section, loads applied to the fa | ice of the truss are noted as front (| F) or back (B). | | | 036322 | 1 E |
| LOAD CASE(S) Stand | ard | | | | 6 | 3 1 | 1. 3 |
| 1) Dead + Roof Live (ba | alanced): Lumber Increase=1.15 | , Plate Increase=1.15 | | | | F. CO . NGINEER | as |
| Vert: 1-7=-6 | 0, 7-10=-60, 2-16=-20, 5-15=-20 | , 8-15=-20, 11-13=-20 | | | | CA CILB | Enn |
| Concentrated Loads Vert: 22=-28 | (lb) 44(B) 23=-289(B) 24=-248(B) | | | | | The GIL | 111 |

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RENGINEERING BY CO

818 Soundside Road Edenton, NC 27932

July 26,2024

| | 3-0-8 | 6-1-0 | 10-10-0 | | 16-0-8 |
|--|--|---|---|---|--|
| | 3-0-8 | 3-0-8 | 4-9-0 | I | 5-2-8 |
| Plate Offsets (X,Y) | [2:0-1-12,Edge], [3:0-2-8,0- | 2-12] | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2015/TPl2 | 2-0-0 CSI. 1.15 TC 0.29 1.15 BC 0.45 NO WB 0.39 2014 Matrix-S | DEFL. in Vert(LL) 0.11 Vert(CT) -0.16 Horz(CT) 0.04 | (loc) l/defl L/d 8-9 >999 240 8-9 >999 240 6 n/a n/a | PLATES GRIP MT20 244/190 Weight: 100 lb FT = 20% |
| LUMBER- TOP CHORD 2x6 S BOT CHORD 2x6 S WEBS 2x4 S 5-6: 2 | SP No.1 SP No.1 SP No.2 *Except* 2x6 SP No.1 | | BRACING- TOP CHORD BOT CHORD WEBS | Structural wood sheathing except end verticals, and Rigid ceiling directly applie 1 Row at midpt | g directly applied or 4-3-6 oc purlins, 2-0-0 oc purlins (4-5-0 max.): 3-5. ed or 7-2-4 oc bracing. 4-6 |
| REACTIONS. (si Max Max Max | ze) 6=0-3-8, 2=0-3-0 Horz 2=71(LC 19) Uplift 6=-512(LC 4), 2=-490(I Grav 6=1219(LC 1), 2=1175 | LC 4) (LC 1) | | | |
| FORCES.(lb) - MaxTOP CHORD2-33BOT CHORD2-93WEBS3-93 | Comp./Max. Ten All force -3039/1239, 3-4=-2825/1190 -1204/2856, 8-9=-1042/2473 -185/484, 4-6=-2449/1032, 4 | es 250 (lb) or less except when sho 0, 5-6=-258/100 3, 6-8=-1042/2473 4-8=-86/382, 4-9=-158/375 | own. | | |
| NOTES- 1) Wind: ASCE 7-10; porch left and right 2) Provide adequate 3) This truss has bee will fit between the 5) Provide mechanic: 6=512, 2=490. 6) Graphical purlin re 7) Hanger(s) or other 6-1-0, 108 lb down lb down and 81 lb down and 50 lb up The design/selecti 8) In the LOAD CASE LOAD CASE(S) Sta 1) Dead + Roof Live I Uniform Loads (plf Vert: 1-3= Concentrated Load Vert: 3=-1 | Vult=130mph Vasd=103mph exposed; Lumber DOL=1.60 drainage to prevent water por n designed for a 10.0 psf bott en designed for a live load of bottom chord and any other al connection (by others) of tru- presentation does not depict connection device(s) shall be and 81 lb up at 8-1-12, 108 up at 14-1-12 on top chord, a at 10-1-12, and 78 lb down a on of such connection devices E(S) section, loads applied to ndard (balanced): Lumber Increase=) -60, 3-5=-60, 2-6=-20 is (lb) 08(B) 7=-39(B) 9=-380(B) 10 | ; TCDL=6.0psf; BCDL=6.0psf; h=1 plate grip DOL=1.60 nding. tom chord live load nonconcurrent i 30.0psf on the bottom chord in all members. uss to bearing plate capable of with the size or the orientation of the p. the provided sufficient to support con lb down and 81 lb up at 10-1-12, a and 380 lb down and 234 lb up at 10 and 50 lb up at 12-1-12, and 78 lb (s) is the responsibility of others. the face of the truss are noted as 1 =1.15, Plate Increase=1.15 =-108(B) 11=-108(B) 12=-108(B) 1 | 15ft; Cat. II; Exp C; Enclosed; with any other live loads. areas where a rectangle 3-6 hstanding 100 lb uplift at joint urlin along the top and/or bott incentrated load(s) 108 lb dow and 108 lb down and 81 lb up 6-1-0, 78 lb down and 50 lb u o down and 50 lb up at 14-1-7 front (F) or back (B). | MWFRS (envelope); -0 tall by 2-0-0 wide (s) except (jt=lb) om chord. n and 81 lb up at 0 at 12-1-12, and 108 up at 8-1-12, 78 lb 12 on bottom chord. | SEAL 036322 |

