

NC.

David D Grorud (919) 601-1406

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Contractor shall verify all conditions and dimensions prior to construction. Any error or missions shall be reported to Dynamic Design Group for corrections or justification Once construction has commenced, the contractor shall assume all responsibility.

Dimensions govern over scale, code governs over dimensions.

PROJECT FOR: ICG

LOCATION: FUQUAY-VARINA _{LOT:} 317

SUBDIVISION: SERENITY

COUNTY: WAKE

S \mathcal{C} 0

NAME

PLAN 3560

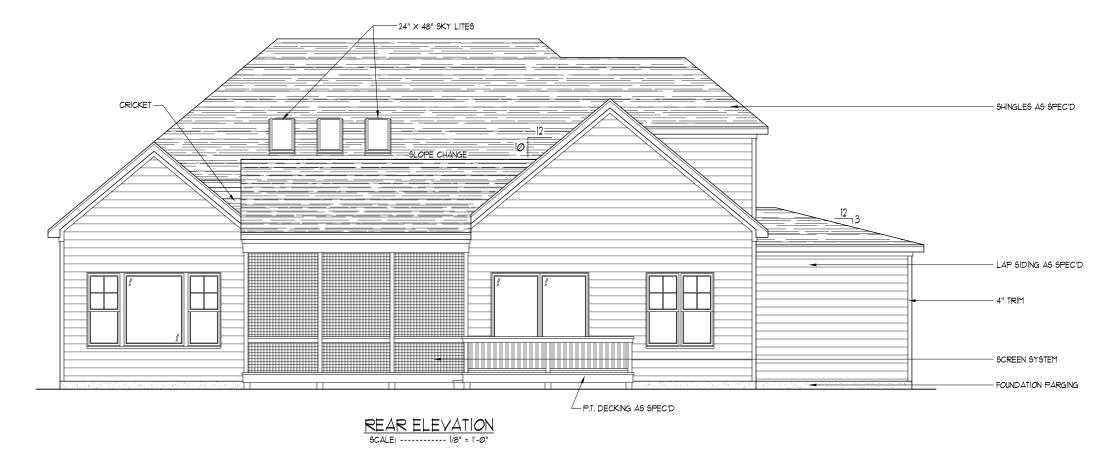
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SHEET

RIGHT SIDE ELEVATION SCALE: ----- 1/8" = 1'-0"







DYNAMIC DESIGN GROUP, INC.

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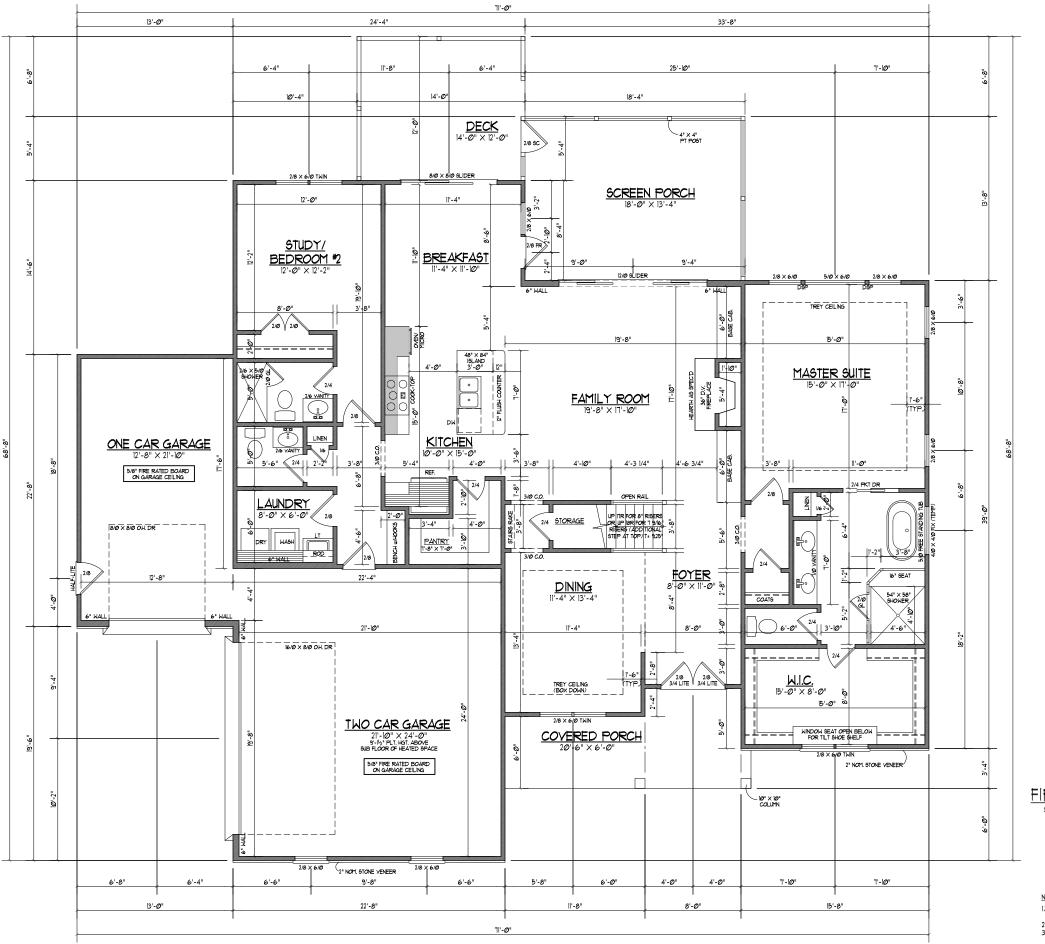
10.31.24

PLAN NAME 1-3560-71GL

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A-1B
SHEET





NC (1)

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LOCATION: FUQUAY-VARINA

{LOT:}317

SUBDIVISION: SERENITY

COUNTY: WAKE

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NAME PLAN

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SHEET

FIRST FLOOR PLAN SCALE: ----- 1/8" = 1'-0"

NOTES:

2058 HEATED SQ. FT. 842 SQ. FT. GARAGE 139 SQ. FT. COVERED PORCH 252 SQ. FT. SCREEN PORCH 166 SQ. FT. DECK

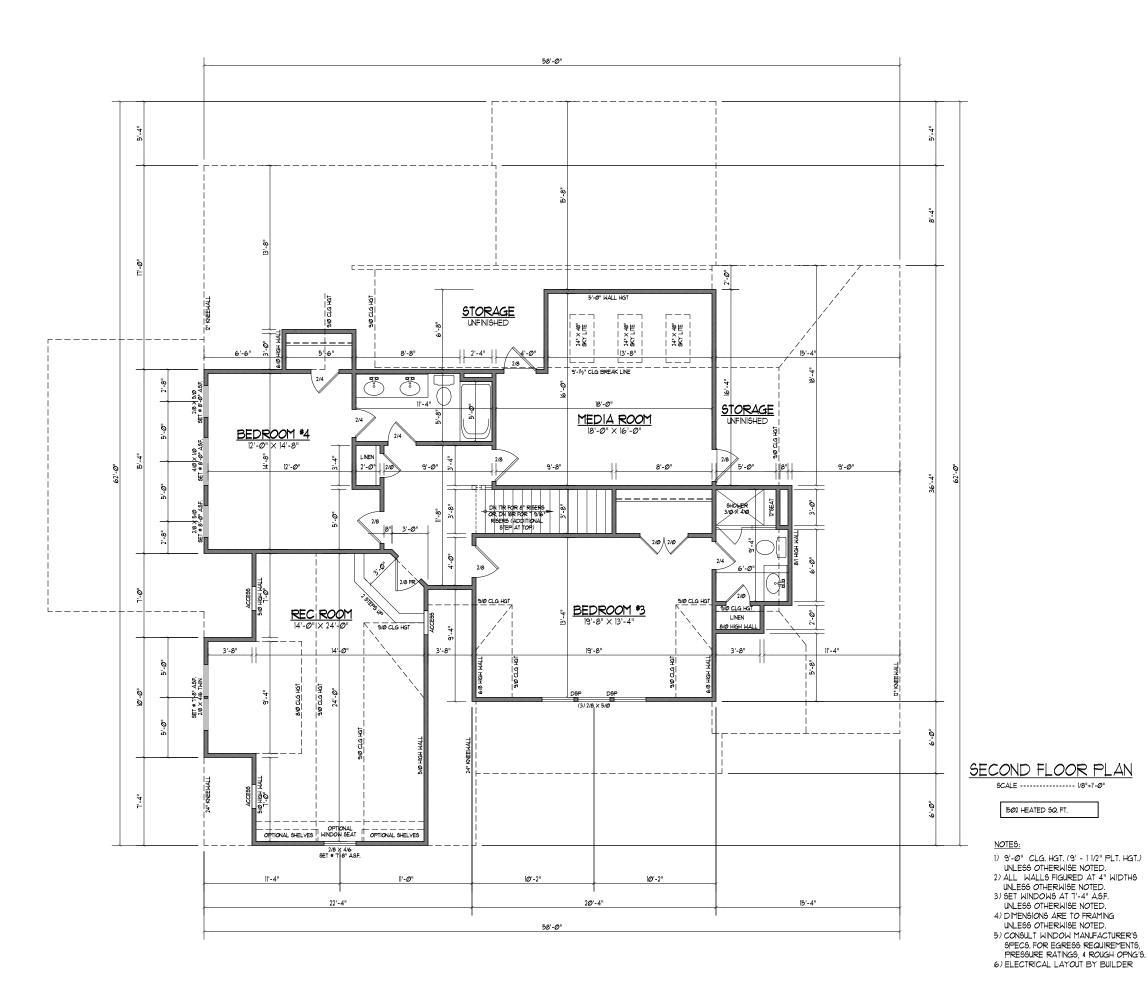
- NOTES:

 1) IØ'-Ø" CLG. HGT. (IØ' 1 I/2" PLT. HGT.)
 UNLESS OTHERWISE NOTED.

 2) ALL WALLS FIGURED AT 4" MIDTHS
 3) SET IMIDOMS AT 3-Ø" ASF.
 UNLESS OTHERWISE NOTED.

 4) DIFENSIONS ARE TO FRAMING
 UNLESS OTHERWISE NOTED.

