Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

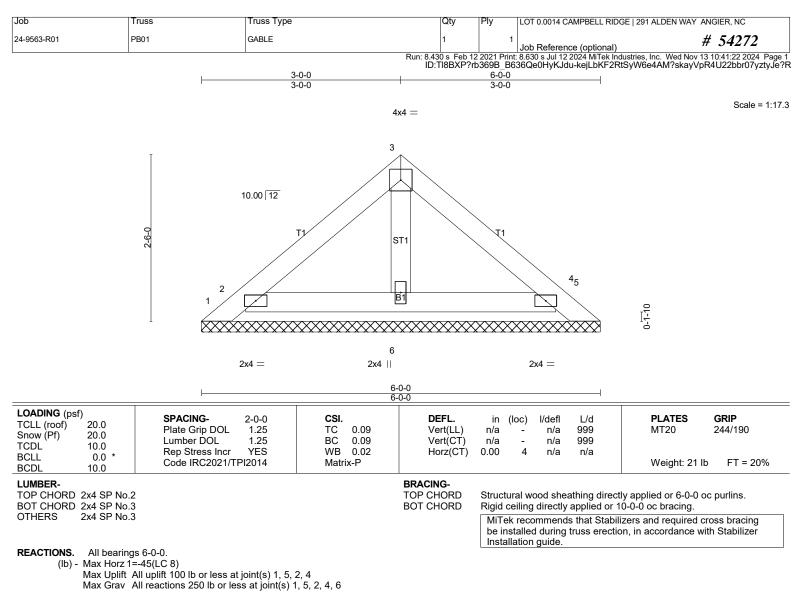
AST #: 54272 JOB: 24-9563-R01 JOB NAME: LOT 0.0014 CAMPBELL RIDGE Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2018 as well as IRC 2021. *37 Truss Design(s)*

Trusses:

PB01, PB02, PB03, PB04, PB05, R01, R02, R03, R04, R05, R06, R08, R09, R10, R11, R13, R14, R15, R17, R19, R20, R21, SP01, SP02, SP02A, V01, V02, V03, VT06, VT07, VT08, VT09,



Warning !--- Verify design parameters and read notes before use.



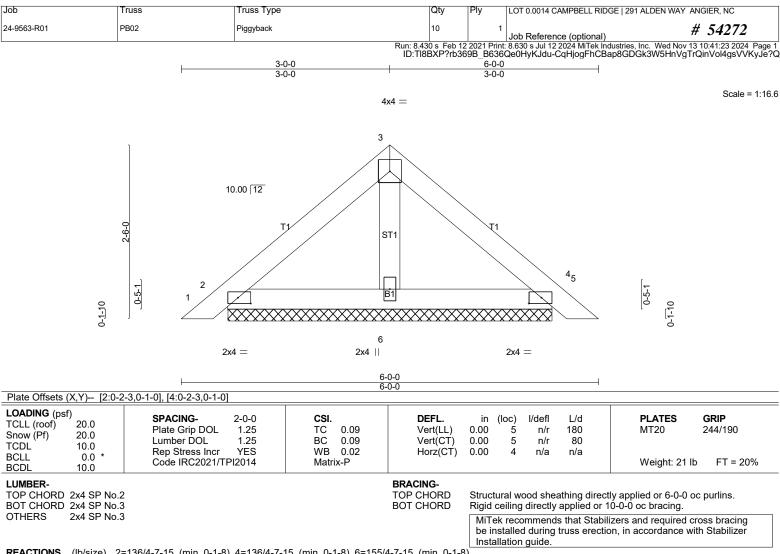
FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (11)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 2-0-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 2, 4.
- 10) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 2=136/4-7-15 (min. 0-1-8), 4=136/4-7-15 (min. 0-1-8), 6=155/4-7-15 (min. 0-1-8) Max Horz 2=-45(LC 10) Max Uplift2=-25(LC 12), 4=-31(LC 13)

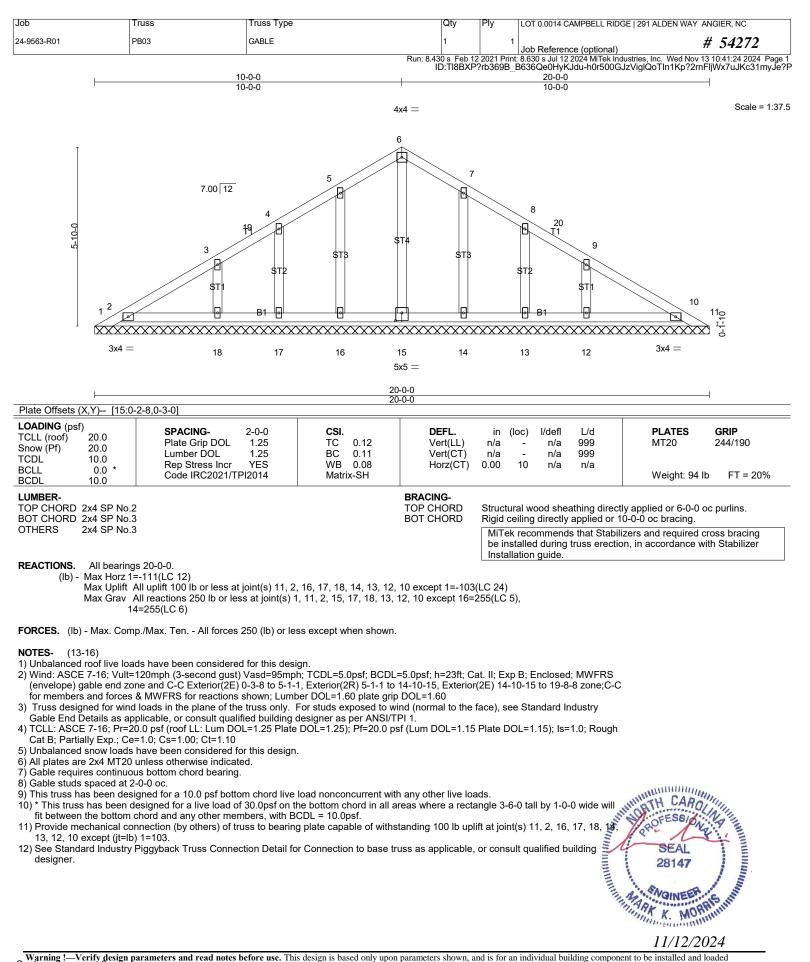
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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LOAD CASE(S) Standard
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Warning !---Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded Continued on page 2. We the state of the

Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALDEN WAY	ANGIER, NC
24-9563-R01	PB03	GABLE	1	1	Job Reference (optional)	# 54272
		Run: 8	430 s Feb 1	2 2021 Print	: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov	13 10:41:24 2024 Page 2

ID:TI8BXP?rb369B_B636Qe0HyKJdu-h0r500GJzViglQoTIn1Kp?2rnFljWx7uJKc31myJe?P 13) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 14) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

15) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 16) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



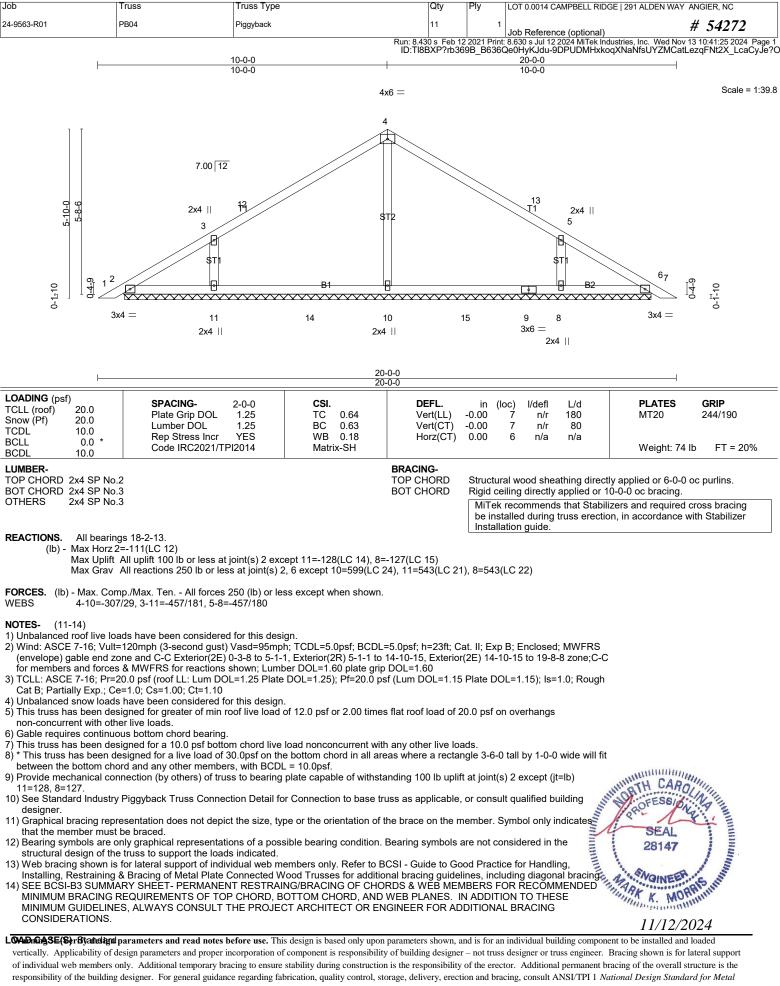
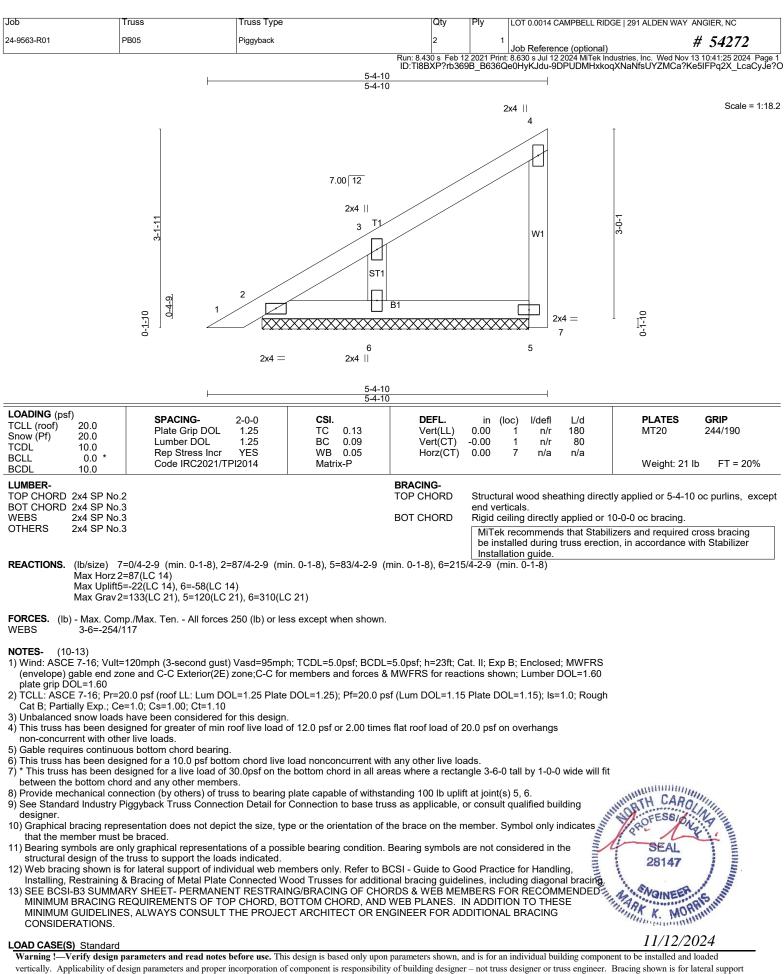


Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job 24-9563-R01		Truss R01	Truss Type GABLE		Qty 1	Ply 1	LOT 0.0014 CAMPBELL RIDG	#	54272
		- <u>0-11</u> 0-11-	0 3-3-12 5-4-5 6-0-14 0 3-3-12 2-0-9 0-8-9	' 2-0-0 '1-0 10-10 =	ID:TI8BXP?1 10-11-8 13-10-2 1-8 12-11-8 14-6 1-0 2-0-0 0-10-10 1-0 0-8 5x8 =	rb369B_B <u>1116-7-4</u> 2-0-9 9	t: 8.630 s Jul 12 2024 MiTek Ind 636Qe0HyKJdu-5bXEe2IBG <u>19-11-0 20-10-0</u> 3-3-12 0-11-0	ustries, Inc. Wed Nov 13 1 GQ4FcuX1zvb1RdgIqSa	10:41:27 2024 Page 1 aHj7DL?lqje5yJe?M Scale = 1:71.4
		4x6 // 2 4x6 // 2 1-0-7 1 	$\begin{array}{c} 00 \overline{12} \\ 39 \\ 31 \\ 31 \\ 4 \\ 39 \\ 31 \\ 4 \\ 39 \\ 4 \\ 4 \\ 39 \\ 4 \\ 39 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ 4 \\ $	23 23 4x6 = 5	$38 30$ $7 \\ 7 \\ 3-0-0$ $3-0-$	₩ 16 17 17 10x10 10x10 1 x6 =	3x4		
Plate Offsets	(X,Y) [2:0-	-2-14,0-2-0], [5:0-4-12,0-2	3-3-12 2-0-90-4-92	2-3-8 2-2-2	$+\frac{12-1-10}{2-2-2}$ $+\frac{14-5-2}{2-3-8}$ 2-0], [21:0-6-0,0-3]	16-7-4 2-2-2 -0]	19-11-0 3-3-12		
LOADING (ps TCLL (roof) Snow (Pf) TCDL BCLL BCDL		SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2021/TI	2-0-0 CSI 1.25 TC 1.25 BC YES WB	0.32 0.95	DEFL. Vert(LL) Vert(CT) Horz(CT) Attic	in (-0.31 -0.54 0.04 -0.28 16	21 >765 240 21 >434 180 13 n/a n/a	PLATES MT20 MT20HS Weight: 219 lb	GRIP 244/190 187/143 • FT = 20%
LUMBER- TOP CHORD BOT CHORD WEBS OTHERS REACTIONS.	2x4 SP SS B2: 2x4 SP 2x4 SP No W3,W6,W4 2x4 SP No (lb/size) 2 Max Horz 2	*Except* No.2 .3 *Except* 4: 2x4 SP No.2 .3 28=1169/0-3-0 (min. 0-1- 28=-127(LC 49)		n. 0-1-12)	BRACING- TOP CHORD BOT CHORD WEBS JOINTS	end ver Rigid co 2-2-0 o 1 Row a 1 Brace MiTek be ins	ral wood sheathing directl ticals. eiling directly applied or 2- c bracing: 16-26 at midpt 2-28, 1 a at Jt(s): 37, 38 c recommends that Stabili talled during truss erectio ation guide.	-2-0 oc bracing. Exce 1-13 zers and required cro	ept:
FORCES. (Ib TOP CHORD BOT CHORD WEBS	 Max. Cor 2-3=-739, 7-8=-466, 11-13=-1 25-27=-1 24-26=-2 16-17=-2 26-27=-7 9-30=-52 	04/640, 23-25=0/2980, 21 516/0, 22-24=-2516/0, 20-	250 (lb) or less except v 90/90, 4-5=-550/178, 5- -589/89, 10-40=-653/69 -23=0/4410, 19-21=0/44 22=-2516/0, 18-20=-25 -530/73, 29-37=-517/79 =0/1055, 20-21=0/387,	6=-466/149,), 10-11=-74 I10, 15-19=(17/0, 17-18=), 37-38=-51 [°] 24-25=-596/	6-7=-463/148, 1/0, 2-28=-1474/0, 0/2980, 14-15=0/54 -2517/0, 7/79, 30-38=-517/7	19,			
 2) Wind: ASC (envelope) zone;C-C fd 3) Truss desi- Gable End 4) TCLL: ASC Cat B; Parti 5) This truss f non-concur 6) Provide add 7) All plates a 8) All plates a 9) Gable stud: 10) This truss 11)* This truss 11)* This truss with the weet ontinue on of individual w responsibility 	d roof live lo E 7-16; Vult gable end z or members gned for win Details as a E 7-16; Pre- ially Exp.; C has been de rent with oth equate drain re MT20 pla re X44 MT20 s spaced at has been d ss has been with gables plicability of d veb members of of the building ted Wood Trus	esigned for a 10.0 psf bot designed for a live load or prhometers and roters design parameters and proper ionly. Additional temporary br g designer. For general guidan as Construction and BCSI 1-0	Vasd=95mph; TCDL=5 -0-9-7 to 4-0-3, Exterior reactions shown; Lumb bruss only. For studs e fied building designer as L=1.25 Plate DOL=1.25 oof live load of 12.0 psf ding. ated. ated. ad. com chord live load nond <u>30.0psf on the bottom of bifere.use. This design is bi- neorporation of component acing to ensure stability duri- ce regarding fabrication, qua</u>	(2R) 4-0-3 tc er DOL=1.6(exposed to w s per ANSI/T); Pf=20.0 p or 2.00 time concurrent w chord in all a ased only upor is responsibilit ing constructio ality control, st	 b) 15-10-13, Exterio c) plate grip DOL=1 c) plate grip DOL=1 c) plate grip DOL=1 c) plate grip DOL=1 c) sf (Lum DOL=1.15 c) s flat roof load of 2 c) sith any other live load of 2 c)	r(2E) 15- .60 face), se Plate D0 20.0 psf c 20.0 psf c	-0 tall by 1-0-0 wide will n overhangs n individual building compone s designer or truss engineer. I ctor. Additional permanent br acing, consult ANSI/TPI 1 <i>Na</i>	Bracing shown is for late acing of the overall struc tional Design Standard j	24 ded ral support true is the for Metal

Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALDEN WAY ANGIER, NC
24-9563-R01	R01	GABLE	1	1	Job Reference (optional) # 54272
					8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov 13 10:41:27 2024 Page 2 36Qe0HyKJdu-5bXEe2IBGQ4FcuX1zvb1RdgIqSaHj7DL?Iqje5yJe?I

NOTES- (16)

- 12) Ceiling dead load (5.0 psf) on member(s). 3-4, 9-10, 4-29, 29-37, 37-38, 30-38, 9-30
- 13) Bottom chord live load (40.0 psf) and additional bottom chord dead load (10.0 psf) applied only to room. 24-26, 22-24, 20-22, 18-20, 17-18, 16-17

14) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 59 lb down and 39 lb up at 19-4-4 on bottom chord. The design/selection of such connection device(s) is the responsibility of others. 15) Attic room checked for L/360 deflection.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-2=-60, 2-3=-60, 3-4=-70, 4-5=-60, 5-8=-60, 8-9=-60, 9-10=-70, 10-11=-60, 11-12=-60, 13-28=-20, 16-26=-40, 4-9=-10 Concentrated Loads (lb)

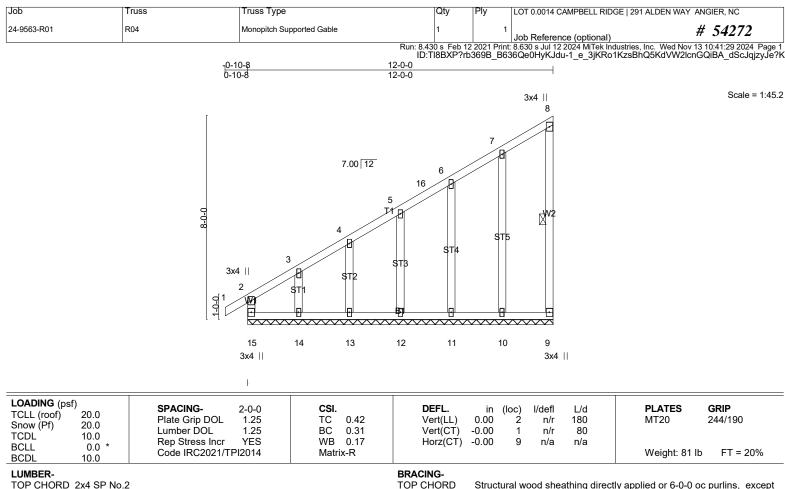
Vert: 41=-59



Job		russ	Truss Type	Qty	Ply		GE 291 ALDEN WAY ANGIER, NC
24-9563-R01		802	Attic	1	1		# 54272
				Run: 8.430 s Feb 1	2 2021 Print	Job Reference (optional) : 8.630 s Jul 12 2024 MiTek Inc	dustries, Inc. Wed Nov 13 10:41:27 2024 Page 1
		-0-11-	6-11-8 <u>) 3-3-12 ₁ 5-4-5 6-0-14 1</u> 1	14-6-	-11 —		GQ4FcuX1zvb1RdglrSaHj7CL?lqje5yJe?N
		- <u>0-11-</u> 0-11-0	0-10-10	0-8-	-9	<u>19-11-0 20-10-</u> 0 3-3-12 0-11-0	01- 4.74.4
			5x8 =	5x8 =			Scale = 1:71.4
		[5		7	J	
		10.			7		
			29 27 3 _T	28	30) _8	
		4x6 🥢			R.	1 4x6 <	
		2		_			
		1 11-8-0		9-2-4		9 10 11 80	
				3-0-0			
		5-10-7 <		3-0-0	vs	W1 1-01-5	
		2 2	W4 🕅 🕅 🕅				
			Bt 22 V2004			<u></u>	
			26 25 23 21		10x10 = 12	Ŕ	
		3:	4 10x10 = 6x6 = 5x12	MT20HS= 6	x6 =	3x4	
				5x8 = 4x6 =			
			5-5-14 <u>3-3-12 5-4-5 7-9-6 9-11-8</u> <u>3-3-12 2-0-90-1-9 2-3-8 2-2-2</u>	<u>12-1-10</u> <u>14-5-2</u> <u>2-2-2</u> <u>2-3-8</u>	16-7-4	<u>19-11-0</u> 3-3-12	
		-14,0-2-0], [5:0-4-12,0-2-	12], [6:0-4-12,0-2-12], [9:0-2-14,0-2				
LOADING (pst TCLL (roof)	f) 20.0	SPACING- Plate Grip DOL	2-0-0 CSI. 1.25 TC 0.32	DEFL. Vert(LL)	in (lo -0.31	oc) l/defl L/d 19 >765 240	PLATES GRIP MT20 244/190
Snow (Pf) TCDL	20.0 10.0	Lumber DOL	1.25 BC 0.95	Vert(CT)	-0.54	19 >434 180	MT20HS 187/143
BCLL BCDL	0.0 * 10.0	Rep Stress Incr Code IRC2021/TF	YES WB 0.76 I2014 Matrix-SH	Horz(CT) Attic	0.04 -0.28 14-	11 n/a n/a 24 566 360	Weight: 197 lb FT = 20%
LUMBER-	10.0			BRACING-			
TOP CHORD BOT CHORD				TOP CHORD	Structur end vert		ly applied or 6-0-0 oc purlins, except
WEBS	B2: 2x4 SP M 2x4 SP No.3			BOT CHORD		iling directly applied or 2 bracing: 14-24	2-2-0 oc bracing. Except:
WEBG		2x4 SP No.2		WEBS	1 Row a	t midpt 4-7, 2-	26, 9-11
							izers and required cross bracing on, in accordance with Stabilizer
REACTIONS.	(lb/size) 26	6=1167/0-3-0 (min. 0-1-1	1), 11=1167/0-3-0 (min. 0-1-11)		Installa	ation guide.	
	Max Horz 26	6=-127(LC 10) 6=1450(LC 3), 11=1450(L					
TOP CHORD	2-3=-738/0	, 3-29=-653/69, 4-29=-58	250 (lb) or less except when shown 9/89, 4-5=-528/166, 5-6=-443/137,				
BOT CHORD			′38/0, 2-26=-1472/0, 9-11=-1472/0 21=0/4410, 17-19=0/4410, 13-17=0	0/2981, 12-13=0/54	48,		
	22-24=-251 14-15=-251		20=-2518/0, 16-18=-2518/0, 15-16=	2518/0,			
WEBS			-537/57, 27-28=-527/59, 7-28=-537 596/0, 13-15=-596/0, 23-24=0/270				
		04/0, 13-14=0/2700		,			
NOTES- (13		de la construcción de la const	for this design				
2) Wind ASC	F 7-16 [.] Vult=	ds have been considered 120mph (3-second gust)	Vasd=95mph ⁻ TCDI =5 0psf ⁻ BCDI	=5.0psf; h=23ft; C	at. II; Exp	B; Enclosed; MWFRS	
(envelope) zone;C-C fo	gable end zor or members a	ne and C-C Exterior(2E) nd forces & MWFRS for	0-9-7 to 4-0-3, Exterior(2R) 4-0-3 to reactions shown; Lumber DOL=1.6	o 15-10-13, Exterio 0 plate grip DOL=1	or(2E) 15-′ ⊨.60	10-13 to 20-8-7	AND
 TCLL: ASC Cat B: Parti 	E 7-16; Pr=20 allv Exp.: Ce=	0.0 psf (roof LL: Lum DO =1.0: Cs=1.00: Ct=1.10	_=1.25 Plate DOL=1.25); Pf=20.0 p	osf (Lum DOL=1.15	Plate DC	L=1.15); ls=1.0; Rough	IN GTH CAHOLIN
4) This truss h	as been desi	gned for greater of min ro	oof live load of 12.0 psf or 2.00 time	es flat roof load of 2	20.0 psf or	n overhangs	2HOT NO. THE
5) Provide ade	equate draina	ge to prevent water pond	ing.			Internet	SEAL
7) All plates al	re 2x4 MT20 piale	unless otherwise indicate	id.			11HU	28147
8) This truss h9) * This truss	has been desi has been des	gned for a 10.0 pst botto signed for a live load of 3	n chord live load nonconcurrent wi 0.0psf on the bottom chord in all ar	th any other live loa reas where a rectai	ads. ngle 3-6-0	tall by 1-0-0 wide will fit	AN NOINEER C
between the 10) Ceiling de	e bottom chor ad load (5.0 p	d and any other member osf) on member(s). 3-4, 7	0-9-7 to 4-0-3, Exterior(2R) 4-0-3 to reactions shown; Lumber DOL=1.6 _=1.25 Plate DOL=1.25); Pf=20.0 p pof live load of 12.0 psf or 2.00 time ing. ated. id. n chord live load nonconcurrent wi 0.0psf on the bottom chord in all ar s. -8, 4-27, 27-28, 7-28			3	ALK MORRININ
11) Bottom ch 15-16, 14-	ord live load ((40.0 psf) and additional	bottom chord dead load (10.0 psf) a	applied only to roor	m. 22-24, 1	20-22, 18-20, 16-18,	SEAL 28147 11/12/2024 ent to be installed and loaded
12) Attic room	checked for I	L/360 deflection.	afono uso. This design is transformer	n norometors -1	nd is form	individual building	11/12/2024
			efore use. This design is based only upor accorporation of component is responsibili			8 1	
of individual w	veb members on	ly. Additional temporary bra	cing to ensure stability during construction	on is the responsibility	of the erec	tor. Additional permanent be	racing of the overall structure is the

