

Elevation A - Sheet List	
Sheet Number	Sheet Name
ElevA-Pg1	Front & Rear Elevations
ElevA-Pg2	Side Elevations
ElevA-Pg3	1st Floor Plan
ElevA-Pg4	Roof Plan
Pg5	Building Section
Pg6	Electrical Plan
Sec-Crawl/Blk 1flr	Typical Wall Section
Structural Pages	by Engineer



1 Front - Elev A  
1/8" = 1'-0"



2 Rear - Elev A  
1/8" = 1'-0"

Area Schedule (Elevation A)	
Name	Area
Heated	
1st Floor	1075 SF
	1075 SF
Unheated	
Front Porch	27 SF
	27 SF
Under Roof	1102 SF

THE JUDY - Elevation A

Front & Rear Elevations

Job #: 2024-SAN-063  
Address: 580 Griffin Rd  
Lillington, NC 27546  
County: Harnett County

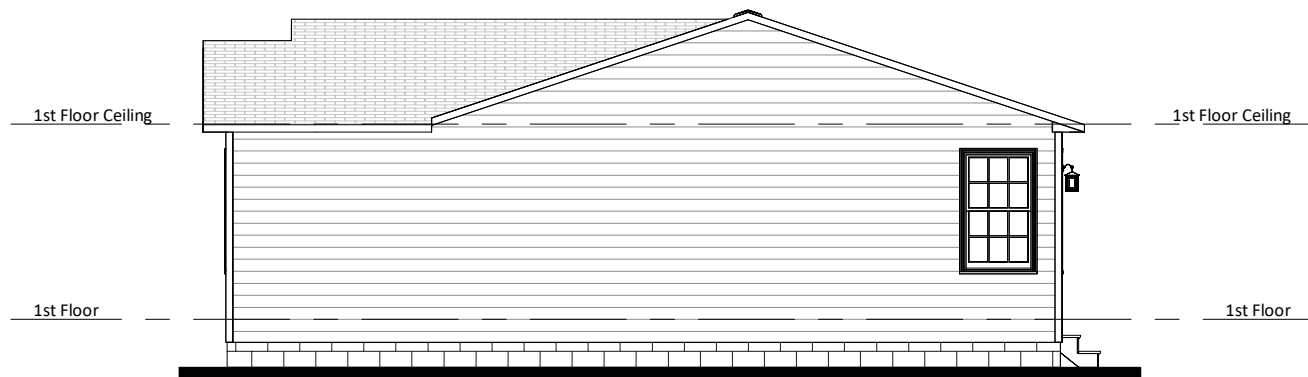
Plan Version Date:  
1-20-21

Job Version Date:  
11-11-24

Sheet #:  
ElevA-Pg1



① Left - Elev A  
1/8" = 1'-0"



② Right - Elev A  
1/8" = 1'-0"

THE JUDY - Elevation A

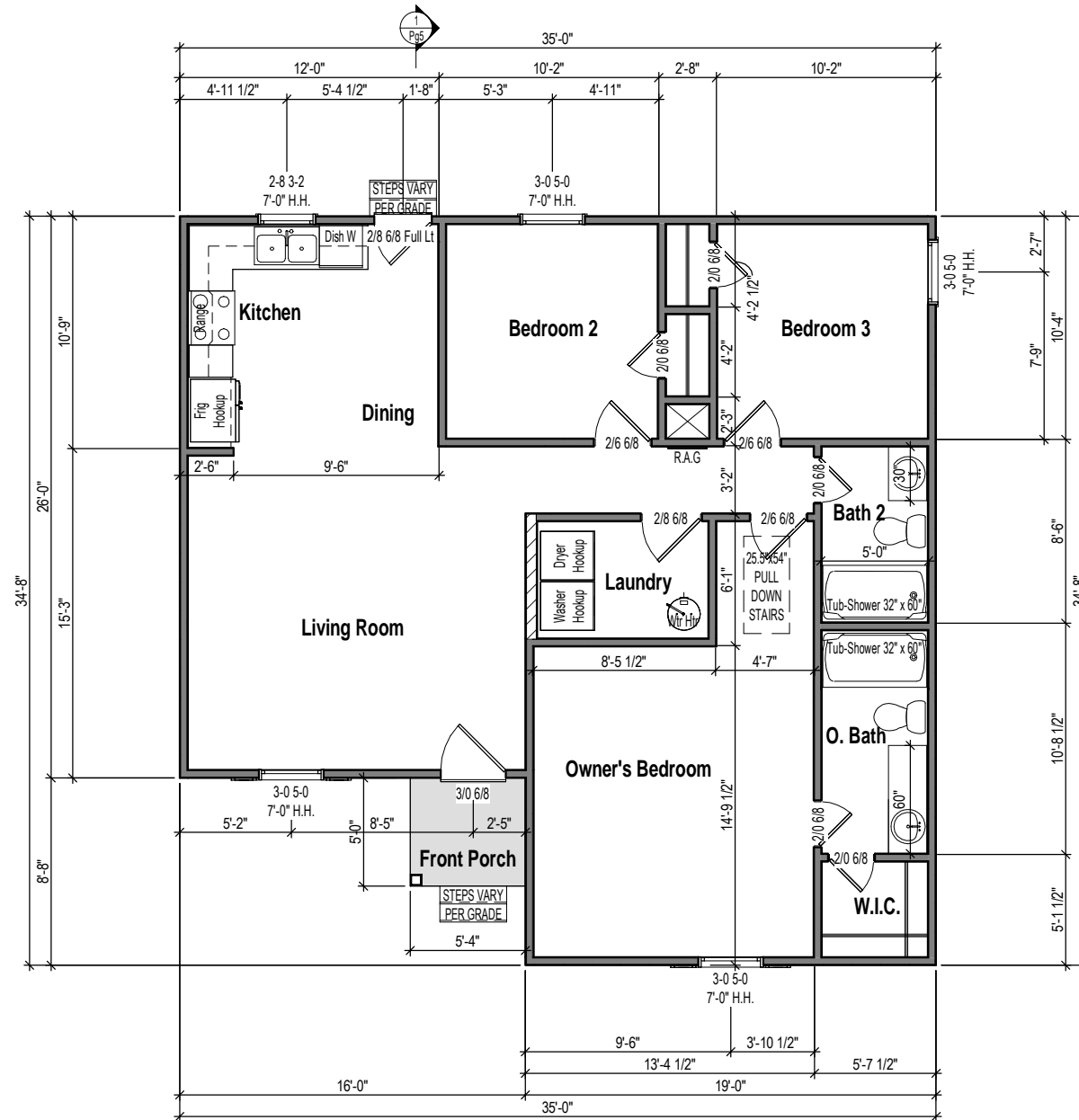
Side Elevations

Job #: 2024-SAN-063  
Address: 580 Griffin Rd  
Lillington, NC 27546  
County: Harnett County

Plan Version Date:  
1-20-21

Job Version Date:  
11-11-24

Sheet #:  
ElevA-Pg2



1 1st Floor Plan - Elev A  
1/8" = 1'-0"

#### WALL STUD SIZES

— = 2x4    ▨ = 2x6

#### DIMENSIONS

- EXTERIOR WALL DIMENSIONS ARE TO FACE OF SHEATHING
- INTERIOR WALL DIMENSIONS ARE TO FACE OF STUD

#### EXTERIOR DOOR ROUGH OPENINGS

- ALL EXTERIOR SWING DOORS HAVE A HEADER HEIGHT = TO 3" HIGHER THAN CALL SIZE
- ALL EXTERIOR SLIDING DOORS HAVE A HEADER HEIGHT = TO CALL SIZE

THE JUDY - Elevation A

1st Floor Plan

Job #: 2024-SAN-063  
Address: 580 Griffin Rd  
Lillington, NC 27546  
County: Harnett County

Plan Version Date:  
1-20-21

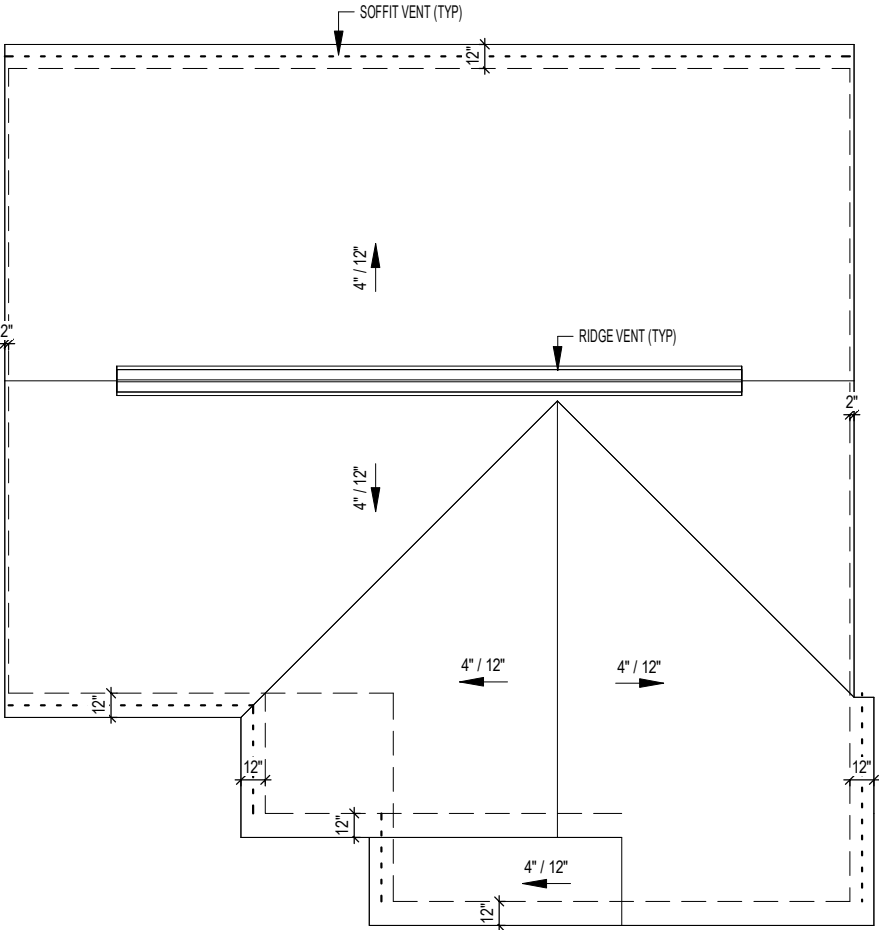
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11-11-24

