# Mark Morris, P.E.

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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 52305 JOB: 24-7828-R01 JOB NAME: LOT 0.0009 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 58 Truss Design(s)

Trusses:

GJ01, GJ02, GJ03, GJ04, GJ05, GJ06, GR01, GR02, GR02A, GV01, J01, J01A, J02, J03, J04, J05, J06, J07, J08, R01, R02, R03, R04, R05, R07, R07A, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, R22, R23, R24, R24A, R24B, R25, R27, R27A, R28, R29, R30, R31, R32,



## Warning !--- Verify design parameters and read notes before use.





Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER, NC	
24-7828-R01	GJ02	Jack-Open Girder	2	1	Job Reference (optional)	# 52305
		Ru	n: 8.430 s Feb 1 ID:PlfkN	2 2021 Prin 15JZRq7i2	t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Se 2cSu5g6whSyi2C3-ywBvJhXe74SNgfk7t0ulle	p 17 10:36:28 2024 Page 2 xYuaf_30selNytlLycmOH

## LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb)

Concentrated Loads (lb) Vert: 11=-13(F) 12=-17(F)









vertically. Applicability of design parameters and read inters before user into seargh is based only upon parameters shown, and is to fail individual voltating component to be instance and rotation of component is responsibility of building designer – not truss designer of truss engineer. Bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIEF	R, NC
24-7828-R01	GR01	Common Girder	1	1	Job Reference (optional)	# 52305
		R	Run: 8.430 s Feb 12	2 2021 Prin	t: 8.630 s Jul 12 2024 MiTek Industries, Inc.	Tue Sep 17 10:36:35 2024 Page

ID:PlfkM5JZRq7/22Su5g6wh5y/2C3+FG7Yn4c1UDKN0kmTo\_WO46kg7PVAC2WgMz8VRycmOA 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

 Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 2-6=-20

Concentrated Loads (lb)

Vert: 15=-49(F) 17=-17(F) 20=-64(F) 21=-64(F) 24=-17(F) 26=-49(F) 27=-42(F) 28=-56(F) 29=-82(F) 30=-109(F) 31=-11(F) 32=-11(F) 33=-109(F) 34=-82(F) 35=-56(F) 36=-42(F)

SEAL 28147 9/16/2024



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Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIEF	R, NC
24-7828-R01	GR02	Common	3	1	Job Reference (optional)	# 52305
		Run: 8	.430 s Feb 1	2 2021 Prir	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc.	Tue Sep 17 10:36:37 2024 Page

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER, NC	
24-7828-R01	GR02A	COMMON	1	1	Job Reference (optional)	# 52305
		Run: 8	.430 s Feb 1 ID:PlfkM5	2 2021 Prin IZRq7i2cS	nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Se Su5g6whSyi2C3-frohP5fvn8jytCV2T635iIMEV	ep 17 10:36:38 2024 Page 2 cynPT562wNP6mycmO7

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 11) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 13) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





LOAD CASE(S) Standard

MORPHS INTERNAL MORPHS INTERNAL MORPHS INTERNAL MORPHS INTERNAL MARKEN INTERNA Autonum Ant Ak NOINEE 9/16/2024







![](_page_18_Figure_0.jpeg)

BCLL BCDL	0.0 * 10.0	Code IRC2021/TPI2014	Matrix-MR	Horz(CT)	0.01 3	n/a	n/a	Weight: 13 lb	FT = 20%
LUMBER- TOP CHOR BOT CHOR	D 2x4 SP No.2 D 2x4 SP No.2		E	BRACING- TOP CHORD	Structural v end vertica	wood she Is.	athing direct	ly applied or 3-3-12 oc	purlins, except
WEBS	2x4 SP No.3		E	BOT CHORD	Rigid ceilin	g directly	applied or 1	0-0-0 oc bracing.	
					MiTek red be install	commend ed during	s that Stabil truss erection	izers and required cros on, in accordance with	ss bracing Stabilizer

Installation guide.

REACTIONS. (lb/size) 5=195/0-3-8 (min. 0-1-8), 3=80/Mechanical, 4=34/Mechanical Max Horz 5=76(LC 12) Max Uplift5=-4(LC 12), 3=-49(LC 12) Max Grav 5=195(LC 1), 3=85(LC 20), 4=58(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

# **NOTES-** (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

- between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 4 lb uplift at joint 5 and 49 lb uplift at joint 3.

LOAD CASE(S) Standard

![](_page_18_Picture_14.jpeg)

![](_page_19_Figure_0.jpeg)

#### REACTIONS. (Ib/size) 4=43/Mechanical, 7=195/0-3-8 (min. 0-1-8), 5=71/Mechanical Max Horz 7=60(LC 12) Max Uplift4=-5(LC 9), 7=-14(LC 12), 5=-24(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 5 lb uplift at joint 4, 14 lb uplift at joint 7 and 24 lb uplift at joint 5.