 5) CONSULT INIDOM MANUFACTURERS
 SEEDE SEESES BED INDEMENTS
- 9PECS FOR EGRESS REQUIREMENTS, PRESSURE RATINGS, & ROUGH OPNG'S. 6) ELECTRICAL LAYOUT BY BUILDER



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SUBDIVISION: SERENITY

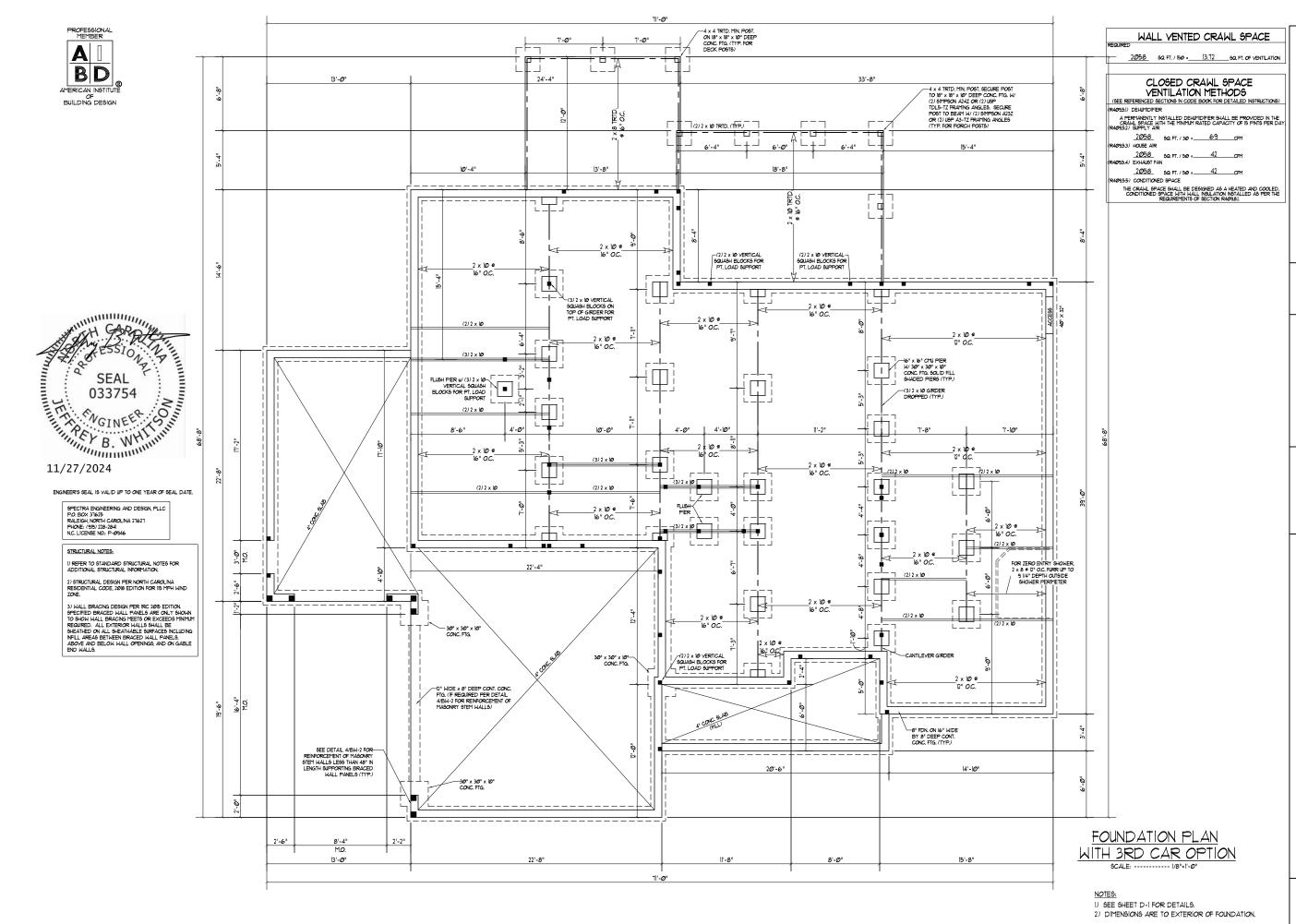
COUNTY: WAKE

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PLAN NAME

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NC. UP SIG (J

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_{LOT:} 317

SUBDIVISION: SERENITY

COUNTY: WAKE

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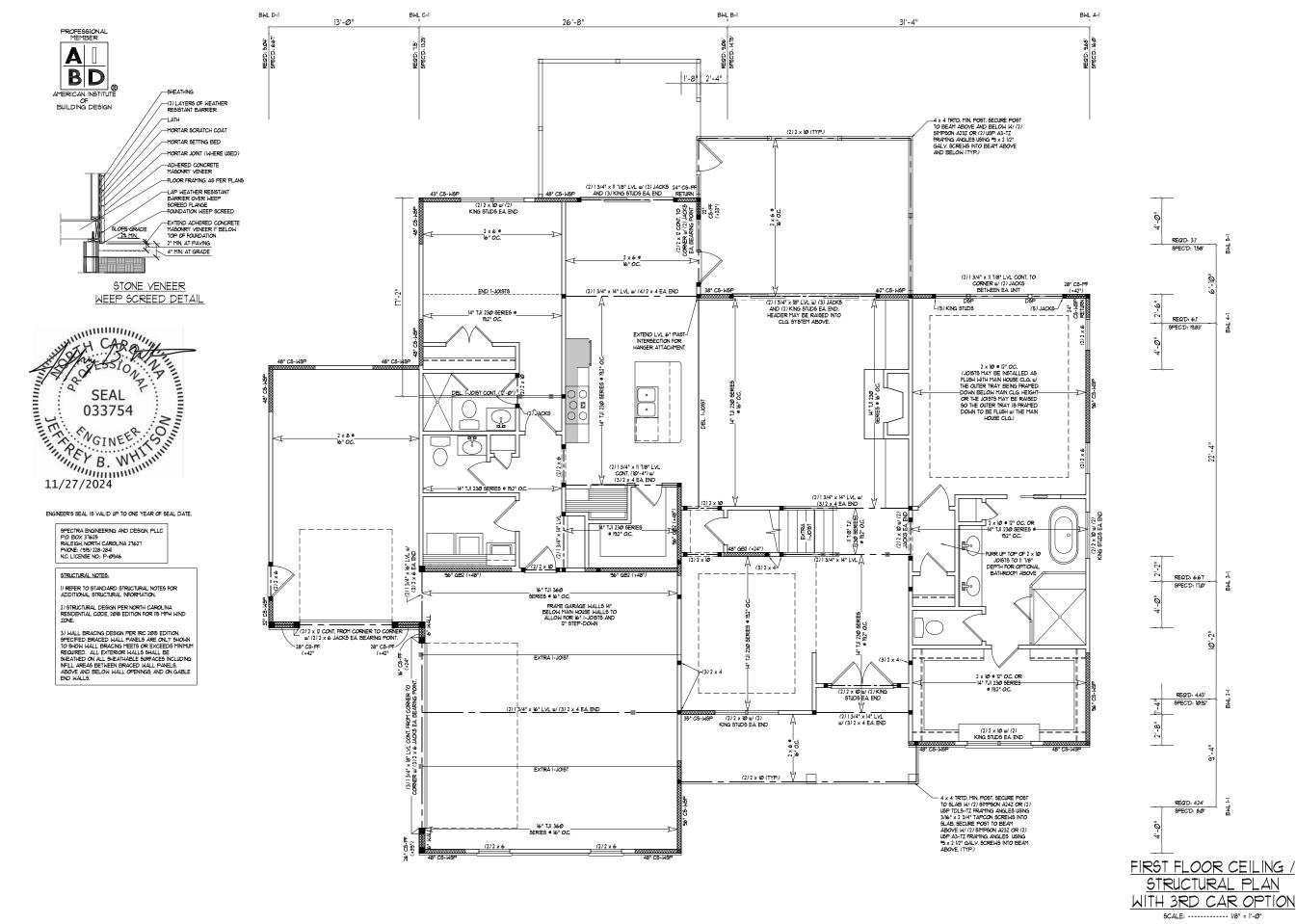
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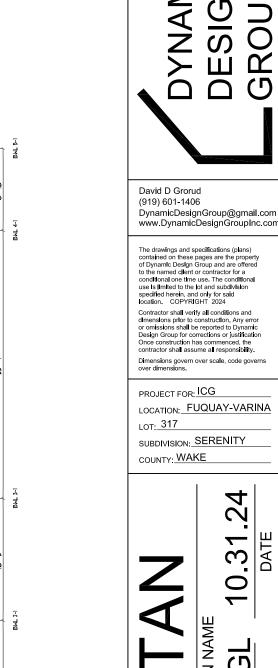
NAME PLAN

560 3

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REQ'D: 3.1'

SPEC'D: 19.83

REQ'D: 6.61' SPEC'D: ITØ'

REQ'D: 4.43' SPEC'D: 10.92'

NOTES:

1) SHADED WALLS DENOTE LOAD BEARING WALLS.

2) DENOTES SOLID STUDS.

NAME PLAN

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ENGINEER'S SEAL IS VALID UP TO ONE YEAR OF SEAL DATE.

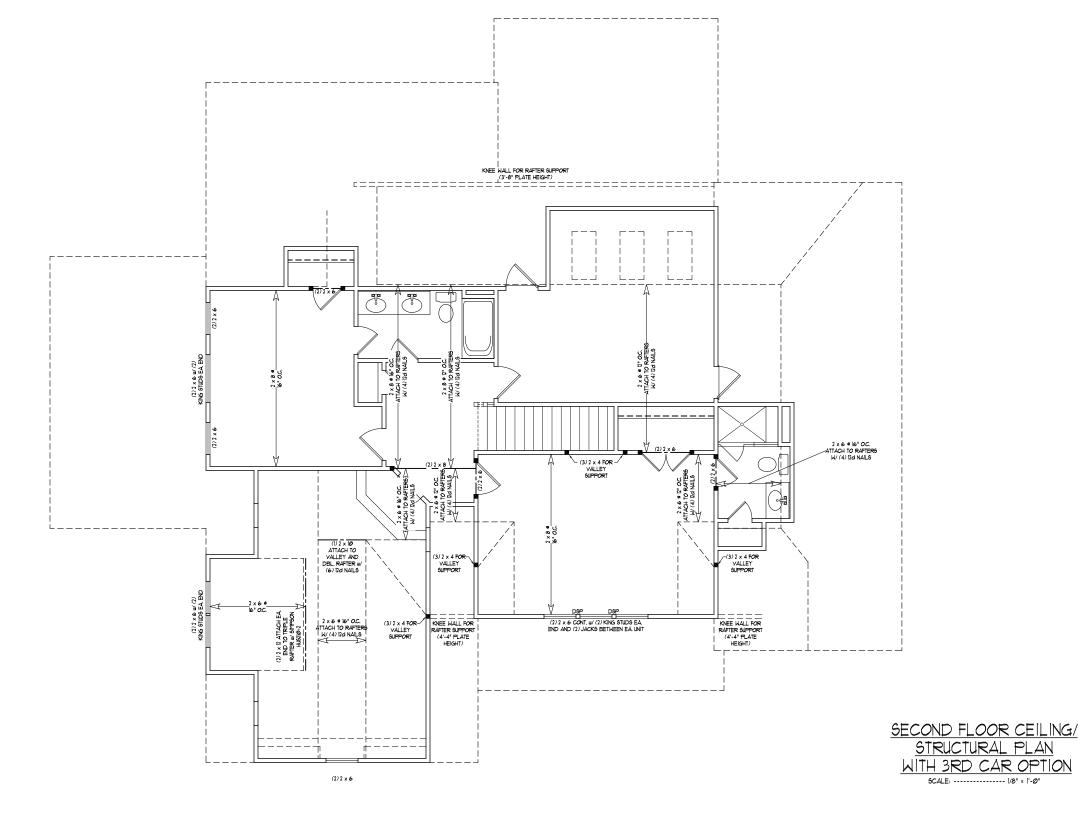
SPECTRA ENGINEERING AND DESIGN, PLLC P.O. BOX 31625 RALEIGH, NORTH CAROLINA 21621 PHONE: 1939 278-264 N.C. LICENSE NO.: P-0946

STRUCTURAL NOTES:

1) REFER TO STANDARD STRUCTURAL NOTES FOR ADDITIONAL STRUCTURAL INFORMATION.

2) STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION FOR 115 MPH WIND ZONE.

3) HALL BRACING DESIGN PER IRC 20th EDITION.
SPECIFIED BRACED HALL PAIRLS ARE ONLY SHOWN.
TO SHOW HALL BRACING PEETS OR EXCEEDS HINDIM
REGUIRED. ALL EXTERIOR WALLS SHALL BE
SHEATHED ON ALL SHEATHED SHEACED HALL PAIRLS,
NIFLL AREAS BETHEEN BRACED HALL PAIRLS,
ABOVE AND BELOW HALL OPENINGS, AND ON GABLE
END HALLS.



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S

03

SUBDIVISION: SERENITY

specified herein, and only for said location. COPYRIGHT 2024

PROJECT FOR: ICG

COUNTY: WAKE

_{LOT:} 317

PLAN NAME 3560 $\overline{}$ \Box

SHEET

1) SHADED WALLS DENOTE LOAD BEARING WALLS. 2) DENOTES SOLID STUDS.