July 26,2024

| | 8-3-10 | <u>16</u> | 5-5-7 1-14 | | 24 | -9-1 3-10 | | |
|---|--|---|---|---|---|---|---|--|
| Plate Offsets (X,Y) | [9:0-4-8,0-2-8] | | 1 14 | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014 | CSI. TC 0.53 BC 0.94 WB 0.70 Matrix-S | DEFL. in Vert(LL) -0.27 Vert(CT) -0.49 Horz(CT) 0.09 Wind(LL) 0.36 | 1 (loc) l/d 7 10-12 >9 9 10-12 >5 5 9 1 6 10-12 >8 | lefi L/d 199 360 187 240 n/a n/a 112 240 | PLATES MT20 M18AHS Weight: 325 lb | GRIP 244/190 186/179 FT = 20% | |
| LUMBER- TOP CHORD 2x6 SP BOT CHORD 2x6 SP WEBS 2x4 SP 2-13,6- | No.1 No.1 No.2 *Except* 9: 2x6 SP No.1 | | BRACING- TOP CHORD BOT CHORD WEBS | 2-0-0 oc pu Rigid ceilin 1 Row at m | urlins (6-0-0 max.): g directly applied of hidpt 5- | 1-7, except end vertic r 10-0-0 oc bracing. 9 | als. | |
| REACTIONS. (size Max U Max G | e) 13=0-3-8, 9=0-3-8 plift 13=-576(LC 4), 9=-411(LC 5) rav 13=1799(LC 1), 9=3077(LC 2) | | | | | | | |
| FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. FOP CHORD 2-13=-1740/660, 2-3=-5897/2190, 3-5=-5897/2190, 5-6=-923/79, 6-9=-337/122 BOT CHORD 12-13=-61/344, 10-12=-2464/6419, 9-10=-2464/6419 WEBS 2-12=-2198/5733, 3-12=-440/184, 5-12=-751/309, 5-10=-548/1245, 5-9=-5698/2463 | | | | | | | | |
| NOTES- 1) 2-ply truss to be con Top chords connect Bottom chords conn Webs connected as 2) All loads are conside ply connections have 3) Wind: ASCE 7-10; V Lumber DOL=1.60 p 4) Provide adequate dr 5) All plates are MT20 6) This truss has been will fit between the b 8) Provide mechanical 13=576, 9=411. 9) Graphical purlin repr 10-4-9, 200 lb dow down and 164 lb uj at 23-2-4 on bottom | nected together with 10d (0.131"x3") na ed as follows: 2x6 - 2 rows staggered at ected as follows: 2x6 - 2 rows staggered follows: 2x4 - 1 row at 0-9-0 oc. ered equally applied to all plies, except it e been provided to distribute only loads 'ult=130mph Vasd=103mph; TCDL=6.0p late grip DOL=1.60 ainage to prevent water ponding. plates unless otherwise indicated. designed for a 10.0 psf bottom chord liv n designed for a live load of 30.0psf on t ottom chord and any other members. connection (by others) of truss to bearin resentation does not depict the size or th connection device(s) shall be provided a n and 164 lb up at 11-2-4, 200 lb down o at 17-2-4, 200 lb down and 164 lb up. | ils as follows: 0-9-0 oc. 1 at 0-6-0 oc. f noted as front (F) or back noted as (F) or (B), unless osf; BCDL=6.0psf; h=15ft; e load nonconcurrent with the bottom chord in all are ing plate capable of withsta ne orientation of the purlin sufficient to support conce and 164 lb up at 13-2-4, 1 at 19-2-4, and 200 lb dow connection device(s) is the | k (B) face in the LOAD (s otherwise indicated. Cat. II; Exp C; Enclosed n any other live loads. as where a rectangle 3- anding 100 lb uplift at joi along the top and/or bo entrated load(s) 665 lb d 200 lb down and 164 lb vn and 164 lb up at 21-: e responsibility of others | CASE(S) sect 1; MWFRS (e 6-0 tall by 2-0 nt(s) except (ttom chord. own and 294 up at 15-2-4 2-4, and 146(| tion. Ply to envelope); 0-0 wide jt=lb) lb up at , 200 lb 3 lb down | SE/ 0363 | AROJULA SIGNAL AL B22 | |
| LOAD CASE(S) Stand 1) Dead + Roof Live (b | dard alanced): Lumber Increase=1.15, Plate | Increase=1.15 | | | | A. C | GILBE | |