of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	[Truss	Truss Type		Qty	Ply	LOT 0.0014 CAMPBELL RIDO	GE 291 ALDEN WAY ANGIER, NC
24-9563-R01		R03	ATTIC		9	1	lob Doference (aution 1)	# 54272
					Run: 8.430 s Feb 1	2 2021 Prin	Job Reference (optional) t: 8.630 s Jul 12 2024 MiTek Ind	lustries, Inc. Wed Nov 13 10:41:28 2024 Page
				6-11-8	14-6	⊢11 [—]		E16EXd6G_rCTcswWSaTUEyaGAXyJe
			<u>3-3-12</u> 3-3-12	2-0-9 0-8-9	<u>2-11-8 13-10-2</u> 6-0-0 0-10-10	2-0-9	<u> 19-11-0 </u>	
				0-10-10 5x8 =	0-٤ = 5x8			Scale = 1:71
				4	5 T2			
			10.00 12	3		6		
				27 25	<u>₩6</u>	28		
			2 5	и		M	7	
		4x6 🧹	. //	3		B	4x6 📎	
			1				8 9	
		11-8-0	X		9-2-4		11-8-0	
					6			
		2-0	W1 N	/ <u>3</u> 1	3-0-0	w	3 / 1	
		5-10-7	w w				3 W1	
				W5 W4 🖂 🖂 🖂	<u>B2</u> 🛛 🖂	<u>⊠ 12</u>		
				BT 20 VE 8W4	WH6W4 MA			
				23 21 19	17 15	$10\dot{x}\dot{1}\dot{0} = 11$	0 9	
			3x4 10x	x10 = 6x6 = 5x12	MT20HS=	3x6 =	3x4	
				4x6 =	5x8 = 4x6 =			
			3-3-12	5-5-14 <u>5-4-5 7-9-6 </u> 9-11-8		16-7-4	19-11-0	
Plate Offsets	(X,Y) [4:0-4	4-12,0-2-12], [5:0-4-12,0	<u>3-3-12</u>)-2-12], [17:0-6	<u>' 2-0-90-4-9 2-3-8 ' 2-2-2</u> 6-0,0-3-0]	2-2-2 2-3-8	2-2-2	3-3-12	
LOADING (ps		SPACING-	2-0-0	CSI.	DEFL.	in (loc) l/defl L/d	PLATES GRIP
TCLL (roof) Snow (Pf)	20.0 20.0	Plate Grip DOL	1.25	TC 0.32	Vert(LL)	-0.31	17 >765 240	MT20 244/190
TCDL	10.0	Lumber DOL Rep Stress Incr	1.25 YES	BC 0.95 WB 0.76	Vert(CT) Horz(CT)	-0.54 0.04	17 >434 180 9 n/a n/a	MT20HS 187/143
BCLL BCDL	0.0 * 10.0	Code IRC2021/T		Matrix-SH	Attic	-0.28 12		Weight: 192 lb FT = 20%
LUMBER-					BRACING-			
TOP CHORD BOT CHORD					TOP CHORD	Structure end ver		y applied or 6-0-0 oc purlins, except
WEBS	B2: 2x4 SP 2x4 SP No.3				BOT CHORD		eiling directly applied or 2 c bracing: 12-22	-2-0 oc bracing. Except:
WED3		: 2x4 SP No.2			WEBS		at midpt 3-6, 1-2	24, 8-9
								zers and required cross bracing n, in accordance with Stabilizer
							ation guide.	
REACTIONS.		4=1112/0-3-0 (min. 0-1- 4=-112(LC 8)	-10), 9=1112/0)-3-0 (min. 0-1-10)				
		4=1403(LC 3), 9=1403(I	LC 3)					
FORCES. (It	o) - Max. Com	p./Max. Ten All forces	s 250 (lb) or le	ss except when shown.				
TOP CHORD), 2-27=-655/53, 3-27=-{ /75, 7-28=-655/53, 7-8=	,	, , ,	5-6=-524/169,			
BOT CHORD	21-23=-10	6/634, 19-21=0/2982, 1	7-19=0/4412,	15-17=0/4412, 11-15=0		52,		
	20-22=-25 12-13=-25	18/0, 18-20=-2518/0, 16 18/0	5-18=-2518/0,	14-16=-2518/0, 13-14=	-2518/0,			
WEBS	22-23=-76	5/0, 10-12=-765/0, 3-25						
		45, 16-17=0/387, 20-21 04/0, 11-12=0/2700	596/0, 11-13	5596/0, 21-22-0/2700), 10-191504/0,			
NOTES- (1	2)							
1) Unbalance	d [´] roof live loa	ds have been considere			5 0			
		120mph (3-second gust one and C-C Exterior(2E						
zone;C-C f	or members a	and forces & MWFRS fo	or reactions sho	own; Lumber DÓL=1.6) plate grip DOL=1	.60 ` Bloto D(N -1 15): la-1 0: Pough	WHITH CARO
Cat B; Parl	tially Exp.; Ce	=1.0; Cs=1.00; Ct=1.10		DOL-1.23), 1 1-20.0 p		i late De	DE- 1.10), 13- 1.0, 1000gl	FESSIO Nall
4) Provide ad	equate draina ire MT20 plat	age to prevent water por es unless otherwise indi	nding. icated.				Inn	and the set
All plates a	re 2x4 MT20	unless otherwise indica	ited.	lood popoono	h ony other live !-	ada		SEAL
5) All plates a 6) All plates a 7) This trues b	ias neeli ües	esigned for a live load of	00.0051 011 01	e bottom chord in all ar	eas where a recta	aus. ngle 3-6-() tall by 1-0-0 wide wij fit	28147
	nus seen ue		ers				IIII	N
between th	e bottom cho	rd and any other membersf) on member(s) 2-3 6	3.7 3.05 05 0	6 6-26			all.	A'L SALOW CA ! S
between th9) Ceiling dea10) Bottom ch	e bottom cho ad load (5.0 p hord live load	rd and any other membe sf) on member(s). 2-3, 6 (40.0 psf) and additiona	6-7, 3-25, 25-2	6, 6-26 d dead load (10.0 psf) a	applied only to roo	n. 20-22,	18-20, 16-18, 14-16,	A NOINEER OR SUIT
between th 9) Ceiling dea 10) Bottom ch 13-14, 12	e bottom cho ad load (5.0 p nord live load -13	sf) on member(s). 2-3, 6 (40.0 psf) and additiona	6-7, 3-25, 25-2	6, 6-26 d dead load (10.0 psf) a	applied only to room	n. 20-22,	18-20, 16-18, 14-16,	A MOREER SHITT
between th 9) Ceiling dea 10) Bottom ch 13-14, 12	e bottom cho ad load (5.0 p hord live load -13 h checked for	sf) on member(s). 2-3, 6	6-7, 3-25, 25-2	6, 6-26 I dead load (10.0 psf) a	applied only to room	n. 20-22,	18-20, 16-18, 14-16,	SEAL 28147 11/12/2024



BOT CHORD

WFBS

BOT CHORD 2x4 SP No.3 WFBS 2x4 SP No.3 2x4 SP No.3 OTHERS

Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing. 1 Row at midpt 8-9

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. All bearings 12-0-0.

- (lb) Max Horz 15=224(LC 14) Max Uplift All uplift 100 lb or less at joint(s) 15, 9, 12, 13, 11, 10 except 14=-177(LC 14) Max Grav All reactions 250 lb or less at joint(s) 15, 9, 12, 13, 14 except 11=281(LC 5), 10=299(LC 5)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