ENGINEER'S SEAL IS VALID UP TO ONE YEAR OF SEAL DATE.

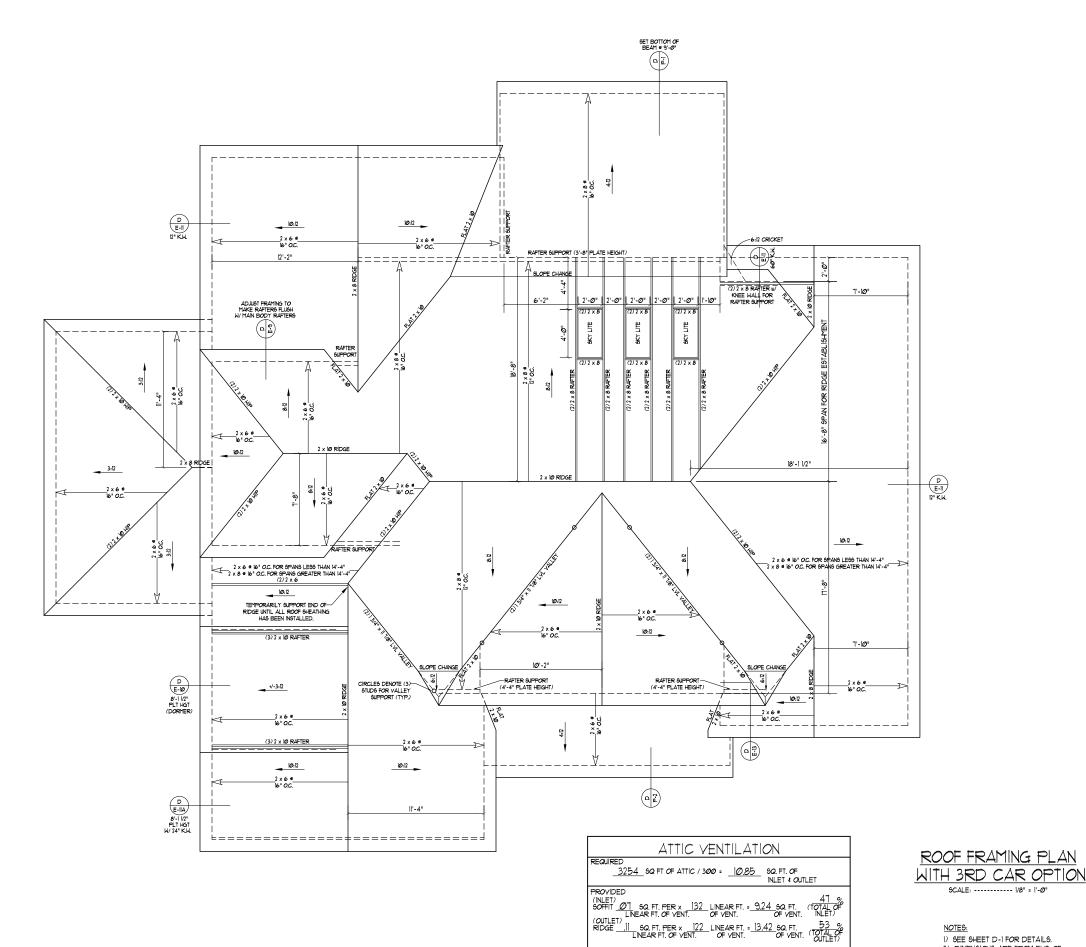
SPECTRA ENGINEERING AND DESIGN, PLLC P.O. BOX 31625 RALEIGH, NORTH CAROLINA 21621 PHONE: (919) 228-2841 N.C. LICENSE NO.: P-0946

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2) STRUCTURAL DESIGN PER NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION FOR 115 MPH WIND ZONE.

3) MALL BRACING DESKN PER IRC 2015 EDITION.
SPECIFED BRACED MALL PANELS ARE ONLY SHOWN
TO SHOM HALL BRACING IFFETS OR EXCEEDS HINMUM
REGUIRED. ALL EXTERIOR MALLS SHALL BE
SHEATHED ON ALL SHEATHED SWEACES INCLUDING
INFILL AREAS BETWEEN BRACED MALL PANELS,
ABOVE AND BELOW HALL OPENINGS, AND ON GABLE
END WALLS.



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_{LOT:_317}

SUBDIVISION: SERENITY

COUNTY: WAKE

4 S

03

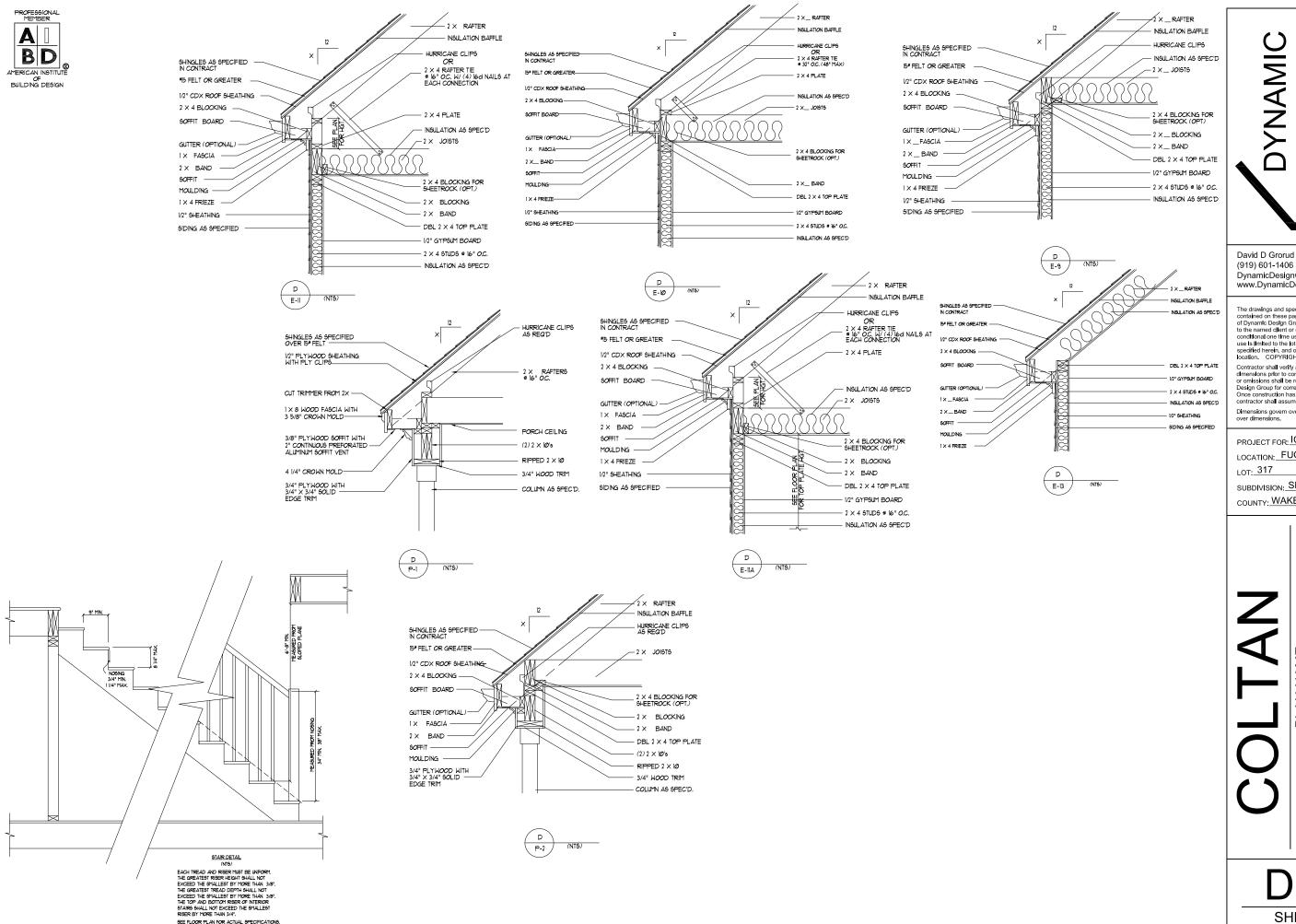
NAME PLAN

3560 $\overline{}$ \Box

SHEET

2) DIMENSIONS ARE FROM EXT. OF FRAMING TO CENTER LINE OF RIDGE.

22.66 TOTAL SQ. FT. OF VENT.



NC. UP SIGI (1)

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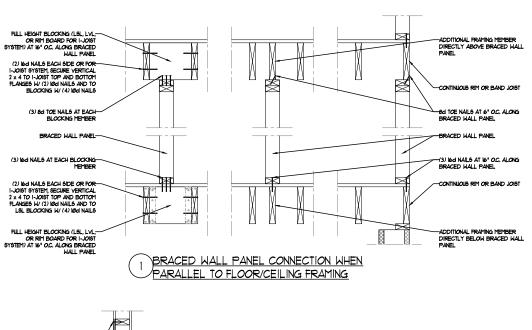
COUNTY: WAKE

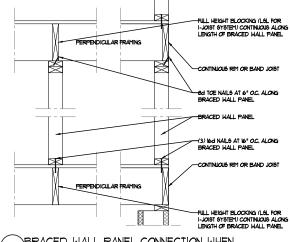
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NAME PLAN

560 3 $\overline{}$ $\mathbf{\omega}$





BRACED WALL PANEL CONNECTION WHEN PERPENDICULAR TO FLOOR/CEILING FRAMING

	BR4		WEL SCHEDULE
ABBREVIATIONS	PANEL TYPE	MATERIAL	FASTENERS .
WSP	INTERMITTENT WOOD STRUCTURAL PANEL	1/16" 08B/ PLYW00D (UNO)	6d or 8d cotton NAILS AT 6" O.C. AT PANEL Edges and 2" O.C. AT Intervediate Supports or 16 ga x 13 4" staples at 3" O.C. AT Panel Edges and 6" O.C. AT Intervediate Supports
GB (1)	INTERMITTENT GYP9UM BOARD (9HEATHING ON ONE FACE OF WALL)	1/2" GYPSUM	I 1/2" GALV, ROOFING NAILS, 6d COMMON NAILS, OR I 1/4" TYPE W DRY WALL SCREWS AT T" O.C. AT PANEL EDGES INCLIDING TOP AND BOTTOM PLATES AND INTERMEDIATE SUPPORTS
GB (2)	INTERMITTENT GYP9UM BOARD (SHEATHING ON BOTH FACES OF WALL)	1/2" GYPSUM	I 1/2" GALV, ROOFING NAILS, 6d COMMON NAILS, OR I 1/4" TYPE WI DRYWALL SCREWS AT T" O.C. AT PANEL EDGES INCLIDING TOP AND BOTTOM PLATES AND INTERMEDIATE SUPPORTS
GB (3)	INTERMITTENT GYP9UM BOARD (SHEATHING ON BOTH FACES OF WALL)	1/2" GYPSUM	I 1/2" GALV, ROOFING NAILS, 6d COMMON NAILS, OR 11/4" TYPE IN DRYWALL SCREWS AT 4" OC. AT PANEL EDGES INCLIDING TOP AND BOTTOM PLATES AND INTERMEDIATE SUPPORTS
LIB	LET-IN-BRACING	1 x 4 WOOD OR SIMPSON CSI6 STRAP	MOOD: (2) 8d NAILS PER STUD INCLUDING TOP AND BOTTOM PLATE. METAL: (1) STRAP EACH DIRECTION, (2) 16d NAILS PER STUD INCLUDING TOP AND BOTTOM PLATE, (20) 16d NAILS MIN. PER STRAP
CS-WSP	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	1/16" 05B/ PLYW00D (UNO)	6d OR 8d COMMON NAILS AT 6" O.C. AT PANEL EDGES AND 1" O.C. AT INTERPEDIATE SUPPORTS OR 16 GA.X 134" STAPLES AT 3" O.C. AT PANEL EDGES AND 6" O.C. AT INTERPEDIATE SUPPORTS
C5-G	CONT. SHEATHED WOOD STRUCTURAL PANEL ADJACENT TO GARAGE	1/16" 0%B/ PLYW00D (UNO)	6d OR 8d COMMON NAILS AT 6" O.C. AT PANEL EDGES AND 1" O.C. AT INTERPEDIATE SUPPORTS OR 16 GA x 1 34" STAPLES AT 3" O.C. AT PANEL EDGES AND 6" O.C. AT INTERPEDIATE SUPPORTS
C5-FF	CONTINUOUS PORTAL FRAME	1/16" 09B/ PLYW00D (UNO)	SEE METHOD CS-PF ON PAGE BW-3
C9-EBWI	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	1/16" 09B/ PLYW00D (UNO)	6d OR 8d CONTON NAILS AT 4" O.C. AT PANEL EDGES AND 8" O.C. AT INTERMEDIATE SUPPORTS
CS-EBW2	CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL	1/16" OSB/ PLYWOOD (UNO)	6d OR 8d COMMON NAILS AT 3" O.C. AT PANEL EDGES AND 6" O.C. AT INTERMEDIATE SUPPORTS

NOTES:	
1) ALL	BR

- NOTES:

 1) ALL BRACED HALL PANELS SHALL HAVE 2X ELOCKING BETIMEDN HALL STUDS AT ALL HORIZONTAL SHEET EDGES.

