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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 54883 JOB: 24-B205-R01 JOB NAME: LOT 0.0015 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2018 as well as IRC 2021. 54 Truss Design(s)

Trusses:

GR01, GR02, GR03, GR04, GR05, GR06, GR07, J01, J02, J04, J05, J06, J07, J09, P01, P02, R01, R02, R03, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R14A, R14B, R14C, R15, R16, R17, R18, R19, R20, R21, R22, R23, R24, R25, R26, R27, R28, V01, V02, V03, V04, V05,



# Warning !--- Verify design parameters and read notes before use.





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Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDER	N WAY ANGIER, NC
24-B205-R01	GR02	Hip Girder	1	2	Job Reference (optional)	# 54883
			D	10 0004 D		0-+ D 7 40.04.00 0004 D

Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Dec 7 18:04:30 2024 Page 2 ID:qqlfH?RqemZ1wWmxuKuRIUzBcTx-CGupUeMyu7VbfqUo9vrrinpkLLN0IKzD\_zmvWwyBIA?

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-5=-60, 5-8=-60, 2-7=-20

Concentrated Loads (lb)

Vert: 4=-83(F) 5=-83(F) 10=-97(F) 3=-48(F) 13=-58(F) 12=-29(F) 11=-29(F) 6=-48(F) 9=-58(F) 14=-63(F) 15=-15(F) 16=-83(F) 17=-83(F) 18=-15(F) 19=-63(F) 20=-43(F) 21=-97(F) 22=-29(F) 23=-29(F) 24=-43(F)





TOP CHORD

BOT CHORD

#### LUMBER-

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2

Left 2x4 SP No.3 1-5-0 SLIDER

REACTIONS. (Ib/size) 4=48/Mechanical, 2=144/0-3-8 (min. 0-1-8), 5=20/Mechanical Max Horz 2=53(LC 12) Max Uplift4=-35(LC 12), 2=-3(LC 12) Max Grav 4=52(LC 20), 2=147(LC 18), 5=40(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 35 lb uplift at joint 4 and 3 lb uplift at joint 2.

# LOAD CASE(S) Standard



Structural wood sheathing directly applied or 2-0-0 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.



- between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 7 and 40 lb uplift at joint 4.
- 10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines
- 11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

#### LOAD CASE(S) Standard

- 1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)
  - Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20

SEAL 28147 2/7/202/ Va and 12/7/2024



BOT CHORD

BOT CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3 Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.

Rigid ceiling directly applied or 6-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 7=259/0-3-8 (min. 0-1-8), 4=108/Mechanical, 5=78/Mechanical Max Horz 7=87(LC 12) Max Uplift4=-27(LC 9), 5=-7(LC 12)

Max Grav 7=259(LC 1), 4=108(LC 1), 5=86(LC 5)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) Provide adequate drainage to prevent water ponding.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

- between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 27 lb uplift at joint 4 and 7 lb uplift at joint 5.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 7=259/0-3-8 (min. 0-1-8), 4=68/Mechanical, 5=117/Mechanical Max Horz 7=125(LC 12) Max Uplift4=-8(LC 9), 5=-58(LC 12) Max Grav 7=259(LC 1), 4=68(LC 1), 5=120(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 8 lb uplift at joint 4 and 58 lb uplift at joint 5.

LOAD CASE(S) Standard





#### NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 132 lb uplift at joint 3.

# LOAD CASE(S) Standard





10) "NAILED" indicates 3-10d (0.148"x3") or 3-12d (0.148"x3.25") toe-nails per NDS guidlines.

11) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-4=-60, 5-7=-20 Concentrated Loads (lb) Vert: 6=-15(F) 9=-15(F) will fit ( 4 and CARO SEAL 28147 SEAL 28147 L2/T/2024 L2/T/2024



Max Uplift4=-15(LC 9), 5=-37(LC 12)

Max Grav 7=221(LC 1), 4=64(LC 1), 5=81(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.





#### **NOTES-** (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 94 lb uplift at joint 3 and 5 lb uplift at joint 4.

8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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LOAD CASE(S) Standard
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Max Uplift4=-66(LC 12), 5=-32(LC 12)

Max Grav 8=221(LC 1), 4=95(LC 20), 5=69(LC 20)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 66 lb uplift at joint 4 and 32 lb uplift at joint 5.

LOAD CASE(S) Standard





Max Uplift5=-19(LC 9), 6=-30(LC 12)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Provide adequate drainage to prevent water ponding.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.8) Refer to girder(s) for truss to truss connections.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 5 and 30 lb uplift at joint 6.

#### LOAD CASE(S) Standard







REACTIONS. (lb/size) 3=53/Mechanical, 2=159/0-3-8 (min. 0-1-8), 4=35/Mechanical Max Horz 2=65(LC 12) Max Uplift3=-30(LC 12), 2=-10(LC 12) Max Grav 3=55(LC 20), 2=159(LC 1), 4=50(LC 5)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### **NOTES-** (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 30 lb uplift at joint 3 and 10 lb uplift at joint 2.

LOAD CASE(S) Standard





2x4 =

2x4 =

⊢		4-	6-0						
4-0-0 Plate Offsets (X,Y) [3:0-3-0,Edge]									
LOADING (psf)           TCLL (roof)         20.0           Snow (Pf)         20.0           TCDL         10.0           BCLL         0.0 *           BCDL         10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.03 BC 0.17 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.00 4 0.00 4 0.00 4	l/defl L/d n/r 180 n/r 80 n/a n/a	PLATES         GRIP           MT20         244/190           Weight: 12 lb         FT = 20%			
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural w Rigid ceiling	ood sheathing dire directly applied o ommends that Stal	ectly applied or 4-6-0 oc purlins. r 10-0-0 oc bracing. bilizers and required cross bracing			
				be installed	d during truss erec a quide	tion, in accordance with Stabilizer			

REACTIONS. (lb/size) 2=148/2-11-0 (min. 0-1-8), 4=148/2-11-0 (min. 0-1-8) Max Horz 2=26(LC 11) Max Uplift2=-17(LC 12), 4=-17(LC 13)

# NOTES- (10)

5) Gable requires continuous bottom chord bearing.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 2 and 17 lb uplift at joint 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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LOAD CASE(S) Standard
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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

<sup>2)</sup> Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

<sup>3)</sup> TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

<sup>4)</sup> This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.



MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation quide.