LOAD CASE(S) Standard

![](_page_19_Picture_15.jpeg)

![](_page_20_Figure_0.jpeg)

![](_page_21_Figure_0.jpeg)

LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied.

Rigid ceiling directly applied.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 3=124/Mechanical, 2=249/0-5-4 (min. 0-1-8), 4=63/Mechanical Max Horz 2=53(LC 10) Max Uplift3=-40(LC 14), 2=-48(LC 10)

Max Grav 3=173(LC 21), 2=335(LC 21), 4=85(LC 7)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 3 and 48 lb uplift at joint 2.

9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

![](_page_21_Picture_21.jpeg)

![](_page_22_Figure_0.jpeg)

Max Grav 3=88(LC 20), 2=191(LC 1), 4=61(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 46 lb uplift at joint 3 and 7 lb uplift at joint 2.

LOAD CASE(S) Standard

![](_page_22_Picture_12.jpeg)

![](_page_23_Figure_0.jpeg)

Max Grav 3=50(LC 20), 2=147(LC 18), 4=36(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 3 and 11 lb uplift at joint 2.

LOAD CASE(S) Standard

![](_page_23_Picture_13.jpeg)

![](_page_24_Figure_0.jpeg)

Scale = 1:30.7

![](_page_24_Figure_2.jpeg)

H	4-10-1 4-10-1	9-4-6 4-6-5		14-0-7 4-8-1		<u>18-0-0</u> 3-11-9	
Plate Offsets (X,Y) [4:0-5	-8,0-2-0], [5:0-2-11,0-1-1]			-			
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2021/TPI2014	CSI. TC 0.63 BC 0.44 WB 0.80 Matrix-MSH	<b>DEFL.</b> Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.09 8 >999 -0.14 8-10 >999 0.02 5 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 106 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x6 SP No.2 WEBS 2x4 SP No.3 W1: 2x6 SP	*Except* No.2		BRACING- TOP CHORD BOT CHORD	Structural wood sh end verticals. Rigid ceiling directl MiTek recommen be installed during Installation guide.	eathing directly y applied or 10 ds that Stabiliz g truss erection	y applied or 3-2-3 oc D-0-0 oc bracing. zers and required cro n, in accordance with	purlins, except ss bracing Stabilizer
REACTIONS. (lb/size) 11=962/0-3-8 (min. 0-1-8), 5=900/0-3-8 (min. 0-1-8) Max Horz 11=-77(LC 10) Max Uplift11=-191(LC 8), 5=-129(LC 13) Max Grav 11=1250(LC 33), 5=1006(LC 34)							
FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       1-15=-1791/254, 15-16=-1791/254, 2-16=-1791/254, 2-17=-2252/321, 17-18=-2252/321, 18-19=-2252/321, 3-19=-2252/321, 3-20=-2252/321, 20-21=-2252/321, 4-21=-2252/321, 4-5=-1694/229, 1-11=-1139/220         BOT CHORD       10-24=-255/1791, 9-24=-255/1791, 8-9=-255/1791, 8-25=-158/1452, 25-26=-158/1452, 7-26=-158/1452, 5-7=-161/1443         WEBS       1-10=-279/1922, 2-10=-759/196, 2-8=-83/522, 3-8=-535/164, 4-8=-173/898							
<ul> <li>BOT CHORD 10-24-2501/191, 92-4-2501/191, 9-3-2501/191, 9-25-1001/452, 25-80-1001/452, 25-80-1001/452, 7-268-158/1452, 5-78-161/1443</li> <li>WEBS 1-10=-279/1922, 2-10=-759/196, 2-8=-83/522, 3-8=-535/164, 4-8=-173/898</li> <li>NOTES- (11) <ol> <li>Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10</li> <li>Unbalanced snow loads have been considered for this design.</li> </ol> </li> <li>This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.</li> <li>Provide adequate drainage to prevent water ponding.</li> <li>This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 11 and 129 lb uplift at joint 5.</li> <li>Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 191 lb uplift at joint 11 and 129 lb uplift at joint 5.</li> <li>NALEDP indicates 3-10d (0.148*x3") or 3-12d (0.148*x3.25") toe-nails per NDS guidlines.</li> <li>In the LOAD CASE(S) Standard <ol> <li>LeoAD CASE(S) Standard</li> <li>Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15</li> <li>Uniform Loads (pf)</li> <li>Vert: 1-4=-60, 4-6=-60, 11-12=-20</li> </ol> </li> </ul>							

Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER	R, NC
24-7828-R01	R01	Hip Girder	1	1	Job Reference (optional)	# 52305
		Dup 9 A	20 a Eab 1	2 2021 Drin	t: 9 620 a Jul 12 2024 MiTak Industriaa Ina	Tuo Son 17 10:26:52 2024 Dogo 1