 2) PROVIDE NAILING/BLOCKING ABOVE AND BELOW ALL BRACED WALL PANELS FER DETAIL I/BIM-1 AND 2/BIM-1.

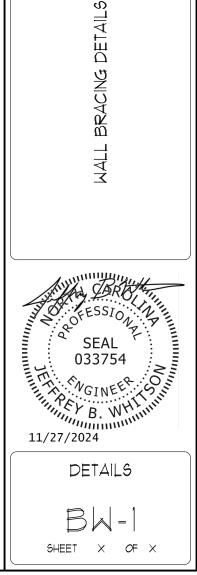
 3) ALL EXTERIOR WALLS OF THE HOUSE AND TO BE SHEATHED IN 1167 OSD OR 5/32" PLTWOOD GECURED FER NOTICE TABLE RESPONZIVE HALL COORNES HEATHEN IS TO BE SECURED AS FER DETAIL 3/BIM-1.

 4) GB (1) AND GB (2) WALL PANELS SHALL BE SECURED AS FER DETAIL 4/BIM-1.

 5) BRACED MALL PANELS ARE PROVIDED AS FER THE NITEMATIONAL RESIDENTIAL CODE, 2/6/8 EDITION, SECTION RESPONZIVE PANEL LENGTHS SHOWN ON PLANS ARE THE MIN. LENGTH RECUIRED.

 6) ALL METHODS SHALL HAVE A GYPSUM BOARD FINISH (OR EQUIVALENT) APPLIED TO THE INSIDE FACE OF THE BRACED WALL PANEL.

PECURE SHEATHING TO MALL FRAMING MV BONALLS AT 6' OC. AT PANEL EDGES AND 2' OC. AT NITER*EDIATE SUPPORTS. OPT. NON-STRUCTURAL FILLER PANEL	GYY AN NC ed ORI CO PAI	MIN MOOD STRUCTURAL PANEL PSMT MALLBOARD AS REQUIRED D INSTALLED IN ACCORDANCE MY ECCHAPTER TO COMPON NAILS AT 2° O.C. BENTATION OF STID MAY YARY NITHAUSH MOOD STRUCTUAL REL BRACED MALL INE ENCRC TABLE R60(3.4(1) FOR TENING			
GYPSUM HALLBOARD AS REQUIRED. AND INSTALLED IN ACCORDANCE MY NORC CHAPTER T	FAS	E NORC TABLE R6623(1) FOR TIPING ITHINGS MODE STRUCTUAL. WELL BRACED WALL LINE IENTATION OF STUD MAY VARY NALLS AT IL' OC. WHE SHEATHING TO MALL. LIVE SHEATHING TO MALL. LIVE SHEATHING TO MALL. WELL BOSES AND "AT INTERIOR SUPPORTS.			—SECURE 1/2" MIN GYPSUM MALLBOARD TO ALL FRAMING PEPTIERES (STUDE, PLATES, AND BLOCKING) IV 1 1/4" TYPE IV SCREING OR BO COCLER NAILS AT "1" O.C. (TYP.) —BRACED WALL
GYPSUM MALLBOARD AS REQUIRED AND INSTALLED IN ACCORDANCE MY NORG CHAPTER T OPT. BLOCKING FOR GYPSUM MALLBOARD FASTENERS ON BOTH STUDS AT EACH PANEL EDGE	FAB 24*	ntinuous wood structual Wel Braced Wall Line	T-PLATE WALL INTERSECTION	3-STUD WALL INTERSECTION	
	AL EXTERIOR CORNER FRA ONTINUOUS SHEATHING	<u>AMING</u>		O GB (1) AND GB (ECTION DETAILS	(2)



spectra spectra

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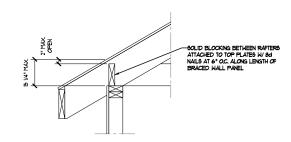
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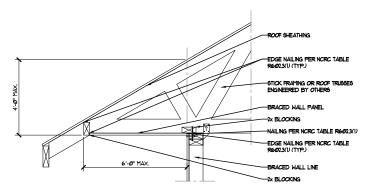
DRAWN BY: 15Z ENGINEERED BY: REVIEWED BY: T

SPECTRA ENGINEERING AND DESIGN, PLLC
P.O. BOX 316.25
RALEIGH, NORTH CAROLINA 216.21
TEL.: (319) 228-2841
LICENSE NO. NC. P-0946 VA: 000462

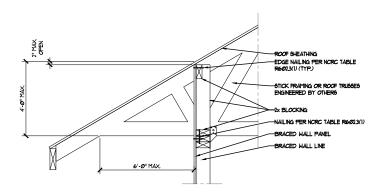
AND



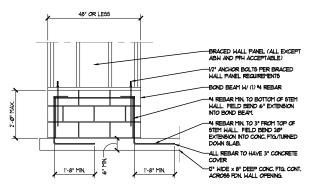
BRACED WALL PANEL CONNECTION TO PERPENDICULAR RAFTERS



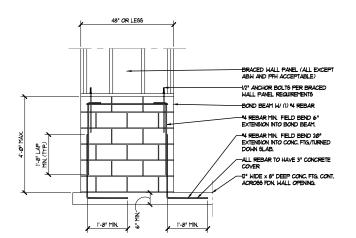
2 BRACED WALL PANEL CONNECTION OPTION TO PERPENDICULAR RAFTERS OR ROOF TRUSSES



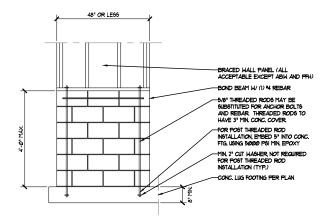
3 BRACED WALL PANEL CONNECTION OPTION TO PERPENDICULAR RAFTERS OR ROOF TRUSSES
PROVIDE VENTING FER NOTE SECTION ROOF (NOT SHOW)



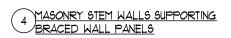
SHORT STEM WALL REINFORCEMENT



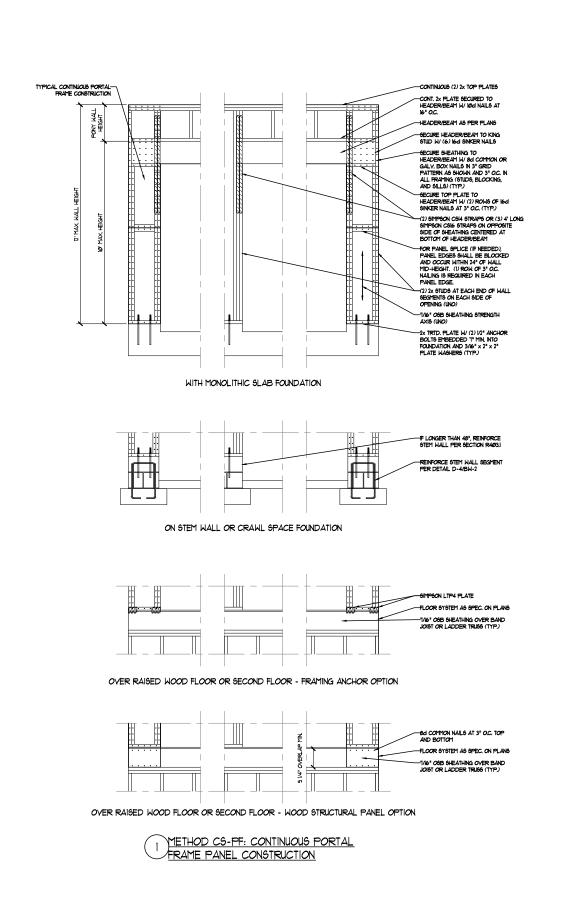
TALL STEM WALL REINFORCEMENT

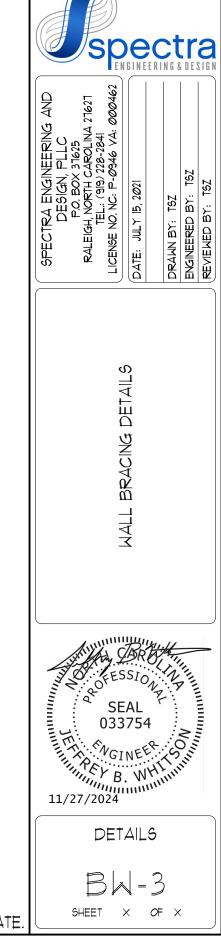


OPT. STEM WALL REINFORCEMENT CONFIGURATION









DISCLAINER - ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF THE NORTH CARCUNA RESIDENTIAL CODE (NORC), 2018 EDITION, PLUS ALL LOCAL CODES AND REGULATIONS. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR, ADD MILL NOT HAVE CONTROL OF, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR SAFETY PRECAUTIONS AND PROCRAMS IN CONNECTION WHITH THE CONSTRUCTION WORK, NOR WILL THE ENGINEER OF RESPONSIBLE FOR THE CONTRACTOR'S FAILURE TO CARRY OUT THE CONSTRUCTION MORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. ENGINEER'S SEAL APPLIES ONLY TO STRUCTURE COMPONIENTS INCLUDING ROSPE BASTERS HIPE AND ITEST, BIDGRES HE ORDER MILL TO STRUCTURAL COMPONENTS INCLIDING ROOF RAFTERS, HIPS, VALLEYS, RIDGES, FLOORS, WALLS, BEAYS, HEADERS, COLUMNS, CANTILEVERS, OFFSET LOAD BEARING WALLS, PIERS, GIRDER SYSTEM AND FOOTING. ENGINEERS SEAL DOES NOT CERTIFY DIMENSIONAL ACCURACY OR ARCHITECTURAL LAYOUT INCLIDING ROOF. ENGINEER'S SEAL DOES NOT APPLY TO I-JOIST OR FLOOR/ROOF TRUSS LAYOUT DESIGN AND ACCURACY.