Continued on page 2

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818 Soundside Road Edenton, NC 27932

July 26,2024

| Job | Truss | Truss Type | Qty | Ply | Lot 71 Magnolia Hills | |
|------------------------|-------------------|-------------|-----|-------------|---|-----------|
| | | | | | | 167090962 |
| J0125-0179 | К1 | Flat Girder | 1 | 2 | | |
| | | | | _ | Job Reference (optional) | |
| Comtech, Inc, Fayettev | ille, NC - 28314, | | | 8.630 s Jul | 12 2024 MiTek Industries, Inc. Wed Jul 24 14:09:35 2024 | Page 2 |
| | | | | | | |

ID:d9Okus??o?Ogeo9B6tgaBuzGAgg-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-60, 2-6=-60, 6-7=-60, 8-14=-20

Concentrated Loads (b) Vert: 15=-665(F) 16=-177(F) 17=-177(F) 18=-177(F) 19=-177(F) 20=-177(F) 21=-177(F) 22=-1168(F)

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| | 7-1-3 | 1 | 12-8-12 5-7-9 | 17-7-8 4-10-12 | 22-9-13 5-2-5 |
|--|--|--|---|--|---|
| Plate Offsets (X,Y) | [2:0-2-13,Edge], [5:0-4-0,0-1-12], [6:0-3- | 0,0-3-8] | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.28 BC 0.17 WB 0.39 Matrix-S | DEFL. in Vert(LL) 0.04 Vert(CT) -0.04 Horz(CT) 0.00 | (loc) l/defl L/d 2-14 >999 240 2-14 >999 240 12 n/a n/a | PLATES GRIP MT20 244/190 Weight: 163 lb FT = 20% |
| LUMBER- TOP CHORD 2x6 BOT CHORD 2x6 WEBS 2x4 7-10 | SP No.1 SP No.1 SP No.2 *Except* : 2x6 SP No.1 | | BRACING- TOP CHORD BOT CHORD | Structural wood sheathing dire except end verticals, and 2-0-0 Rigid ceiling directly applied or 6-0-0 oc bracing: 11-12. | ectly applied or 6-0-0 oc purlins, 0 oc purlins (6-0-0 max.): 5-6. r 10-0-0 oc bracing, Except: |
| REACTIONS. (s Max Max | size) 10=Mechanical, 2=0-3-0, 12=0-5-8 (Horz 2=180(LC 12) (Uplift 10=-47(LC 12), 2=-172(LC 8), 12=-3 | 347(LC 8) | | | |

Max Grav 10=268(LC 1), 2=455(LC 1), 12=1156(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-511/339, 3-5=-425/463
- BOT CHORD 2-14=-502/429, 12-14=-328/340
- WEBS 3-14=-357/324, 3-12=-856/738, 5-12=-705/403, 6-10=-255/81

NOTES-

1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 15-1-4, Exterior(2) 15-1-4 to 21-4-11, Interior(1) 21-4-11 to 22-9-13 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) Provide adequate drainage to prevent water ponding.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide 4) will fit between the bottom chord and any other members.

5) Refer to girder(s) for truss to truss connections.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10 except (jt=lb) 2=172, 12=347.

7) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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818 Soundside Road

| F | 7-1-3 7-1-3 | | 12-6-0 5-4-13 | 12 _] 0-2 | 8 ₁ 12 2-12 | | | 22-9-13 10-1-1 | 3 | |
|--|--|--|------------------|---|--------------------------------------|---|--|---|--|-------------------------------------|
| Plate Offsets (X,Y) | [2:0-2-13,Edge], [5:0-3-0,0-2-12], [9:0-1 | 8,0-2-0] | | | | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCodeIRC2015/TPI2014 | CSI. TC 0.35 BC 0.21 WB 0.44 Matrix-S | | DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL) | in -0.05 -0.10 0.00 0.04 | (loc) 9-10 9-10 9 2-12 | l/defl >999 >999 n/a >999 | L/d 360 240 n/a 240 | PLATES MT20 Weight: 152 lb | GRIP 244/190 FT = 20% |
| LUMBER- TOP CHORD 2x6 BOT CHORD 2x6 WEBS 2x4 6-9: | SP No.1 SP No.1 SP No.2 *Except* 2x6 SP No.1 | | | BRACING- TOP CHOR BOT CHOR | D D | Structu except Rigid c 6-0-0 c | end wood s end vertic eiling dire oc bracing: | sheathing direct cals, and 2-0-0 o ctly applied or 1 : 9-10. | ly applied or 6-0-0 o oc purlins (6-0-0 ma 0-0-0 oc bracing, 1 | oc purlins, x.): 5-7. Except: |
| REACTIONS. (| size) 9=Mechanical, 2=0-3-0, 10=0-3-8 | | | | | | | | | |