ID:PlfkM5JZRq7i2cSu5g6whSyi2C3-FXe\_LuqhUSUzYMZkH2JNGixaGFolhgjAH6m9byycmNv

# LOAD CASE(S) Standard

Concentrated Loads (lb) Vert: 1=-50(B) 4=-24(B) 11=-28(B) 9=-20(B) 7=-20(B) 15=-24(B) 16=-24(B) 17=-24(B) 19=-24(B) 20=-24(B) 21=-24(B) 22=-20(B) 23=-20(B) 24=-20(B) 25=-20(B) 26=-20(B) 26=-20(B)

> SEAL 28147 9/16/2024

![](_page_26_Figure_0.jpeg)

![](_page_27_Figure_0.jpeg)

![](_page_28_Figure_0.jpeg)

![](_page_29_Figure_0.jpeg)

![](_page_30_Figure_0.jpeg)

![](_page_31_Figure_0.jpeg)

![](_page_32_Figure_0.jpeg)

9/16/2024

![](_page_33_Figure_0.jpeg)

![](_page_34_Figure_0.jpeg)

![](_page_35_Figure_0.jpeg)

Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Appliedability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER, NC	
24-7828-R01	R11	Common Girder	1	2	Job Reference (optional)	# 52305
		Run: 8.4 ID	30 s Feb 1 PlfkM5JZI	2 2021 Prin Rq7i2cSu	t: 8.630 s Jul 12 2024 MiTek Industries, Inc.  Tue Se 5g6whSyi2C3-ncc1iM0kjMVhTpnpDPc7w3b0	ep 17 10:37:08 2024 Page 2 OOiBXRx5Xybe?90ycmNf

## LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 11-14=-20

Concentrated Loads (lb)

Vert: 7=-574(B) 10=-508(B) 17=-658(B) 18=-508(B) 19=-587(B) 20=-587(B) 21=-574(B) 22=-574(B) 23=-575(B)

![](_page_36_Picture_6.jpeg)

![](_page_37_Figure_0.jpeg)

![](_page_38_Figure_0.jpeg)

![](_page_39_Figure_0.jpeg)

![](_page_40_Figure_0.jpeg)

vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and loaded vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and loaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

![](_page_41_Figure_0.jpeg)

![](_page_42_Figure_0.jpeg)

![](_page_43_Figure_0.jpeg)

**NOTES-** (9)

LOAD CASE(S) Standard

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; porch left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

6) Refer to girder(s) for truss to truss connections.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 4.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

![](_page_43_Figure_10.jpeg)

![](_page_44_Figure_0.jpeg)

- 14) Bearing at joint(s) 12, 11, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify

Weenagily Of detarities is a factor of the second s of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive Madison WI 53719

9/16/2024

Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER, NC	
24-7828-R01	R22	GABLE COMMON	1	1	Job Reference (optional)	# 52305
	·	Run: 8.4 ID	30 s Feb 1 PlfkM5JZ	2 2021 Prin Rg7i2cSu	it: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue S I5g6whSyi2C3-RwLZDT9Gu2? vfi7wxqxPb	ep 17 10:37:20 2024 Page 2 5QbXPiFX8IjTYdaKycmN1

**NOTES-** (17) 15) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 17, 16, 15, 14, 12, 18 except (jt=lb) 10=130. 16) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 17, 16, 15, 14, 12, 11, 18, 10.

LOAD CASE(S) Standard

![](_page_45_Picture_3.jpeg)