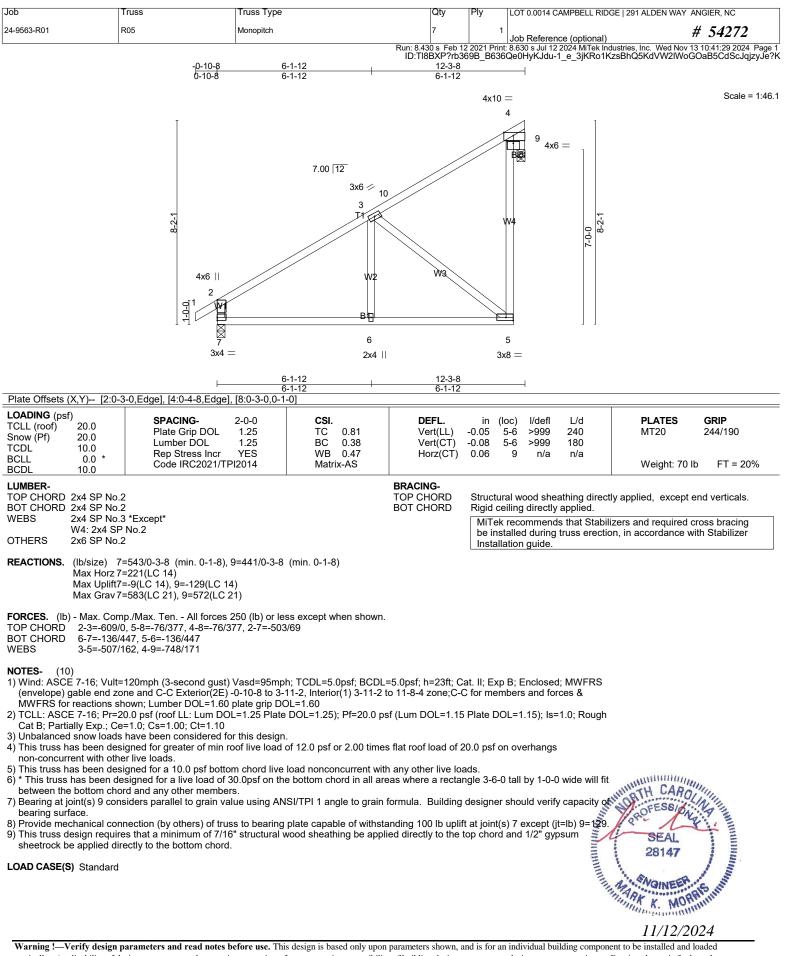
TOP CHORD 2-3=-386/175. 3-4=-266/122

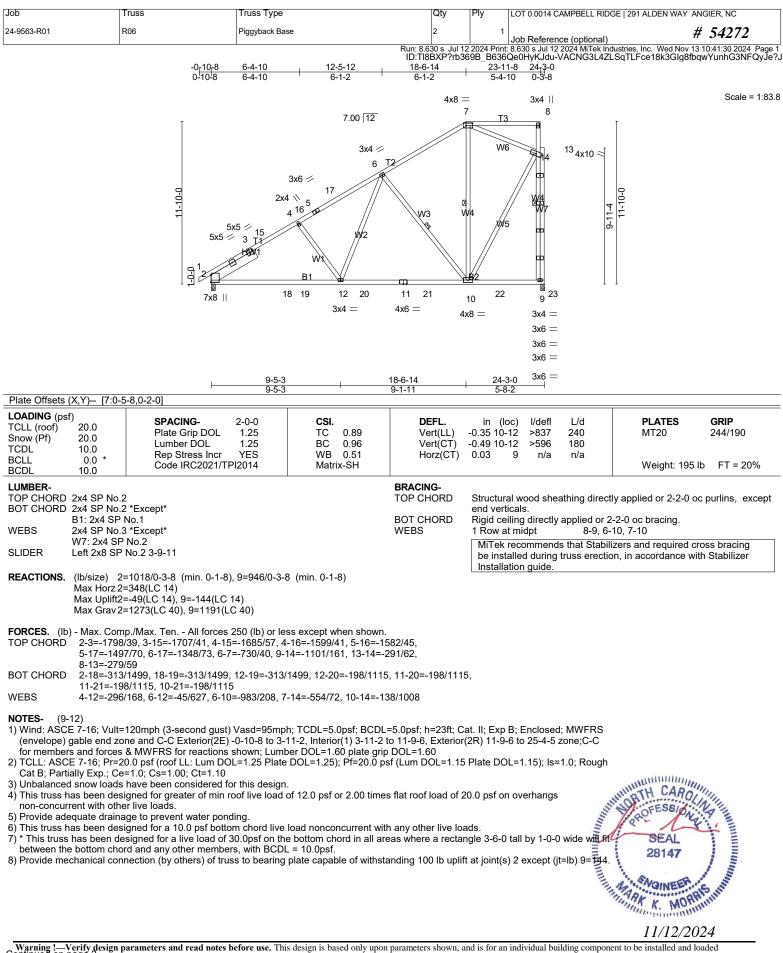
NOTES-(13)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Exterior(2N) 4-0-0 to 7-0-10, Corner(3E) 7-0-10 to 11-10-4 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- 8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A Decode against lateral movement (i.e. diagonal web).
 I his truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will be used for a members, with BCDL = 10.0psf.
 Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at lateration.
 * D CASE(P) of (11)
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 9, 12, 13, 11, 20

LOAD CASE(S) Standard

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Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 AI	LDEN WAY ANGIER, NC
24-9563-R01	R06	Piggyback Base	2	1	Job Reference (optional)	# 54272
					: 8.630 s Jul 12 2024 MiTek Industries, Ind	

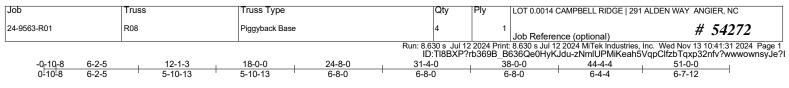
ID:TI8BXP?rb369B_B636Qe0HyK.du-VACNG3L4ZLSqTLFce18k3Glg8fbqwYunhG3NFQyJe?J 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 10) Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

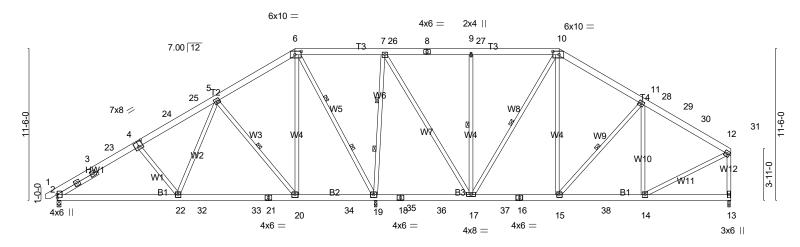
 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Scale = 1:87.2



⊢	9-1-12		18-0-0	24-1		31-4-0	38-0-0	44-4-4	51-0-0	
Plate Offsets	9-1-12 (X Y) [2:0-3		8-10-4 -0,0-4-8], [6:0-5-12,0	6-1- -3-01 [10:0-		7-2-12	6-8-0	6-4-4	6-7-12	
	<u>, , , , ,</u>	2,00 - 1, [0 -	0,0 4 0], [0.0 0 12,0		,00					
LOADING (ps TCLL (roof) Snow (Pf)	it) 20.0 20.0	SPACING Plate Grip	DOL 1.25	CSI. TC	0.45	DEFL. Vert(LL)	in (loc) l/defl -0.12 20-22 >999	240	PLATES MT20	GRIP 244/190
TCDL`´ BCLL	10.0 0.0 *	Lumber E Rep Stres Code IRC			0.47 0.84 ix-SH	Vert(CT) Horz(CT)	-0.17 20-22 >999 0.03 13 n/a		Weight: 447 lb	FT = 20%
BCDL	10.0									
LUMBER- TOP CHORD BOT CHORD WEBS		2				BRACING- TOP CHORD BOT CHORD	end verticals.		y applied or 6-0-0 oc)-0-0 oc bracing, Ex	
SLIDER	Left 2x4 SP					WEBS	6-0-0 oc bracing: 1 1 Row at midpt	17-19.	0.	
						WEB5	2 Rows at 1/3 pts	5-20, 9- 6-19, 7-	-17, 10-17, 11-15 -19	
								g truss erectior	zers and required cro n, in accordance with	
REACTIONS.	Max Horz 2= Max Uplift2=	=206(LC 11) =-73(LC 14), 19=	. 0-1-8), 19=2262/0- =-103(LC 11), 13=-8′ 9=2853(LC 44), 13=	(LC 15)	,-	962/0-3-8 (min. 0-1-	8)			
FORCES. (IL TOP CHORD	2-3=-1493/ 5-25=-105 9-27=-569/	87, 3-23=-1410/ 5/116, 5-6=-444/ 160, 9-10=-569/	ll forces 250 (lb) or le /89, 4-23=-1337/105, /122, 6-7=0/411, 7-20 /160, 10-11=-907/163 011/83, 12-31=-1078	4-24=-1282 6=-569/160, 3, 11-28=-88	/99, 24-2 8-26=-56 4/103, 2	25=-1170/100, 39/160, 8-27=-569/16 8-29=-892/99,	0,			
BOT CHORD	2-22=-213/ 20-34=-54/ 17-36=-336	(1299, 22-32=-1) (337, 19-34=-54/ 6/105, 17-37=-9/	04/881, 32-33=-104/ /337, 19-35=-336/10 /719, 16-37=-9/719,	381, 21-33=- 5, 18-35=-33	104/881 6/105, 1	, 20-21=-104/881, 8-36=-336/105,				
WEBS	7-19=-1766	159, 5-22=-38/6 6/186, 7-17=-72/	613, 5-20=-946/198, (/1262, 9-17=-724/14 00/92, 12-14=-37/95	3. 10-17=-66	9/56.10	-15=-38/624.				
NOTES- (1 1) Unbalance 2) Wind: ASC (envelope) 25-2-9 to 3 forces & M 3) TCLL: ASC Cat B: Part	1-14) d roof live load E 7-16; Vult= gable end zoi 0-9-7, Exterioi WFRS for rea E 7-16; Pr=20 jally Exp.: Ces	ds have been co 120mph (3-seco ne and C-C Exte r(2R) 30-9-7 to 4 ctions shown; L ctions shown; L 0.0 psf (roof LL: =1.0; Cs=1.00: C	onsidered for this des and gust) Vasd=95mp erior(2E) -0-8-8 to 4-4 45-2-9, Interior(1) 45- umber DOL=1.60 pla Lum DOL=1.25 Plat 2t=1.10	ign. bh; TCDL=5. I-11, Interior 2-9 to 45-9- ite grip DOL e DOL=1.25	0psf; BC (1) 4-4-1 1, Exteric =1.60); Pf=20.0	DL=5.0psf; h=23ft; C 1 to 10-9-7, Exterior(or(2E) 45-9-1 to 50-1(0 psf (Lum DOL=1.15 mes flat roof load of 2	at. II; Exp B; Enclos 2R) 10-9-7 to 25-2-9 0-4 zone;C-C for me 5 Plate DOL=1.15); I	ed; MWFRS , Interior(1) mbers and s=1.0; Rough	SEAL 28147	Minimum Minimum
4) Unbalance 5) This truss I non-concu	d snow loads has been desi rrent with othe	have been cons gned for greater r live loads.	idered for this design of min roof live load	n. of 12.0 psf o	or 2.00 ti	mes flat roof load of 2	20.0 psf on overhang	gs annun	ARA K. MORR	
<i>i j A</i> li plates a	10 373 101 20		s maicateu.						11/12/202	01
						with any other live lo		.1.1.		
Warning !— Continued on Vertically. Ap	verity design p page 2 plicability of de	arameters and re- sign parameters an	ad notes before use. The d proper incorporation of the design of the des	1s design is ba f component i	sed only u s responsil	pon parameters shown, a pility of building designe	and 1s for an individual b r – not truss designer or	truss engineer. E	nt to be installed and loa Bracing shown is for late	ded ral support
of individual	web members on	ly. Additional terr	porary bracing to ensure	e stability duri	ng constru	ction is the responsibility	of the erector. Additio	nal permanent bra	acing of the overall struc	ture is the

responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

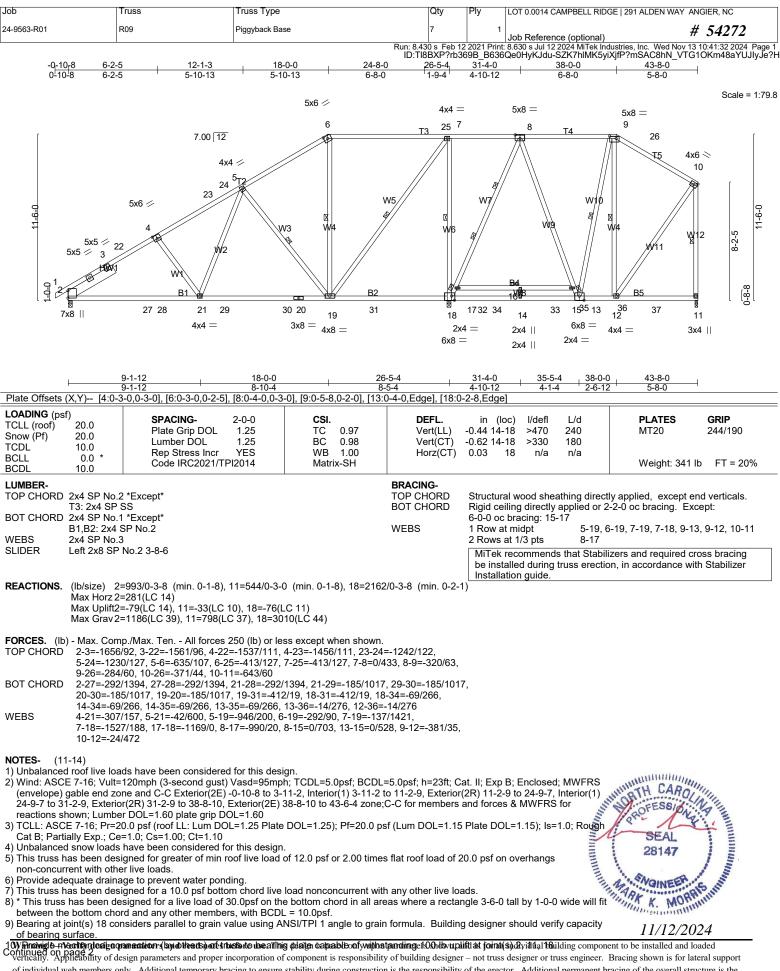
[Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALDEN WAY ANGIER, NC
	24-9563-R01	R08	Piggyback Base	4	1	Job Reference (optional) # 54272
						8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov 13 10:41:31 2024 Page 2 636Qe0HyKJdu-zNmIUPMiKeah5VqpClfzbTqxp32nfv?wwwownsyJe?I

NOTES- (11-14)

- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 13 except (it=lb) 19=103.
- 11) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 12) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated
- 13) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALDEN W	/AY ANGIER, NC
24-9563-R01	R09	Piggyback Base	7	1	Job Reference (optional)	# 54272
					: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed I	