- REACTIONS. (lb/size) 2=123/2-3-8 (min. 0-1-8), 4=123/2-3-8 (min. 0-1-8) Max Horz 2=-22(LC 10) Max Uplift2=-16(LC 12), 4=-16(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

#### NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 16 lb uplift at joint 2 and 16 lb uplift at joint 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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LOAD CASE(S) Standard
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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583









Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDG	E   271 ALDEN WAY AND	GIER, NC
24-B205-R01	R10	Half Hip Girder	1	1	Ich Reference (antional)	#	54883
			Run: 8.430 s Feb	12 2021 Pri	int: 8.630 s Jul 12 2024 MiTek Ir	ndustries, Inc. Sat Dec 71	18:05:00 2024 Page 1
-0 <mark>-10<sub>7</sub>8 2-3-8</mark>	6-6-0 10-1-12	13-6-0 15-7-12 20-	6-14 <u> </u>	5-6-0		35-6-0	IIKIOeuu I VZBYBI9A
0-10-8 2-3-8	4-2-8 3-7-12	' 3-4-4 ' 2-1-12 ' 4-1	1-2 ' 4	-11-2	4-11-2	5-0-14	
							Scale = 1:65.9
		NAILED NAILED NAILED	NAILED NAILE	ED NAILEI		) NAILED NAILED	
		$2x4 \parallel 4x4 = 5x8 = 14$	4x4 = 3x	8 =	2x4    NAUED 4x4 =	= 4x4 =	
	4 32	5 33 6 34 7 35	1LED 36 <sup>8</sup> 37 9	38 39	10 40 41 - 11	42 43 12	
8.00 12	NAILED		u up u				
NAILED	31						
2-11-2		B4 W3 W5	v6 W5 💙	V76	W5 W6 W5	W6 W5	
¥ 2 3	B2						
N 1 81 6	UU					B6 6	0-0
	44 <sup>22</sup> 45	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	<u>u u</u> 49. 50	<u>51 52</u>		55 56	<u>l</u>
23		$4x8 = \frac{19}{12} + \frac{13}{12} $	17 ILED 4-0	01 02	$16^{-16} = 4x8 = 14$		I
$4x6 = \frac{2x4}{4x6} =$	NAILED	3x6    6x12 =  NAILED			4x8 = 4x6		
2x4		NAILED					
NAILE	ED						
2.2.0	6 6 0 10 1 13	12 6 0 15 7 12 20	e 14	560	20 5 2	35.6.0	
2-3-8	4-2-8 3-7-12	3-4-4 2-1-12 4-1	1-2 4	-11-2	4-11-2	5-0-14	
Plate Offsets (X,Y) [2:0-	<u>-2-4,0-2-0], [3:0-3-0,0-1-8],</u>	[4:0-7-12,0-2-0], [18:0-3-8,0-4-8]	1				
TCLL (roof) 20.0	SPACING-	2-0-0 <b>CSI</b> .	DEFL.	in (le	oc) I/defl L/d	PLATES	GRIP
Snow (Pf) 20.0	Lumber DOL	1.15 BC 0.57	Vert(CT)	-0.17	23 >999 240 23 >999 180	WIT20	244/190
BCLL 0.0 *	Rep Stress Incr	NO WB 0.75	Horz(CT)	0.07	18 n/a n/a	Weight: 256 lb	FT = 20%
BCDL 10.0							
LUMBER- TOP CHORD 2x4 SP No	.2 *Except*		BRACING- TOP CHORD	Structur	al wood sheathing directl	v applied or 6-0-0 oc	purlins, except
T1: 2x6 SP	No.2		BOT CHORD	end vert	ticals.		
B3,B4: 2x4	SP No.2		BOTCHORD	MiTek	recommends that Stabiliz	zers and required crc	ss bracing
WEBS 2x4 SP No	.3			be inst	alled during truss erectio	n, in accordance with	n Stabilizer
REACTIONS. (Ib/size)	13=791/Mechanical, 2=525	/0-3-8 (min. 0-1-8), 18=2671/0-3-8	(min. 0-3-2)	Installa	alion guide.		
Max Horz	2=141(LC 9) 13=-438(LC 6) 2=-120(LC	10) 18=-1230/1 ( 7)					
Max Grav	13=815(LC 26), 2=525(LC	1), 18=2671(LC 1)					
FORCES. (lb) - Max. Cor	mp./Max. Ten All forces 2	50 (lb) or less except when shown.					
TOP CHORD 2-3=-269	/119, 3-30=-438/193, 30-3	=-407/164, 4-31=-351/169, 6-34=-4	127/1028,				
7-34=-42 10-40=-6	7/1028, 8-38=-625/436, 9-3 25/436, 40-41=-625/436, 1	38=-625/436, 9-39=-625/436, 10-39 1-41=-625/436, 11-42=-648/399, 42	=-625/436, 2-43=-648/399,				
12-43=-6 2 44- 10	48/399, 12-13=-745/444	45- 202/256 21 45- 202/256 21	46- 1011/410				
20-46=-1	011/419, 6-20=-968/459, 1	8-48=-1243/512, 48-49=-1243/512,	49-50=-1243/512,				
17-50=-1 WEBS 4-22=-12	243/512, 16-53=-352/642, 1/387	15-53=-352/642, 15-54=-352/642, 1  =-271/172_6-21=-522/1167_18-20	4-54=-352/642 =-1291/538				
7-20=-23	1/438, 7-18=-1815/951, 7-1	7=-754/1613, 8-17=-985/551, 8-16	=-384/857,				
10-16=-3	81/269, 11-14=-415/342, 1	2-14=-482/852					
<b>NOTES-</b> (11)	-100mmh (2 accord suct)	lasd-ofmak, TODI -F Oraf, DODI		- 4 U. E			
(envelope) gable end z	one; cantilever left and right	it exposed ; end vertical left and rig	ht exposed; Lumbe	er DOL=1	.60 plate grip DOL=1.60		
2) TCLL: ASCE 7-16; Pr=	20.0 psf (roof LL: Lum DOL	=1.15 Plate DOL=1.15); Pf=20.0 ps	sf (Lum DOL=1.15	Plate DC	0L=1.15); ls=1.0; Rough	WINNITH CARO	11.
3) This truss has been de	signed for greater of min ro	of live load of 12.0 psf or 2.00 time	s flat roof load of 2	0.0 psf or	n overhangs	FESSIA	Valle
4) Provide adequate drain	her live loads. hage to prevent water pond	ina			Inn	Port May	
5) This truss has been de	signed for a 10.0 psf bottor	n chord live load nonconcurrent with	h any other live loa	ids.		SEAL	1111
between the bottom ch	ord and any other members	0.0pst on the bottom chord in all are 5.	eas where a rectan	igie 3-6-0	tall by 1-0-0 wide will fit	28147	
7) Refer to girder(s) for trues	uss to truss connections.	e to bearing plate conchie of with the	anding 100 lb unliff	t at inint/-	a) except (it-lb) 12-12	De la cal	1 million
2=120, 18=1239.		s to bearing plate capable of withsta	anding 100 lb uplitt	r ar joint(S	o except (It-ID) 13=438	APLOINEE	Summer
9) "NAILED" indicates 3-1	0d (0.148"x3") or 3-12d (0.	148"x3.25") toe-nails per NDS guid	lines. ont (E) or back (B)			Mining K. MORTH	Nº.
	,					12/7/202/	1
LUAD CASE(S) Standard Warning !—Verify design	parameters and read notes b	efore use. This design is based only upon	parameters shown, an	nd is for ar	individual building compone	ent to be installed and $\log \frac{12}{12024}$	ided
Continued on page 2	•	5 · · · · · · · · · · · · · · · · · · ·	-				

Continued on page 2 with the second s

Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDE	EN WAY ANGIER, NC
24-B205-R01	R10	Half Hip Girder	1	1	Job Reference (optional)	# 54883
		Run:	3.430 s Feb	12 2021 Pri	nt: 8.630 s Jul 12 2024 MiTek Industries. Inc.	Sat Dec 7 18:05:01 2024 Page 2

Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Dec 7 18:05:01 2024 Page 2 ID:qqIfH?RqemZ1wWmxuKuRIUzBcTx-oDGX\_EkObePCtaMITwu6SKb?Fru?3COn6Hm25dyBI9W

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-4=-60, 4-12=-60, 23-24=-20, 20-27=-20, 13-19=-20

Concentrated Loads (lb)

Vert: 4=-24(F) 23=-36(F) 22=-39(F) 21=-39(F) 5=-24(F) 11=-40(F) 14=-23(F) 30=-43(F) 31=-5(F) 32=-24(F) 33=-24(F) 34=-40(F) 35=-40(F) 36=-40(F) 37=-40(F) 38=-40(F) 39=-40(F) 40=-40(F) 41=-40(F) 42=-40(F) 43=-40(F) 44=-63(F) 45=-39(F) 46=-39(F) 47=-23(F) 48=-23(F) 49=-23(F) 50=-23(F) 51=-23(F) 52=-23(F) 53=-23(F) 54=-23(F) 55=-23(F) 56=-23(F) 56=





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDEN WAY ANGIER, NC
24-B205-R01	R12	Нір	1	1	Job Reference (optional) # 54883

Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Dec 7 18:05:06 2024 Page 2 ID:qqlfH?RqemZ1wWmxuKuRIUzBcTx-8B3Q1ynXQB1U\_MFjGTUH9NJu9sW\_kPsWGZUpnryBl9R

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDEN WAY ANGIER, NC
24-B205-R01	R13	PIGGYBACK BASE	1	1	(ab Bafaranaa (antianal) # 54883
			Run: 8.430 s Feb	12 2021 Pr	int: 8,630 s Jul 12 2024 MiTek Industries, Inc. Sat Dec 7 18:05:09 2024 Page 1
-0-	10 <sub>1</sub> 82-3-8 7-10-12	15-6-0	ID:qqlfH?Rq 20-0-0	1em∠1wW -27	mxuKuRIUzBcTx-ZmlZtzqPi6P3rpztxb1_n0xJ43WFxoOzyXTTN9yBl9O 8-935-6-0
0-	10-8 2-3-8 5-7-4	7-7-4	4-6-0	7-8	3-9 7-9-7
		<u></u>			Scale = 1:72.7
		6x8 =	6x8 =	=	
T -		8.00   12 7	тз 8		Ŧ
				$\sim$	
				72	
		3x0		$\langle \rangle >$	3x8 📎
	5x	.8 6 // //	wa		9 2x4 //
9-0 9-0		5 W6 M	x/		
-1-0-		// WB		Wild	27
4x	б ∕/ 3х4    26 <sub>∏1</sub>			//	W11 14
	4 W3	W4 W5 // //	//		
	3		/		
		16 W7 🕞	B5	•	
0 1	´⊠ 5x10 = 19	5x8 = 2x4    15 14	28	13	3 12 29 30 <sup>🛛 🖓 8</sup>
	4x4 = 6x6 =	3x4    6x8 =	=	3x8	$s = 4x4 = 4x6 \parallel$
	<u>2-3-8</u> 7-10-12 2-3-8 5-7-4	<u> </u>	<u>25-6-0</u> 10-0-0		<u> </u>
Plate Offsets (X,Y) [3:0	-1-4,0-1-12], [5:0-2-10,0-1-	12], [7:0-4-13,Edge], [8:0-4-13,Edge	], [16:0-2-4,0-2-12	2], [18:0-4	-4,0-3-4], [19:0-3-0,0-3-12]
LOADING (psf)	SPACING-	2-0-0 CSI.	DEFL.	in (l	oc) I/defl L/d PLATES GRIP
Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.87 1.15 BC 1.00	Vert(LL)	-0.44 12	-14 >971 240 MT20 244/190 -14 >624 180
TCDL 10.0 BCU 0.0 *	Rep Stress Incr	YES WB 0.81	Horz(CT)	0.23	11 n/a n/a
BCDL 10.0	Code IRC2021/TP	I2014 Matrix-AS			Weight: 227 lb FT = 20%
LUMBER-	0		BRACING-	Christen and the set	
BOT CHORD 2x4 SP No	0.2 0.1 *Except*		BOT CHORD	Rigid ce	iling directly applied.
B1: 2x6 SF WEBS 2x4 SP No	P No.2, B3,B4: 2x4 SP No.2 3 *Excent*	2	WEBS	1 Row a	It midpt 7-14, 8-14
W2: 2x4 S	P No.2			be inst	alled during truss erection, in accordance with Stabilizer
WEDGE Right: 2x4 SP No.3				Installa	ation guide.
	0-1170/0 0 0 (min 0 1 10	(14-1440)(0, 2, 0, (min, 0, 4, 42))			
Max Horz	2=1473/0-3-8 (mm. 0-1-12) 2=208(LC 9)	), 11–1419/0-3-8 (mm. 0-1-13)			
Max Uplift Max Grav	2=-107(LC 12), 11=-92(LC 2=1473(LC 1), 11=1530(LC	13)			
TOP CHORD 2-3=-204	mp./Max. Ten All forces 2 ·6/134, 3-4=-3991/369, 4-26	250 (lb) or less except when shown. 6=-4297/469, 5-26=-4261/491, 5-6=-	1885/186,		
6-7=-176	4/223, 7-8=-1296/228, 8-9=	1993/255, 9-10=-2113/217, 10-27=	-2061/182,		
BOT CHORD 2-19=-20	.241/159 19/1613, 18-19=-88/772, 17	-18=-190/2172, 16-17=-190/2173, 1	5-16=-265/0,		
14-28=0/ 11-30=-7	1283, 13-28=0/1283, 12-13 6/1787	3=0/1283, 12-29=-76/1787, 29-30=-7	76/1787,		
WEBS 5-17=0/3	09, 5-16=-808/215, 14-16=	0/1548, 7-16=-115/1052, 7-14=-499	/150, 8-12=-146/93	35,	
10-12=-4	416/242, 3-18=-323/2774, 3	3-19=-1174/142, 5-18=-320/1935			
NOTES- (10)	and the second second second second	Constitution of the stress			
2) Wind: ASCE 7-16; Vult	t=120mph (3-second gust)	Vasd=95mph; TCDL=5.0psf; BCDL=	=5.0psf; h=23ft; Ca	at. II; Exp	B; Enclosed; MWFRS
(envelope) gable end z	cone and C-C Exterior(2E) -	0-10-8 to 3-11-2, Interior(1) 3-11-2 t	o 8-8-9, Exterior(2	R) 8-8-9	to 26-9-7, Interior(1)
members and forces &	MWFRS for reactions show	wn; Lumber DOL=1.60 plate grip DC	)L=1.60	and right	exposed, c-c toi
3) TCLL: ASCE 7-16; Pr=	20.0 psf (roof LL: Lum DOL ce=1 0: Cs=1 00: Ct=1 10	_=1.15 Plate DOL=1.15); Pf=20.0 ps	f (Lum DOL=1.15	Plate DC	DL=1.15); Is=1.0; Rough
4) This truss has been de	signed for greater of min ro	of live load of 12.0 psf or 2.00 times	flat roof load of 20	0.0 psf oi	n overhangs
non-concurrent with otl 5) Provide adequate drair	ner live loads. hage to prevent water pond	ing.			28147
6) This truss has been de	esigned for a 10.0 psf bottor	n chord live load nonconcurrent with	any other live loa	ids.	Itall by 1.0.0 wide will fit
between the bottom ch	ord and any other members	s, with BCDL = 10.0psf.	as where a recian	igie 3-0-0	A CAN BY 1-0-0 WILL WILLAL NONEED S
8) Provide mechanical co	nnection (by others) of trus	s to bearing plate capable of withsta	inding 100 lb uplift	at joint(s	s) 11 except (jt=lb)
<ul><li>9) This truss design requi</li></ul>	ires that a minimum of 7/16	structural wood sheathing be appli	ed directly to the to	op chord	and 1/2" gypsum
sheetrock be applied d	irectly to the bottom chord.	e mi i i i i i		1: 0	12///2024
warning !— Verify design LOAD CASE(S). Standard Vertically. Applicability of	parameters and read notes b design parameters and proper in	corporation of component is responsibility	parameters shown, ar of building designer	nd is for ar – not truss	a marviaual building component to be installed and loaded s designer or truss engineer. Bracing shown is for lateral support
of individual web members	only. Additional temporary bra	cing to ensure stability during construction	is the responsibility	of the erec	tor. Additional permanent bracing of the overall structure is the

responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE	271 ALDEN WAY ANGIER, NC		
24-B205-R01	R14	PIGGYBACK BASE	1	1		# 51883		
			Bup: 9,420 a Eab	12 2021 Dri	Job Reference (optional)	π <b>J+</b> 00 <b>J</b>	Dogo 1	
			ID:qqlfH?R	qemZ1wW	mxuKuRIUzBcTx-zLRil?sl?1n	ieiHitdkbhPeZqNHX?88nPeVx7_U	JyBI9L	
-0-10-	82-3-8 7-10-12	15-6-0	20-0-0	27-8-9	35-6	-0		
0-10-0	0 2-0-0	1-1-4	4-0-0	7-0-3	1-3	.,		
		5x8 =	<b>F</b> -0 —			Scale =	1:72.2	
		0.00 40	5x8 =					
1		8.00   12 7	<sup>8</sup>			т		
		тэ		ТЭ				
	3)	(8 // // // // // // // // // // // // //		- He				
	5x8 ~			、 、 、 、				
N	5		wø/	$\langle \rangle$	9 204 //	N		
+++++++++++++++++++++++++++++++++++++++	/		Ø/		A			
9	. ///	1 // WB		VV10	// 27	10		
4xo 🍫	3x4    26T1		//		W11 T4			
	W3	W4 W5 // //	/					
	3			\	$\langle   $			
ې 1 <sup>2</sup>			B5		B6			
	$ = - \frac{1}{2} $	17 5x8 =		40	<u>⊌</u> D0			
-	19	2x4    15 14	28	13	12 29 30			
4x4	4 = 6x6 =	3x4    6x8 =		3x8 =	4x4 =	4x0		
F	<u>2-3-8</u> <u>7-10-12</u> 2-3-8 <u>5-7-4</u>	13-6-0 15-6-0	25-6-0		35-6-0			
Plate Offsets (X,Y) [3:0-	-1-4,0-1-12], [5:0-2-10,0-1-	12], [7:0-5-8,0-1-12], [8:0-5-12,0-2-0	)], [16:0-2-4,0-2-12	], [18:0-4	5,0-3-4], [19:0-2-12,0-4-0]			
LOADING (psf)	SPACING	200 CSI	DEEL	in (le	) //defl //d			
TCLL (roof) 20.0	Plate Grip DOL	1.15 TC 0.87	Vert(LL)	-0.44 12-	14 >968 240	MT20 244/190		
Snow (Pt) 20.0 TCDI 10.0	Lumber DOL	1.15 BC 0.99	Vert(CT)	-0.69 12-	14 >621 180			
BCLL 0.0 *	Rep Stress Incr Code IRC2021/TP	YES WB 0.84 2014 Matrix-AS	Horz(CT)	0.24	11 n/a n/a	Weight: 228 lb FT = 20%	6	
BCDL 10.0								
LUMBER-	0		BRACING-	Ctructure	al wood aboathing directly a	applied		
BOT CHORD 2x4 SP No	.2 *Except*		BOT CHORD	Rigid ce	iling directly applied.	ipplied.		
B1: 2x6 SF	No.2, B5,B6: 2x4 SP No.1		WEBS	1 Řow a	t midpt 7-14, 8-14	1		
WEBS 2x4 SP No W2: 2x4 SI	.3 *Except* P No 2			MiTek	recommends that Stabilize	rs and required cross bracing		
WEDGE	110.2			Installa	tion guide.	in accordance with Stabilizer		
Right: 2x4 SP No.3					J			
REACTIONS. (Ib/size)	2=1473/0-3-8 (min. 0-1-12)	, 11=1419/0-3-8 (min. 0-1-13)						
Max Horz	2=210(LC 9)							
Max Uplift Max Grav	2=-106(LC 12), 11=-92(LC 2=1473(LC 1)_11=1530(LC	13) 25)						
		20)						
FORCES. (lb) - Max. Cor	mp./Max. Ten All forces 2	50 (lb) or less except when shown.	1991/196					
6-7=-176	1/224, 7-8=-1283/229, 8-9=	4291/407, 3-204233/489, 3-0	=-2063/183,					
11-27=-2	243/159							
BOT CHORD 2-19=-20 14-28=0/	9/1611, 18-19=-91//9/, 1/- 1271 13-28=0/1271 12-13	18=-192/2177, 16-17=-192/2177, 1 =0/1271_12-29=-77/1790_29-30=-`	5-16=-266/0, 77/1790					
11-30=-7	7/1790		,					
WEBS 5-17=0/3	09, 5-16=-831/219, 14-16=0	0/1535, 7-16=-116/1058, 7-14=-493	8/152, 8-12=-150/9	57,				
10-124	14 1/240, 3-10335/2600, 3	-191221/146, 5-16319/1927						
<b>NOTES-</b> (10)								
<ol> <li>Unbalanced root live lo</li> <li>Wind: ASCE 7-16: Vult</li> </ol>	ads have been considered	for this design. /asd=95mph; TCDI =5 0psf; BCDI ;	=5 0nsf: h=23ft: C:	at II: Exp	B: Enclosed: MW/ERS			
(envelope) gable end z	one and C-C Exterior(2E) -	0-10-8 to 3-11-2, Interior(1) 3-11-2	to 8-8-9, Exterior(2	R) 8-8-9 1	o 26-9-7, Interior(1)	MUMMMMM		
26-9-7 to 30-8-6, Exteri	ior(2E) 30-8-6 to 35-6-0 zor	e; cantilever left and right exposed	; end vertical left a	and right e	exposed;C-C for	ATH CARO		
3) TCLL: ASCE 7-16: Pr=	20.0 psf (roof LL: Lum DOL	=1.15 Plate DOL=1.15): Pf=20.0 ps	sf (Lum DOL=1.15	Plate DO	L=1.15): Is=1.0: Rough	OFESSION		
Cat B; Partially Exp.; C	e=1.0; Cs=1.00; Ct=1.10				in the second second	and Agent		
<ol> <li>This truss has been de non concurrent with oth</li> </ol>	signed for greater of min ro	of live load of 12.0 psf or 2.00 times	s flat roof load of 2	0.0 psf or	overhangs	SEAL		
5) Provide adequate drain	hage to prevent water pondi	ng.				28147		
6) This truss has been de	signed for a 10.0 psf bottor	n chord live load nonconcurrent with	h any other live loa	ids.				
/) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the potential of the potential								
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 11 except (jt=lb)								
2=106. 9) This trues design requires that a minimum of 7/16" structural wood sheathing he applied directly to the tap shord and 1/2" growing that a minimum of 7/16" structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the tap shord and 1/2" growing the structural wood sheathing he applied directly to the structural wood she								
9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord. 12/7/2024								
Warning !	parameters and read notes be	fore use. This design is based only upon	parameters shown, a	nd is for an	individual building component	to be installed and loaded		
Vertically. Applicability of	lesign parameters and proper in	corporation of component is responsibility	y of building designer	- not truss	designer or truss engineer. Bra	cing shown is for lateral support		
of individual web members	only. Additional temporary brac	cing to ensure stability during construction	n is the responsibility	of the erec	or. Additional permanent braci	ng of the overall structure is the		

Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDG	E   271 ALDEN WAY ANGIER. NC
24-B205-R01	R14A	Piggyback Base	2	1		# 51002
			Run: 8 430 c. Ech	12 2021 Dei	Job Reference (optional)	# <b>J400J</b>
-		45.0.0	ID:qqIfH?Rqen	nZ1wWm	kuKuRIUzBcTx-s6gC7Mvo3	BFI3Au0esZfeZUJXRuv24yE?Z7vL7FyBl9H
-0-1 0-1	0-8 2-3-8 7-10-12 0-8 2-3-8 5-7-4	7-7-4	<u>4-6-0</u>	7-8-9	<del>9 3</del>	<u>3-0-0 3φ-4-</u> δ '-9-7 0-10-8
		5×0 —				Scale = 1:73.4
		— 6XC	5x8 =			
		8.00 12 7	тз 8			
Ī		<b>H</b>				Ī
		тэ ////		<b>T</b> 2		
		3x8 🚧 🔰		, k	3v8	
	5x8	<i>6</i>		$\langle \rangle$	9 2x4 //	
		5	VV9/		10	-1-2
10-1		W6 🛛		wia	19	10-1
5x6	3 // 2×4 II 31T1		//		W12	<b>1</b> 32
	3x4    W3	W4 W5 // //	/			
	3		Re	```		
			B5 W	<b>[4</b> ]	B7	
1 <sup>1</sup> <sup>1</sup>	$\boxed{1}$ $\overrightarrow{3}$ $5x10 = \boxed{1}$	$6x8 = 2x4 \parallel 6x8 = 20 10$	1833 37 17	14	<b>38</b> 1315 35 36	
	6x6 =	2x = 7x8 =	2x4	3x8 =	= 4x4 =	5x6
	4x4 =	2x4 =	= 2x4		2x4 =	
	2-3-8 7-10-12	13-6-0 15-6-0	20-6-0	25-6-0	35-6-0	
Plate Offsets (X,Y) [2:0	<u>2-3-8 5-7-4</u> )-0-0,0-0-7], [3:0-1-4,0-2-8],	<u>-7-4</u> <u>-2-0-0</u> [5:0-2-10,0-1-12], [7:0-5-8,0-1-12], [	<u> </u>	5-0-0 1:0-2-4,0	-3-4], [23:0-4-2,0-3-3], [24	, 4:Edge,0-4-0]
LOADING (psf)	SPACING	2-0-0 CSI	DEF	in ///	c) l/defl L/d	
TCLL (roof) 20.0 Snow (Pf) 20.0	Plate Grip DOL	1.15 TC 0.80	Vert(LL)	-0.61	17 >702 240	MT20 244/190
TCDL 10.0	Lumber DOL Rep Stress Incr	1.15 BC 0.98	Vert(CT)	-0.95	17 >450 180 11 n/a n/a	
BCLL 0.0 * BCDI 10.0	Code IRC2021/TP	I2014 Matrix-AS	1012(01)	0.20	in ind ind	Weight: 243 lb FT = 20%
LUMBER-			BRACING-			
TOP CHORD 2x4 SP No	D.2 *Except*		TOP CHORD	Structura	al wood sheathing directl	y applied.
BOT CHORD 2x4 SP No	p.2 *Except*		BOT CHORD	5-10-0 o	ing directly applied. Exc ic bracing: 15-18	серг.
B1: 2x6 SI	P No.2, B2,B3: 2x4 SP No.1	, B5,B7: 2x4 SP SS	WEBS	1 Row a	t midpt 7-19, 8	-18
WEBS 2x4 SP No W2: 2x4 S	SP No.2			MiTek be inst	recommends that Stabiliz alled during truss erectio	zers and required cross bracing n. in accordance with Stabilizer
WEDGE Right: 2x4 SP No 3				Installa	ition guide.	
REACTIONS. (lb/size) Max Horz	2=1550/0-3-8 (min. 0-1-15) 2=-214(LC 10)	), 11=1579/0-3-8 (min. 0-2-2)				
Max Uplif	t2=-68(LC 12), 11=-53(LC 1	3)				
Max Grav	2=1623(LC 20), 11=1808(L	C 21)				
FORCES. (lb) - Max. Co	mp./Max. Ten All forces 2	250 (lb) or less except when shown.	212/106			
6-7=-20	92/144, 7-8=-1552/166, 8-9=	473/333, 3-314737/337, 3-02	=-2469/82,			
11-32=-2 BOT CHORD 2-24=-15	2649/57 59/1789 23-24=-65/867 22	-23=-116/2484 21-22=-116/2485 2	0-21=-398/0			
19-33=0	/1598, 17-33=0/1598, 17-34	=0/1598, 14-34=0/1598, 13-14=0/1	598, 13-35=0/2121	,		
35-36=0 WEBS 5-22=0/3	/2121, 11-36=0/2121 320, 5-21=-865/212, 19-21=	0/1864. 7-21=-99/1079. 7-19=-429/2	219. 8-15=-103/114	49.		
13-15=-	133/997, 10-13=-426/249, 3	-23=-240/3125, 3-24=-1310/107, 5-	23=-266/2059	- ,		
<b>NOTES-</b> (10)						
1) Unbalanced roof live lo	bads have been considered	for this design.	-5 Opef: h=23ft: Ca	t II. Evo	B. Enclosed: MW/ERS	ANNIHITAD.
(envelope) gable end 2	zone and C-C Exterior(2E) -	0-10-8 to 3-11-2, Interior(1) 3-11-2 to	o 8-8-9, Exterior(2)	R) 8-8-9 t	to 26-9-7, Interior(1)	MUNITH CARO
26-9-7 to 31-6-14, External forces &	erior(2E) 31-6-14 to 36-4-8 MWERS for reactions show	zone; cantilever left and right expose	ed ; end vertical lef	ft and righ	nt exposed;C-C for	OFESSIDA NATIN
3) TCLL: ASCE 7-16; Pr	=20.0 psf (roof LL: Lum DOL	.=1.15 Plate DOL=1.15); Pf=20.0 ps	f (Lum DOL=1.15	Plate DO	L=1.15); ls=1.0; Rough	and the second
Cat B; Partially Exp.; ( 4) This truss has been de	ce=1.0; Cs=1.00; Ct=1.10	of live load of 12.0 psf or 2 00 times	flat roof load of 20	),0 psf or	n overhangs	SEAL
non-concurrent with ot	her live loads.					28147
<ul><li>b) Provide adequate drai</li><li>6) This truss has been de</li></ul>	nage to prevent water pond esigned for a 10.0 psf bottor	ing. n chord live load nonconcurrent with	any other live loa	ds.	Anna	Non al I
7) * This truss has been	designed for a live load of 3	0.0psf on the bottom chord in all are	as where a rectan	gle 3-6-0	tall by 1-0-0 wide will fit	ARESOR
8) Provide mechanical co	ond and any other members onnection (by others) of trus	s, with BCDL = 10.0pst. s to bearing plate capable of withsta	nding 100 lb uplift	at joint(s	) 2, 11.	Manager Monant
9) This truss design requ	ires that a minimum of 7/16	structural wood sheathing be appli	ed directly to the to	op chord	and 1/2" gypsum	12/7/2024
Warning !—Verify design	anectly to the boltom chord.	efore use. This design is based only upon	parameters shown ar	nd is for an	individual building compone	nt to be installed and loaded
LOAD CASE(S) Standar	design parameters and proper in	corporation of component is responsibility	of building designer	– not truss	designer or truss engineer. I	Bracing shown is for lateral support
of individual web members	only. Additional temporary bra	cing to ensure stability during construction	is the responsibility	of the erect	tor. Additional permanent br	acing of the overall structure is the
Plate Connected Wood Tru	as Construction and BCSI 1-03	Guide to Good Practice for Handling, In.	stalling & Bracing of	Metal Pla	te Connected Wood Trusses	from Truss Plate Institute, 583