Job Truss ITruss Type IOfv IPIv II OT O 0000 CAMPRELL RI	DGE   ANGIER NC
24-7828-R01 R23 Half Hip 1 1	# 57205
Run: 8,430 s Feb 12 2021 Print: 8,630 s Jul 12 2024 MiTek	) π 32303 Industries, Inc. Tue Sep 17 10:37:22 2024 Page 1
ID:PIfkM5JZRq7i2cSu5g6whSyi2C3-NISKe8BW0 -0-11-0 7-4-8 , 14-8-15 ,16-3-7, 20-9-0 ,	QgFi9zsV2MsPU0BdgLy7jEpbAn1kfCycmNR
0-11-0 7-4-8 7-4-8 1-6-8 4-5-9	
7 00 [12] $6x8 = 2x4    5x5 =$	Scale = 1:65.8
$5 \frac{176}{T3}$ T3	
3x6 - T2 W1 W3 B3 88 [∓	r
$4x6 = 4$ $9_{7x8} = 3x4$	
$3 \frac{16}{3} \frac{10}{10} \frac{10}{4x6}$	
10 15 T1 W1 11 7.00 12	4
5x6 / 4x6 / 4x6	6
1 2 W2 B1 12 5x5 -	
white	
51	
	l
13 3x4	
7-4-8 14-8-15 16-3-7 20-9-0 7-4-8 7-4-8 1-6-8 4-5-9	
Plate Offsets (X,Y) [2:0-3-0,0-1-8], [5:0-5-0,0-1-12], [7:0-2-8,0-1-12], [9:0-5-4,Edge]	
LOADING (psf) TCLL (roof) 20.0 SPACING- 2-0-0 CSI. DEFL. in (loc) I/defl L/d	PLATES GRIP
Snow (Pf)         20.0         Plate Grip DOL         1.15         TC         0.95         Vert(LL)         -0.22 10-12         >999         240           Snow (Pf)         20.0         Lumber DOL         1.15         BC         0.77         Vert(CT)         -0.37 10-12         >660         180	MT20 244/190
BCLL 0.0 * Rep Stress Incr YES WB 0.88 Horz(CT) 0.18 8 n/a n/a	Weight: 116 lb ET = 20%
TOP CHORD 2x4 SP No.2 *Except* TOP CHORD Structural wood sheathing dire	ctly applied, except end verticals.
BOT CHORD 2x4 SP No.1 BOT CHORD CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2	ilizers and required cross bracing
WEBS 2x4 SP No.3 *Except* be installed during truss erect be erected be installed during truss erect be erected be erected by the trust erected by	tion, in accordance with Stabilizer
Max Horz 13=228(LC 14)	
Max Uplitt13=-40(LC 14), 8=-115(LC 14) Max Grav 13=1161(LC 36), 8=900(LC 36)	
FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown.	
TOP CHORD 2-13=-1154/222, 2-14=-3001/417, 14-15=-2881/419, 3-15=-2727/439, 3-16=-2463/297, 4 16= 2352/207, 4 5= 2354/231, 5 17= 2114/230, 6 17= 2114/230, 6 7= 2000/236	
4-10	
WEBS 2-12=-291/2441, 3-12=-613/169, 3-10=-542/226, 5-10=-28/357, 5-9=-71/577, 6-9=-276/207,	
7-9=-327/2129	
NOTES- (12)	
<ul> <li>2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS</li> <li>(wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS</li> </ul>	
(envelope) gable end zone and C-C Exterior(2E) -0-11-0 to 3-10-10, interior(1) 3-10-10 to 7-11-8, Exterior(2R) 7-11-8 to 15-9-10, Exterior(2E) 15-9-10 to 20-7-4 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces δ	x
MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16: Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15): Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15): Is=1.0: Rough	1
Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10	ANNUM CARCINE
<ul><li>5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs</li></ul>	STAN SEESSIG
6) Provide adequate drainage to prevent water ponding.	and have the
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will f	SEAL
between the bottom chord and any other members.	28147
of bearing surface.	ANDINER
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13 except (jt=lb) 8=115.	TAK K MORALININ
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord	White the second states
	9/16/2024

![](_page_47_Figure_0.jpeg)

![](_page_48_Figure_0.jpeg)

vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

![](_page_49_Figure_0.jpeg)

![](_page_50_Figure_0.jpeg)