STRUCTURAL DESIGN - STRUCTURAL DESIGN AS PER NCRC, INCLUDING CHAPTER 45 FOR CONSTRUCTION IN 180, 140, AND 150 MPH WIND ZONES. DESIGN LOADS ARE AS FOLLOWS:

	LIVE LOAD	DEFLECTION	
	(PSF)	(LL)	
ATTIC WITH LIMITED STORAGE	2Ø	L/24Ø	
ATTIC WITHOUT STORAGE	10	L/36Ø	
DECK5	40	L/36Ø	
EXTERIOR BALCONIES	60	L/36Ø	
FIRE ESCAPES	40	L/36Ø	
GUARDRAILS AND HANDRAILS	200	L/36Ø	
PASSENGER VEHICLE GARAGES	50	L/36Ø	
ROOMS OTHER THAN SLEEPING ROOMS	40	L/36Ø	
SLEEPING ROOMS	3Ø	L/36Ø	
STAIRS	40	L/36Ø	
SNOW	2Ø	L/36Ø	
LIND LOAD (BASED ON ILLALI	AND BOOK OL	ADDING DESIGN LOADS!	

WIND LOAD (BASED ON "WALL AND ROOF CLADDING DESIGN LOADS' TABLE, WIND ZONE, MEAN ROOF HEIGHT AND EXPOSURE)

- STICK FRAMED SYSTEMS ARE DESIGNED WITH 10 PSF DEAD LOAD.
- I-JOIST SYSTEMS ARE DESIGNED WITH 12 PSF DEAD LOAD. - FLOOR TRUSS SYSTEMS ARE DESIGNED WITH IS PSF DEAD LOAD.

HIGH WIND ZONES - CONSTRUCTION IN 13:0, 14:0, AND 15:0 MPH WIND ZONES SHALL BE IN ACCORDANCE WITH CHAPTER 45 OF THE NORC. CONSTRUCTION IN THE COASTAL AND FLOOD PLAINS SHALL BE IN ACCORDANCE WITH CHAPTER 46 OF THE NCRC.

CONCRETE FOOTING AND SLAB PREPARATION - FOR ALL CONCRETE SLABS AND FOOTINGS, THE AREA WITHIN THE PERIMETER OF THE BUILDING ENVELOPE SHALL HAVE ALL VEGETATION, TOP SOIL AND FOREIGN MATERIAL, REMOVED. FILL MATERIAL, SHALL BE FREE OF VEGETATION AND FOREIGN MATERIAL. THE FILL SHALL BE COMPACTED TO ASSURE UNIFORM SUPPORT OF THE SLAB, AND EXCEPT WHERE APPROVED, THE FILL DEPTHS SHALL NOT EXCEED 24" FOR CLEAN SAND OR GRAVEL AND 8" FOR EARTH, A 4" THICK BASE COURSE CONSISTING OF CLEAN GRADED SAND, GRAVEL, OR CRUSHED BLAST -RIPNACE SLAG PASONIG A 2" SIEVE SHALL BE PLACED ON THE PREPARED SUBGRADE WHEN THE SLAB IS BELOW GRADE. A BASE COURSE IS NOT REQUIRED WHEN A CONCRETE SLAB IS NSTALLED ON WELL-DRAINED OR SAND-GRAVEL MIXTURE SOILS CLASSIFIED AS GROUP I ACCORDING TO THE UNITED BOIL CLASSIFICATION SYSTEM IN ACCORDANCE WITH TABLE
R4051 OF THE NCRC. PROPERLY DEWATER EXCAVATION PRIOR TO POURING CONCRETE WHEN
BOTTOM OF CONCRETE SLAB IS AT OR BELOW WATER TABLE.

<u>SOIL BEARING CAPACITY</u> - THE ALLOWABLE MINIMUM BEARING CAPACITY FOR SOIL IS ASSUMED TO BE 2000 PSF. CONTACT GEOTECHNICAL ENGINEER IF BEARING CAPACITY IS NOT ACHIEVED.

CONCRETE - CONCRETE SHALL CONFORM TO SECTION R4022 OF THE NCRC. CONCRETE REINFORCING CONCRETE COVER AROUND REINFORCING STEEL OF 3" IN FOOTINGS AND 1 1/2" IN SLABS, FOR POURED

CONCRETE WALLS, CONCRETE COVER FOR REINFORCING STEEL MEASURED FROM THE INSIDE FACE OF

SECURE ALL BEAMS SUPPORTING ROOF TRUSSES OR RAFTERS TO THEIR RESPECTIVE BEARING

CONCRETE WALLS, CONCRETE COVER FOR REINFORCING STEEL MEASURED FROM THE INSIDE FACE OF

SUPPORT MEMBERS WITH (1) SIMPSON CSIG STRAP PER CONNECTION LAPPING 14" MIN. ONTO EACH THE WALL SHALL NOT BE LESS THAN 3/4". CONCRETE COVER FOR REINFORCING STELL MEASURED FROM THE OUTSIDE FACE OF THE WALL SHALL NOT BE LESS THAN 1 1/2" FOR "5 BARS OR SMALLER, AND NOT LESS THAN 2" FOR "6 BARS OR LARGER.

CONCRETE CONTROL JOINTS - IF APPLICABLE, CONTROL JOINTS ARE TO BE SAMED TO A DEPTH OF 25% OF SLAB THICKNESS MITHIN 4 TO 12 HOURS OF CONCRETE FINISHING. CONTROL JOINTS SHOULD BE SPACED NO MORE THAN 12'-0" APART AND SECTIONS SHOULD BE RECTANGULAR WITH SIDE RATIOS

MASONRY - MASONRY UNITS TO CONFORM TO ACE 530/ASCE 5/TMS 402, MORTAR SHALL CONFORM TO ASTM C210, REINFORCING STEEL TO BE ASTM A615 GRADE 60.

REBAR LAP SPLICES - REINFORCEMENT SHALL BE THE LONGEST LENGTHS PRACTICAL OR BE LAP SPLICED 30° MINIMUM FOR "4 REBAR 38" MINIMUM FOR "5 REBAR 45" MINIMUM FOR "6 REBAR OR THE MINIMUM REQUIRED LAP SPLICE LENGTH OF THE SMALLER BAR AS PER FIGURE R6005.4(1) OF THE

CONCRETE AND MASONRY FOUNDATION WALLS - ALL CONCRETE AND MASONRY FOUNDATION WALLS ARE TO BE CONSTRUCTED IN ACCORDANCE WITH THE PROVISIONS OF SECTION RADA OF THE NCRC. OR IN ACCORDANCE WITH ACI 318, ACI 323, NCMA TREAS-A OR ACE 580/ACSE 59176 402, MASONRY FOUNDATION WALLS ARE TO BE REINFORCED FER TABLE RADALICID THROUGH RADALICA) OF THE NCRC. CONCRETE FOUNDATION WALLS ARE TO BE REINFORCED FER TABLE RADALICA(1) THROUGH RADALICA) OF THE NCRC. PRECAST CONCRETE FOUNDATION WALLS ARE TO BE REINFORCED FOR TABLE RADALICA(1) THROUGH RADALICS OF THE NCRC. PRECAST CONCRETE FOUNDATION WALLS ARE TO CONFORM TO SECTION RADAS OF THE NCRC. STEP CONCRETE FOUNDATION WALLS TO 2 x 6 FRAMED WALLS AT 16" O.C. WHERE GRADE

PIERS - THE UNSUPPORTED HEIGHT OF MASONRY PIERS SHALL NOT EXCEED 1/0 TIMES THEIR LEAST DIMENSION. WHEN STRUCTURAL CLAY TILE HOLLOW CONCRETE MASONRY UNITS ARE USED FOR ISOLATED PIERS TO SUPPORT BEAMS AND GIRDERS, THE CELLULAR SPACES SHALL BE FILLED SOLIDLY WITH CONCRETE OR TYPE M OR 9 MORTAR, EXCEPT UNFILLED HOLLOW PIERS MAY BE USED HIP 9PLICES ARE TO BE 9PACED A MINIMUM OF 8"-0". FASTEN MEMBERS WITH THREE IF THEIR UNSUPPORTED HEIGHT IS NOT MORE THAN FOUR TIMES THEIR LEAST DIMENSION. HOLLOW ROWS OF 12d NAILS AT 16" OC. SOLID MINISUPPORTED HEIGHT IS NOT MORE THAN FOUR TIMES THEIR LEAST DIMENSION. HOLLOW PIERS SHALL BE CAPPED WITH 4" OF SOLID MASONRY OR CONCRETE FOR DIME STORY AND 8" OF SOLID MASONRY OR CONCRETE FOR DIME STORY AND 8" OF SOLID MASONRY OR CONCRETE FOR TWO STORY AND TWO AND ONE-HALF STORY OR SHALL HAVE CAVITIES OF THE TOP COURSE FILLED WITH CONCRETE OR GROUT OR OTHER APPROVED METHODS. SHADED OR NOTED PIERS ARE TO BE FILLED SOLID WITH CONCRETE OR GROUT OR OTHER

<u>PIERGIRDER LOCATION</u> - THE CENTER OF EACH PIER SHALL BEAR IN THE MIDDLE THIRD OF ITS RESPECTIVE FOOTING. EACH GIRDER SHALL BEAR IN THE MIDDLE THIRD OF EACH PIER.

FOUNDATION ANCHORAGE - FOR 115, 120, AND 130 MPH WIND ZONES, THE WOOD SOLE PLATE AT EXTERIOR WALLS ON MONOLITHIC SLABS, NOOD SOLE PLATES OF BRACED WALL PANELS AT BUILDING INTERIORS ON MONOLITHIC SLAB, AND ALL WOOD SILL PLATES SHALL BE ANCHORED TO THE FOUNDATION WITH ANCHOR BOLITS SPACED A MAXIMUM OF 6 '0" OC. (4"-0" OC. FOR 130 MPH MIND ZONE) AND NOT MORE THAN 12" FROM THE CORNER THERE SHALL BE A MINIMUM OF THO BOLTS FER PLATE SECTION. BOLTS SHALL BE AT LEAST 1/2" IN DIAMFETER AND SHALL EXTEND A MINIMUM OF "I" INTO MASONRY OR CONCRETE (B" INTO MASONRY FOR 130 MPH WIND ZONE). BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. INTERIOR BEARING WALL SOLE PLATES ON MONOLITHIC SLAB FOUNDATIONS NOT PART OF A BRACED WALL PANEL SHALL BE POSITIVELY ANCHORED WITH APPROVED FASTENERS, FOR 140 MPH AND 150 MPH WIND ZONES, FOUNDATION ANCHORAGE IS TO COMPLY WITH SECTION 4504 OF THE NORC.

FRAMING LUMBER - ALL FRAMING LUMBER SHALL BE 9 SYP MINIMUM (Fb = 150 PS), Fv = 115 PS), E = 1400000 PS)) UNLESS NOTED OTHERWISE (UNO). ALL TREATED LUMBER SHALL BE 9 SYP MINIMUM (Fb = 150 PS), Fv = 115 PS), E = 1400000 PS)) UNLESS NOTED OTHERWISE (UNO).

ENGINEERED LUMBER - LAMINATED VENEER LUMBER (LYL) SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES. Pb = 2600 P61, Fv = 285 P61, E = 19000000 P61. LAMINATED 5TRAND LUMBER (LSL) SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES. Pb = 2325 P61, E = 1550000 P61. PARALLEL STRAND LUMBER (P61, JW TO T" DEPTH SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES. Pc = 2500 P61, E = 18000000 P61. PARALLEL STRAND LUMBER (P61, JWORE THAN T" DEPTH SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES. Pc = 2900 P61, E = 2000000 P61. NSTALL ALL CONNECTIONS PER MANIFACTURER'S SPECIFICATIONS.

STEEL BEAMS - ALL STRUCTURAL STEEL SHALL BE ASTM A36. STEEL BEAMS SHALL BE SUPPORTIED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3 1/2" AND FULL FLANGE WIDTH (UND). PROVIDE SOLID BEARING FROM BEAM'S SUPPORT TO FOUNDATION. BEAM'S SHALL BE ATTACHED TO EACH SUPPORT WITH TWO LAG SCREWS (1/2" DIAMETER X 4" LONG). LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE 2X NAILER ON TOP OF THE STEEL BEAM, AND THE 2X NAILER (S) ECURED TO THE BEAM FLANGE OR THE TOP OF THE STEEL BEAM IS INSTALLED WITHIN I 1/2" OF THE TOP OF THE JOISTS.