D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   27	71 ALDEN WAY ANGIER, NC
24-B205-R01	R14B	PIGGYBACK BASE	1	1		# 54883
			Run: 8.430 s Feb	 12 2021 Pr	JOD Reference (optional) int: 8.630 s Jul 12 2024 MiTek Industr	ies, Inc. Sat Dec 7 18:05:19 2024 Page 1
-Q-1	0 <sub>7</sub> 82-3-8 7-10-12	ال 	D:qqIfH?RqemZ1 <sup>:</sup> 0-0	wWmxuK 27-8-9	uRIUzBcTx-GhMLmOxhMAge2N 9	/kDXiDLB7L2R5wlHJ?RF57?kayBl9E
0-1	0-8 2-3-8 5-7-4	7-7-4 4-6	6-0	7-8-9	7-9-7	I
		5x8 =	<b>F</b> 0			Scale = 1:73.8
		8 00 12 7	5x8 =			
T			3 8			Ţ
	3	1x8 / T2 ////		12		
	578				3x8 📎	
2	0,0	6 5 (1) W	vø/	/	9 284 //	Ņ
-11-		We a 1		MAN	B	-11-
₹ 5x6	v ///			~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	31	7
	3x4    30 <sub>T1</sub>	W4 W5 // //			W12 14	
	3			/		
مب <sub>ت 1</sub> 2			86 85 Welde			1 HAVE
1 <sup>2</sup> 0	5x10 =	21 $6x8 = 173^{\circ}$	2 36	13	37/121/ 3/ 35	
	23	2x4    19 18 173	<sup>2</sup> 30 16	3x8 =	4x4 =	5x6
2	4x4 = 0x0 -	3x4 = 7x8 =	2x4    2x4	0,10	2x4 =	
		284 —	284			
	2-3-8 7-10-12	13-6-0 15-6-0 20	-6-0	25-6-0	35-6-0	
	2-3-8 5-7-4	5-7-4 2-0-0 5- 5-7-4 2-0-0 5-	0-0	5-0-0		
Plate Offsets (X, Y) [2:0	<u>)-0-0,0-0-7], [3:0-1-4,0-2-8],</u>	[5:0-2-10,0-1-12], [7:0-5-8,0-1-12], [8:0	)-5-12,0-2-0], [2	0:0-2-4,0	-3-4], [22:0-4-3,0-3-3], [23:E0	ge,0-4-0]
TCLL (roof) 20.0	SPACING-	2-0-0 <b>CSI.</b>	DEFL.	in (l	oc) I/defl L/d	PLATES GRIP
Snow (Pf) 20.0	Lumber DOL	1.15 BC 0.98	Vert(CT)	-0.95	16 >448 180	WIT20 244/190
BCLL 0.0 *	Rep Stress Incr Code IRC2021/TP	YES WB 0.87 2014 Matrix-AS	Horz(CT)	0.25	11 n/a n/a	Weight <sup>.</sup> 242 lb FT = 20%
BCDL 10.0		-				
TOP CHORD 2x4 SP No	0.2 *Except*	B T	OP CHORD	Structur	al wood sheathing directly ap	plied.
	P No.1	В	OT CHORD	Rigid ce	iling directly applied. Except:	
B01 CHORD 224 3F NG B1: 2x6 SI	P No.2, B2,B3: 2x4 SP No.1	, B5,B7: 2x4 SP SS W	<b>VEBS</b>	1 Row a	at midpt 7-18, 8-17	
WEBS 2x4 SP No W2: 2x4 SP	0.3 *Except* EP No 2			MiTek	recommends that Stabilizers	and required cross bracing
WEDGE	1 110.2			Installa	ation guide.	accordance with Stabilizer
Right: 2x4 SP No.3						
REACTIONS. (lb/size)	2=1550/0-3-8 (min. 0-1-15	), 11=1526/0-3-8 (min. 0-2-1)				
Max Uplifi	2=210(LC 11) t2=-68(LC 12), 11=-38(LC 1	3)				
Max Grav	2=1624(LC 20), 11=1759(L	C 21)				
FORCES. (lb) - Max. Co	mp./Max. Ten All forces 2	50 (lb) or less except when shown.				
TOP CHORD 2-3=-227 6-7=-209	/3/75, 3-4=-4487/259, 4-30= 93/149. 7-8=-1552/168. 8-9=	:-4768/344, 5-30=-4732/366, 5-6=-221 :-2408/163. 9-10=-2524/124. 10-31=-2	3/111, 471/88.			
11-31=-2 2 22= 40	2651/64	00- 105/0470 00 01- 405/0470 10 0	20- 200/0			
18-32=0	/1593, 16-32=0/1593, 16-33	=0/1593, 13-33=0/1593, 12-13=0/1593	20=-398/0, 3, 12-34=0/2123	8,		
34-35=0/3	/2123, 11-35=0/2123	0/1850 7 20- 104/1080 7 18- 420/21	10			
8-14=-10	)3/1151, 12-14=-134/1000,	10-12=-426/249, 3-22=-252/3117, 3-23	3=-1306/113,			
5-22=-27	/3/2054					
<b>NOTES-</b> (10)		for this desire				
2) Wind: ASCE 7-16; Vul	t=120mph (3-second gust) '	/asd=95mph; TCDL=5.0psf; BCDL=5.	0psf; h=23ft; Ca	it. II; Exp	B; Enclosed; MWFRS	TH CARD
(envelope) gable end z	zone and C-C Exterior(2E) -	0-10-8 to 3-11-2, Interior(1) 3-11-2 to 8	8-8-9, Exterior(2)	R) 8-8-9	to 26-9-7, Interior(1)	FESSIO No"
members and forces 8	MWFRS for reactions show	wn; Lumber DOL=1.60 plate grip DOL=	=1.60			and the state
3) TCLL: ASCE 7-16; Pr= Cat B: Partially Exp.: 0	=20.0 psf (roof LL: Lum DOL Ce=1.0: Cs=1.00: Ct=1.10	.=1.15 Plate DOL=1.15); Pf=20.0 psf (I	Lum DOL=1.15	Plate DC	DL=1.15); Is=1.0; Rough	SEAL
4) This truss has been de	esigned for greater of min ro	of live load of 12.0 psf or 2.00 times fla	at roof load of 20	0.0 psf oi	n overhangs 🔤 🚦	28147
5) Provide adequate drai	ner live loads. nage to prevent water pond	ng.				SALA A
6) This truss has been de	esigned for a 10.0 psf bottor	n chord live load nonconcurrent with an	ny other live loa	ds. de 3 6 0	tall by 1.0.0 wide will fit	ONE ORB UNIT
between the bottom ch	ord and any other members	s, with BCDL = $10.0$ psf.	where a rectari	gie 5-0-0		M. K. MOLIMAN
<ol> <li>8) Provide mechanical co</li> <li>9) This truss design regulation</li> </ol>	onnection (by others) of trus ires that a minimum of 7/16	s to bearing plate capable of withstand structural wood sheathing be applied	ling 100 lb uplift directly to the to	at joint(s	s) 2, 11. and 1/2" gypsum	12/7/2024
	iparameterseand read ables b	efore use. This design is based only upon par	rameters shown, ar	nd is for ar	individual building component to	be installed and loaded
Continued on page 2 vertically. Applicability of	design parameters and proper in	corporation of component is responsibility of	building designer	- not truss	s designer or truss engineer. Bracin	ng shown is for lateral support
of individual web members responsibility of the buildin	only. Additional temporary bra g designer. For general guidance	cing to ensure stability during construction is e regarding fabrication quality control storage	the responsibility of the responsibility of the responsibility of the rest of	of the erec	tor. Additional permanent bracing cing, consult ANSI/TPI 1 National	of the overall structure is the l Design Standard for Metal
Plate Connected Wood Tru	ss Construction and BCSI 1-03	Guide to Good Practice for Handling, Insta	lling & Bracing of	Metal Pla	te Connected Wood Trusses from	Truss Plate Institute, 583
D'Onofrio Drive, Madison,	WI 53719.					

	Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDEN WA	Y ANGIER, NC
	24-B205-R01	R14B	PIGGYBACK BASE	1	1	Job Reference (optional)	# 54883
1			Run	3430 s Feb	12 2021 Pri	nt: 8 630 s. Jul 12 2024 MiTek Industries Inc. Sat F	ec 7 18:05:19 2024 Page 2

LOAD CASE(S) Standard

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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.











Scale = 1:59.4

12/7/2024



I	<u> </u>	9-3-12		17-3-0			5-2-4			34-6-0	
Plate Offset	ts (X,Y) [4:0-4	-13,Edge], [6:0-4-13,Ed	ge]	7 11 4						0012	
LOADING (p TCLL (roof) Snow (Pf) TCDL BCLL BCDL	psf) 20.0 20.0 10.0 0.0 * 10.0	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/T	2-0-0 1.15 1.15 YES PI2014	<b>CSI</b> . TC BC WB Matri	0.67 0.73 0.44 x-AS	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) -0.19 10-12 -0.33 10-12 0.09 8	l/defl >999 >999 n/a	L/d 240 180 n/a	<b>PLATES</b> MT20 Weight: 186 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHOR BOT CHOR WEBS WEDGE Left: 2x4 SP	D 2x4 SP No.2 D 2x4 SP No.2 2x4 SP No.3 ? No.3 , Right: 2	2 2 3 2 2 3 2 2 3 2 2 3 2 2 3 2 2 3 2 2 3 2 2 3 2 2 3 2 3 2 3 2 3 3 2 3 3 2 3				BRACING- TOP CHORD BOT CHORD	Structural w Rigid ceiling MiTek reco be installed Installation	ood shea directly a ommends d during t guide.	thing directl applied. that Stabili russ erectio	y applied. zers and required cros n, in accordance with	ss bracing Stabilizer
REACTIONS	REACTIONS.       (lb/size)       2=1432/0-3-8       (min. 0-1-11),         Max Horz 2=-131(LC 10)       Max Uplift2=-67(LC 12),       8=-67(LC 13)										
FORCES. ( TOP CHOR BOT CHOR WEBS NOTES- ( 1) Unbalanc 2) Wind: AS (envelope Exterior(2 exposed; 3) TCLL: AS Cat B; Pa 4) This truss non-conc 5) Provide a 6) This truss 7) * This trus between 1 8) Provide n 9) This truss sheetrock	<ul> <li>(b) - Max. Com</li> <li>D 2-21=-205; 5-23=-196</li> <li>D 2-14=-143; 11-26=-38; 4-14=0/390</li> <li>(10)</li> <li>(10)</li> <li>(10)</li> <li>(10)</li> <li>(10)</li> <li>(10)</li> <li>(10)</li> <li>(10)</li> <li>(11)</li> <li>(11)&lt;</li></ul>	p./Max. Ten All forces 2/211, $3-21=-1940/231$ , 4/264, $6-23=-1962/265$ , (1636, 13-14=-114/1532) (1532, 10-11=-38/1532), 0, 4-12=-148/658, 5-12= ds have been considere 120mph (3-second gust ne and C-C Exterior(2E) 30-6-14, Exterior(2E) $30-6-14$ , Exterior (2E)	250 (lb) or le: 3-4=-1872/22 6-7=-1872/22 , 13-25=-114/ 8-10=-114/16 -561/189, 6-1 d for this desi ) Vasd=95mp -0-10-8 to 35-4-i 6-14 to 35-4-i 5 for reactions DL=1.15 Plate roof live load ding. om chord live 30.0psf on the rs, with BCDI iss to bearing 6" structural v i.	ss except w 3, 4-22=-19 3, 7-24=-19 1532, 12-25 36 2=-148/658 gn. n; TCDL=5.( 11-2, Exterio 3 zone; cant 3 zone; cant 3 zone; cant 5 shown; Lur DOL=1.15) of 12.0 psf c load noncor e bottom ch . = 10.0psf. plate capab wood sheath	hen shown. 62/265, 5-2: 40/231, 8-24 5=-114/1532 , 6-10=0/394 0psf; BCDL: or(2R) 3-11- tilever left an mber DOL= y; Pf=20.0 ps or 2.00 times hourrent with ord in all are ble of withsta- ning be appli	2=-1964/264, 4=-2052/211 , 12-26=-38/1532, ) =5.0psf; h=23ft; C 2 to 16-1-3, Interio dright exposed ; 1.60 plate grip DO sf (Lum DOL=1.15 s flat roof load of 2 h any other live loa eas where a rectar anding 100 lb uplif ed directly to the f	at. II; Exp B; E or(1) 16-1-3 to end vertical la L=1.60 i Plate DOL=1 20.0 psf on ov ads. ngle 3-6-0 tall it at joint(s) 2, top chord and	Enclosed; 18-4-13, eft and rig .15); Is=1 erhangs by 1-0-0 8. 1/2" gyps	ght 1.0; Rough wide will fit	SEAL 28147	and a state of the
LOAD CASE	<b>E(S)</b> Standard								In the second se	ARK K. MORRI	anna anna anna anna anna anna anna ann



of individual web members only. Additional temporary bracing to ensure stability during designed – not tuss designed of tuss engineer. Bracing shown is to hateral support responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDEN WAY ANGIER, NC
24-B205-R01	R24	Hip Girder	1	1	Job Reference (optional) # 54883
		Run	8.430 s Feb	12 2021 Pri	int: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Dec 7 18:05:49 2024 Page 3