Max Horz 2=171(LC 8) Max Uplift 9=-14(LC 9), 2=-175(LC 8), 10=-332(LC 8) Max Grav 9=309(LC 24), 2=472(LC 23), 10=1118(LC 23)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-513/375, 3-5=-373/391

BOT CHORD 2-12=-506/424, 10-12=-209/293

WEBS 3-12=-453/319, 3-10=-807/662, 5-10=-574/440

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 17-1-4, Exterior(2) 17-1-4 to 22-9-13 zone; porch left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Provide adequate drainage to prevent water ponding.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=175, 10=332.

8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

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| | 7-1-3 | | 12-6-0 | <u>12-8-12</u> 0-2-12 | | 22-9-13 10-1-1 | |
|--|---|---|--------|--|---|---|---|
| Plate Offsets (X,Y) | [2:0-2-1,Edge], [5:0-3-0,0-2-12], [9:0-1-8 | 3,0-2-0] | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.43 BC 0.27 WB 0.21 Matrix-S | | DEFL. in Vert(LL) -0.07 Vert(CT) -0.12 Horz(CT) 0.00 Wind(LL) 0.05 | (loc) l/defl L 9-10 >999 36 9-10 >977 24 9 n/a n 2-12 >999 24 | /d PLATES 50 MT20 10 /a 10 Weight: 156 lb | GRIP 244/190 FT = 20% |
| LUMBER- TOP CHORD 2x6 S BOT CHORD 2x6 S WEBS 2x6 S C4X S 6-9: 2 REACTIONS. (sia Max M Max M | P No.1 P No.2 *Except* x6 SP No.1 ze) 9=Mechanical, 2=0-3-0, 10=0-3-8 -forz 2=190(LC 8) Jplift 9=-17(LC 12), 2=-176(LC 8), 10=-3: Grav 9=332(LC 2), 2=483(LC 23), 10=10 | 21(LC 8) 93(LC 23) | | BRACING- TOP CHORD BOT CHORD WEBS | Structural wood shea except end verticals, Rigid ceiling directly T-Brace: Fasten (2X) T and I (0.131"x3") nails, 6ir Brace must cover 90 | athing directly applied or 6-0-0 and 2-0-0 oc purlins (6-0-0 ma applied or 10-0-0 oc bracing. 2x4 SPF No.2 - 5-10 braces to narrow edge of web o.c.,with 3in minimum end dis % of web length. | oc purlins, ax.): 5-7. with 10d tance. |
| FORCES.(lb) - MaxTOP CHORD2-3=BOT CHORD2-12WEBS3-12 | . Comp./Max. Ten All forces 250 (lb) or 503/370, 3-5=-313/320 ≥=-512/408 ≥=-521/366, 3-10=-787/594, 5-10=-452/35 | less except when sho | own. | | | | |
| NOTES- 1) Unbalanced roof liv | e loads have been considered for this de | sign. | | | | | |

- 2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -0-11-5 to 3-5-8, Interior(1) 3-5-8 to 19-1-4, Exterior(2) 19-1-4 to 22-9-13 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Provide adequate drainage to prevent water ponding.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=176, 10=321.
- 8) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 9) Warning: Additional permanent and stability bracing for truss system (not part of this component design) is always required.

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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 6-8=-178/332, 5-8=-178/332

BOT CHORD 6-7=-200/265 WEBS 4-6=-367/294, 5-9=-421/229

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-0-15 to 3-3-14, Interior(1) 3-3-14 to 10-6-4 zone; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2 except (jt=lb) 9=167.

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1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) 0-5-12 to 4-10-9, Interior(1) 4-10-9 to 7-4-8, Exterior(2) 7-4-8 to 11-9-5, Interior(1) 11-9-5 to 14-3-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=104, 6=104.

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TOP CHORD

BOT CHORD

TOP CHORD 2x4 SP No.1 BOT CHORD

2x4 SP No.1 OTHERS 2x4 SP No.2

REACTIONS. 1=8-7-3, 3=8-7-3, 4=8-7-3 (size) Max Horz 1=-64(LC 8) Max Uplift 1=-27(LC 12), 3=-33(LC 13) Max Grav 1=169(LC 1), 3=170(LC 1), 4=280(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3. 6) Non Standard bearing condition. Review required.