Job	Truss	Truss Type	Qtv	Plv		GEL ANGIER NC
24-7828-R01	R27	Monopitch	1	1		# 52305
2110201101			Run: 8.430 s Feb		lob Reference (optional) 8.630 s Jul 12 2024 MiTek Ir	# 32303 ndustries, Inc. Tue Sep 17 10:37:29 2024 Page 1
		-0-11-0 7-11-8	ID:PlfkM5JZR	q7i2cSu5g6w	vhSyi2C3-gfNz6XGvmp8	jU1usyKU2GVzqMAKasTednMEcOlycmNk
		0-11-0 7-11-8		3-0-0		
			7.00 4	4	4x6 🖉	Scale = 1:65.9
	т		7.00   1.	2	5	
	11-10	9 71	3x6 = 10 2x4    4	T2 W4	WV5	
	4 7 7 7	5x5 = 1 2 1 2 1 2 8 3x4	$\frac{1}{7}$		6 3x4 =	
		7.11.8	11	5-11-8		
		7-11-0 7-11-8		3-0-0		
LOADING (nsf)	<u>J-2-0,0-1-12], [5:0-2-14,0-2-</u>					
TCLL (roof) 20.0 Snow (Pf) 20.0	SPACING- Plate Grip DOL	2-0-0 <b>CSI.</b> 1.15 TC 0.86	DEFL. Vert(LL)	in (loc -0.29 6-7	c) l/defl L/d 7 >641 240 7 >403 180	PLATES         GRIP           MT20         244/190
TCDL         10.0           BCLL         0.0 *           BCDL         10.0	Rep Stress Incr Code IRC2021/TP	1.15         BC         0.85           YES         WB         0.55           I2014         Matrix-AS	Horz(CT)	-0.38 6-7	7 >493 180 6 n/a n/a	Weight: 115 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No BOT CHORD 2x4 SP No WEBS 2x4 SP No W5: 2x4 SP	0.2 0.2 0.3 *Except* P No.2		BRACING- TOP CHORD BOT CHORD WEBS	Structural Rigid ceilin 1 Row at r MiTek re be instal Installatio	wood sheathing direct ng directly applied. midpt 5-6, 5- ecommends that Stabil led during truss erection on guide.	tly applied, except end verticals. 7 izers and required cross bracing on, in accordance with Stabilizer
REACTIONS. (ID/size) Max Horz Max Uplif Max Grav	6=625/Mechanical, 8=693/ 8=348(LC 13) 6=-143(LC 14), 8=-39(LC 1 6=865(LC 24), 8=718(LC 2	0-3-0 (min. 0-1-8) 14) 11)				
FORCES. (Ib) - Max. Co	mp./Max. Ten All forces 2	250 (lb) or less except when show	vn.			
TOP CHORD 2-9=-660 2-8=-644 BOT CHORD 7-8=-329	)/84, 3-9=-566/104, 3-4=-69 I/137 9/492	93/198, 4-10=-599/210, 5-10=-52	8/234, 5-6=-698/188	3		
WEBS 3-7=-595	5/256, 5-7=-246/862, 2-7=-4	13/469				
<ol> <li>Wind: ASCE 7-16; Vul (envelope) gable end 2 cantilever left and righ Lumber DOL=1.60 pla</li> <li>TCLL: ASCE 7-16; Pre Cat B; Partially Exp.; C</li> <li>Unbalanced snow load</li> <li>This truss has been de pop concurrent with of</li> </ol>	t=120mph (3-second gust) zone and C-C Exterior(2E) - t exposed ; end vertical left te grip DOL=1.60 =20.0 psf (roof LL: Lum DOI 2e=1.0; Cs=1.00; Ct=1.10 ds have been considered fo ssigned for greater of min re bet live loads	Vasd=95mph; TCDL=5.0psf; BC -0-11-0 to 3-10-10, Interior(1) 3-1 and right exposed;C-C for memb L=1.15 Plate DOL=1.15); Pf=20.0 r this design. pof live load of 12.0 psf or 2.00 tir	DL=5.0psf; h=23ft; C 0-10 to 11-0-2, Exter ers and forces & MV ) psf (Lum DOL=1.1t nes flat roof load of 2	cat. II; Exp B rior(2E) 11-0 VFRS for rea 5 Plate DOL: 20.0 psf on c	; Enclosed; MWFRS 0-2 to 15-9-12 zone; actions shown; =1.15); ls=1.0; Rough overhangs	
<ul> <li>5) This truss has been de</li> <li>6) * This truss has been de</li> <li>6) * This truss has been de</li> <li>7) Refer to girder(s) for tr</li> <li>8) Provide mechanical co</li> <li>9) This truss design requires the strust design requires the strust design requires the struct be applied of</li> </ul>	esigned for a 10.0 psf botton designed for a live load of 3 nord and any other member uss to truss connections. Donnection (by others) of trus ires that a minimum of 7/16 directly to the bottom chord.	m chord live load nonconcurrent 00.0psf on the bottom chord in all s, with BCDL = 10.0psf. s to bearing plate capable of with " structural wood sheathing be a	with any other live lo areas where a recta nstanding 100 lb upli oplied directly to the	ads. ngle 3-6-0 ta ft at joint(s) t top chord ar	all by 1-0-0 wide will fit 8 except (jt=lb) 6=143. nd 1/2" gypsum	SEAL 28147
LOAD CASE(S) Standar	d				Another States	ANGINEER AS
						9/16/2024

![](_page_52_Figure_0.jpeg)

![](_page_53_Figure_0.jpeg)

![](_page_54_Figure_0.jpeg)

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER, NC	
24-7828-R01	R29	Half Hip Supported	1	1	Job Reference (optional)	# 52305
		Run: 8 ID:F	.430 s Feb 1 IfkM5JZRq	2 2021 Prin 7i2cSu5g6	t: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Se SwhSyi2C3-UolENbLgMfusDyL?JabSWmDqn	p 17 10:37:35 2024 Page 2 aTyGGQV9lhwcyycmNE

NOTES- (14) 13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11, 15, 16, 17, 14, 13, 12 except (jt=lb) 19=127, 18=209.