Sheet #:  
ElevA-Pg3

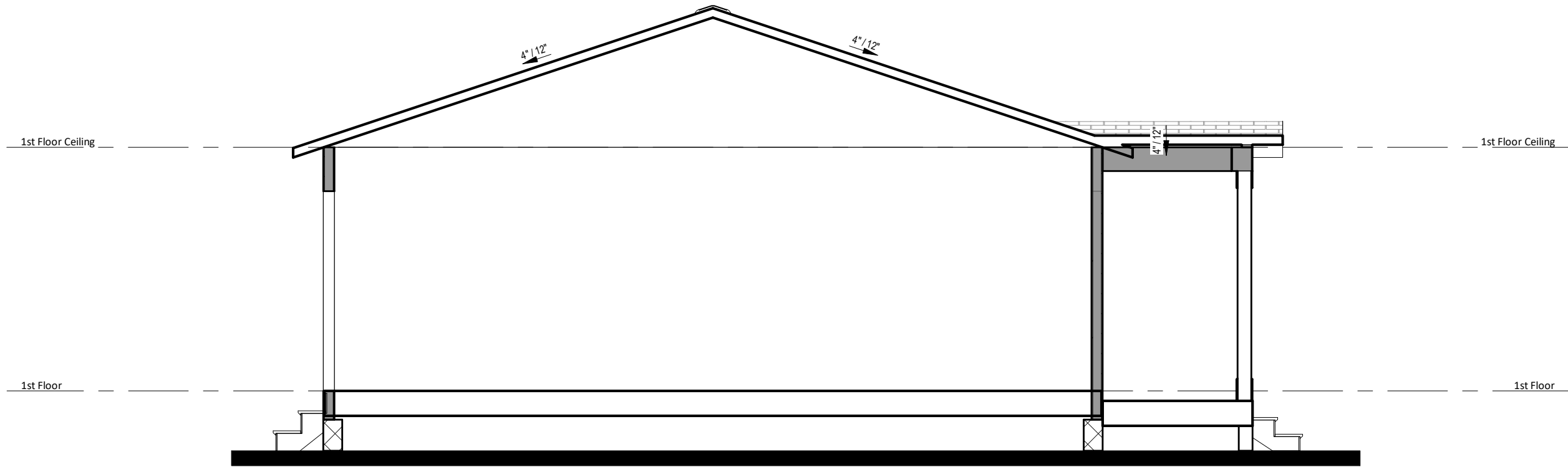
Attic Ventilation Calcs 1/300 (sq.in.)										
Name	Area	Ventilation Required (sq.in.)	Max Upper (sq.in.)	Min Upper (sq.in.)	Upper Ventilation (sq.in.)	Lower Ventilation (sq.in.)	Total Ventilation (sq.in.)	Ridge Vent (ln.ft.)	Roof Vents (ea)	Soffit Vents (sq.ft.)
Main Roof	1075 SF	516	413	258	390	372	762	26	0	62

CALCS BASED ON THE FOLLOWING VALUES



















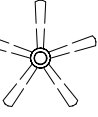



- Ridge Vents = 15 in<sup>2</sup> of net free area per linear foot
- Roof Vents = 50 in<sup>2</sup> of net free area per unit
- Soffit Vents = 6 in<sup>2</sup> of net free area per square foot



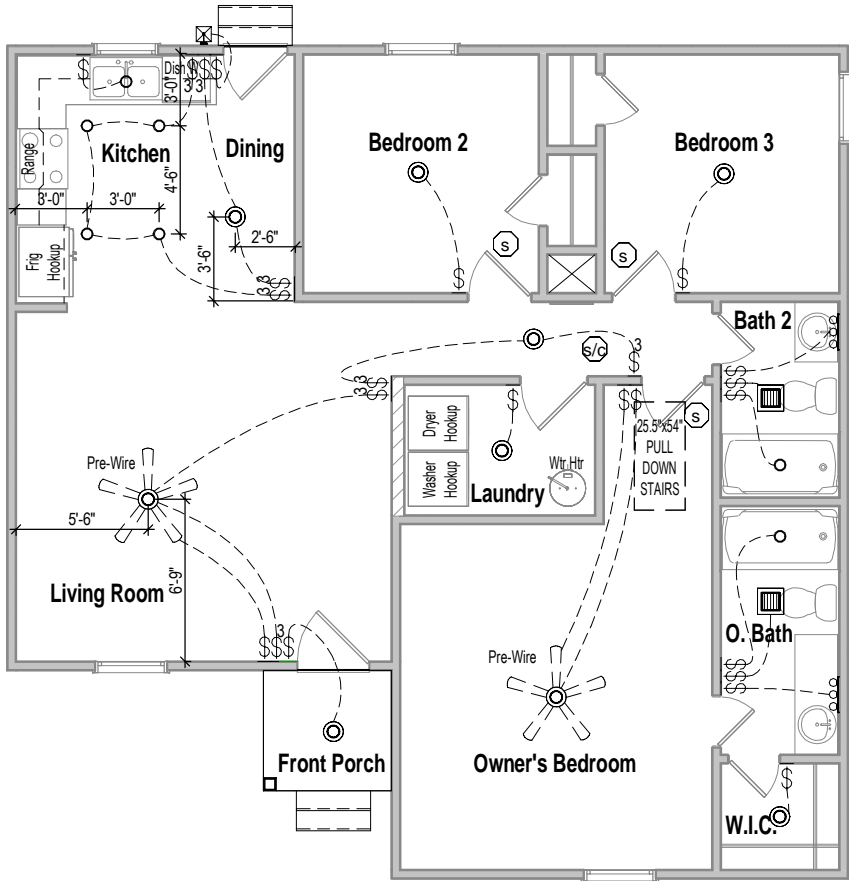
1 Roof Plan - Elev A  
1/8" = 1'-0"




1 Building Section  
1/4" = 1'-0"

ELECTRICAL LEGEND					
WALL MOUNTED FIXTURES			CEILING MOUNTED FIXTURES		
 OUTLET - 110V	 OUTLET - TV	 GARAGE DOOR OPENER PREWIRE	 KEYLESS	 SMOKE DETECTOR	
 OUTLET - 110V GROUND FAULT INTERRUPTER	 OUTLET - PHONE	 18" LIGHT BAR	 FLUSH MOUNT	 SMOKE DETECTOR/CO2	
 OUTLET - 110V GROUND FAULT INTERRUPTER WATER PROOF	 SWITCH - SINGLE POLE	 COACH LIGHT - FRONT DOOR	 SURFACE LIGHT	 BATHROOM EXHAUST FAN	
 OUTLET - 220V	 SWITCH - 3 WAY	 COACH LIGHT - REAR DOOR	 FLUSH MOUNT W-FAN PREWIRE	 FLUORESCENT 4' - 2 LAMPS	
	 SWITCH - 4 WAY			 FLUORESCENT 2' - 1 LAMP	

Outlets shown on the electrical layout are in addition to the outlets that shall be provided in accordance with International Residential Code Sections E3901.2 through E3901.11.