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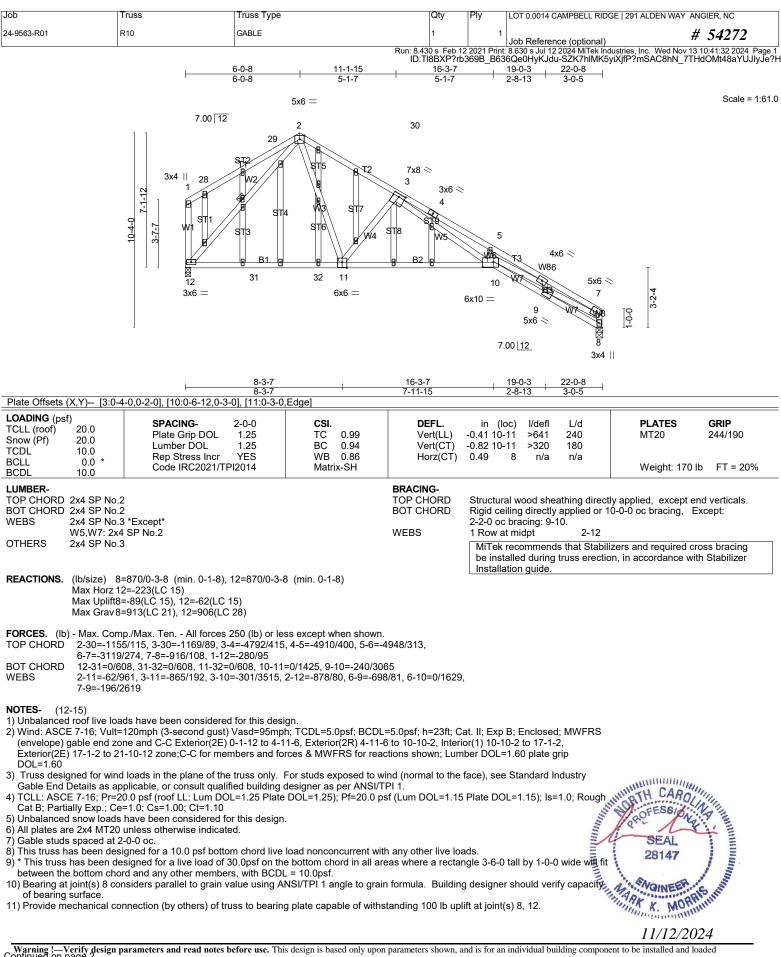
Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
 Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALDEN WAY	ANGIER, NC
24-9563-R01	R10	GABLE	1	1	Job Reference (optional)	# 54272
					: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov	

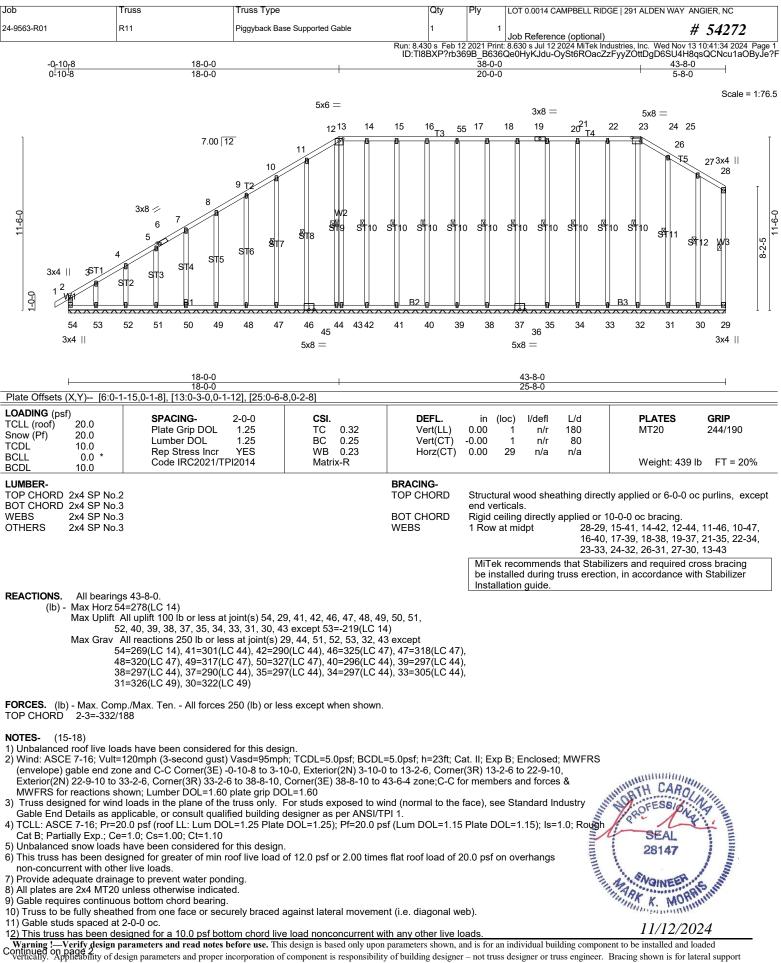
12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. (13) Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

14) Web blacking shown is to hater support of individual web individual web individual web individual to be of a boot individual to be of a boot individual to be of a boot individual web individual web individual to be of a boot individual to boot individual to be of a boot individual to be of a boot ind

LOAD CASE(S) Standard





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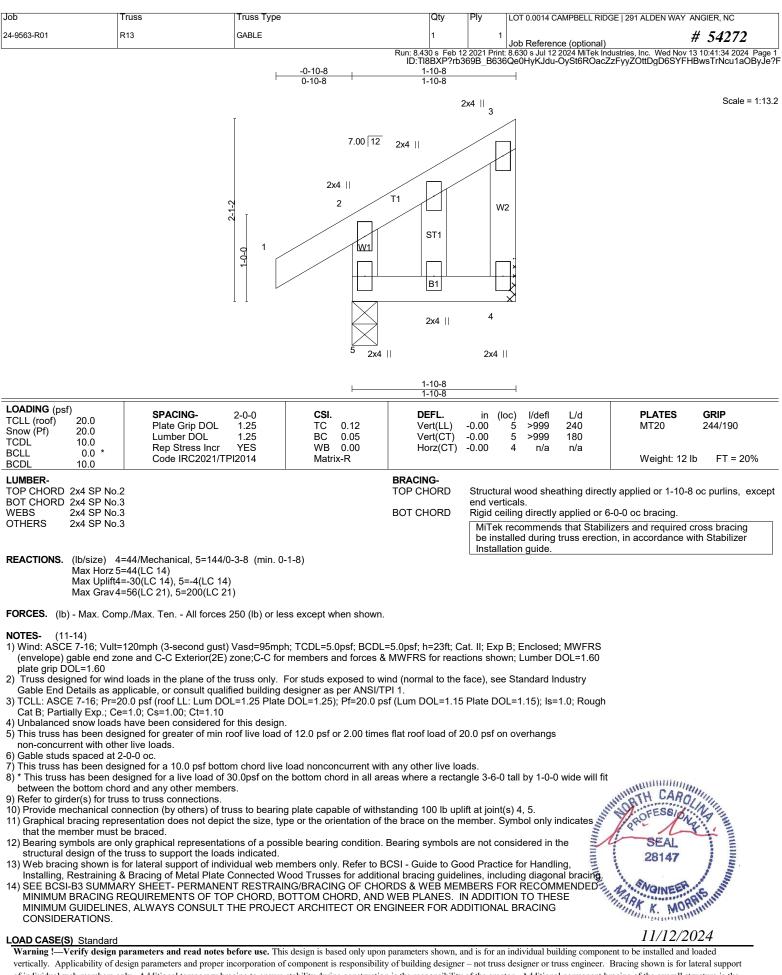
Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALDEN WAY	ANGIER, NC
24-9563-R01	R11	Piggyback Base Supported Gable	1	1	Job Reference (optional)	# 54272
					8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov Qe0HyKJdu-OySt6ROacZzFyyZOttDgD6SU4H	

NOTES- (15-18)

- 13) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 54, 29, 41, 42, 46, 47, 48, 49, 50, 51, 52, 40, 39, 38, 37, 35, 34, 33, 31, 30, 43 except (it=lb) 53=219.
- 15) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 16) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 17) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 18) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

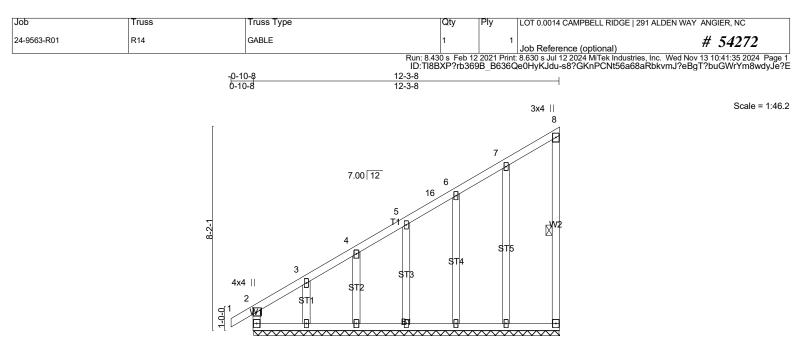




Plate Offsets (X,Y)-- [2:0-2-0.0-1-12]

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.25 Lumber DOL 1.25 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.43 BC 0.32 WB 0.18 Matrix-R	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl -0.00 2 n/r -0.00 1 n/r -0.00 9 n/a	L/d 180 80 n/a	PLATES MT20 Weight: 83 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3			BRACING- TOP CHORD	Structural wood she end verticals.	athing direct	tly applied or 6-0-0 o	c purlins, excep
WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3			BOT CHORD WEBS	Rigid ceiling directly 1 Row at midpt	applied or 1 8-9	0-0-0 oc bracing.	
						izers and required cr on, in accordance wit	0

Installation guide

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SEAL

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NOINEE

K. MORR

11/12/2024

A BUILDING

REACTIONS. All bearings 12-3-8

(lb) - Max Horz 15=229(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 15, 9, 12, 13, 11, 10 except 14=-176(LC 14) Max Grav All reactions 250 lb or less at joint(s) 15, 9, 12, 13, 14 except 11=274(LC 5), 10=314(LC 5)

ł

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

TOP CHORD 2-3=-387/176, 3-4=-266/122

NOTES-(13)

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-1-12, Exterior(2N) 4-1-12 to 7-4-2, Corner(3E) 7-4-2 to 12-1-12 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

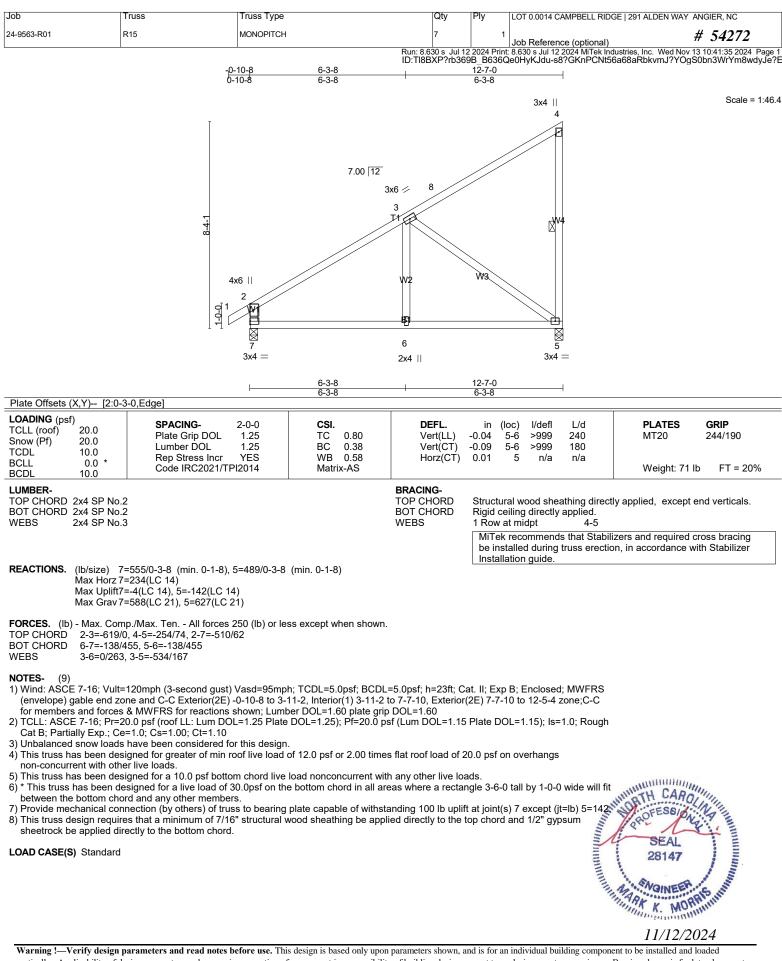
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