LOAD CASE(S) Standard

![](_page_55_Picture_3.jpeg)

![](_page_56_Figure_0.jpeg)

Scale = 1:31.0

![](_page_56_Figure_2.jpeg)

Plate Grip Differis (XY)-       I2:03-12:07-13[       I4:04-00-1-12]       ICO         LOADING (psf)       SPACING-       2:0-0       TC       0.49       Vert(L)       -0.10 10:11-999       240         VEX.L (roof)       2:0       Plate Grip DOL       1.15       BC       0.85       Vert(CT)       -0.19 10:11-999       180         BCDL       1:0:0       Code IRC2021/TPI2014       WB       0.85       Horz(T)       0.19 10:11-999       240         LUMEER-       1:0:0       Code IRC2021/TPI2014       WB       Matmx-MSH       For OP CHORD       24.67       N'a       N'a         UMEES-       2:45 SP No.2       WB       BCOL       TOP CHORD       24.65 P No.2       Structural wood sheathing directly applied or 3-1-11 oc purlins, exce         WEBS       2:44 SP No.3       BOT CHORD       2:38-2799/466, 3-14-1625/363, 4-14-1772/369, 4-156-1357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-16-3357/286, 5-17-3357/286,	ļ	5-0-0	8-10-5		13-5-2	18-0	)-0 -14
LOADING (psr) TGL (por)       SPACING- 2:0:0       2:0:0       SPACING- Plate Grip DOL       1:1:5       TC       0:49       DEFL.       in (loc)       I/det       L/d         MIZO       20:0       Plate Grip DOL       1:15       BC       0:85       Vert(C1)       0:01       1:15       PLATES       GRIP         MIZO       20:0       1:0:0       Code IRC202/TIPI2014       Watrw-MSH       DEFL.       0:0:0:1:1:1 >999       240         Vert(C1)       0:0:0       Code IRC202/TIPI2014       Watrw-MSH       BC       0:0:0:1:1 >999       240         UMBER.       Code IRC202/TIPI2014       Matrw-MSH       BC       0:0:0:1:1 >999       240         UMBER.       TOP CHORD       2:4:5:0:0:0:0:0:1:1:5:0:0:0:0:0:0:0:0:0:0:0	Plate Offsets (X,Y)	2:0-3-12,0-1-8], [4:0-6-0,0-1-12]	0 10 0			40	
LUMBER       BRACING-         TOP CHORD 2x6 SP No.2       TOP CHORD         WEBS       2x4 SP No.3         WEBS       2x4 SP No.3         BOT CHORD       2x6 SP No.2         WEBS       2x4 SP No.3         BOT CHORD       2x6 SP No.2         BOT CHORD       2x6 SP No.3         BOT CHORD       2x6 SP No.3         BOT CHORD       2x6 SP No.3         BOT CHORD       2x4 SP No.3         BOT CHORD       2x8 SP No.3         BOT CHORD       2x3-2795/466, 3-14=-185/256, 01-18-1357/286, 5-16=-1357/286, 5-16=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-17=-1357/286, 5-10=-327/1730, 20-21=-	LOADING (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr NO Code IRC2021/TPI2014	<b>CSI.</b> TC 0.49 BC 0.85 WB 0.63 Matrix-MSH	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.10 10-11 >999 -0.19 10-11 >999 0.04 7 n/a	L/d <b>Pi</b> 240 M 180 n/a W	LATES GRIP IT20 244/190 Veight: 102 lb FT = 20%
<ul> <li>REACTIONS. (Ib/size) 7-882/Mechanical, 2=931/0-5-4 (min. 0-1-8) Max Horz 2=88(LC 52) Max Uplif7=-198(LC 8), 2=-180(LC 8) Max Grav 7=1016(LC 33), 2=1113(LC 34)</li> <li>FORCES. (Ib) - Max. Comp./Max. Ten All forces 250 (Ib) or less except when shown.</li> <li>TOP CHORD 2-3=-2795/466, 3-14=-1825/363, 4-14=-1772/369, 4-15=-1357/286, 15-16=-1357/286, 5-16=-1357/286, 5-17=-1357/286, 1-71=-1357/286, 6-7=-039/214</li> <li>BOT CHORD 2-11=-433/2658, 10-19=-453/2658, 9-10=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 8-21=-327/1730</li> <li>WEBS 3-10=-1015/143, 4-10=-26/515, 4-8=-612/97, 5-8=-491/180, 6-8=-296/1519</li> <li>NOTES- (13)</li> <li>1) Unbalanced roof live loads have been considered for this design.</li> <li>2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23f; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15); PI=20.0 psf (Lum DOL=1.15); Is=1.0; Rough Cat B; Partially Exp; Ce=1.0; Cs=1.00; CH=1.0</li> <li>4) Unbalanced now loads have been considered for this design.</li> <li>5) This truss has been designed for a live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.</li> <li>6) Provide adequate drainage to prevent water ponding.</li> <li>7) This truss has been designed for a 10.0 psf bottom chord live load on nonconcurrent with any other live loads.</li> <li>8) This truss has been designed for a 10.0 psf obttom chord live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live load 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will be the totom chord and any other members.</li> <li>9) Refer to girder(5) for truss to truss connections.</li> <li>10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100</li></ul>	LUMBER- TOP CHORD 2x4 SF BOT CHORD 2x6 SF WEBS 2x4 SF	No.2 No.2 No.3		BRACING- TOP CHORD BOT CHORD	Structural wood she end verticals. Rigid ceiling directly MiTek recommend be installed during Installation guide.	athing directly applied applied or 10-0-0 oc is that Stabilizers and truss erection, in acc	d or 3-1-11 oc purlins, excep bracing. I required cross bracing cordance with Stabilizer