1 1st Floor Electrical  
1/8" = 1'-0"



3015 Jefferson Davis Hwy, Sanford, NC 27332

THE JUDY - Elevation A

Electrical Plan

Job #:  
2024-SAN-063

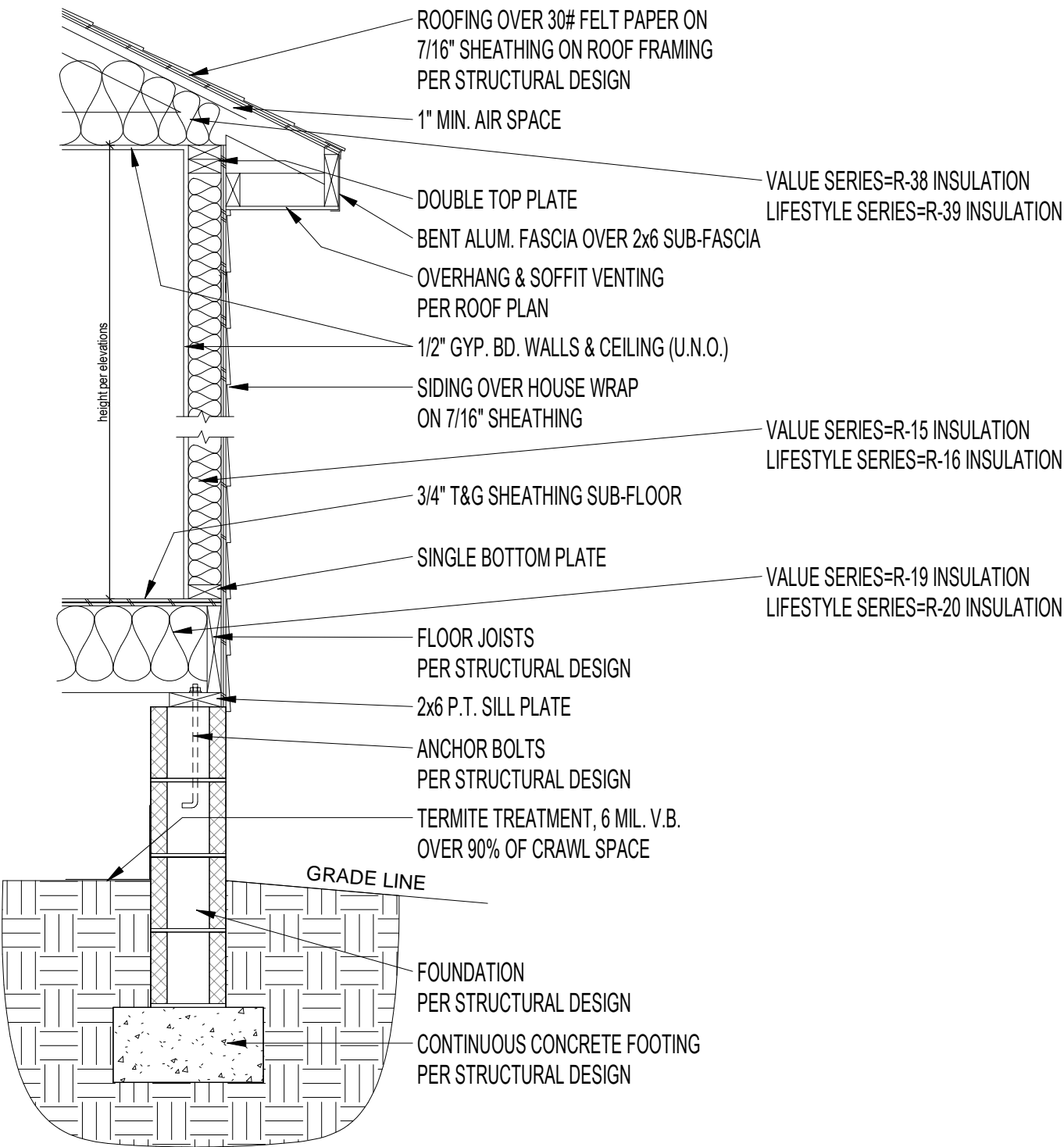
Address:  
580 Griffin Rd  
Lillington, NC 27546

County:  
Harnett County

Plan Version Date:  
1-20-21

Job Version Date:  
11-11-24

Sheet #:  
Pg6



1 Typical Wall Section - Block Fnd 1 floor  
3/4" = 1'-0"

THE JUDY - Elevation A

Typical Wall Section

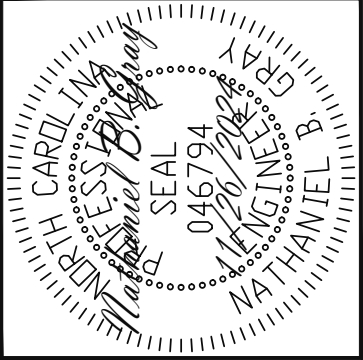
Job #: 2024-SAN-063  
Address: 580 Griffin Rd  
Lillington, NC 27546  
County: Harnett County

Plan Version Date:  
1-20-21

Job Version Date:  
11-11-24

Sheet #:  
Sec-Crawl/Blk 1flr

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Raleigh, North Carolina 27609  
ASSOCIATES, P.A. Phone (919) 844-1661

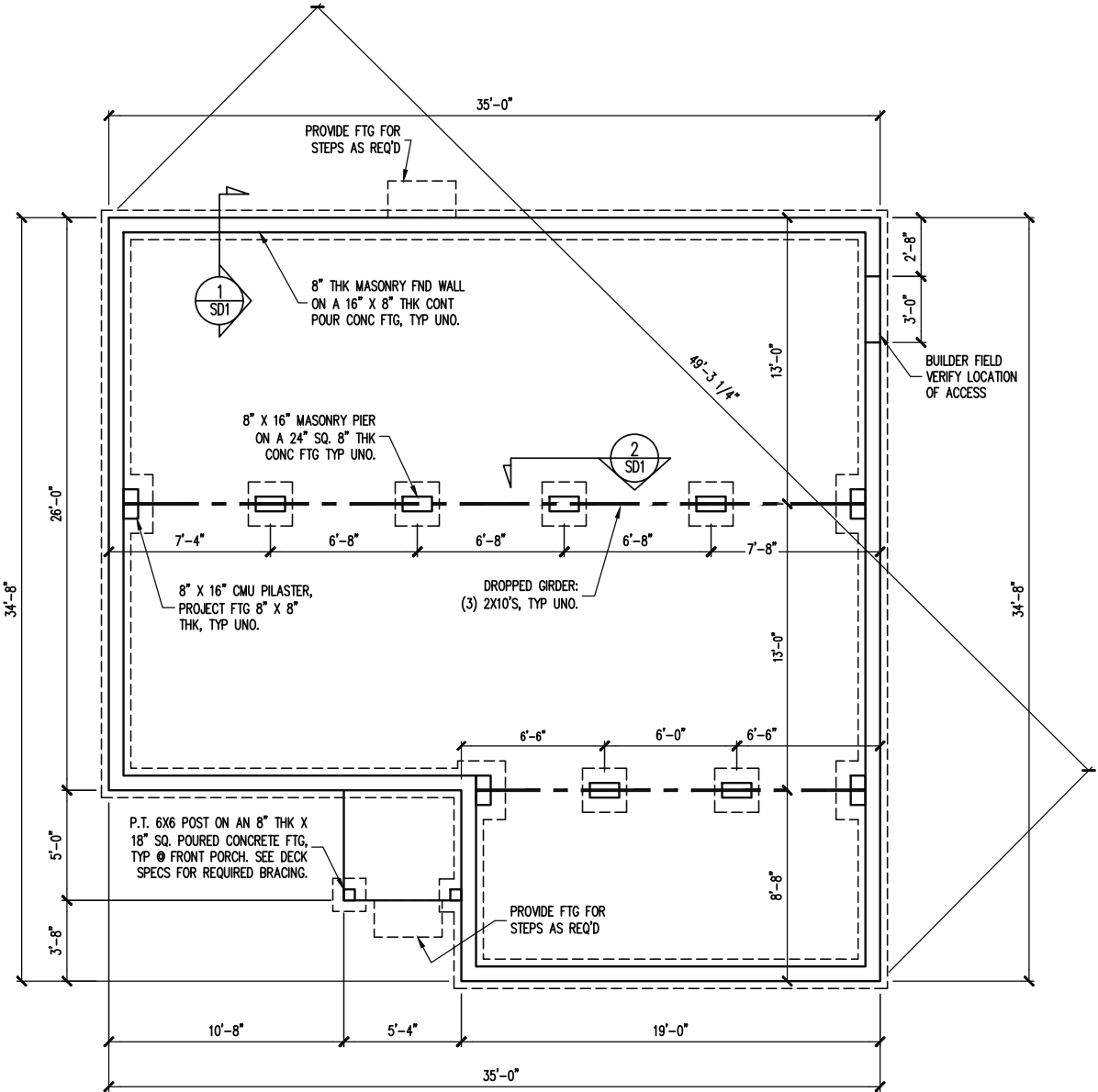
VALUE BUILD HOMES			
STRUCTURAL ADDENDUM			
SCOPE			
LOC	580 GRIFFIN RD	REV 1	EAF 07-16-2024
	LILLINGTON, NC		
JOB# 2024-SAN-063 VASQUEZ			

