# Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 54645 JOB: 24-9832-R01 JOB NAME: LOT 0.0035 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2018 as well as IRC 2021. 33 Truss Design(s)

Trusses:

J01, J02, J03, R01, R02, R02A, R02B, R02C, R03, R03A, R03B, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R19, R20, R21, VT01, VT02, VT03, VT04, VT05, VT06,



## Warning !--- Verify design parameters and read notes before use.



responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHE	ELBY MEADOW LANE ANGIER, NO
24-9832-R01	J02	Monopitch Structural Gable	1	1	Job Reference (optional)	# 54645
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LOAD CASE(S) Standard

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#### LOAD CASE(S) Standard

Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

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Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHELBY M	EADOW LANE ANGIER, NO
24-9832-R01	R02	Roof Special	3	1	Job Reference (optional)	# 54645
		Run: 8. ID:V	630 s Jul 12 VI8rkg6BK	2024 Print SaRYCY	: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed No Gf9_0xywFJ5-GnWnkaTqkIKrq1PZReogebfB	v 27 20:44:04 2024 Page 2 mgeaB03tN?_C2dyEtsP

**NOTES-** (12) 11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

LOAD CASE(S) Standard





vertically. Applicability of design parameters and read notes before use. This begin is obset only upon parameters shown, and is for an individual voluting component to be instanted and loaded of individual web members only. Additional permanent bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Trusse Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.







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Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHELBY	MEADOW LANE ANGIER, NC
24-9832-R01	R07	COMMON GIRDER	1	2	Job Reference (optional)	# 54645

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### LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-51, 3-5=-51, 9-12=-20 Concentrated Loads (b)

Vert: 15=-1101(F) 16=-1102(F) 17=-1102(F) 18=-1102(F) 19=-1102(F) 20=-1092(F) 21=-1092(F) 22=-1192(F) 23=-1192(F)







of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHE	LBY MEADOW LANE ANGIER, NO
24-9832-R01	R10	GABLE	1	1	Job Reference (optional)	# 54645
		Run 8 f	630 s. Jul 12	2024 Print	8 630 s Jul 12 2024 MiTek Industries Inc. V	Ved Nov 27 20:44:26 2024 Page 2

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- 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate
- Connected Wood Trustees for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHELBY MEAD	OW LANE ANGIER, NO
24-9832-R01	R11	Common	6	1	Job Reference (optional) #	4 54645
		Run: 8	.630 s Jul 12 ID:pMqJz	2024 Print 2024 Print	: 8.630 s Jul 12 2024 MiTek Industries, Inc. Wed Nov 27 WiSfiGO4QyyWlk-ZPzrmnlt5gMRijFBrgDIdfeQ9Ls	20:44:28 2024 Page 2 sEp0cPvkoTvEyEts1

LOAD CASE(S) Standard



Job	Truss	Truss Type		Qty	Ply	LOT 0.0035 HC	NEYCUTT HI	LLS   304 SHELBY MEADOW	LANE ANGIER, N
24-9832-R01	R12	GABLE		1	1			#	54645
		-0-10 <sub>1</sub> 8 0-10-8	<u>6-8-1</u> 6-8-1	Run: 8.630 s Jul 1 ID:Wl8rkg6BK 13-{ 6-9	2 2024 Print 5SaRYCY 5-8 -7	Job Referenc :: 8.630 s Jul 12 2 Gf9_0xywFJ5- 	e (optional) 2024 MiTek Ind 1bXDz7mVr_	Justries, Inc. Wed Nov 27 20: UIKtqNOOIX9tBYWIIWY	44:29 2024 Page 1 S3Y7OY1RgyEts0
					3x4	- 11			Scale = 1:61.1
		1		9.00 12		4			
		9-2-00 	5x6 ≠ 3 5 1 5 1 5 1 5 1 2 6 98	T ST4 ST4 ST5 ST 7 27	6 ST8	₩ <sup>3</sup> 3x4 =			
		4,44 —	0.01	10.7		10 5 0			
Plata Offaata (X.V.)		<u>  2-2-</u> 2-2-	0 6-8-1 0 4-6-1	5-11-7	7	13-5-8 0-10-0			
LOADING (psf) TCLL (roof) Snow (Pf/Pg) 15.4/ TCDL BCLL BCDL	20.0 /20.0 10.0 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.46 BC 0.48 WB 0.21 Matrix-AS	DEFL. Vert(LL Vert(CT Horz(C	in ) -0.02 ) -0.05 Г) 0.01	(loc) l/defl 6-7 >999 6-7 >999 5 n/a	L/d 240 180 n/a	PLATES MT20 Weight: 126 I	<b>GRIP</b> 244/190 b FT = 20%
LUMBER-	10.0			BRACING-					
TOP CHORD 2x4 BOT CHORD 2x4 WEBS 2x4	SP No.2 SP No.3 SP No.3			TOP CHORD BOT CHORD WEBS	Structur Rigid ce 1 Row a	al wood shea ailing directly a at midpt	thing directl applied. 4-5, 3-4	ly applied, except end 5	verticals.
OTHERS 2x4 WEDGE Left: 2x4 SP No.3	SP No.3				MiTek be inst Installa	recommends alled during t ation guide.	that Stabili russ erectio	zers and required cross on, in accordance with S	s bracing Stabilizer
REACTIONS. AI (Ib) - Ma: Ma: Ma:	l bearings 2-5-8 ex x Horz 2=322(LC 1 x Uplift All uplift 10 x Grav All reaction 1)	ccept (jt=length) 5=1-1-8, 8=0- 2) 00 lb or less at joint(s) 8 excep ns 250 lb or less at joint(s) 9, 8	3-8, 6=0-3-8. ot 5=-261(LC 12), 9=-2 3 except 2=473(LC 24	276(LC 12) ), 5=468(LC 24),	6=269(LC	5), 2=396(L0	C		
FORCES. (Ib) - M TOP CHORD 2 BOT CHORD 2 6 WEBS 3-	ax. Comp./Max. Te 3=-592/0 9=-152/463, 8-9=- <sup>-</sup> 27=-152/461, 5-6= 7=0/292, 3-5=-579	en All forces 250 (lb) or less 152/463, 8-26=-152/463, 7-26 152/461 /189	except when shown. =-152/463, 7-27=-152	/461,					
NOTES- (11-14) 1) Wind: ASCE 7-1 (envelope) gable members and fo	16; Vult=120mph ( e end zone and C- prces & MWFRS fo	3-second gust) Vasd=95mph; C Exterior(2E) -0-10-8 to 3-11 r reactions shown; Lumber D(	TCDL=5.0psf; BCDL= -2, Interior(1) 3-11-2 to DL=1.60 plate grip DC	=5.0psf; h=23ft; C o 8-6-2, Exterior(: DL=1.60	at. II; Exp 2E) 8-6-2	B; Enclosed; to 13-3-12 zo	MWFRS ne;C-C for		

Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
 TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15);

Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.

5) All plates are 2x4 MT20 unless otherwise indicated.

6) Gable studs spaced at 2-0-0 oc.

7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 8 except (jt=lb) 5=261, 9=276

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

PROFESS MANNAHIH WALL SEAL 28147 MORAS MULTINIA AND NOINER K. MORR 11/27/2024

Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHE	LBY MEADOW LANE ANGIER, NO
24-9832-R01	R12	GABLE	1	1	Job Reference (optional)	# 54645
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- Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
  Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
   SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS
- 14) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard





of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison WI 53719



LOADING (psf) TCLL (roof) 20.0 Snow (Pf/Pg) 15.4/20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	<b>CSI.</b> TC 0.17 BC 0.19 WB 0.06 Matrix-S	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) 0.01 1 0.01 1 0.00 7	l/defl n/r n/r n/a	L/d 180 80 n/a	PLATES MT20 Weight: 41 lb	<b>GRIP</b> 244/190 FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3 OTHERS 2x4 SP No.3		<b>B</b>	RACING- OP CHORD S OT CHORD F	Structural wood end verticals. Rigid ceiling di MiTek recomi be installed d Installation qu	d sheath rectly ap mends th uring tru lide.	ing directly plied or 10 nat Stabiliz ss erection	r applied or 6-0-0 oc pr -0-0 oc bracing. ers and required cross n, in accordance with S	urlins, except s bracing Stabilizer

REACTIONS. All bearings 9-11-8.

(lb) - Max Horz 2=84(LC 13)

Max Uplift All uplift 100 lb or less at joint(s) 7, 2, 10, 9, 8 Max Grav All reactions 250 lb or less at joint(s) 7, 2, 9, 8 except 10=315(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(12-15)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) -0-10-8 to 4-0-0, Exterior(2N) 4-0-0 to 5-0-2, Corner(3E) 5-0-2 to 9-9-12 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10; Min. flat roof snow load governs.
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 6) All plates are 2x4 MT20 unless otherwise indicated.
- 7) Gable requires continuous bottom chord bearing.

- (1) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  (1) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will CAROUT if between the bottom chord and any other members.
  (1) Provide mechanical connection (by others) of truss to bearing plate complete structure.
- 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the
- structural design of the truss to support the loads indicated.
- 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
- AUDRITURN STATE 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS
- MORPHS INTERNAL STREET Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded LOAD DASE(\$)). Standardlesign parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive Madison WI 53719

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Job	Truss	Truss Type	Qty	Ply	LOT 0.0035 HONEYCUTT HILLS   304 SHELBY M	IEADOW LANE ANGIER, NO
24-9832-R01	R15	Monopitch	5	1	Job Reference (optional)	# 54645

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8) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).

9) Gable studs spaced at 2-0-0 oc.

- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- \* This muss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads. \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will the second s 11)
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 16, 10, 14, 15, 12 11

LOAD CASE(S) Standard

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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=129, 6=129.

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LOAD CASE(S) Standard
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FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES- (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



<sup>1)</sup> Unbalanced roof live loads have been considered for this design.

<sup>2)</sup> Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



Max Grav 1=85(LC 2), 3=85(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15);

Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

**NOTES-** (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

## NOTES- (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



<sup>1)</sup> Unbalanced roof live loads have been considered for this design.



Max Uplift1=-7(LC 12), 3=-7(LC 13) Max Grav 1=101(LC 2), 3=101(LC 2)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