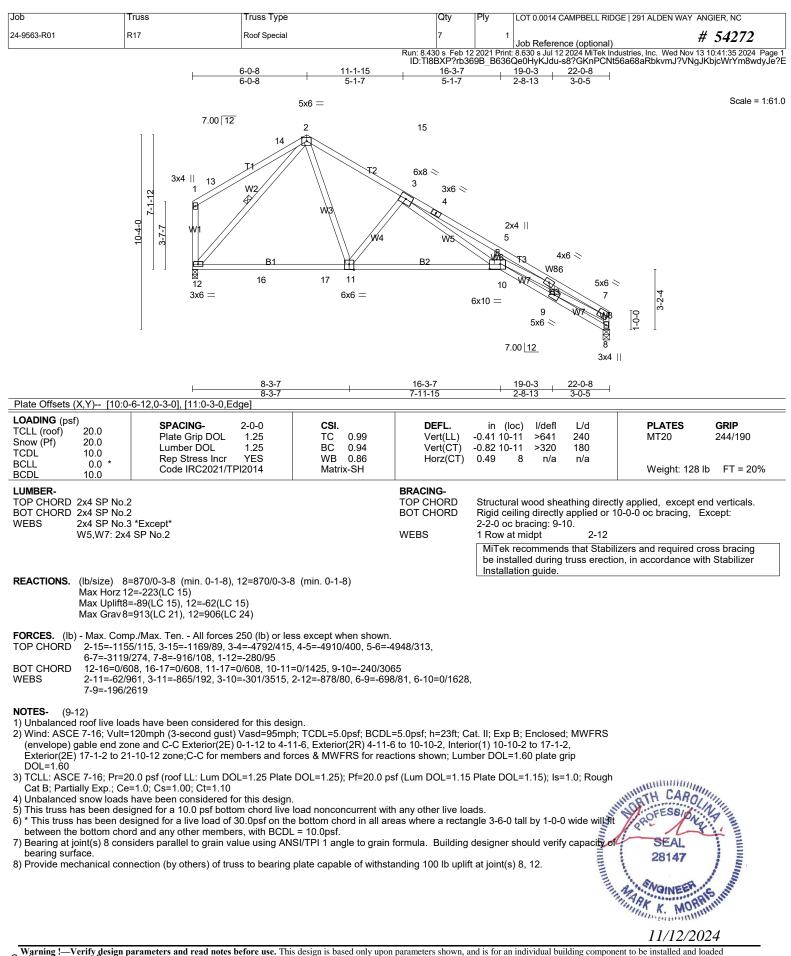
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

- non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.
- Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 9) Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15, 9, 12, 13, 11, 10 All ANA ANA except (jt=lb) 14=176.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0014 CAMPBELL RIDGE 291 ALC	DEN WAY ANGIER, NC	
24-9563-R01	R17	Roof Special	7	1	Job Reference (optional)	# 54272	
Run: 8.430 s Feb 12 2021 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov 13 10:41:36 2024 Page							

9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.

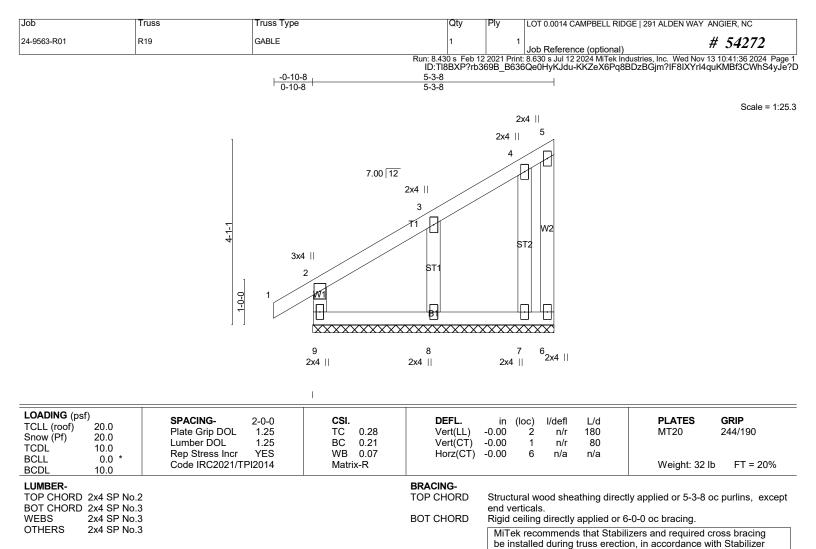
10) Bearing symbols are only graphical representation does not depict the size, type of the orientation of the brace of the member. Symbol only indicates that the member must be braced. Ioabs indicated.

 Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS

12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Installation guide

REACTIONS. All bearings 5-3-8.

(lb) - Max Horz 9=105(LC 14)

Max Uplift All uplift 100 lb or less at joint(s) 6 except 8=-104(LC 14) Max Grav All reactions 250 lb or less at joint(s) 9, 6, 7 except 8=297(LC 21)

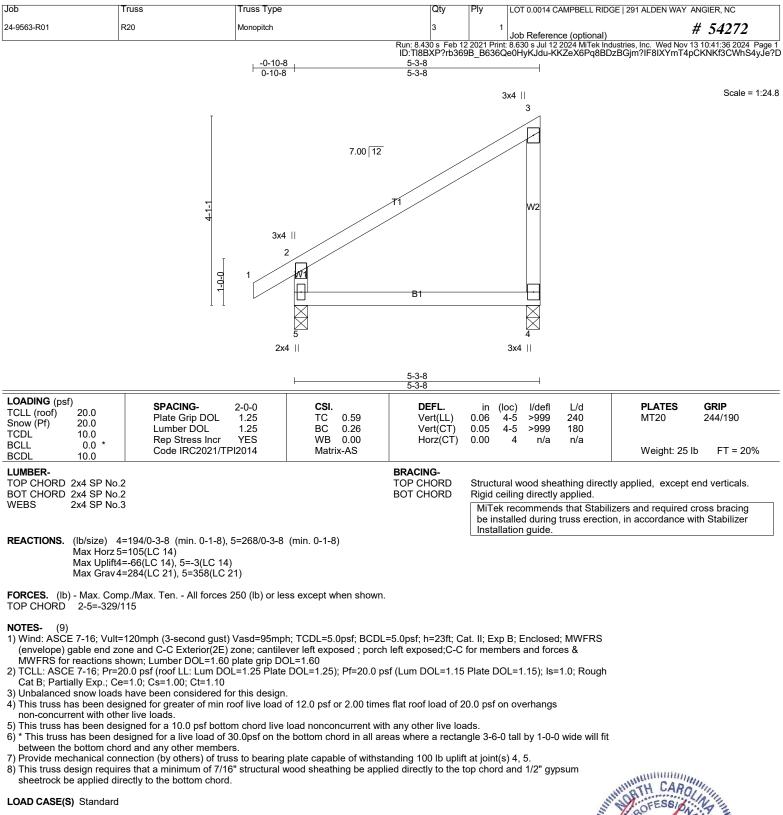
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (12)

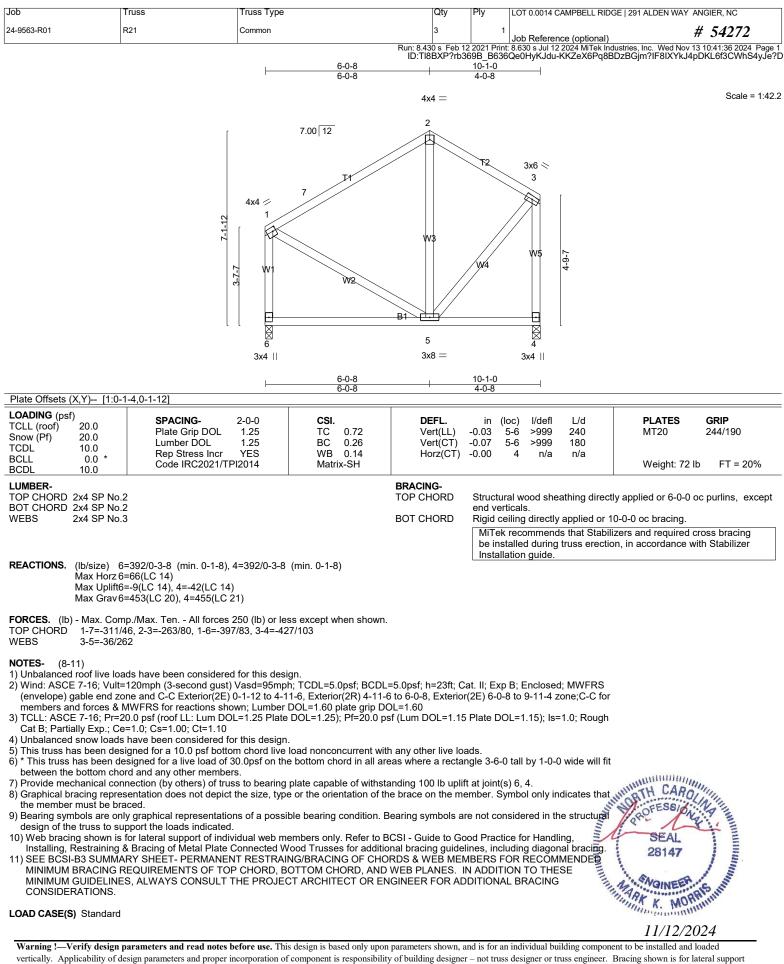
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) Gable requires continuous bottom chord bearing.
- 7) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
- 8) Gable studs spaced at 2-0-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6 except (jt=lb) 8=104.