ENG	PAL/NBG
DATE	11/26/2024

PLAN
JUDY

PROJECT NO.
24-26-163

SHEET NO.
S1
1 of 7

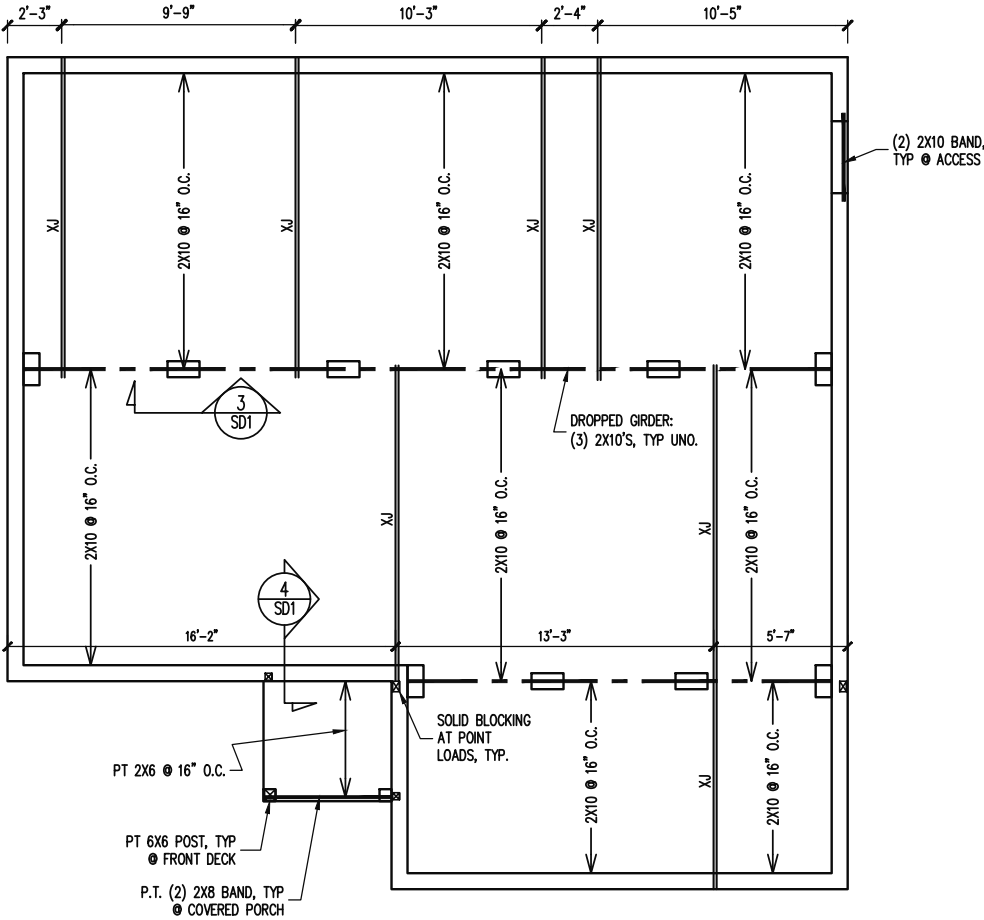
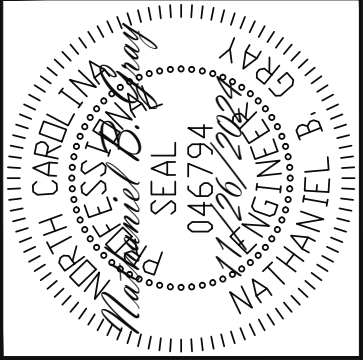


NOTES:  
-HEIGHT AND BACKFILL LIMITATIONS FOR FOUNDATION WALLS ARE TO BE GOVERNED BY THE NCSBC, LATEST EDITION. REINFORCEMENT AND GROUTING SHALL BE DETERMINED BY FINAL SITE CONDITIONS.  
-BUILDER TO FIELD LOCATE CRAWLSPACE ACCESS OPENING WITH MINIMUM DIMENSIONS OF 18X24. DO NOT LOCATE ACCESS OPENING BELOW POINT LOADS FROM ABOVE WITHOUT ENGINEER APPROVAL.

FOUNDATION PLAN  
1/8" = 1'-0"



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ASSOCIATES, P.A.

VALUE BUILD HOMES			
SCOPE	STRUCTURAL ADDENDUM		
	REV 1	EAF	07-16-2024
LOC	580 GRIFFIN RD		
	LILLINGTON, NC		
JOB# 2024-SAN-063 VASQUEZ			

ENG	PAL/NBG
DATE	11/26/2024

PLAN
JUDY

PROJECT NO.
24-26-163

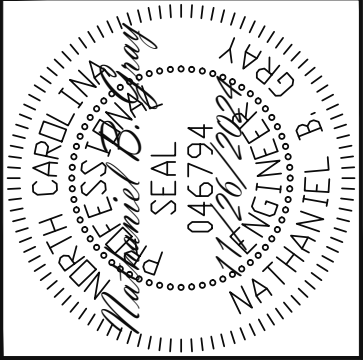
SHEET NO.
S2
2 of 7

VALUE BUILD HOMES			
SCOPE:	STRUCTURAL ADDENDUM		
LOC:	580 GRIFFIN RD	REV 1	EAF 07-16-2024
	LILLINGTON, NC		
	JOB# 2024-SAN-063 VASQUEZ		

PLAN
JUDY

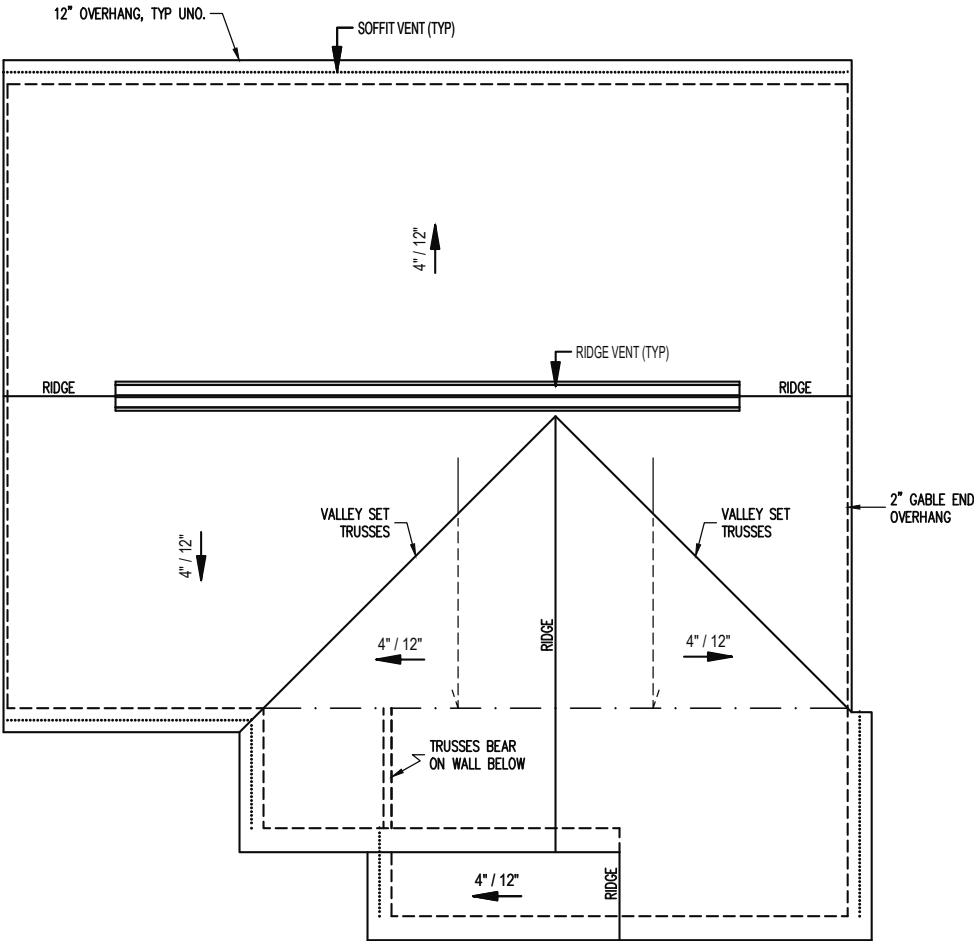
SHEET NO.
S3

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TRUSS UPLIFT CONNECTORS

EXPOSURE B, 120 MPH, ANY PITCH  
24" O.C. MAX ROOF TRUSS SPACING

TRUSSES SHALL BE ATTACHED TO SUPPORT WALL FOR UPLIFT RESISTANCE. CONTINUOUS OSB WALL SHEATHING BELOW PROVIDES CONTINUOUS UPLIFT RESISTANCE TO FOUNDATION. ALL TRUSSES SUPPORTED BY INTERMEDIATE SUPPORT WALLS, KNEEWALLS OR BEAMS SHALL BE ATTACHED TO SUPPORTING MEMBER PER SCHEDULE BELOW.

ROOF SPAN IS MEASURED HORIZONTALLY BETWEEN FURTHEST SUPPORT POINTS.