Structural wood sheathing directly applied or 6-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

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| | | | 4-1-10 | | 0-0-6 | |
|--|---|---|---|--------------------------------------|---|---|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.05 BC 0.02 WB 0.01 Matrix-P | DEFL. Vert(LL) Vert(CT) Horz(CT) | in (loc) n/a - n/a - 0.00 3 | l/defl L/d n/a 999 n/a 999 n/a n/a | PLATES GRIP MT20 244/190 Weight: 16 lb FT = 20% |

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LUMBER-
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TOP CHORD2x4 SP No.1BOT CHORD2x4 SP No.1OTHERS2x4 SP No.2

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 4-2-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 1=4-1-4, 3=4-1-4, 4=4-1-4 Max Horz 1=-42(LC 8) Max Uplift 1=-15(LC 13), 3=-15(LC 13) Max Grav 1=84(LC 1), 3=84(LC 1), 4=108(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope)

and C-C Exterior(2) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

| | | 6-1-0 6-1-0 | | | | | |
|--|---|--|---|---|--|--|--|
| Plate Offsets (X,Y) | [2:0-2-15,0-0-5] | | | | | | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 | SPACING-2-0-0Plate Grip DOL1.15Lumber DOL1.15Rep Stress IncrYESCode IRC2015/TPI2014 | CSI. TC 0.46 BC 0.14 WB 0.00 Matrix-P | DEFL. in (loc) I/defl L/d Vert(LL) -0.02 2-4 >999 360 Vert(CT) -0.03 2-4 >999 240 Horz(CT) -0.00 3 n/a n/a Wind(LL) 0.04 2-4 >999 240 | PLATES GRIP MT20 244/190 Weight: 26 lb FT = 20% | | | |

LUMBER-

TOP CHORD2x4 SP No.1BOT CHORD2x6 SP No.1

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=72(LC 8) Max Uplift 3=-68(LC 12), 2=-135(LC 8), 4=-30(LC 8)

Max Grav 3=168(LC 1), 2=325(LC 1), 4=118(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

 Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 6-0-4 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

4) Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=135.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

818 Soundside Road

| | | | 1-11-15 | |
|---------------|---------------------------------------|-------------|----------------------------------|-----------------------------|
| LOADING (psf) | SPACING- 2-0-0 Plate Grip DOI 1 15 | CSI. | DEFL. in (loc) I/defl L/d | PLATES GRIP MT20 244/190 |
| TCDL 10.0 | Lumber DOL 1.15 | BC 0.03 | Vert(CT) -0.00 2-4 >999 240 | WI120 2++/100 |
| BCDL 10.0 | Code IRC2015/TPI2014 | Matrix-P | Wind(LL) 0.00 2-4 >999 240 | Weight: 8 lb FT = 20% |

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LUMBER-
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TOP CHORD 2x4 SP No.1 BOT CHORD 2x4 SP No.1 BRACING-

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 1-11-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) 3=Mechanical, 2=0-3-0, 4=Mechanical Max Horz 2=33(LC 8) Max Uplift 3=-20(LC 12), 2=-84(LC 8), 4=-10(LC 8) Max Grav 3=35(LC 1), 2=176(LC 1), 4=39(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)

| | H | | <u>3-11-15</u> <u>3-11-15</u> | I |
|--|---|---|---|---|
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2015/TPI2014 | CSI. TC 0.15 BC 0.13 WB 0.00 Matrix-P | DEFL. in (loc) l/defl L/d Vert(LL) -0.01 2-4 >999 360 Vert(CT) -0.02 2-4 >999 240 Horz(CT) -0.00 3 n/a n/a Wind(LL) 0.02 2-4 >999 240 | PLATES GRIP MT20 244/190 Weight: 14 lb FT = 20% |

LUMBER-

TOP CHORD 2x4 SP No 1 BOT CHORD 2x4 SP No.1 BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 3-11-15 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. 3=Mechanical, 2=0-3-0, 4=Mechanical (size) Max Horz 2=51(LC 8) Max Uplift 3=-44(LC 12), 2=-108(LC 8), 4=-20(LC 8)

Max Grav 3=100(LC 1), 2=246(LC 1), 4=76(LC 3)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-10; Vult=130mph Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=15ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) and C-C Exterior(2) -1-2-8 to 3-2-5, Interior(1) 3-2-5 to 3-11-3 zone; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4 except (jt=lb) 2=108.