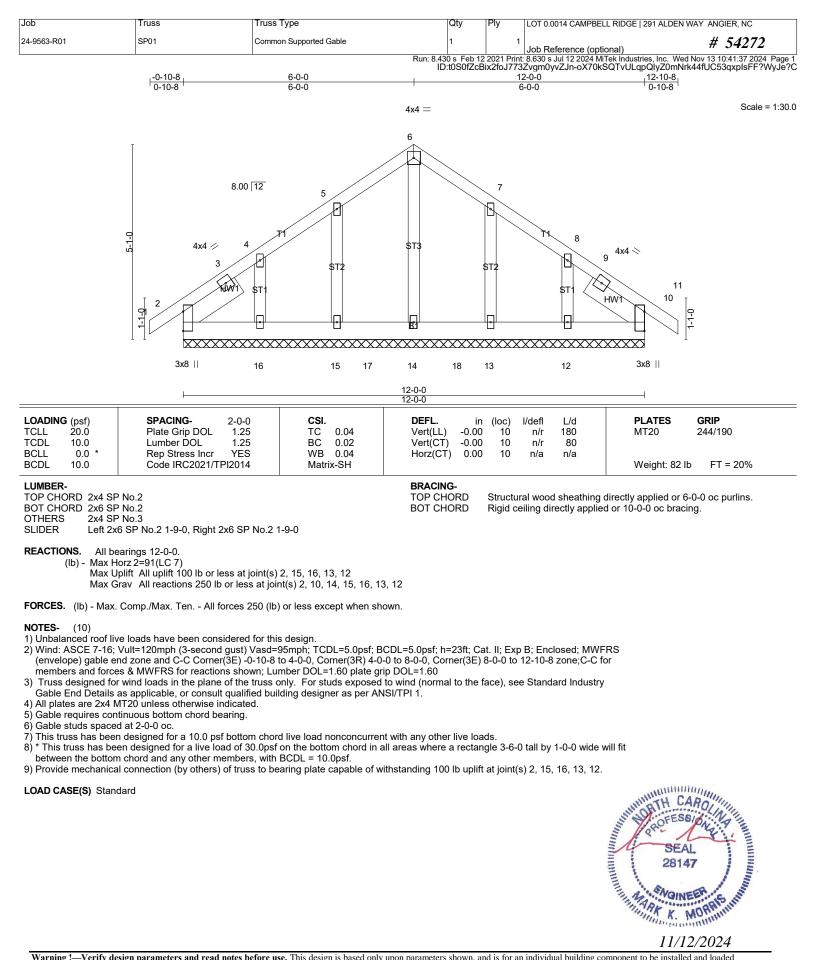
LOAD CASE(S) Standard



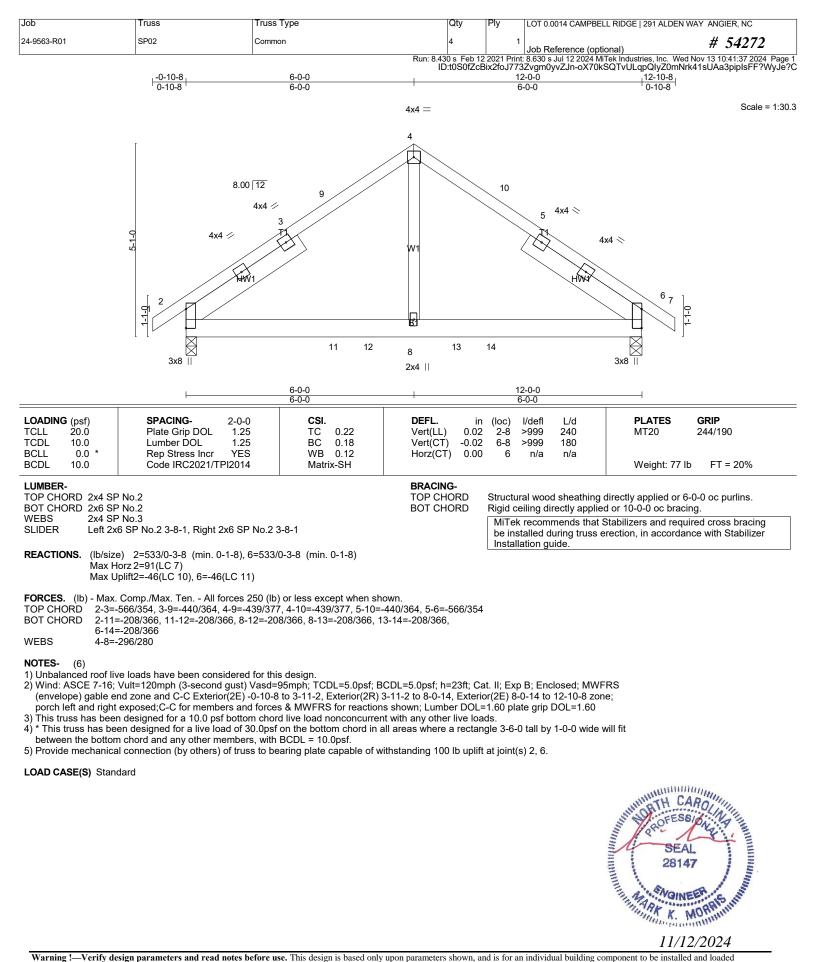






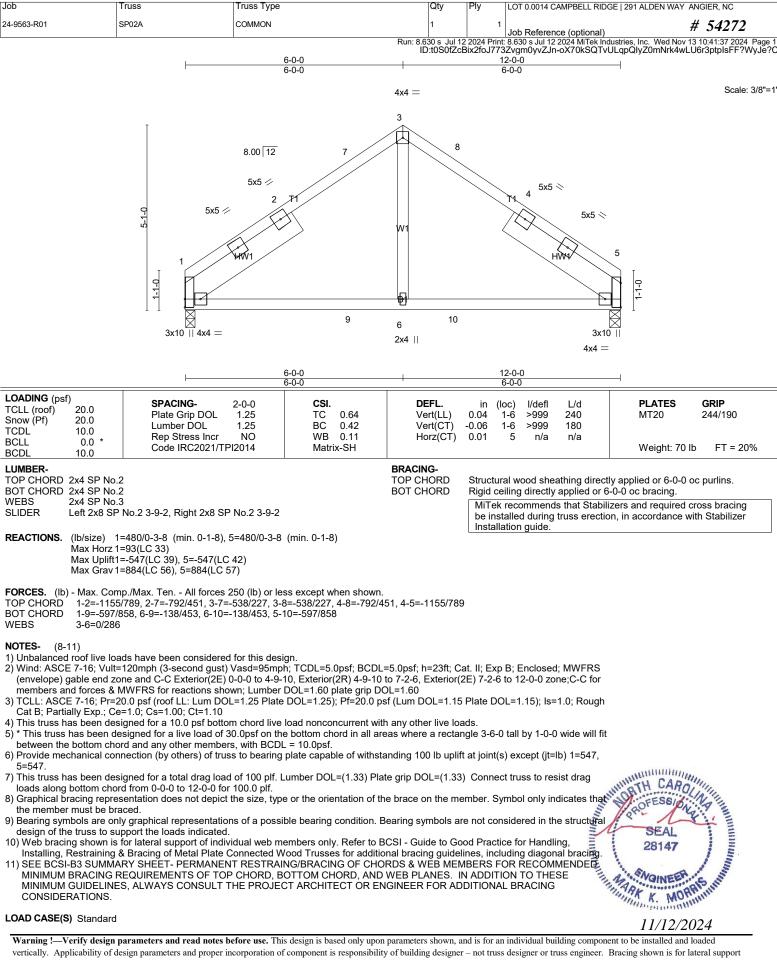


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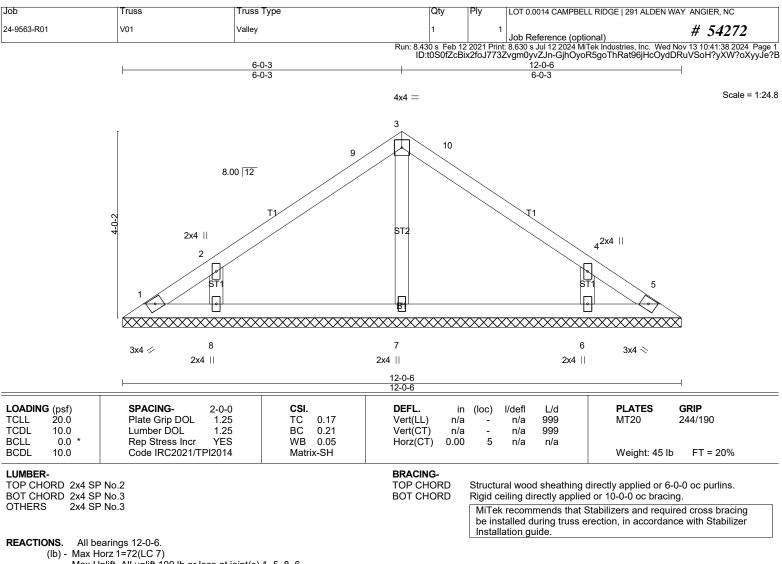


Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

11/12/2024



of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designet. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Trusse Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Trusse Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Max Uplift All uplift 100 lb or less at joint(s) 1, 5, 8, 6

Max Grav All reactions 250 lb or less at joint(s) 1, 5 except 7=267(LC 1), 8=298(LC 17), 6=298(LC 18)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-5-12 to 5-3-6, Exterior(2R) 5-3-6 to 6-9-0, Exterior(2E) 6-9-0 to 11-6-10 zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

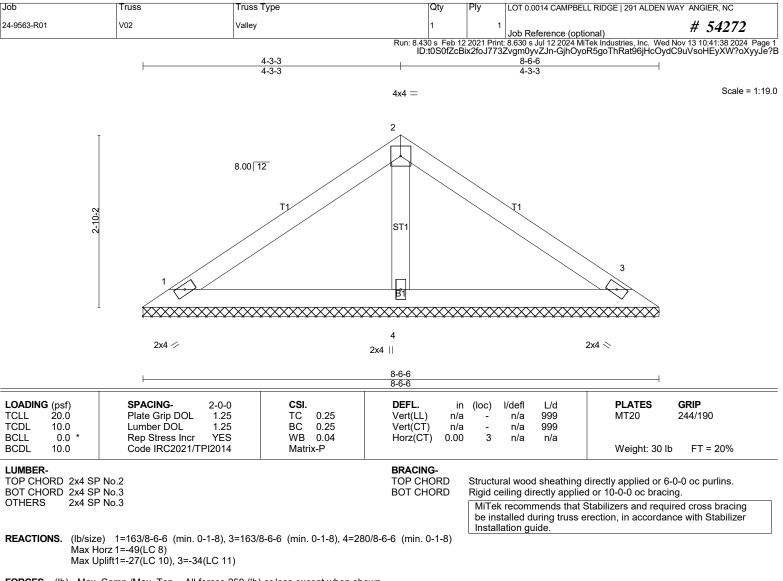
3) Gable requires continuous bottom chord bearing.

4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members. 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5, 8, 6.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

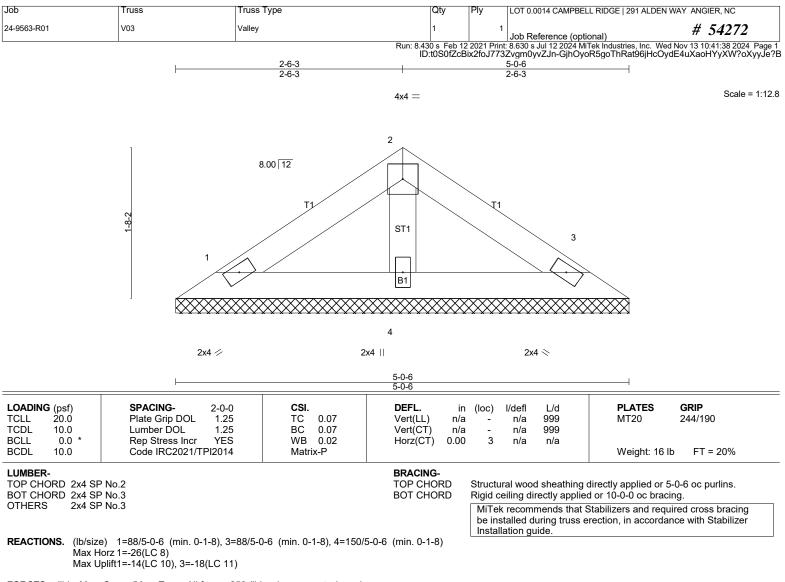
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (7)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph, TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

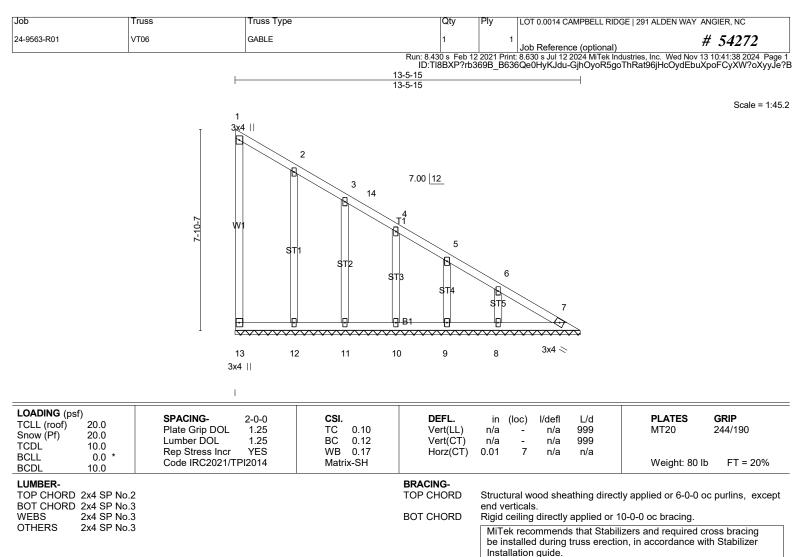
3) Gable requires continuous bottom chord bearing.

- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.

6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. All bearings 13-5-15

(lb) - Max Horz 13=-228(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 13, 12, 11, 10, 9, 8

Max Grav All reactions 250 lb or less at joint(s) 13, 7, 10, 9, 8 except 12=328(LC 6), 11=266(LC 6)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-1-12 to 4-11-6, Interior(1) 4-11-6 to 8-1-14, Exterior(2E) 8-1-14 to 12-11-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) All plates are 2x4 MT20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

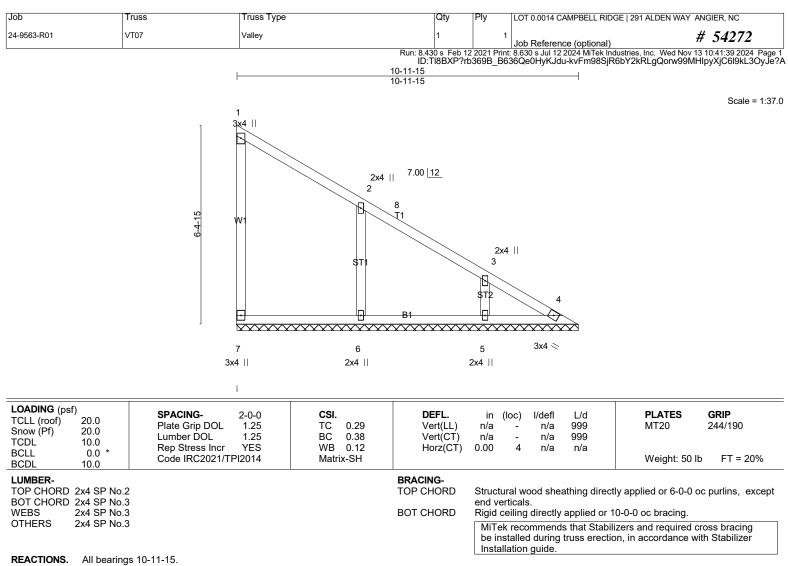
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 7) between the bottom chord and any other members, with BCDL = 10.0psf.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 13, 12, 11, 10, 9, 8.