ROOF SPAN UP TO 18' CONNECTOR NAILING PER TABLE 602.3(1) NCRBC 2018 EDITION

OVER 18' (1) SIMPSON H2.5A HURRICANE CLIP TO DBL TOP PLATE OR BEAM

FRAMING NOTES

ROOF ONLY

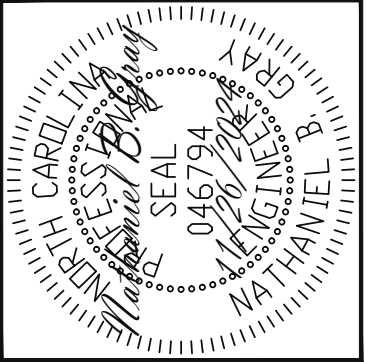
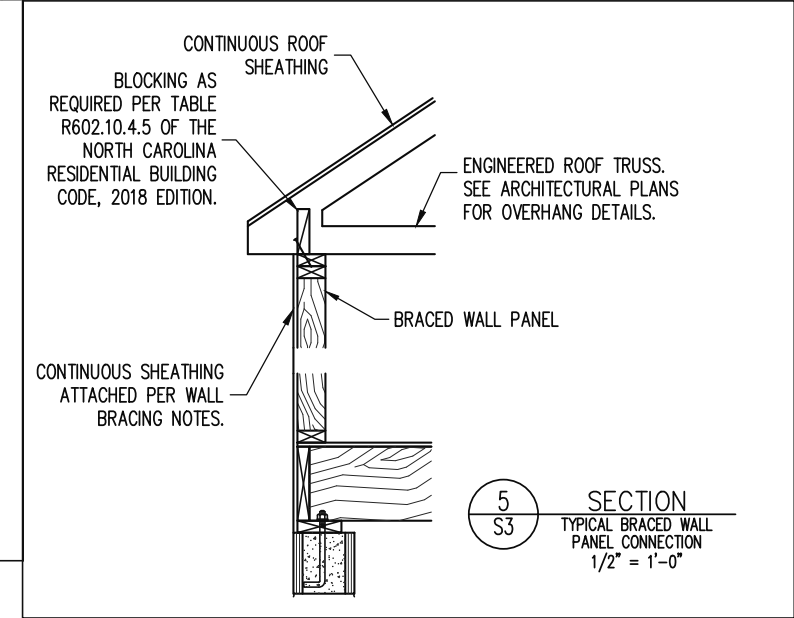
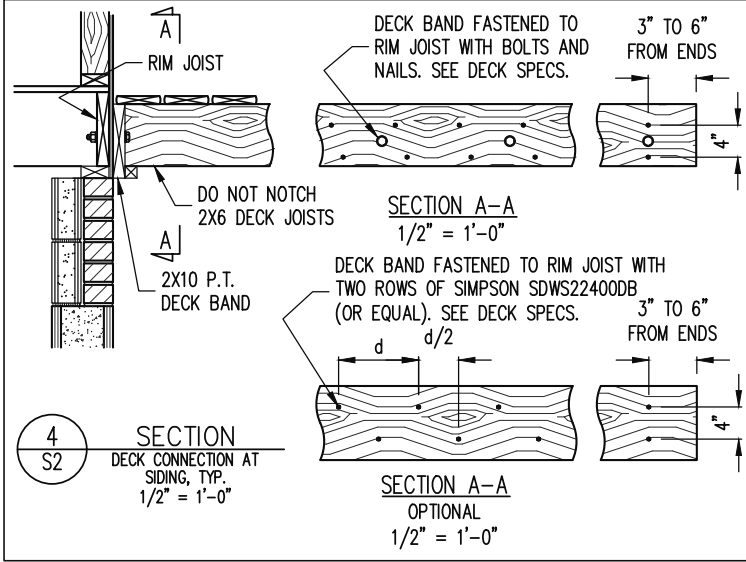
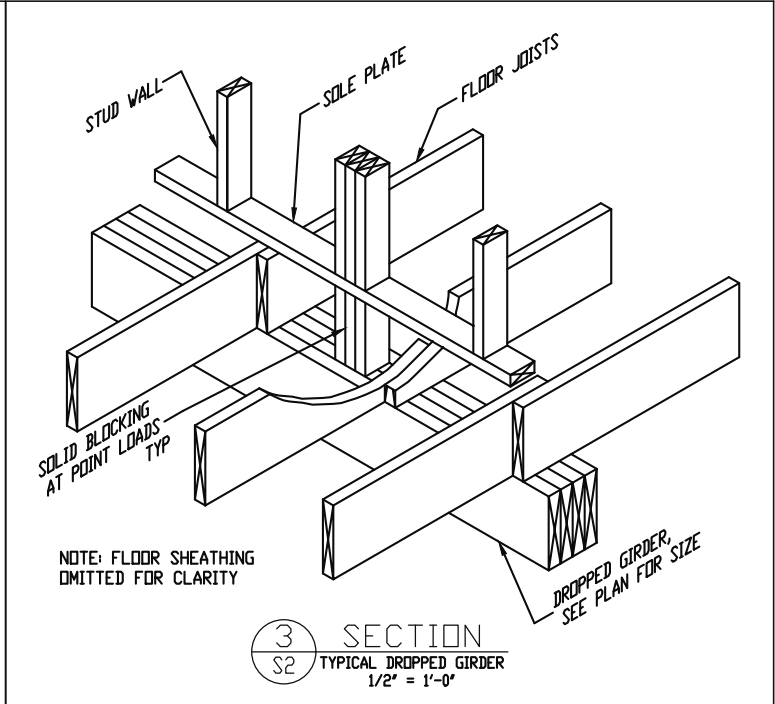
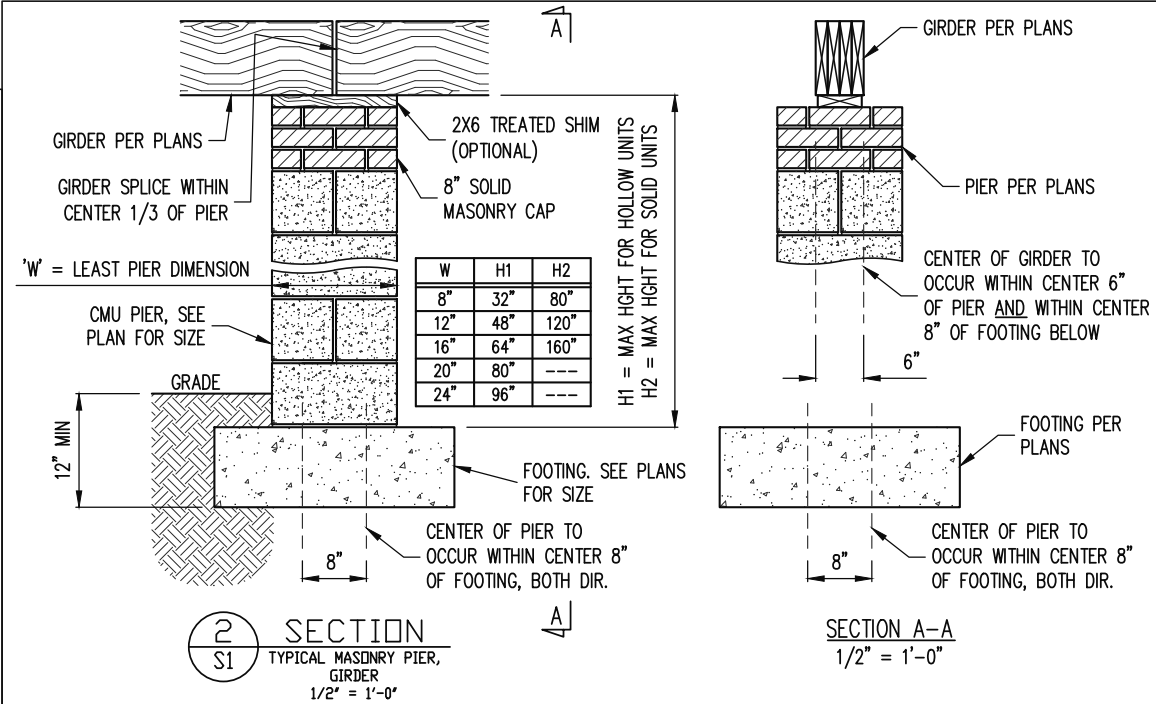
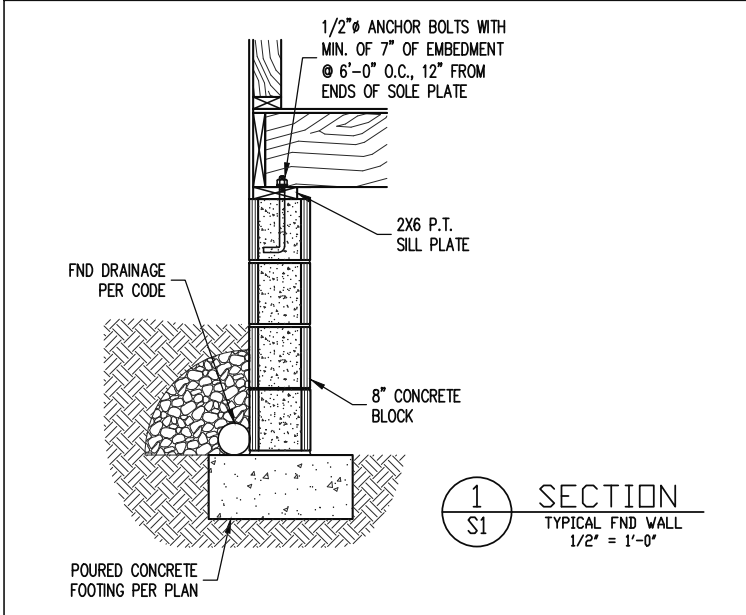
-ROOF TRUSSES PER MANUFACTURER, TYP U.N.O.