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| | H | 5-1-4 | | | | 8-5-12 | |
|--|--|---|---|-----------------------------------|--|---|------------------------------------|
| Plate Offsets (X,Y) | [2:0-3-11,0-0-3], [7:0-1-8,0-2-0] | 5-1-4 | | | | 3-4-0 | |
| LOADING (psf) TCLL 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0 | SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2015/TPI2014 | CSI. TC 0.20 BC 0.14 WB 0.12 Matrix-P | DEFL. in Vert(LL) 0.02 Vert(CT) -0.02 Horz(CT) 0.00 | (loc) 2-8 2-8 7 | l/defl L/d >999 240 >999 240 n/a n/a | PLATES MT20 Weight: 44 lb | GRIP 244/190 FT = 20% |
| LUMBER- TOP CHORD 2x4 S BOT CHORD 2x6 S WEBS 2x4 S 4-7: 2 | P No.1 P No.1 P No.2 *Except* x6 SP No.1 | | BRACING- TOP CHORD BOT CHORD | Structura except e Rigid ce | al wood sheathing di and verticals. iling directly applied | irectly applied or 6-0-0 or 10-0-0 oc bracing. | oc purlins, |
| REACTIONS. (siz Max Max Max (| ze) 7=Mechanical, 2=0-3-14 Horz 2=71(LC 19) Uplift 7=-150(LC 4), 2=-203(LC 4) Grav 7=370(LC 1), 2=458(LC 1) | | | | | | |
| FORCES. (lb) - Max TOP CHORD 2-3= BOT CHORD 2-8= WEBS 3-7= | . Comp./Max. Ten All forces 250 (lb 639/228 255/587, 7-8=-255/587 631/274 |) or less except when shown. | | | | | |
| NOTES- 1) Wind: ASCE 7-10; porch left and right | Vult=130mph Vasd=103mph; TCDL= | 5.0psf; BCDL=6.0psf; h=15ft; Cat | . II; Exp C; Enclosed | ; MWFRS | 6 (envelope); | | |

2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

3) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.

Refer to girder(s) for truss to truss connections.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7=150, 2=203.

6) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 17 lb down and 17 lb up at 2-10-15, 17 lb down and 17 lb up at 2-10-15, and 40 lb down and 54 lb up at 5-8-14, and 40 lb down and 54 lb up at 5-8-14 on top chord, and 3 lb down and 22 lb up at 2-10-15, 3 lb down and 22 lb up at 2-10-15, and 21 lb down and 22 lb up at 5-8-14, and 21 lb down and 42 lb up at 5-8-14 on top chord, and 42 lb up at 5-8-14 on totom chord. The design/selection of such connection device(s) is the responsibility of others.
7) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-5=-60, 2-6=-20 Concentrated Loads (lb) Vert: 10=-31(F=-15, B=-15) 12=-20(F=-10, B=-10)

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ENGINEERING BY A MITEK Atfiliate

818 Soundside Road

| isDesign | Client: Project: Address: | | Date: Input by: Job Nam | 1/10/2025 Neal Baggett ie: Lot 71 Magnolia Hills | Page 2 of 10 |
|---|---|----------------------------------|----------------------------------|--|-----------------|
| BM1 Kerto-S LV | L 1.750" X 9.250 |)" 2-Ply | Project # | Level: Level | |
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| | • • | • | • | • • | |
| | • • | • | • | • • | • <u> </u> |
| 1 SPF 0-3-8 | | 8' 1/2" | | 2 SPF 0- | 3-8 1 3 1/2" |
| { | | 8' 1/2" | | | |
| Multi-Ply Analysis | | | | | |
| Fasten all plies using 3 rows of Capacity 7 | of 10d Box nails (.128x3") at 1 | 2" o.c Maximu | m end distance n | ot to exceed 6". | |
| Yield Limit per Foot 2 Yield Limit per Fastener 9 | 82.4 PLF 4.1 lb. | | | | |
| Cm 1 Yield Mode IV Edge Distance 1 | / 1/2" | | | | |
| Min. End Distance 3 Load Combination D | " | | | | |
| Duration Factor 1 | .15 | | | | |
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| Notes Calculated Structured Designs is responsible only of the | chemicals Handling & Installation | 6. For flat roofs provid ponding | de proper drainage to prevent | Manufacturer Info Metsä Wood | _ |
| structural adequacy of this component based on the design criteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended component suitability of the intended | 1. LVL beams must not be cut or drilled 2. Refer to manufacturer's product informat regarding installation requirements, multi- fastening details, beam strength values, and cc | ion ply ode | | 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 | |
| application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive | approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to av lateral displacement and rotation | oid | - 11-1 11 - 0/00/2000 | www.metsawood.com/us | |