PONT LOADS - SQUARES DENOTE POINT LOADS WHICH REQUIRE SOLID BLOCKING TO GIRDER OR FOUNDATION. SHADED SQUARES DENOTE POINT LOADS FROM ABOVE WHICH REQUIRE SOLID BLOCKING TO SUPPORTING MEMBER BELOW.

LOAD BEARN'S HEADERS - ALL LOAD BEARING HEADERS ARE TO CONFORM TO TABLES R602.1(1), R602.1(2) AND R602.1(3) OR BE (2) 2×10 WITH (1) JACK AND (1) KING STUD EACH END (INO), WHICHEVER IS GREATER. ALL HEADERS ARE TO BE SECURED TO EACH JACK STUD WITH (4) 8d NAILS. ALL BEAMS ARE TO BE SUPPORTED WITH (2) STUDS AT EACH BEARING POINT (UNO).

BEAM BEARING: - ALL BEAMS, HEADERS, OR GIRDER TRUSSES PARALLEL TO BEARING WALL ARE TO BEAR FULLY ON (1) JACK OR (2) STUDS MINIMIM OR THE NUMBER OF JACKS OR STUDS NOTED. ALL BEAMS OR GIRDER TRUSSES PERPENDICULAR TO WALL AND SUPPORTED BY (3) STUDS OR LESS ARE TO HAVE 119," MINIMIM BEARING (WIND.) ALL BEAMS OR GIRDER TRUSSES PERPENDICULAR TO WALL AND SUPPORTED BY MORE THAN (3) STUDS OR OTHER NOTED COLUMN ARE TO BEAR FULLY ON SUPPORT COLUMN FOR ENTIRE WALL DEPTH (UNO). BEAM ENDS THAT BUTT INTO ONE ANOTHER ARE

STEEL FLITCH PLATE BEAM - STEEL FLITCH PLATE BEAMS SHALL BE BOLTED TOGETHER USING 1/2" DIAMETER BOLTS (ASTM A3Ø1) WITH WASHERS PLACED AT THREADED END OF BOLT. BOLTS SHALL BE SPACED AT 24" CENTERS (MAXIMUM), AND STAGGERED AT TOP AND BOTTOM OF BEAM (2" EDGE DISTANCE), WITH (2) BOLTS LOCATED 6" FROM EACH END (UNO).

I-JOIST/TRUSS LAYOUTS - ALL I-JOIST OR TRUSS LAYOUTS ARE TO BE IN COMPLIANCE WITH THE OVERALL DESIGN SPECIFIED ON THE PLANS. ALL DEVIATIONS ARE TO BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD PRIOR TO INSTALLATION.

WALL BRACING - BRACED WALL PANELS SHALL BE CONSTRUCTED ACCORDING TO SECTION R602.10
OF THE INTERNATIONAL RESIDENTIAL CODE, 2015 EDITION. THE LENGTH OF BRACING IN EACH BRACED WALL LINE SHALL COMPLY WITH TABLE R602.103(1) OR R602.103(3) OF THE INTERNATIONAL RESIDENTIAL CODE, 2015 EDITION, WHICHEVER IS GREATER. REFER TO WALL BRACING DETAILS WHEN

UPLIFT CONNECTIONS - SECURE ALL RAFTERS TO EXTERIOR WALL OR SUPPORTING BEAM WITH SIMPSON H25A HURRICANE TIE, EQUIVALENT CONNECTOR OR ALTERNATE CONNECTION CONFORMING TO THE NORC, SECURE EACH ROOF TRUSS TO EXTERIOR HALL OR BUPPORTING BEAM WITH UPLIFT CONNECTOR RATED AT OR ABOVE UPLIFT LOAD SHOWN ON TRUSS PROFILE. INSTALL ALL RAFTER/ROOF TRUSS-TO-WALL CONNECTORS DIRECTLY TO WALL FRAMING THROUGH EXTERIOR SHEATHING. WHERE CONNECTORS ARE INSTALLED TO INSIDE FACE OF TOP PLATES, INSTALL UPLIFT CONNECTOR SECURING RAFTER/ROOF TRUSS DIRECTLY TO WALL STUD BELOW OR INSTALL ADDITIONAL EQUIVALENT CONNECTOR SECURING THE TOP PLATE TO THE WALL STUD.

FRAMING MEMBER OR (2) SIMPSON MTS12 TWIST STRAPS (TYP, UNLESS NOTED OTHERWISE.)

BRACED WALL PANELS LOCATED AT EXTERIOR WALLS SUPPORTING RAFTERS OR ROOF TRUSSES, INCLUDING STORIES BELOW TOP STORY, SHALL BE CONSTRUCTED TO RESIST UPLIFT FORCES CONTINUOUS FROM ROOF TO FOUNDATION. EXTERIOR SHEATHING SHALL SECURE STORY ABOVE AND BELOW FLOOR BAND BY LAPPING ONTO OR ACROSS BAND. WHERE EXTERIOR SHEATHING IS INSTALLED WITH HORIZONTAL JOINT SPLICE AT THE TOP AND/OR BOTTOM OF THE FLOOR BANDS, SECURE EXTERIOR SHEATHING AND/OR BAND ACROSS SPLICE AT THE BRACED WALL PANELS WITH SIMPSON LTP4 FRAMING PLATES AT 24" O.C. MAX. OR SIMPSON CSIG COIL STRAPS AT 48" O.C. MAX. (TWO STRAPS MIN. PER BRACED WALL PANEL) LAPPING THE WALL FRAMING 14" MIN.

MALLS PARALLEL TO JOISTS. - PROVIDE DOUBLE JOIST UNDER ALL WALLS PARALLEL TO FLOOR JOISTS. DOUBLE JOISTS SEPARATED TO PERMIT THE INSTALLATION OF PIPING OR VENTS SHALL BE FULL DEPTH SOLID BLOCKED WITH LUMBER NOT LESS THAN 2" SPACED NOT MORE THAN 4"-0" OC. PROVIDE SUPPORT UNDER ALL WALLS PARALLEL TO FLOOR TRUSSES OR I-JOISTS PER MANUFACTURER'S SPECIFICATIONS. INSTALL BLOCKING BETHERN JOISTS OR TRUSSES FOR POINT LOAD SUPPORT FOR ALL POINT LOADS ALONG OFFSET LOAD LINES.

PRICK SUPPORT: - FOR ALL HEADERS SUPPORTING BRICK VENEER THAT ARE LESS THAN 8'-0" IN LENGTH, REST A 6" x 4" x 5/16" STEEL, ANGLE NITH 4" MINIMUM EMBEDMENT AT SUDES FOR BRICK SUPPORT. FOR ALL HEADERS 8'-0" AND GREATER IN LENGTH, BOLT A 6" x 4" x 5/16" STEEL ANGLE TO HEADER WITH 10" LAG SCREUS AT 12" OC. STAGGERED FOR BRICK SUPPORT. FOR ALL BRICK SUPPORT AT ROOF LINES, BOLT A 6" x 4" x 5/6" STEEL ANGLE TO 2 x 10" BLOCKING INSTALLED BETWEEN WALL STUDS WITH 12" LAG SCREWS AT 12" O.C. STAGGERED AND IN ACCORDANCE WITH SECTION RT03822 OF THE 2018 NCRC.

ROOF MEMBER SUPPORT - FOR STICK FRAMED ROOFS: CIRCLES DENOTE (3) 2×4 POSTS FOR ROOF MEMBER SUPPORT

DORMER FRAMING - FRAME DORMER WALLS ON TOP OF DOUBLE OR TRIPLE RAFTERS AS SHOWN 1. FRAME DORMER WALLS ON TOP OF 2 × 4 LADDER FRAMING AT 24" O.C. BETWEEN ADJACENT TRUSSES, STICK FRAME OVER-FRAMED ROOF SECTIONS WITH 2 × 8 RIDGES, 2 × 6 RAFTERS AT 6" O.C. AND FLAT 2 x 10 VALLEYS (UNO)

 $extstyle{DECK5}$ - ALL DECK FRAMING, LATERAL BRACING, GUARDRAIL CONSTRUCTION, ATTACHMENT TO THE HOUSE STRUCTURE AND THE CONNECTIONS WITHIN THE DECK FRAMING ARE TO COMPLY WITH APPENDIX M OF THE NORC.

ENERGY EFFICIENCY - ENERGY EFFICIENCY COMPLIANCE TO BE IN ACCORDANCE WITH CHAPTER II OF THE NORG. THE BUILDING THERMAL ENVELOPE SHALL MEET THE REQUIREMENTS OF TABLE NII02.12 BASED ON THE CLIMATE ZONE SPECIFIED.

WIND ZONE AND CLIMATE ZONE BY COUNTY

		WIND ZONE (MPH.	ν	WIND ZONE (MPH)/
	COUNTY	CLIMATE ZONE	COUNTY	CLIMATE ZONE
	ALAMANCE	115 / 4	JOHNSTON	120 / 3
	ALEXANDER	IB / 4	JONES	140/3
	ALLEGHANY	SMR / 5	LEE	115 / 4
	ANSON	II5 / 3	LENOIR	130/4
	ASHE	SMR / 5	LINCOLN	115 / 4
	AVERY	SMR/5	MACON	115 / 4
	BEAUFORT	130/3	MADISON	SMR / 4
	BERTIE ^a	120/130 / 4	MARTIN ⁹	120/130 / 3
	BLADEN ^b	130/140 / 3	MCDOWELL	115 / 4
	BRUNSWICK ^c	140/150 / 3-WHC	MECKLENBURG	115 / 3
	BUNCOMBE	SMR / 4	MITCHELL	9MR / 5
	BURKE	115 / 4	MONTGOMERY	115/3
	CABARRUS	II5 / 4	MOORE	115 / 3
	CALDWELL	II5 / 4	NASH	115 / 4
			NEW HANOVER ^h	
	CAMDEN	130 / 3 150 / 3-WHC		140/150 / 3-WHC
	CARTERET		NORTHAMPTON	115 / 4
	CASWELL	115 / 4	ONSLOW!	13@/14@/15@ / 3-WHO
	CATAWBA	115 / 4	ORANGE DAMILICO	115 / 4
	CHATHAM	115 / 4	PAMLICO	140/3
	CHEROKEE	15 / 4	PASQUOTANK	130/3
	CHOWAN	130 / 3	PENDER J	130/140/150 / 3-WH
,	CLAY	115 / 4	PERQUIMANS	130/3
_	CLEVELAND	115 / 4	PERSON	115 / 4
E	COLUMBUS	140 / 3-WHC	PITT	130/3
	CRAVEN	140/3	POLK	115 / 4
	CUMBERLAND ^d	120/130 / 3	RANDOLPH	115 / 3
	CURRITUCK	130/3	RICHMOND	120/3
	DARE	130/140 / 3	ROBESON	130/3
	DAVIDSON	115 / 3	ROCKINGHAM	115 / 4
	DAVIE	IB / 4	ROWAN	115 / 3
	DUPLIN	130/3	RUTHERFORD	115 / 4
	DURHAM	IB / 4	SAMPSON	130/3
	EDGECOMBE	II5 / 3	SCOTLAND	120/3
	FORSYTH	IB / 4	STANLY	115 / 3
	FRANKLIN	IB / 4	STOKES	115 / 4
	GASTON .	115 / 3	SURRY	115 / 4
	GATES	120 / 4	SWAIN	SMR / 4
	GRAHAM	SMR / 4	TRANSYLVANIA	115 / 4
,	GRANVILLE	IB / 4	TYRRELL	130/3
D	GREENE	130/3	union	115 / 3
	GUILFORD	IB / 4	VANCE	115 / 4
N	HALIFAX	II5 / 4	WAKE	115 / 4
	HARNETT	II5 / 4	WARREN	115 / 4
	HAYWOOD	SMR / 4	Washington	130/3
	HENDERSON	IB / 4	WATAUGA	SMR / 5
0	HERTFORD	li5 / 4	WAYNE	130/3
	HOKE.	120/3	WILKES	115 / 4
	HYDE	130/140 / 3	WILSON	120/3
	IREDELL	IB / 4	YADKIN	115 / 4
	JACKSON	SMR / 4	YANCEY	SMR / 5