-VERIFY ROOF PITCHES, OVERHANG LENGTHS, AND KNEEWALL FRAMING HGTS WITH ARCHITECTURAL DRAWINGS, TYPICAL

ROOF FRAMING PLAN  
1/8" = 1'-0"

VALUE BUILD HOMES			
SCOPE	STRUCTURAL ADDENDUM		
LOC	580 GRIFFIN RD	REV 1	EAF
			07-16-2024
JOB# 2024-SAN-063 VASQUEZ			

ENG	PAL/NBG
DATE	11/26/2024
PLAN	
JUDY	
PROJECT NO.	
24-26-163	
SHEET NO.	
S4	
4 of 7	



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VALUE BUILD HOMES				
STRUCTURAL ADDENDUM				
SCOPE				
LOC	580 GRIFFIN RD	REV 1	EAF	07-16-2024
LILLINGTON, NC				
JOB# 2024-SAN-063 VASQUEZ				

ENG: PAL/NBG  
DATE: 11/26/2024

PLAN  
JUDY

PROJECT NO.  
24-26-163

SHEET NO.  
SD1  
5 of 7

CONSTRUCTION SPECIFICATIONS

PART 1: GENERAL

- 1.01 CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE NORTH CAROLINA RESIDENTIAL CODE, 2018 EDITION.
- 1.02 DIMENSIONS SHOWN SHALL GOVERN OVER SCALE ON THESE DRAWINGS.
- 1.05 METHODS, PROCEDURES AND SEQUENCES OF CONSTRUCTION ARE THE RESPONSIBILITY OF THE CONTRACTOR, WHO SHALL TAKE ALL NECESSARY PRECAUTIONS TO MAINTAIN AND INSURE THE INTEGRITY OF THE STRUCTURE AT ALL STAGES OF CONSTRUCTION.

PART 2: DESIGN LOADS

- 2.01 DESIGN LOADS SHALL CONFORM WITH THE TABLE BELOW:
- | USE   | LIVE LOAD (PSF) | DEAD LOAD (PSF)    |
|---|-----------------|--------------------|
| BALCONIES, DECKS, ATTICS WITH FIXED STAIR ACCESS, DWELLING UNITS INCLUDING ATTICS WITH FIXED STAIR ACCESS, STAIRS, FIRE ESCAPES | 40              | 10                 |
| GARAGES (PASSENGER CARS ONLY)   | 50              | --                 |
| ATTICS (NO STORAGE, LESS THAN 5' HEADROOM)  | 10              | 10                 |
| ATTICS (WITH STORAGE)   | 20              | 10                 |
| ROOF  | 20              | 10 (15 FOR VAULTS) |

NOTES: - INDIVIDUAL STAIR TREADS ARE TO BE DESIGNED FOR THE UNIFORMLY DISTRIBUTED LIVE LOAD OF 40 PSF OR A 300 LB. CONCENTRATED LOAD ACTING OVER AN AREA OF 4 SQ. WHICHEVER PRODUCES THE GREATER STRESS.  
- BUILDER TO VERIFY DEAD LOAD DOES NOT EXCEED 10 PSF WHEN HEAVY FLOOR OR ROOF FINISHES SUCH AS TILE OR SLATE ARE UTILIZED. NOTIFY ENGINEERING UNDER THESE CONDITIONS

- 2.02 INTERIOR WALLS: 5 PSF LATERAL.
- 2.03 BASIC WIND DESIGN VELOCITY OF 120 MPH.
- 2.04 SOIL BEARING CAPACITY 2000 PSF (PRESUMPTIVE).

PART 3: STRUCTURAL STEEL

- 3.01 WIDE FLANGE BEAMS AND TEE SECTIONS SHALL CONFORM TO ASTM A992 MINIMUM GRADE
- 3.02 SQUARE AND RECTANGULAR TUBING SHALL CONFORM TO ASTM A500 GRADE B MINIMUM GRADE.
- 3.03 STEEL PIPE SHALL CONFORM TO ASTM A53 GRADE B, TYPE S, MINIMUM GRADE
- 3.04 ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36 MINIMUM GRADE
- 3.05 STRUCTURAL STEEL CONSTRUCTION SHALL MEET THE REQUIREMENTS OF THE AISC SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS.

PART 4: WELDING

- 4.01 WELDING ELECTRODES SHALL BE E70XX AND ALL WELDING SHALL BE PERFORMED BY AN AWS CERTIFIED WELDER

PART 5: CONCRETE AND SLABS ON GRADE

- 5.01 CAST IN PLACE CONCRETE SHALL BE OF NORMAL WEIGHT, 6% AIR ENTRAINMENT, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS TYP UNO. ALL CONCRETE, INCLUDING CONCRETE FOR FOOTINGS, IS TO BE CAST IN PLACE, TYP UNO.
- 5.02 REINFORCED CAST IN PLACE CONCRETE SHALL BE PROPORTIONED, MIXED AND PLACED IN ACCORDANCE WITH THE SPECIFICATIONS OF ACI 318, LATEST EDITION.
- 5.03 SLABS ON GRADE, IF ANY, SHALL CONTAIN SYNTHETIC POLYPROPYLENE FIBRILLATED MICRO FIBERS, FIBER LENGTH 1 1/2", DOSAGE RATE 1 1/2 LBS/CU YD. SLAB TO BE PLACED ON A 6 MIL VAPOR BARRIER ON 2" MIN GRANULAR FILL ON SOIL WITH 90% MIN STANDARD PROCTOR DENSITY. VAPOR BARRIER MAY BE OMITTED FOR SLABS NOT IN ENCLOSED AREAS

PART 6: REBAR AND WIRE REINFORCEMENT

- 6.01 REBAR SHALL BE DEFORMED STEEL CONFORMING TO ASTM A615 GRADE 60 TYP UNO
- 6.02 LAP SPLICES SHALL BE CLASS B AS DEFINED BY ACI 318, TYP UNO
- 6.03 WIRE REINFORCEMENT SHALL BE 9 GA AND SHALL CONFORM TO ASTM A1064.

PART 7: MASONRY

- 7.01 CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 AND C55, NORMAL WEIGHT, f'm = 1,500 PSI MIN
- 7.02 CLAY MASONRY UNITS SHALL CONFORM TO ASTM C62-17 GRADE SW
- 7.03 MORTAR SHALL BE TYPE S. MORTAR AND GROUT SHALL CONFORM TO ASTM C476, MIN COMPRESSIVE STRENGTH OF 2000 PSI.
- 7.04 MASONRY CONSTRUCTION SHALL CONFORM TO THE SPECIFICATIONS OF ACI 530
- 7.05 LADDER WIRE REINFORCEMENT SHALL CONFORM TO ASTM A951. 6" MIN LAPS FOR CONTINUOUS WALL APPLICATIONS

PART 8: BOLTS AND LAG SCREWS

- 8.01 BOLTS SHALL CONFORM TO ASTM A307 MINIMUM GRADE TYP UNO. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR THE NUT / BOLT HEAD WHEN BOLTING WOOD MEMBERS
- 8.02 LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981. PILOT HOLES SHALL BE USED FOR LAG SCREW INSTALLATION AND SHALL BE BORED ACCORDING TO NDS SPECIFICATIONS. INSTALL STANDARD STEEL WASHERS (ASTM F844-07a) FOR SCREW HEAD
- 8.03 ANCHOR RODS AND BOLTS SHALL CONFORM TO ASTM F1554-15 GRADE 36 UNO. BENT ANCHOR BOLTS SHALL HAVE A 2" MIN HOOK UNO

PART 9: DRIVEN FASTENERS

- 9.01 NAILS, SPIKES AND STAPLES SHALL CONFORM TO ASTM F 1667- 05. NAILS ARE TO BE COMMON WIRE OR BOX

PART 10: DIMENSIONAL LUMBER

- 10.01 SOLID SAWN WOOD FRAMING DESIGN IS BASED ON NO. 2 SPRUCE PINE FIR OR SYP #2 FOR JOISTS, RAFTERS, GIRDERS, BEAMS, STUDS, ETC.

PART 11: ENGINEERED LUMBER

- 11.01 LVL OR PSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS:  
E= 1.9 X 10E6 PSI, Fb = 2600 PSI, Fv = 285 PSI, Fc = 750 PSI  
LSL MINIMUM ALLOWABLE DESIGN STRESSES ARE AS FOLLOWS:  
E= 1.3 X 10E6 PSI, Fb = 1700 PSI, Fv = 400 PSI, Fc = 680 PSI
- 11.02 LVL OR PSL MEMBERS MAY BE RIPPED FROM DEEPER MEMBERS TO MATCH THE MEMBER DEPTH SPECIFIED IN THE PLANS

PART 12: PRESSURE TREATED LUMBER

- 12.01 LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR MASONRY SHALL BE PRESSURE TREATED IN ACCORDANCE WITH AWPA STANDARD C-15. ALL OTHER EXPOSED LUMBER SHALL BE TREATED IN ACCORDANCE WITH AWPA STANDARD C-2 OR BY ANY METHOD GIVING EQUAL PROTECTION. THE BUILDING CODE OFFICE MAY ALSO APPROVE A NATURAL DECAY RESISTANT WOOD PER SECTION 19-6(A)

PART 13: STEEL FLITCH PLATE BEAMS

- 13.01 FLITCH PLATE BEAMS SHALL CONSIST OF A CONTINUOUS STEEL PLATE BOLTED BETWEEN TWO PIECES OF CONTINUOUS LUMBER AS SIZED ON THE PLANS. BOLT PIECES TOGETHER USING 1/2" Ø BOLTS SPACED AT 24" O.C. STAGGERED TOP TO BOTTOM OF THE BEAM. MAINTAIN A 2" EDGE DISTANCE. PLACE TWO BOLTS, ONE ABOVE THE OTHER, 6" ± 2" FROM EACH END OF THE BEAM.