| 2 | | | Client: Project: | | | | | Date: Input by: | 1/10/2025 Neal Baggett | Page 4 of 1 |
|----------------------------|------------------------------|-------------------------|-------------------------|---------------|--|----------------|----------------|-------------------------|-----------------------------------|-----------------------|
| ļ | isDesign | | Address: | | | | | Job Name: Project #: | Lot 71 Magnolia Hills | |
| BM3 | Kerto-S | LVL | 1.750" | X 11.87 | 5" 2 | -Ply | - PA S | SSED└ | evel: Level | |
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| 1 SPF | End Grain 0-3-0 | | | | | | | | 2 SPF End Grain 0-5-0 | |
| | | | | | 16'2 1/2" 16'2 1/2" | | | | | 3 1/2" |
| • | | | | | | | | | | |
| Multi-Ply | / Analysis | | | | | | | | | |
| asten all | plies using 2 ro | ows of 100 | d Box nails (.1 | 128x3") at 12 | " o.c Ma | aximum | end dis | stance no | t to exceed 6". | |
| apacity oad | | 0.0 % 0.0 PLF | | | | | | | | |
| ield Limit pe | er Foot er Fastener | 163.7 P 81 9 lb | LF | | | | | | | |
| ieid Liniit pe M | | 1 | | | | | | | | |
| ield Mode dge Distand | ce. | IV 1 1/2" | | | | | | | | |
| in. End Dis | tance | 3" | | | | | | | | |
| oad Combir | nation stor | 1 00 | | | | | | | | |
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| Notes Calculated Struct | tured Designs is responsible | che only of the Hand | emicals | ı | For flat r ponding | oofs provide p | proper drainag | e to prevent | Metsä Wood | |
| structural adequ | acy of this component base | ed on the 1. LV | L beams must not be cut | or drilled | | | | | 301 Merritt 7 Building, 2nd Floor | |

| Notes | chemicals | 6. For flat roofs provide proper drainage to prevent | Manufacturer Info | |
|---|---|--|--|--|
| Calculated Structured Designs is responsible only of the structural adequacy of this component based on the design oriteria and loadings shown. It is the responsibility of the customer and/or the contractor to ensure the component suitability of the intended application, and to verify the dimensions and loads. Lumber 1. Dry service conditions, unless noted otherwise 2. LVL not to be treated with fire retardant or corrosive | Handling & Installation 1. UVL beams must not be cut or drilled 2. Refer to manufacturer's product information regarding installation requirements, multi-ply fastening details, beam strength values, and code approvals 3. Damaged Beams must not be used 4. Design assumes top edge is laterally restrained 5. Provide lateral support at bearing points to avoid lateral displacement and rotation | ponding This design is valid until 6/28/2026 | Metsä Wood 301 Merritt 7 Building, 2nd Floor Norwalk, CT 06851 (800) 622-5850 www.metsawood.com/us | |

| Í | isDesign | | Client: Project: Address: | | | | Date: Input by: Job Name | 1/10/2025 Neal Baggett c: Lot 71 Magnolia Hills | Pa | age 6 of 10 |
|--------------------------------|------------------------|------------------------|---------------------------------|--------------|------------|--------------------------|--------------------------------|---|-----|-------------|
| BM4 | SP #2 | 2.000" | X 12.0 | 00" 2 | 2-Ply · | PASSE | Project #: | Level: Level | | |
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| • | • | • | • | • | • | • | • | | · | 11 1/4" |
| | F End Grain 0-3-0 | | | | 11' | | | 2 SPF End Grair | | |
| / | | | | | 11' | | | | ł | |
| Multi-Ply | y Analysis | ows of 10d B | ov nails (1) | 28v3") at 1 | 2" o c . M | lavimum end | distance n | at to exceed 6" | | |
| Fasten an Capacity Load | i pilės using 2 r | 0.0 % 0.0 PLF | ox nails (. i. | 28x3) at 1. | 2 O.C IV | iaximum end | distance no | ot to exceed 6 . | | |
| Yield Limit p Yield Limit p | er Foot er Fastener | 202.6 PLF 101.3 lb. | | | | | | | | |
| Cm Yield Mode | | 1 IV | | | | | | | | |
| Edge Distan Min. End Dis | ice stance | 1 1/2" 3" | | | | | | | | |
| Load Combi Duration Fac | ination ctor | 1.00 | | | | | | | | |
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| 1 | | | | | This d | lesign is valid until 6/ | 28/2026 | | | |