-SMR DESIGNATES "SPECIAL MOUNTAIN REGION"

-SMR DEGIGNATES "SPECIAL MOINTAIN REGION"
-INHC DEGIGNATES "MARM-HUMID COUNTY"

B. 120 MPH ZONE MEST OF HUY TI, 120 MPH ZONE EAST OF HUY TI.

C. 130 MPH ZONE WEST OF HUY TOI, 130 MPH ZONE EAST OF HUY TOI.

C. 140 MPH ZONE WEST OF HUY TI, 150 MPH ZONE EAST OF HUY TI, 150

MPH ZONE WEST OF HUY TI, 150 MPH ZONE EAST OF HUY TI, 150

C. HAD TIPH ZONE MEDI OF HAIT II, IDM TIPH ZONE EADI OF HAIT II, IDM MPH ZONE ON BALD HEAD ISLAND. d. 120 MPH ZONE WEST OF I-95, 130 MPH ZONE EAST OF I-95. e. 130 MPH ZONE WEST OF US ROUTE 264, 140 MPH ZONE EAST OF US

F. 130 MPH ZONE WEST OF US ROUTE 264, 140 MPH ZONE EAST OF US ROUTE 264.

ROUTE 264. 9, 120 MPH ZONE WEST OF HAY 11, 130 MPH ZONE EAST OF HAY 11. 14 MB MPH ZONE WEST OF HAY 11, 150 MPH ZONE EAST OF HAY 11, 1. 130 MPH ZONE WEST OF HAY 11, 140 MPH ZONE EAST OF HAY 11 TO THE MTRACOASTAL WATERNAY, 150 MPH ZONE EAST OF THE INTRACOASTAL

MALERMAN.

J. 140 MPH ZONE IN THE TOWNSHIP OF TOPSAIL WEST OF THE
INTRACOASTAL MATERWAY, 150 MPH ZONE EAST OF THE INTRACOASTAL
WATERWAY, 130 MPH ZONE IN THE REMAINDER OF THE COUNTY.

WALL AND ROOF CLADDING DESIGN LOADS (POSITIVE AND NEGATIVE PSF)

MEAN ROOF			SF) BY	WALL CLADDING
HEIGHT (FT)	Ø < X < 2.5	25 < X < T	1 < X < 12	(PSF)
< 3Ø	10.0, -36.0	10.0, -33.0	13.1, -16.0	14.3, -19.0
3Ø < h < 35	10.5, -37.8	10.5, -34.7	13.8, -16.8	15.Ø, -2Ø.Ø
35 < h < 40	10.9, -39.2	10.9, -36.0	14.3, -17.4	15.6, -20.7
4Ø < h < 45	112, -403	11.2, -37.0	14.7, -17.9	16 <i>Ø</i> , -21,3
₹3Ø	10.0, -39.0	10.0, -36.0	14.2, -18.0	15.5, -2 <i>0.0</i>
3Ø < h < 35	10.5, -4L0	10.5, -36.5	14.9, -18.9	16.3, -21.0
35 < h < 40	10.9, -42.5	10.9, -37.9	15.5, -19.6	16.9, -21.8
4Ø < h < 45	11.2, -43.7	11.2, -39.0	15.9, -20.2	17.4, -22.4
∢3Ø	100, -460	10.5, -43.0	16.7, -21.Ø	18.2, -24.Ø
3Ø < h < 35	10.5, -48.3	11.0, -45.2	17.5, -22.1	19.1, -25.2
35 < h < 40	10.9, -50.1	11.4, -46.9	18.2, -22.9	19.8, -26.2
4Ø < h < 45	11.2, -51.5	11.8, -48.2	18.7, -23.5	20.4, -26.9
∢3Ø	IØ.Ø, 53.Ø	12.2, -49.0	19.4, -24.0	212, -28.0
3Ø < h < 35	10.5, -55.7	12.8, -51.5	20.4, -25.2	22.3, -29.4
35 < h < 40	10.9, -57.8	13.3, -53.4	2LI, -26.2	23.1, -30.5
4Ø < h < 45	11.2, -59.4	13.7, -54.9	21.7, -26.9	23.7, -31.4
₹3Ø	9.9, -610	14.0, -57.0	22.2, -28.Ø	24.3, -32.Ø
3⁄0 < h < 35	10.4, -64.1	14.7, -59.9	23.3, -29.4	25.5, -33.6
35 < h < 40	10.8, -66.5	15.3, -62.1	24.2, -30.5	26.5, -34.9
4Ø < h < 45	11.168.3	15.1, -63.8	24.9, -31.4	27.2, -35.8
	ROOF +HEIGHT (FT) (30) < h < 35 35 < h < 40 40 < h < 45 (30) 30 < h < 35 35 < h < 40 40 < h < 45 (30) 30 < h < 35 35 < h < 40 40 < h < 45 (30) 30 < h < 35 35 < h < 40 40 < h < 45 (30) 30 < h < 35 35 < h < 40 30 < h < 35 35 < h < 40 30 < h < 35 35 < h < 40 30 < h < 35 35 < h < 40 35 < h < 40	ROOF HEIGHT (FT)	ROOF PITCH ROOF PITCH REGHT (FT)	ROOF ROOF PICH

WIND ZONE	MEAN ROOF				WALL CLADDING
(MPH)	HEIGHT (FT)	Ø < X < 2.5	2.5 < X < T	1 < X < 12	(PSF)
	₹3Ø	10.0, -36.0	10.0, -33.0	13.1, -16.0	14.3, -19.0
115	3Ø < h < 35	10.5, -37.8	10.5, -34.7	138, -168	15 <i>0</i> , -2 <i>00</i>
כוו	35 < h < 40	10.9, -39.2	10.9, -36.0	14.3, -17.4	15.6, -20.7
	4Ø < h < 45	112, -403	112, -37Ø	14.7, -17.9	16.Ø, -21.3
	₹3Ø	10.0, -39.0	10.0, -36.0	14.2, -18.0	15.5, -2 <i>0.0</i>
120	3Ø < h < 35	10.5, -4L0	10.5, -36.5	14.9, -18.9	16.3, -21.Ø
1200	35 < h < 40	10.9, -42.5	10.9, -37.9	15.5, -19.6	16.9, -21.8
	4Ø < h < 45	11.2, -43.7	11.2, -39.0	15.9, -20.2	17.4, -22.4
	₹3Ø	10.0, -46.0	10.5, -43.0	16.T, -21.Ø	18.2, -24.Ø
130	3Ø < h < 35	10.5, -48.3	11.0, -45.2	17.5, -22.1	19.1, -25.2
שכו	35 < h < 40	10.9, -50.1	11.4, -46.9	18.2, -22.9	19.8, -26.2
	4Ø < h < 45	11.2, -51.5	11.8, -48.2	18.7, -23.5	20.4, -26.9
	₹3Ø	IØ.Ø, 53.Ø	12.2, -49.0	19.4, -24.0	21.2, -28.0
140	3Ø < h < 35	10.5, -55.7	12.8, -51.5	20.4, -25.2	22.3, -29.4
140	35 < h < 40	10.9, -57.8	13.3, -53.4	2l.l, -26.2	23.1, -30.5
	40 < h < 45	11.2, -59.4	13.7, -54.9	21.7, -26.9	23.7, -31.4
	∢3Ø	9.9, -610	14.0, -57.0	22.2, -28.Ø	24.3, -32.Ø
150	3Ø < h < 35	10.4, -64.1	14.7, -59.9	23.3, -29.4	25.5, -33.6
שפו	35 < h < 40	10.8, -66.5	15.3, -62.1	24.2, -30.5	26.5, -34.9
	40 < h < 45	11.1, -68.3	15.7, -63.8	24.9, -31.4	27.2, -35.8

TABLE NIØ2.12

	INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT									
CLIMATE ZONE	FENESTRATION U-FACTOR 6, J	SKYLIGHT ^b U-FACTOR	GLAZED FENESTRAŢIQN SHGC	CEILING R-VALUE [®]	WOOD FRAME WALL R-YALUE®	MASS WALL R-VALUE ¹	FLOOR R-VALUE	BASEMENT WALL ^{C, O} R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE ^C WALL R-VALUE
3	0.35	Ø.55	0.30	38 OR 30 CI	15 OR 13+2.5 h	5/13 OR 5/10 CI	19	5/13 ^f	ø	5/13
4	Ø.35	0.55	030	38 OR 30 CI	15 OR 13+25 ^h	5/13 OR 5/10 CI	19	10/13	10 ^d	10/13
5	Ø.35	Ø.55	NR	38 OR 30 CI	19, 1345 ^h OR 1543 ^h	13/17 OR 13/12.5 CI	3Ø ⁹	10/13	10 ^d	10/19

- a. R-VALUES ARE MINMIMS. U-FACTORS AND SHGC ARE MAXIMMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE
- b. THE FENESTRATION U-FACTOR COLUMN EXCLUDES SKYLIGHTS. THE SHGC COLUMN APPLIES TO ALL GLAZED FENESTRATION.

 c. "IØ/IB" MEANS R-IØ CONTINUOUS INSULATED SHEATHING ON THE INTERIOR OF EXTERIOR OF THE HOME OR R-IB CAVITY INSULATION AT THE INTERIOR OF
- THE BASEMENT WALL OR CRAWL SPACE WALL. THE DADETIEN! MALL OR CRANL SPACE MALL.

 A.R.5 SHALL BE ADDET OT THE REQUIRED SLAB EDGE R-VALUES FOR HEATED SLABS. FOR MONOLITHIC SLABS, INSULATION SHALL BE APPLIED FROM THE INSPECTION GAP DOWNLARD TO THE BOTTOM OF THE FOOTING OR A MAXIMUM OF 24" BELOW GRADE, WHICHEVER IS LESS, FOR FLOATING SLABS, INSULATION SHALL EXTEND TO THE BOTTOM OF THE FOUNDATION WALL OR 24", WHICHEVER IS LESS. (SEE APPENDIX O)
- e. DELETED F BASEMENT WALL INSULATION IS NOT REQUIRED IN WARM-HUMID
- T. DASCHEM WALL MOULAHOU IS NOT RECUIRED IN WARFHAMILD LOCATIONS AS DEFINED BY FIGURE NIBOLT AND TABLE NIBOLT.

 G. OR NSULATION SUFFICIENT TO FILL THE FRAMING CAVITY, R-19 MINIMUM.

 THE FIRST VALUE 19 CAVITY INSULATION, THE SECOND VALUE 19 CONTINUOUS INSULATION, 50 "13-5" INEANS R-13 CAVITY INSULATION PLUS R-5 CONTINUOUS INSULATION, IF STRUCTURAL SHEATHING COVERS 25% OR LESS OF THE EXTERIOR, INSULATING SHEATHING IS NOT REQUIRED WHERE STRUCTURAL SHEATHING IS USED. IF STRUCTURAL SHEATHING COVERS MORE THAN 25% OF EXTERIOR, STRUCTURAL SHEATHING SHALL BE SUPPLEMENTED WITH INSULATED SHEATHING OF AT LEAST R-2.