PART 14: STUD SUPPORTS FOR BEAMS

- 14.01 STEEL, ENGINEERED LUMBER, AND FLITCH PLATE BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:  
  
1-WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR FULL WIDTH ON THE SUPPORTING WALL INDICATED AND SHALL BE SUPPORTED BY A MINIMUM OF THREE GANGED STUDS, OR A GANGED STUD COLUMN WITH A NUMBER OF STUDS SUCH THAT THE STUD COLUMN IS AT LEAST AS WIDE AS THE TRUE WIDTH OF THE BEAM BEING SUPPORTED, WHICHEVER IS GREATER, TYP UNO. FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM  
2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 4 1/2" ONTO THE WALL AND BE SUPPORTED BY A TRPL STUD GANGED COLUMN TYP UNO.
- 14.02 DIMENSIONAL LUMBER BEAMS BEARING ON A STUD WALL SHALL BEAR AS FOLLOWS:  
  
1-WHEN THE BEAM IS PERPENDICULAR TO, OR SKEWED RELATIVE TO THE WALL, THE BEAM SHALL BEAR FULL WIDTH ON THE SUPPORTING WALL INDICATED (LESS 1 1/2" TO ALLOW FOR A CONTINUOUS RIM JOIST WHERE APPLICABLE) AND SHALL BE SUPPORTED BY A

GANGED STUD COLUMN THE SAME WIDTH AS THE BEAM TYP UNO. (E.G. A TRIPLE 2X10 IS TO BE SUPPORTED BY (3) STUDS). FOR THE SKEWED CONDITION PARTICULAR CARE SHALL BE TAKEN TO ENSURE STUD COLUMN IS CENTERED ON THE BEAM  
2-BEAMS BEARING ONTO THE END OF A STUD WALL PARALLEL TO THE BEAM SHALL BEAR A MINIMUM OF 3" ONTO THE WALL AND BE SUPPORTED BY A DBL STUD GANGED COLUMN TYP UNO.

- 14.03 EXTRA JOISTS BEARING ON A STUD WALL PERPENDICULAR TO OR SKEWED RELATIVE TO THE BEAM SHALL BE SUPPORTED BY ONE ADDITIONAL STUD.
- 14.04 STUDS THAT ARE GANGED TO FORM A COLUMN SHALL HAVE ADJACENT STUDS WITHIN THE COLUMN NAILED TOGETHER WITH ONE ROW OF 10d NAILS AT 8" O.C. (TWO ROWS OF 10d NAILS @ 8" O.C., 3" APART, FOR 2X8 OR 2X10 STUDS) ALL COLUMNS SHALL BE CONTINUOUS DOWN TO THE FOUNDATION OR OTHER PROPERLY DESIGNED STRUCTURAL ELEMENT SUCH AS A BEAM. COLUMNS TRANSFERRING LOADS THROUGH FLOOR LEVELS SHALL BE SOLIDLY BLOCKED FOR THE FULL WIDTH OF THE STUD COLUMN WITHIN THE CAVITY FORMED BY THE FLOOR JOISTS.

PART 15: NAILING OF MULTI PLY WOOD BEAMS

- 15.01 SOLID SAWN LUMBER JOISTS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM NAILED TOGETHER WITH THREE ROWS OF 10d NAILS @ 16" O.C. FOR 2X10 OR LARGER, TWO ROWS OF 10d NAILS @ 16" O.C. FOR 2X8, ONE ROW OF 10d NAILS @ 16" O.C. FOR 2X6 OR SMALLER. STAGGER ROWS 5" MIN.
- 15.02 LVL MEMBERS THAT ARE GANGED TO FORM A BEAM SHALL HAVE ADJACENT MEMBERS IN THE BEAM FASTENED TOGETHER PER MANUFACTURERS RECOMMENDATIONS, TYP UNO

PART 16: WALL FRAMING AND BRACING

- 16.01 STUD WALLS SHALL CONSIST OF 2X4 STUDS SPACED AT 16" O.C. UNO. STUDS SHALL BE CONTINUOUS FROM SOLE PLATE AT FLOOR TO DOUBLE TOP PLATE AT THE CEILING OR ROOF. NO INTERMEDIATE BANDS OR PLATES SHALL CAUSE DISCONTINUITIES IN A STUD WALL EXCEPT AS REQUIRED FOR DOOR OR WINDOW OPENINGS. THE KING STUDS FOR SUCH OPENINGS SHALL BE CONTINUOUS, TYP UNO.  
**MAX ALLOWABLE WALL HEIGHTS** FOR EXTERIOR STUD WALLS, WITH SOLE PLATE AND DBL TOP PLATE AND 7/16" OSB EXTERIOR BRACING AND ROW OF 2X4 / 2X6 PURLINS AT 8' HEIGHT (AND AT 16' HEIGHT FOR TALL WALLS), TYP UNO:  
2X4 @ 16" O.C.: 11'-1 1/2"    2X6 @ 16" O.C.: 17'-0"  
2X4 @ 12" O.C.: 12'-1 1/2"    2X6 @ 12" O.C.: 18'-8"  
DBL 2X4 @ 16" O.C.: 13'-4"    DBL 2X6 @ 16" O.C.: 21'-0"
- 16.02 FOR WALL BRACING THE FOLLOWING SHALL APPLY:  
-BLOCKING AT UNSUPPORTED PANEL EDGES IS REQUIRED TYP UNO.  
-WALL BRACING IS BY ENGINEERED DESIGN AND NOT PRESCRIPTIVE PER SECTION 602.10 OF THE 2018 NCR. CONTINUOUS SHEATHING HAS BEEN PROVIDED, ALONG WITH ALTERNATIVE METHODS TO INSURE THE MINIMUM INTENT OF SECTION 602.10 OF THE 2018 NCR. HAS BEEN MET AND EXCEEDED.  
-BRACED WALL PANELS SHALL BE FASTENED IN ACCORDANCE WITH TABLE 602.3(1) TO PROVIDE CONTINUOUS PANEL UPLIFT RESISTANCE AND COMPLIANCE WITH NCRBC R602.3.5 AND R802.11 UNLESS NOTED OTHERWISE ON STRUCTURAL PLANS.  
-MAY SUBSTITUTE WSP FOR GB  
-SINGLE JOIST, CONTINUOUS RIM JOIST, OR BLOCKING OF EQUAL DEPTH IS REQUIRED ABOVE AND BELOW ALL BRACED WALLS. NAIL BLOCKING ABOVE WALL TO TOP PLATE WITH 16d TOE NAILS @ 6" O.C. NAIL SOLE PLATE OF BRACED WALL TO BLOCKING BELOW WITH (3) 16d NAILS @ 16" O.C. BLOCKING AT HORIZONTAL JOINTS IN BRACED WALL LINES ONLY REQUIRED AT SHADED WALLS, UNO.

PART 17: KING STUDS

- 17.01 KING STUDS FOR OPENINGS IN EXTERIOR WALLS SHALL BE AS FOLLOWS:
- |                   | NUMBER OF KING STUDS |       |        |        |        |
|-------------------|----------------------|-------|--------|--------|--------|
| MAX OPENING WIDTH | 5'-0"                | 9'-0" | 13'-0" | 17'-0" | 21'-0" |
| 2X4               | 1                    | 2     | 3      | 4      | 5      |
| 2X6               | 1                    | 1     | 2      | 2      | 2      |
| 2X8               | 1                    | 1     | 1      | 1      | 2      |

PART 18: SUBSTITUTIONS

- 18.01 MATERIAL OR MEMBER SIZE SUBSTITUTIONS OR PLAN DEVIATIONS REQUIRE THE WRITTEN AUTHORIZATION OF THE DESIGNERS. UNAUTHORIZED DEVIATIONS ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

PART 19: OWNERSHIP OF STRUCTURAL DESIGN

- 19.01 THE STRUCTURAL DESIGN OF THIS PLAN IS THE PROPERTY OF ENGINEERING TECH ASSOCIATES (ETA). THESE PLANS ARE FOR THE ONE TIME USE AT THE LOCATION INDICATED AND FOR THE CLIENT LISTED. ETA ASSUMES NO LIABILITY FOR THESE PLANS IF THEY ARE REPRODUCED, IN WHOLE OR IN PART, FOR CONSTRUCTION AT ANY OTHER LOCATION WITHOUT WRITTEN PERMISSION FROM ETA

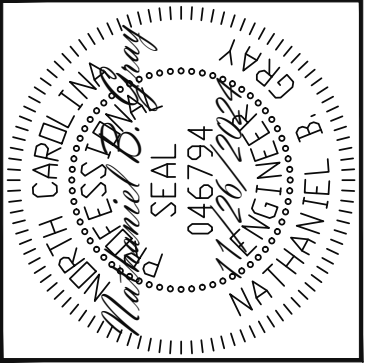
ABBREVIATIONS

FOUNDATION	TJ	TRIPLE JOIST
FOOTING	TYP	TYPICAL
HOT DIPPED	TRPL	TRIPLE
GALVANIZED	TSP	TRIPLE STUD POCKET
HANGER	UNO	UNLESS NOTED OTHERWISE
LAMINATED VENEER LUMBER	XJ	EXTRA JOIST
NOT TO SCALE		
ON CENTER		
PARALLEL STRAND LUMBER		
PRESSURE TREATED		
QUAD JOIST		
STUD POCKET		
SQUARE		