| 1 | isDesign | Client: Project: Address: | | | Date: Input by Job Nan | 1/10/2025 : Neal Baggett ne: Lot 71 Magnolia Hills # | | Page 8 of 10 |
|---|---|---|--|---|---------------------------------|---|-------------|--------------|
| BM2 | Kerto-S L | VL 1.750 |)" X 9.250 | " 2-Ply | - PASSED | ≁. Level: Level | | |
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| • | ٠ | • | • | • | • | • • | 1 1/2" | 9 1/ |
| | • PF End Grain 0-3-0 | • | • | • | • | • • • • • • • • • • • • • • • • • • • | | |
| | | | 7'7 7'7 | | | | | 3 1/2" |
| Multi-Ply | y Analysis | | | | | | | |
| Fasten all Capacity | l plies using 2 rows | s of 10d Box nails | s (.128x3") at 12 | " o.c Maxim | um end distance r | not to exceed 6". | | |
| Load Yield Limit p Vield Limit p | per Foot | 0.0 PLF 163.7 PLF 81.9 lb | | | | | | |
| CM Yield Mode | er Fastener | 1 IV | | | | | | |
| Edge Distan Min. End Dis | nce stance | 1 1/2" 3" | | | | | | |
| Load Combin Duration Fac | ination ctor | 1.00 | | | | | | |
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| | | | | | | Manufacturer Info | | |
| Notes Calculated Structural adeque | ctured Designs is responsible only o uacy of this component based or | chemicals of the Handling & Install | ation | For flat roofs pro- ponding | vide proper drainage to prevent | Metsä Wood | | |
| design criteria responsibility of ensure the co | a and loadings shown. It is f the customer and/or the contract omponent suitability of the inte | the 2. Refer to manufac or to regarding installation nded fastening details. here | turer's product information trer's product information on requirements, multi-pl on strength values, and code | 1 | | Norwalk, CT 06851 (800) 622-5850 | | |
| application, and Lumber | to verify the dimensions and loads. | approvals 3. Damaged Beams mus 4. Design assumes top e | it not be used idge is laterally restrained | | | www.metsawood.com/us | | |
| 2. LVL not to b | be treated with fire retardant or corre | 5. Provide lateral support lateral displacement a | ort at bearing points to avoid nd rotation | This design is | valid until 6/28/2026 | | | |

| P | | Client: | | Date: | 1/10/2025 | Page 10 of 7 |
|--|---|---|--|-----------------------|---|----------------|
| 1 | isDesign | Project: Address: | | Input by: Job Name | iveai Baggett Lot 71 Magnolia Hills | |
| | | | | Project #: | | |
| GDH | Kerto-S LV | L 1.750" X 11.87 | 5" 2-Ply - PASS | SED ^L | evel: Level | |
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| | | | | | | ↓ ↓ 11 7/8" |
| | • • • | • • • • | • • • | • • | • • • • | |
| | End Grain 0-5-0 | | | | | |
| 1 | | | 16'10" | | | ່ 1 1/3 1/2" |
| 1 | | | 16'10" | | | 1 |
| | | | | | | |
| ⁄lulti-Ply | / Analysis | | | | | |
| asten all | plies using 2 rows | of 10d Box nails (.128x3") at | 12" o.c Maximum end c | listance no | t to exceed 6". | |
| oad | | 5.3 % 10.0 PLF | | | | |
| ield Limit pe | er Foot | 188.3 PLF | | | | |
| eia Limit pe M | er Fastener | 94.1 ID. 1 | | | | |
| ield Mode | | IV | | | | |
| dge Distand lin End Dis | ce itance | 1 1/2" 3" | | | | |
| oad Combir | nation | D+S | | | | |
| Juration Fac | tor | 1.15 | | | | |
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| Notes | | chemicals | 6. For flat roofs provide proper drain | nage to prevent | Manufacturer Info | |
| Calculated Struct structural adequa | tured Designs is responsible only of acy of this component based on | the Handling & Installation the 1. LVL beams must not be cut or drilled | ponunty | | Metsä Wood 301 Merritt 7 Building, 2nd Floor | |
| esign criteria esponsibility of ensure the cor | and loadings shown. It is the customer and/or the contracto mponent suitability of the inten- | r to regarding installation requirements, mu | ation Iti-ply | | Norwalk, CT 06851 | |
| application, and t | to verify the dimensions and loads. | approvals 3. Damaged Beams must not be used | uuu | | www.metsawood.com/us | |
| 1. Dry service of | onditions, unless noted otherwise | Design assumes top edge is laterally restrained Provide lateral support at bearing points to | avoid | | | |
| ∠. LVL not to be | e ueated with fire retardant or corros | lateral displacement and rotation | This design is valid until 6/2 | 8/2026 | | 1 |