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#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-9=-60, 9-12=-60, 22-25=-20

Concentrated Loads (lb) Vert: 18=-23(B) 20=-23(B) 19=-23(B) 4=-40(B) 5=-40(B) 9=-40(B) 14=-23(B) 28=-37(B) 29=-10(B) 30=-40(B) 31=-40(B) 32=-40(B) 33=-40(B) 34=-40(B) 35=-40(B) 51=-59(B) 52=-42(B)





	BCDL	10.0					
	LUMBER-				BRACING-		
	TOP CHORD BOT CHORD	2x4 SP No.2 2x4 SP No.3			TOP CHORD	Structural wood sheathing direct end verticals.	ly applied or 6-0-0 oc purlins, except
W	WEBS	2x4 SP No.3 2x4 SP No.3	3		BOT CHORD	Rigid ceiling directly applied or 6	-0-0 oc bracing.
	OTHERS				MiTek recommends that Stabil be installed during truss erection Installation guide.	izers and required cross bracing on, in accordance with Stabilizer	

REACTIONS. All bearings 12-0-0.

(lb) - Max Horz 20=120(LC 11) Max Uplift All uplift 100 lb or less at joint(s) 20, 12, 17, 15, 16 except 19=-112(LC 12), 13=-109(LC 13) Max Grav All reactions 250 lb or less at joint(s) 20, 12, 18, 19, 14, 13, 17, 15, 16

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-2-0, Corner(3R) 4-2-0 to 7-10-0, Corner(3E) 7-10-0 to 12-10-8 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

6) Provide adequate drainage to prevent water ponding.

All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

9) Truss to be tuing structure.
10) Gable studs spaced at 2-0-0 oc.
11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with end.
12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-0-0 tail by a fit between the bottom chord and any other members, with BCDL = 10.0psf.
13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 12, 17, 15, 18 except (jt=lb) 19=112, 13=109.

MORPHS MULTURE Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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D'Onofrio Drive Madison WI 53719

Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583

Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDEN WA	AY ANGIER, NC
24-B205-R01	R26	Hip Girder	1	2	Job Reference (optional)	# 54883
Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat Dec 7 18:05:53 ID:qqlfH?RqemZ1wWmxuKuRIUzBcTx-GEPCu0M0Ldyp7aLIWrqJZHIzHobTFoOR4						Dec 7 18:05:53 2024 Page 2 zHobTFoOR4NMowcyBl8

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-4=-60, 4-5=-60, 5-7=-60, 10-14=-20

Concentrated Loads (lb)

Vert: 19=-795(B) 20=-1399(B) 21=-1399(B) 23=-1399





BCDL	10.0				-
LUMBER-			BRACING-		
TOP CHORE	2x4 SP No.	2	TOP CHORD	Structural wood sheathing direct	ly applied or 6-0-0 oc purlins, except
BOT CHORE	2x4 SP No.	3		end verticals.	
WEBS	2x4 SP No.	3	BOT CHORD	Rigid ceiling directly applied or 6	-0-0 oc bracing.
OTHERS	2x4 SP No.3	3		MiTek recommends that Stabili be installed during truss erection Installation guide.	zers and required cross bracing n, in accordance with Stabilizer

#### REACTIONS. All bearings 12-3-8.

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(14)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-1-12, Corner(3R) 4-1-12 to 8-1-12, Corner(3E) 8-1-12 to 13-2-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry

Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

- 6) Provide adequate drainage to prevent water ponding.
- 7) All plates are 2x4 MT20 unless otherwise indicated.

8) Gable requires continuous bottom chord bearing.

Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

- A COPEO 9) Truss to be tuing structure.
  10) Gable studs spaced at 2-0-0 oc.
  11) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with end.
  12) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-0-0 tail by a fit between the bottom chord and any other members, with BCDL = 10.0psf.
  13) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 20, 12, 17, 15, 18 except (jt=lb) 19=115, 13=113.

MORPHS MULTURE Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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12/7/2024

<sup>(</sup>lb) - Max Horz 20=123(LC 11) Max Uplift All uplift 100 lb or less at joint(s) 20, 12, 17, 15, 16 except 19=-115(LC 12), 13=-113(LC 13) Max Grav All reactions 250 lb or less at joint(s) 20, 12, 18, 19, 14, 13, 17, 15, 16



Job	Truss	Truss Type	Qty	Ply	LOT 0.0015 CAMPBELL RIDGE   271 ALDEN WA	Y ANGIER, NC
24-B205-R01	R28	Hip Girder	1	2	Job Reference (optional)	# 54883
		Rui	: 8.430 s Feb qqlfH?Rqen	12 2021 Pri Z1wWmx	int: 8.630 s Jul 12 2024 MiTek Industries, Inc. Sat E uKuRIUzBcTx-kQza6MNe6w4gkkvx4YLY6V	0ec

#### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-3=-60, 3-4=-60, 4-6=-60, 9-13=-20

Concentrated Loads (lb)

Vert: 17=-1505(B) 19=-1498(B) 20=-1503(B) 21=-1503(B) 23=-1503(B)





Max Grav 5=79(LC 19), 3=210(LC 19), 4=449(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

LOAD CASE(S) Standard





Max Grav 5=76(LC 19), 3=154(LC 23), 4=266(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 3, 4.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 4=165/4-7-8 (min. 0-1-8), 3=165/4-7-8 (min. 0-1-8) Max Horz 4=-54(LC 8) Max Uplift4=-13(LC 13), 3=-8(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES- (8

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 3.

#### LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=85/2-10-0 (min. 0-1-8), 3=85/2-10-0 (min. 0-1-8) Max Horz 1=-21(LC 8) Max Uplift1=-4(LC 13), 3=-4(LC 12)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



- NOTES-(8)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 5-11-11, Exterior(2E) 5-11-11 to 10-9-4 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





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REACTIONS. (lb/size) 1=204/5-9-8 (min. 0-1-8), 3=204/5-9-8 (min. 0-1-8)
Max Horz 1=-50(LC 8)
Max Uplift1=-10(LC 12), 3=-10(LC 13)
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3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

<sup>2)</sup> Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



REACTIONS. (lb/size) 1=97/3-1-8 (min. 0-1-8), 3=97/3-1-8 (min. 0-1-8) Max Horz 1=24(LC 11) Max Uplift1=-5(LC 12), 3=-5(LC 13)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

<sup>1)</sup> Unbalanced roof live loads have been considered for this design.