ABOVE	ABV
BOTH	B.
BOTH ENDS	B.E.
BETWEEN	BTWN
CAST IN PLACE	CIP
CONCRETE	CONC
CONTINUOUS SHEATHING	CS
DIAMETER	D/A
DOUBLE	DBL
DOUBLE JOIST	DJ
DOUBLE STUD POCKET	DSP
EQUAL	EQ
EACH	EA
FLANGE	FLG
FLITCH PLATE	FL PL
FLOOR	FLR

NOTES

THE BUILDER IS RESPONSIBLE FOR REVIEWING PLANS PRIOR TO CONSTRUCTION. THE BUILDER SHALL IMMEDIATELY CONTACT THE ENGINEER OF RECORD (EOR) BEFORE PROCEEDING IF THE FOLLOWING CONDITIONS ARE NOTED BEFORE OR DURING CONSTRUCTION:  
1) THE WORKING PLANS DO NOT BEAR THE SEAL OF THE EOR  
2) THE PLANS CONTAIN DISCREPANT OR INCOMPLETE INFORMATION  
ANY ERRORS DUE TO A FAILURE TO FOLLOW THE ABOVE PROCEDURES SHALL NOT BE THE RESPONSIBILITY OF THE EOR. FURTHERMORE, IT IS THE RESPONSIBILITY OF THE BUILDER TO ENSURE THAT ANY REVISIONS ISSUED BY THE EOR ARE PROMPTLY DISTRIBUTED TO THE SUBCONTRACTORS  
THE EOR DOES NOT PERFORM FENESTRATION OR VENTING CALCULATIONS OR ANY OTHER CALCULATIONS THAT ARE NOT DIRECTLY RELATED TO STRUCTURAL ENGINEERING.  
ROOF AND FLOOR TRUSSES TO BE DESIGNED BY AN ENGINEER REGISTERED BY THE STATE. FINAL TRUSS DRAWING SHOULD BE SUBMITTED TO THE EOR FOR REVIEW



Engineering

STRUCTURAL ENGINEERS

License No. C-3870

318 W MILLBROOK RD, SUITE 201

Raleigh, North Carolina 27609

Phone (919) 844-1661

ASSOCIATES, P.A.

VALUE BUILD HOMES	
STRUCTURAL ADDENDUM	
SCOPE	REV 1
LOC	580 GRIFFIN RD
	LILLINGTON, NC
	JOB# 2024-SAN-063 VASQUEZ

ENG	PAL/NBG
DATE	11/26/2024

PLAN
JUDY

PROJECT NO.
24-26-163

SHEET NO.
SPECS1

DECK SPECIFICATIONS

1. A DECK IS AN EXPOSED EXTERIOR WOOD FLOOR STRUCTURE WHICH MAY BE ATTACHED TO A STRUCTURE OR BE FREE STANDING. ROOFED PORCHES, OPEN OR SCREENED IN, MAY BE CONSTRUCTED USING THESE PROVISIONS.
2. SUPPORT POSTS SHALL BE SUPPORTED BY A FOOTING.
3. WHEN ATTACHED TO A STRUCTURE, THE STRUCTURE TO WHICH ATTACHED SHALL HAVE A TREATED WOOD BAND FOR THE LENGTH OF THE DECK, OR CORROSION RESISTANT FLASHING SHALL BE USED TO PREVENT MOISTURE FROM COMING IN CONTACT WITH THE UNTREATED FRAMING OF THE STRUCTURE. THE DECK BAND AND THE STRUCTURE BAND SHALL BE CONSTRUCTED IN CONTACT WITH EACH OTHER EXCEPT AT BRICK VENEER AND WHERE PLYWOOD SHEATHING IS REQUIRED AND PROPERLY FLASHED. SIDING SHALL NOT BE INSTALLED BETWEEN THE STRUCTURE AND THE DECK BAND. IF ATTACHED TO A BRICK STRUCTURE, NEITHER FLASHING NOR A TREATED BAND FOR THE BRICK STRUCTURE IS REQUIRED. IN ADDITION, THE TREATED DECK BAND SHALL BE CONSTRUCTED IN CONTACT WITH THE BRICK
4. WHEN THE DECK IS SUPPORTED AT THE STRUCTURE BY ATTACHING THE DECK TO THE STRUCTURE, THE FOLLOWING ATTACHMENT SCHEDULES SHALL APPLY FOR ATTACHING THE DECK BAND TO THE STRUCTURE:

A. ALL STRUCTURES EXCEPT BRICK STRUCTURES

REQUIRED FASTENERS	JOIST LENGTH	
	UP TO 8' MAX.	UP TO 16' MAX.
	ONE- 5/8" Ø BOLT @ 42" O.C. AND (2) ROWS OF 12d NAILS @ 8" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 32" O.C. STAGGERED	ONE- 5/8" Ø BOLT @ 20" O.C. AND (3) ROWS OF 12d NAILS @ 6" O.C. OR TWO ROWS OF SIMPSON SDWS22400DB @ d = 16" O.C. STAGGERED

A . BRICK VENEER STRUCTURES

REQUIRED FASTENERS	JOIST LENGTH	
	UP TO 8' MAX.	UP TO 16' MAX.
	ONE- 5/8" Ø BOLT @ 28" O.C.	ONE- 5/8" Ø BOLT @ 16" O.C.

5. IF THE DECK BAND IS SUPPORTED BY A 1/2" MINIMUM MASONRY LEDGE ALONG THE FOUNDATION WALL, 5/8" Ø BOLTS SPACED @ 48" O.C. MAY BE USED FOR SUPPORT.
6. OTHER MEANS OF SUPPORT, SUCH AS JOIST HANGERS, MAY BE USED TO CONNECT DECK JOISTS TO A TREATED STRUCTURE BAND
7. GIRDERS SHALL BEAR DIRECTLY ON POSTS OR BE BE CONNECTED TO THE SIDES OF POSTS WITH 2- 5/8" Ø BOLTS
8. FLOOR DECKING SHALL BE NO. 2 GRADE TREATED SOUTHERN PINE OR EQUIVALENT. THE MINIMUM FLOOR DECKING THICKNESS SHALL BE AS FOLLOWS:

JOIST SPAN	DECKING
12" O.C.	1" S4S
16" O.C.	1" T&G
24" O.C.	1 1/4" S4S
32" O.C.	2" S4S

9. MAXIMUM HEIGHT OF DECK SUPPORT POSTS IS AS FOLLOWS:

POST SIZE	MAX POST HEIGHT
4X4	8'
6X6	20'
ENGINEERED	20' +

NOTES: 1) THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS.  
2) THIS TABLE IS BASED ON A MAXIMUM TRIBUTARY AREA OF 128 SQ. FT.  
3) POST HEIGHT IS FROM TOP OF FOOTING TO BOTTOM OF GIRDER.

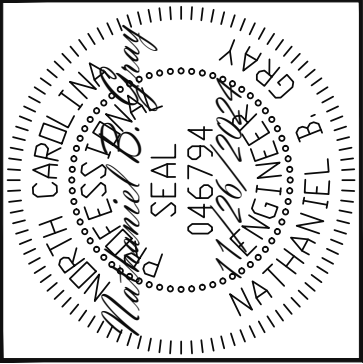
10. DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF THE FOLLOWING METHODS:

- A. WHEN THE DECK FLOOR HEIGHT IS LESS THAN 4'-0" AND THE DECK IS ATTACHED TO THE STRUCTURE IN ACCORDANCE WITH SECTION 4, LATERAL BRACING IS NOT REQUIRED.
- B. 4X4 WOOD KNEE BRACES MAY BE PROVIDED ON EACH COLUMN IN BOTH DIRECTIONS. THE KNEE BRACES SHALL ATTACH TO EACH POST AT A POINT NOT LESS THAN 1/3 OF THE POST LENGTH FROM THE TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL. KNEE BRACES SHALL BE ATTACHED AT THE ENDS TO THE GIRDER AND THE POST WITH ONE - 5/8"Ø BOLT
- C. FOR FREE STANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN CONCRETE IN ACCORDANCE WITH THE FOLLOWING:

POST SIZE	TRIBUT. AREA	POST HEIGHT	EMB. DEPTH	CONC. DIAM.
4X4	48 SQ. FT.	4'-0"	2'-6"	1'-0"
6X6	120 SQ. FT.	6'-0"	3'-6"	1'-8"

- D. 2X6 DIAGONAL VERTICAL CROSS BRACING SHALL BE PROVIDED IN TWO PERPENDICULAR DIRECTIONS FOR FREE STANDING DECKS OR PARALLEL TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS. THE BRACES SHALL BE ATTACHED TO THE POSTS WITH ONE - 5/8" Ø BOLT AT EACH END OF THE BRACE.

NOTES: 1) ALL NAILS AND BOLTS ARE TO BE HOT DIPPED GALVANIZED.  
2) MINIMUM EDGE DISTANCE FOR BOLTS IS 2 1/2".  
3) NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF 1 1/2".



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318 W MILLBROOK RD, SUITE 201  
Raleigh, North Carolina 27609  
Phone (919) 844-1661

Engineering Associates, P.A.

VALUE BUILD HOMES	
SCOPE	STRUCTURAL ADDENDUM
	REV 1 EAF 07-16-2024
LOC	580 GRIFFIN RD LILLINGTON, NC
JOB# 2024-SAN-063 VASQUEZ	

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SHEET NO.  
SPECS2  
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