<ul> <li>FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.</li> <li>TOP CHORD 2-3=-2795/466, 3-14=-1722/369, 4-14=-1772/369, 4-15=-1357/286, 5-7=-939/214</li> <li>BOT CHORD 2-11=-453/2868, 11-19=-453/2658, 10-19=-453/2658, 9-10=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 9-21=-327/1730, 9-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 8-21=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 8-21=-327/1730, 8-21=-327/1730, 8-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 8-21=-327/1730, 20-21=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-21=-327/1730, 8-21=-327/1730, 20-20, 20-327/1730, 20-21=-327/1730, 20-21=-327/1730, 20-20, 20-327/1730, 20-21=-327/1730, 20-20, 20-327/1730, 20-20, 20-327/1730, 20-20, 20-327/1730, 20-20, 20-327/1730, 20-20, 20-327/1730, 20-20, 20-327/1730, 20-20, 20-327/1730, 20</li></ul>	REACTIONS. (Ib/size Max H Max U Max G	) 7=892/Mechanical, 2=931/0-5-4 rz 2=88(LC 52) lift7=-196(LC 8), 2=-180(LC 8) av 7=1016(LC 33), 2=1113(LC 34)	(min. 0-1-8)				
<ul> <li>NOTES- (13)</li> <li>1) Unbalanced roof live loads have been considered for this design.</li> <li>2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60</li> <li>3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10</li> <li>4) Unbalanced snow loads have been considered for this design.</li> <li>5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.</li> <li>6) Provide adequate drainage to prevent water ponding.</li> <li>7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>8) * This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.</li> <li>8) * This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the between the bottom chord and any other members.</li> <li>9) Refer to girder(s) for truss to truss connections.</li> <li>10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 7= 196, 28147</li> <li>11) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.</li> <li>12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).</li> </ul>	FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-2795/466, 3-14=-1825/363, 4-14=-1772/369, 4-15=-1357/286, 15-16=-1357/286, 5-16=-1357/286, 5-16=-1357/286, 5-17=-1357/286, 17-18=-1357/286, 6-18=-1357/286, 6-7=-939/214         BOT CHORD       2-11=-453/2658, 11-19=-453/2658, 10-19=-453/2658, 9-10=-327/1730, 9-20=-327/1730, 20-21=-327/1730, 8-21=-327/1730         WEBS       3-10=-1015/143       4-10=-26/515       4-8=-109/1180       6-8=-296/1519						
LOAD CASE(S) Standard 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	<ul> <li>NOTES- (13)</li> <li>1) Unbalanced roof lix</li> <li>2) Wind: ASCE 7-16; (envelope) gable e</li> <li>3) TCLL: ASCE 7-16; Cat B; Partially Exg</li> <li>4) Unbalanced snow I</li> <li>5) This truss has been non-concurrent witi</li> <li>6) Provide adequate of</li> <li>7) This truss has been</li> <li>8) * This truss has been</li> <li>9) Refer to girder(s) for</li> <li>10) Provide mechanic</li> <li>2=180.</li> <li>11) "NAILED" indicate</li> <li>12) In the LOAD CASE</li> <li>LOAD CASE(S) Stand</li> <li>1) Dead + Snow (bala)</li> </ul>	e loads have been considered for th /ult=120mph (3-second gust) Vasd- d zone; cantilever left and right exp Pr=20.0 psf (roof LL: Lum DOL=1.1: ; Ce=1.0; Cs=1.00; Ct=1.10 wads have been considered for this designed for greater of min roof live other live loads. rainage to prevent water ponding. designed for a 10.0 psf bottom cho n designed for a live load of 30.0ps chord and any other members. truss to truss connections. al connection (by others) of truss to s 3-10d (0.148"x3") or 3-12d (0.148" (S) section, loads applied to the fac ard need): Lumber Increase=1.15, Plate	is design. 95mph; TCDL=5.0psf; BCDL osed ; end vertical left and rig 5 Plate DOL=1.15); Pf=20.0 p design. a load of 12.0 psf or 2.00 time rd live load nonconcurrent wit f on the bottom chord in all ar bearing plate capable of withs x3.25") toe-nails per NDS gui ze of the truss are noted as from Increase=1.15	=5.0psf; h=23ft; C ht exposed; Lumb sf (Lum DOL=1.1 th any other live lo reas where a recta standing 100 lb up idlines. ont (F) or back (B)	Cat. II; Exp B; Enclosed ber DOL=1.60 plate grij 5 Plate DOL=1.15); Is= 20.0 psf on overhangs rads. Ingle 3-6-0 tall by 1-0-0 lift at joint(s) except (jt	d; MWFRS p DOL=1.60 =1.0; Rough 0 wide will fit =lb) 7=196,	SEAL 28147