- 1. THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR OF THE MASS WALL.
- I IN ADDITION TO THE EXEMPTION IN SECTION NII/233 A MAXIMUM OF TWO J. IN ADDITION TO THE EXEMPTION IN SECTION NIW233, A MAXIMUM OF THO GLAZED FRESTRATION PRODUCT ASSEMBLES HAVING A U-FACTOR NO GREATER THAN #955 SHALL BE PERMITTED TO BE SUBSTITUTED FOR HIMINUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES HITHOUT PENALTY. IN ADDITION TO THE EXEMPTION IN SECTION NIW233, A MAXIMUM OF THO GLAZED FENESTRATION PRODUCT ASSEMBLIES HAVING A SHIGC NO GREATER THAN #216 SHALL BE FERTITIED TO BE SUBSTITUTED FOR MINIMUM CODE COMPLIANT FENESTRATION PRODUCT ASSEMBLIES WITHOUT PENALTY.
- I. R-39 SHALL BE DEEPTED TO SATISFY THE CELING INSULATION REQUIREMENT IMHEREVER THE FULL HEIGHT OF UNCOMPRESSED R-39 INSULATION EXTENDS OVER THE WALL TOP PLATE AT THE EAVES. OTHERNISE R-39 INSULATION IS REQUIRED WHERE ADEQUATE CLEARANCE
- OTHERWISE R-35 INSULATION IS REQUIRED WHERE AUGUSTIC CLEARANCE EXISTS OR INSULATION MUST EXTEND TO EITHER THE INSULATION BAFFLE OR WITHIN I' OF THE ATTIC ROOF DECK.

 III. TABLE VALUE REQUIRED EXCEPT FOR ROOF EDGE WHERE THE SPACE IS III. TABLE VALUE PROBLEMEN EXCEPT FOR ROPE FLORE WHERE THE STACE IN HIMTED BY THE PITCH OF THE ROOF, THERE THE NOULATION MUST FILL THE SPACE UP TO THE AIR BAFFLE. A. R. PS FIBERGLASS BATTS COMPRESSED AND NSTALLED IN A NOMINAL 2x6 FRAMING CAVITY IS DEEMED TO COMPLY. FIBERGLASS BATTS RATED
- R-19 OR HIGHER COMPRESSED AND INSTALLED IN A 2x4 MALL IS NOT DEEPED TO COMPLY. O. BASEMENT HALL MEETING THE MINIMUM MASS MALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM

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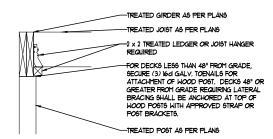
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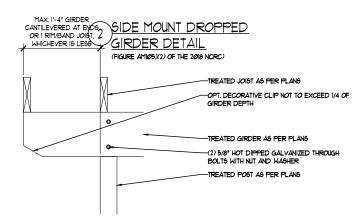
STANDARD STRUCTURAL NOTES

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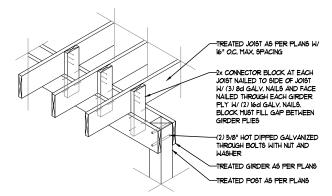
TOP MOUNT/FLUSH GIRDER DETAIL (FIGURE AMIØ5.I(1) OF THE 2018 NCRC)



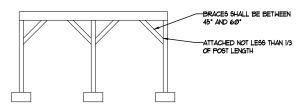


CANTILEVERED DROPPED GIRDER DETAIL

(FIGURE AMIØ5 I(4) OF THE 2018 NORC) -CANTILEVERED GIRDER LIMITED TO FLOOR LOADS ONLY. ROOF LOADS PROHIBITED ON CANTILEVERED GIRDER APPLICATION



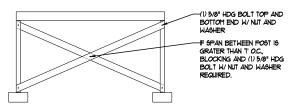
SPLIT GIRDER DETAIL (FIGURE AMIØ5.I(3) OF THE 2018 NCRC) -SPLIT GIRDER LIMITED TO FLOOR LOADS ONLY AND CANTILEVER GIRDER ENDS ALLOWED PER FIGURE



WOOD KNEE BRACING DETAIL (FIGURE AMIØ9.I(2) OF THE 2018 NCRC)

1) FREE STANDING DECKS REQUIRING BRACING SHALL BE INSTALLED IN BOTH DIRECTIONS OF EACH POST.

2) DECKS ATTACHED TO STRUCTURE REQUIRE DIAGONAL BRACING ONLY AT OUTSIDE GIRDER LINE PARALLEL WITH STRUCTURE.



DIAGONAL VERTICAL CROSS 6 BRACING DETAIL

 $\underline{\it RAIL\ POSTS}$ - CANNOT EXCEED 8' O.C. SPACING AND SHALL BE ATTACHED W/ (2) 3/8" GALV. BOLTS W/ NUT AND WASHER TO OUTER BANDS.

STAIR HANDRAIL - HEIGHT BETWEEN 34"-38" IN ACCORDANCE W/ R311.1.8.1 AND R312.1. OPENINGS ON SIDE OF STAIRS REQUIRING GUARDS SHALL NOT ALLOW A SPHERE W/ 4 3/8" DIAMETER TO PASS IN ACCORDANCE W/ R3I2.13, EXCEPTION 2.

<u>\$1AIR TREADS, AND RISERS</u> - PER R3II.15.1 (8 I/4" MAX. RISER) AND R3II.15.2 (9" MIN. TREAD DEPTH). \$1AIRWAYS 36" MIN. WIDTH PER R3II.1.1 (RAIL PROJECTIONS ALLOWED).

RISER OPENINGS - STAIRS W/ A 30° OR MORE VERTICAL RISE MUST HAVE SOLID RISERS OR OPENING RESTRICTED TO PREVENT A 4° DIAMETER SPHERE FROM PASSING PER

GUARDS - AT A 36" MIN, HEIGHT REQUIRED IN ACCORDANCE W/ R312.12 W/ 30" DROP AND OPENING LIMITS PER R312.13, TOP RAIL AND POST TO SUPPORT 200 LBS W/ INFILL TO MEET 50 LBS IN ACCORDANCE W/ TABLE R3015 AND FOOTNOTES.

DECKING - PER AMIØT FOR 12 SYP AND ATTACHED W/ (2) 8d GALV. NAILS AT EACH JOIST OR APPROVED SCREIG. OTHER MATERIALS PER MANIFACTURER'S INSTALLATION BASED UPON JOISTS OC. SPACING. ALTERNATE MATERIAL ATTACHED PER MANIFACTURER'S INSTALLATION INSTRUCTIONS. DECKS ARE TO BE CONSTRUCTED AS PER APPENDIX M OF THE 2018 NORTH CAROLINA

PECK ATTACHMENT: - AS PER SECTION AMOST OF THE 2018 NORC, WHEN A DECK SHALL BE SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE, SECURE DECK TO STRUCTURE AS PER TABLE AMOST, (1), TABLE AMOST, (2), METHOD 3 OR METHOD 4 BELOW:

TABLE AMIDA.(1)
ALL STRUCTURES EXCEPT BRICK VENEER STRUCTURES

FASTENERS	ENERS 8' MAX, JOIST SPAN®		
5/8" HDG BOLTS W/ NUT AND WASHER ^B	1 # 3'-6" O.C.	1 • 1'-8" O.C.	
AND	AND	AND	
12d COMMON HDG NAILS ^C	2 # 8" O.C.	3 # 6" O.C.	
	OR		
SELF-DRILLING SCREW FASTENER ^d	12" O.C. STAGGERED	6" O.C. STAGGERED	

- 8. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOISTS SPAN IS ALLOWED. b. MIN. EDGE DISTANCE FOR BOLTS IS 2 1/2".
- b. MIN. EDGE DISTANCE FOR BOLTS 15 2 (7).
 c. NALIS MIST PENETRATE THE SUPPORTING STRUCTURE BAND A MIN OF 1 (1/2", d. SELF-DRILLING SCREW FASTENER HAVING A MINIMIM SHANK DIAMETER OF Ø.195" AND A LENGTH LONG ENOUGH TO PENETRATE THROUGH THE SUPPORTING STRUCTURE BAND. THE STRUCTURE BAND SHALL HAVE A MINIMIM DEPTH OF 1 (1/8"). SCREW SHALL BE EVALUATED BY AN APPROVED TESTING AGENCY FOR ALLOWABLE SHEAR LOAD FOR SYP TO SYP LUMBER OF 250 LBS. AND SHALL HAVE A CORRECSION-RESISTANT. FINISH EQUIVALENT TO HOT DIP GAL VANIZED. MINIMUM EDGE DISTANCE FOR SCREWS IS I 1/16". A MAXIMUM OF 1/2" THICK WOOD STRUCTURAL PANEL IS PERMITTED TO BE LOCATED BETWEEN THE DECK LEDGER AND THE STRUCTURE BAND.

TABLE AMI@4.(2) BRICK VENEER STRUCTURES

FASTENERS	8' MAX, JOIST SPAN ⁸	16" MAX, JOIST SPAN [®]
5/8" HDG BOLTS W/ NUT AND WASHER ^B	1 # 2'-4" O.C.	1 # 1'-4" O.C.

a, ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOISTS SPAN IS ALLOWED b. MIN, EDGE DISTANCE FOR BOLTS 1S 2 $1/2^\circ$

METHOD 3) IF THE DECK BAND IS SUPPORTED BY A MIN, OF 1/2" MASONRY LEDGE ALONG THE RONDATION WALL, SECURE DECK TO STRUCTURE W/ 5/8" HDG BOLTS W/ WASHERS SPACED AT 48" OC.

 $\underline{\text{METHOD 4.}} \text{ JOIST HANGERS OR OTHER MEANS OF ATTACHMENT MAY BE CONNECTED TO HOUSE BAND AND SHALL BE PROPERLY FLASHED.}$

 $\underline{\text{DECK BRACNG}}$ - AS PER SECTION AMIØ3 OF THE 2018 NCRC, THE DECK SHALL BE LATERALLY BRACED AS PER ONE OF THE FOLLOWING:

1) WHEN THE DISTANCE FROM THE TOP OF THE DECK FLOOR TO THE FINISHED GRADE IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION AMOU LISTED ABOVE, LATERAL BRACING IS NOT REQUIRED. LATERAL BRACING IS NOT REQUIRED FOR FREE STANDING DECKS WITH A DECK FLOOR HEIGHT OF 30" OR LESS ABOVE FINISHED GRADE.

 $2)\,4\times4$ Treated Mood knee Braces May be provided on each column in both directions. The knee Braces shall attach to each post at a point not less than 1/3 of the post length from the top of the post, and the braces shall be ANGLED BETHEBY 45° AND 60° RROM THE HORIZONTAL. KNEE BRACES SHALL BE BOLTED TO THE POST AND THE GIRDER/DOUBLE BAND W/ (1) 5/6° HDG BOLT WITH NUT AND WASHER AT BOTH ENDS OF THE BRACE PER DETAIL 5.

3) FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POST IN ACCORDANCE WITH TABLE AMIOSLS. DECKS ATTACHED TO STRUCTURE CAN ALSO BE BRACED ON EXTERIOR GIRDER LINE W/ EMBEDMENT OPTION.

TABLE AMIØ9.13

POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT ^a	EMBEDMENT DEPTH	CONCRETE DIAMETER
4 x 4	48 5Q. FT.	4'-@"	2'-6"	1'-0"
6 x 6	12Ø 5Q. FT.	6'-0"	3'-6"	1'-8"

a. FROM TOP OF FOOTING TO TOP OF DECKING

4) 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE 2 x 6's SHALL BE ATTACHED TO THE POSTS W/ (1) 5/8" HDG BOLT W/ NUT AND WASHER AT EACH END OF EACH BRACING MEMBER PER DETAIL 6.

5) FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.



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