LOAD CASE(S) Standard





(lb) - Max Horz 7=-184(LC 15)

Max Uplift All uplift 100 lb or less at joint(s) 7, 6, 5

Max Grav All reactions 250 lb or less at joint(s) 7, 4 except 6=500(LC 6), 5=281(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-6=-381/139

NOTES- (8)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

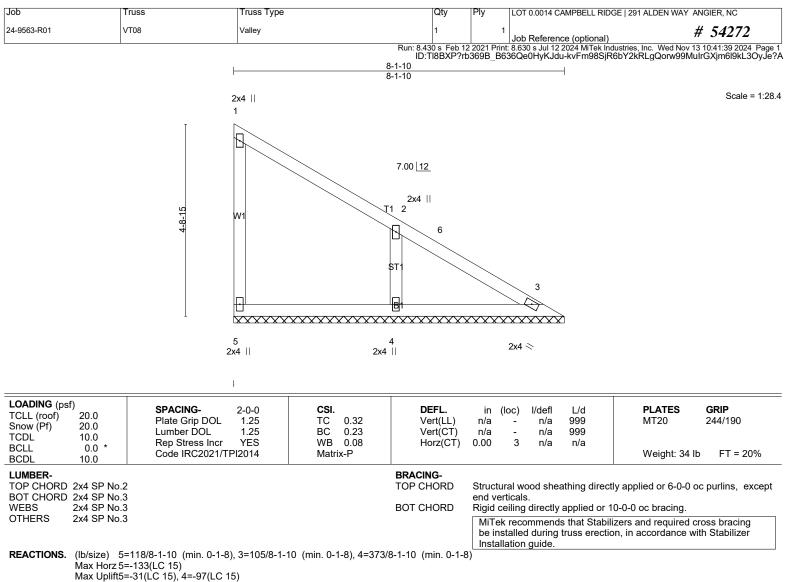
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 6, 5.

LOAD CASE(S) Standard





Max Grav 5=177(LC 21), 3=105(LC 1), 4=492(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

WFBS

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

2-4=-398/169

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

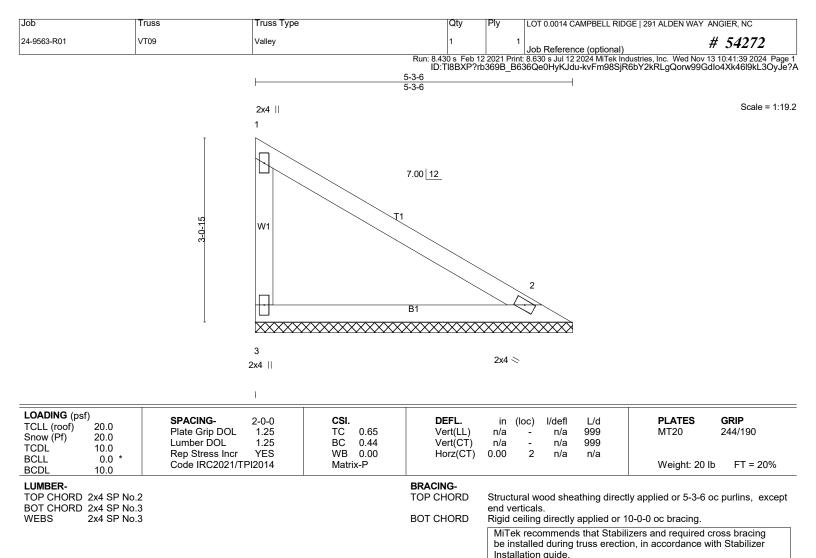
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 3=184/5-3-6 (min. 0-1-8), 2=184/5-3-6 (min. 0-1-8) Max Horz 3=-82(LC 15) Max Uplift3=-48(LC 15) Max Grav 3=264(LC 21), 2=264(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

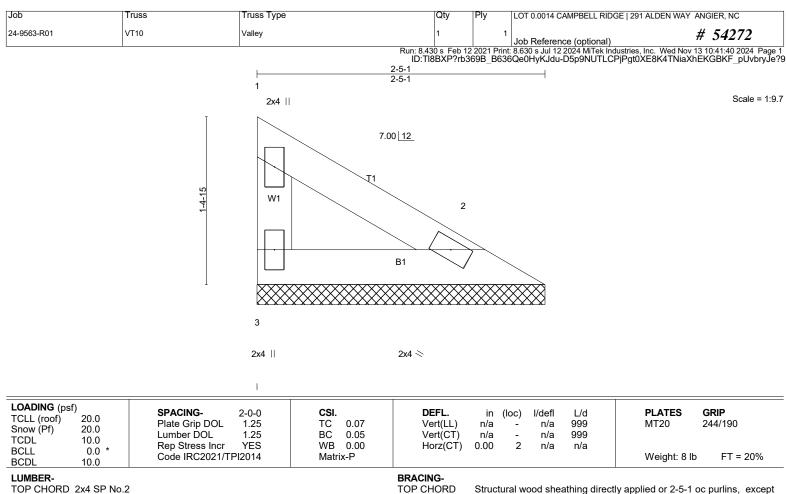
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

LOAD CASE(S) Standard





BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 BRACING-TOP CHORD

Structural wood sheathing directly applied or 2-5-1 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 3=69/2-5-1 (min. 0-1-8), 2=69/2-5-1 (min. 0-1-8) Max Horz 3=-31(LC 15) Max Uplift3=-18(LC 15) Max Grav 3=88(LC 21), 2=88(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

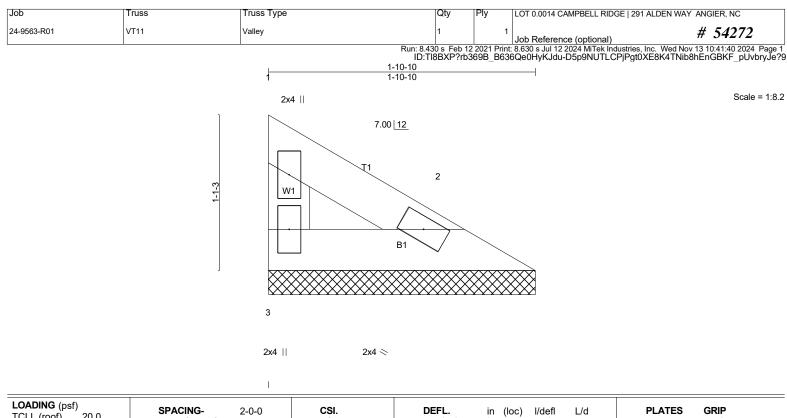
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

LOAD CASE(S) Standard





LOADING (pst TCLL (roof) Snow (Pf) TCDL BCLL BCDL	f) 20.0 20.0 10.0 0.0 * 10.0	SPACING-2-0-0Plate Grip DOL1.25Lumber DOL1.25Rep Stress IncrYESCode IRC2021/TPI2014	CSI. TC 0.03 BC 0.02 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) n/a - n/a - 0.00 2	l/defl n/a n/a n/a	L/d 999 999 n/a	PLATES MT20 Weight: 6 lb	GRIP 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing directly applied or 1-10-10 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.					
					MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.				

REACTIONS. (lb/size) 3=48/1-10-10 (min. 0-1-8), 2=48/1-10-10 (min. 0-1-8) Max Horz 3=-21(LC 15) Max Uplift3=-13(LC 15) Max Grav 3=59(LC 21), 2=59(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

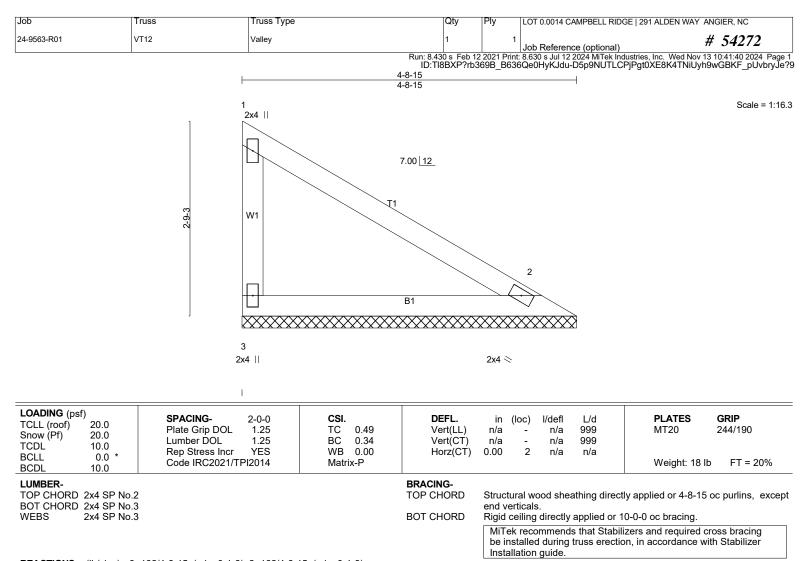
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 3=162/4-8-15 (min. 0-1-8), 2=162/4-8-15 (min. 0-1-8) Max Horz 3=-72(LC 15) Max Uplift3=-42(LC 15) Max Grav 3=229(LC 21), 2=229(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

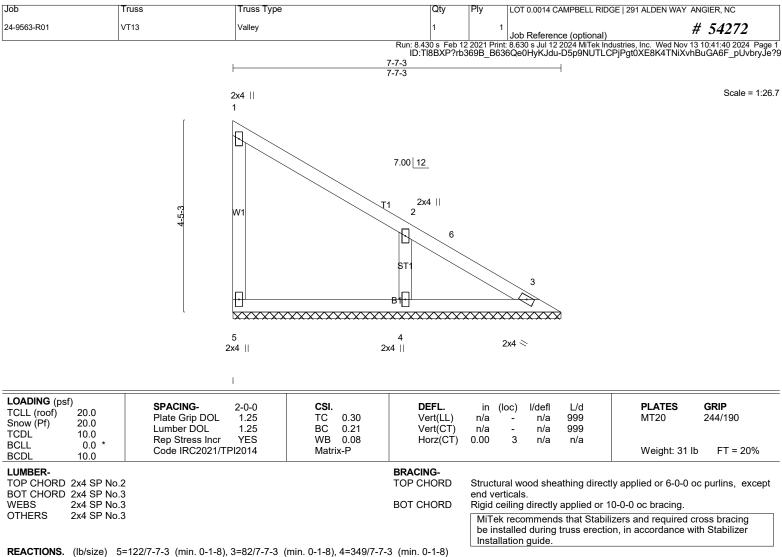
- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3.

LOAD CASE(S) Standard





REACTIONS. (Ib/size) 5=122/7-7-3 (min. 0-1-8), 3=82/7-7-3 (min. 0-1-8), 4=349/7-7-3 (min. 0-1-8) Max Horz 5=-123(LC 15) Max Uplift5=-32(LC 15), 4=-91(LC 15)

Max Grav 5=181(LC 21), 3=88(LC 26), 4=470(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-4=-383/163

NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

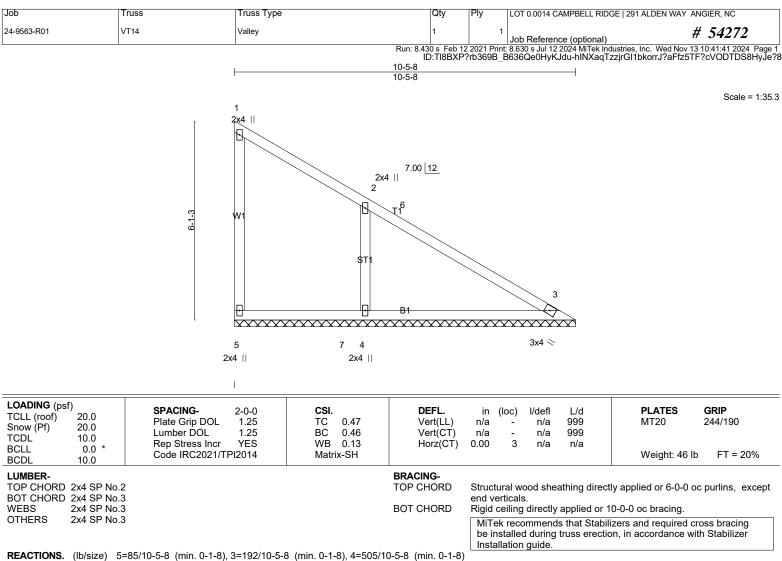
6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 4.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 5=85/10-5-8 (min. 0-1-8), 3=192/10-5-8 (min. 0-1-8), 4=505/10-5-8 (min. 0-1-8) Max Horz 5=-174(LC 15) Max Uplift5=-23(LC 15), 4=-131(LC 15)

Max Grav 5=203(LC 6), 3=192(LC 1), 4=613(LC 21)

NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 4=131.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown. WEBS 2-4=-468/182