Job	Truss	Truss Type	Qty	Ply	LOT 0.0009 CAMPBELL RIDGE   ANGIER, NC	
24-7828-R01	R30	Half Hip Girder	1	1	Job Reference (optional)	# 52305
		Dum Q	120 a Eab 4	0.0004 Daim	to 0,000 a Jul 40,0004 MiTal/ Industrias Inc. Tue C	- 17 10.07.00 0001 Dama

Run: .430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 Mi Tek Industries, Inc. Tue Sep 17 10:37:39 2024 Page 2 ID:PIfkM5JZRq7i2cSu5g6whSyi2C3-Na\_ICyOAPuOIhafnYQfOgcOejCkuC\_y54wf8ljycmNA

# LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-4=-60, 4-6=-60, 2-7=-20

Concentrated Loads (lb) Vert: 3=-43(F) 11=-72(F) 10=-14(F) 4=-20(F) 14=-3(F) 15=-20(F) 16=-20(F) 17=-20(F) 18=-22(F) 19=-51(F) 20=-14(F) 21=-14(F) 22=-14(F) 23=-15(F)

![](_page_57_Picture_6.jpeg)

![](_page_58_Figure_0.jpeg)

![](_page_59_Figure_0.jpeg)

![](_page_60_Figure_0.jpeg)

![](_page_61_Figure_0.jpeg)

![](_page_62_Figure_0.jpeg)

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

![](_page_63_Figure_0.jpeg)

Max Uplift All uplift 100 lb or less at joint(s) 4, 6 except 7=-131(LC 18), 5=-119(LC 13) Max Grav All reactions 250 lb or less at joint(s) 7, 4 except 6=394(LC 20), 5=426(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 3-5=-297/205

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 0-9-10, Exterior(2R) 0-9-10 to 4-1-8, Exterior(2E) 4-1-8 to 8-11-1 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 6 except (jt=lb) 7=131, 5=119.

## LOAD CASE(S) Standard

![](_page_63_Picture_12.jpeg)

![](_page_64_Figure_0.jpeg)

Max Hol2 / - 122(LC 6)
 Max Uplift All uplift 100 lb or less at joint(s) 7, 4, 6 except 5=-107(LC 13)
 Max Grav All reactions 250 lb or less at joint(s) 7, 4, 6 except 5=333(LC 24)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 3-5=-260/208

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 0-9-10, Exterior(2R) 0-9-10 to 2-4-8, Exterior(2E) 2-4-8 to 7-2-1 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 4, 6 except (jt=lb) 5=107.

# LOAD CASE(S) Standard

![](_page_64_Picture_12.jpeg)

![](_page_65_Figure_0.jpeg)

Max Uplift5=-123(LC 5), 3=-16(LC 13) Max Grav 3=175(LC 1), 4=321(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3 except (jt=lb) 5=123.

LOAD CASE(S) Standard

![](_page_65_Picture_12.jpeg)

![](_page_66_Figure_0.jpeg)

REACTIONS. (lb/size) 5=0/4-1-14 (min. 0-1-8), 3=109/4-1-14 (min. 0-1-8), 4=173/4-1-14 (min. 0-1-8) Max Horz 5=-53(LC 8) Max Uplift5=-32(LC 5), 3=-11(LC 13)

Max Grav 5=9(LC 19), 3=109(LC 1), 4=178(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3.

LOAD CASE(S) Standard

![](_page_66_Picture_14.jpeg)

![](_page_67_Figure_0.jpeg)

BRACING-

TOP CHORD

BOT CHORD

Installation guide

REACTIONS.	(lb/size)	1=69/2-4-14	(min. 0-1-8), 3=69/2-4-14	(min. 0-1-8)
			· · · · · · · · · · · · · · · · · · ·	· /

BCLL

BCDL

LUMBER-

0.0

10.0

TOP CHORD 2x4 SP No.2

BOT CHORD 2x6 SP No.2

Max Horz 1=-13(LC 10) Max Uplift1=-4(LC 12), 3=-5(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

Code IRC2021/TPI2014

#### NOTES- (8)

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Matrix-P

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

![](_page_67_Picture_13.jpeg)

Weight: 9 lb

Structural wood sheathing directly applied or 2-4-14 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer

FT = 20%

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.