

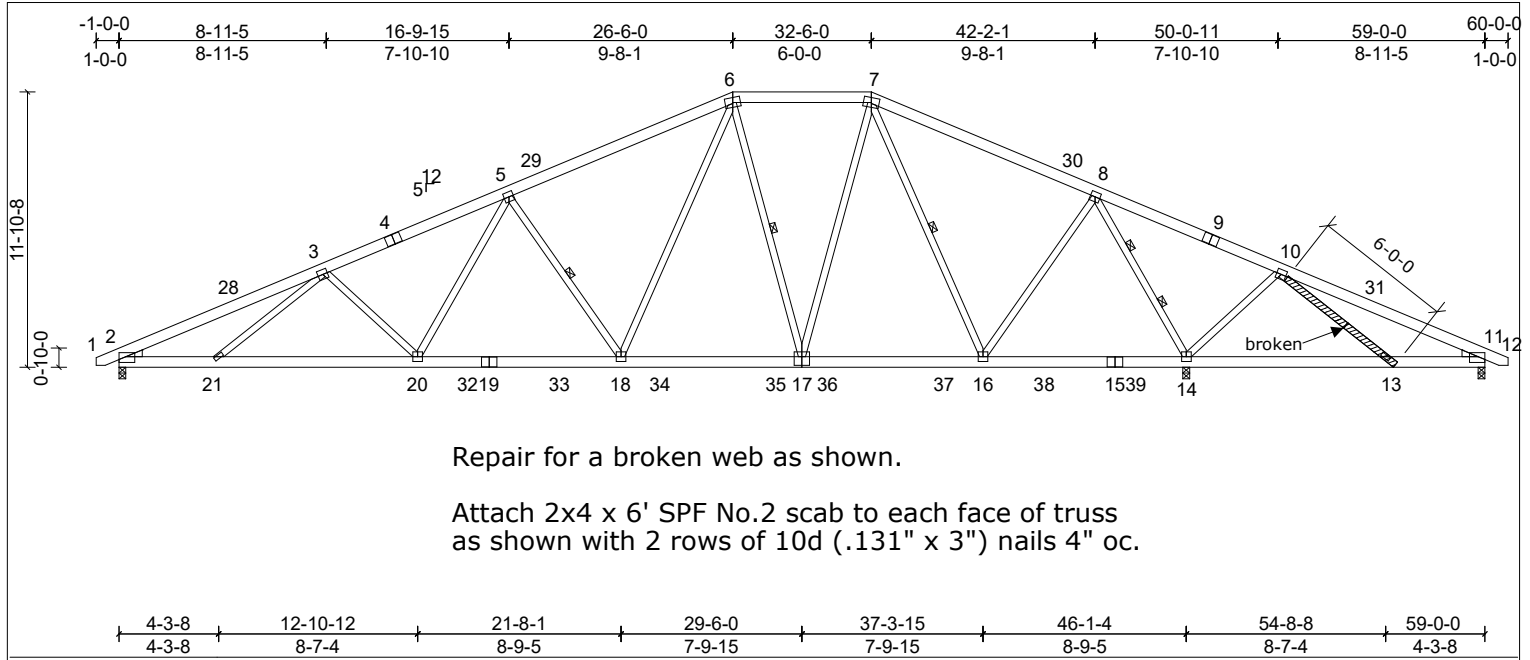
Job 72433825REP1	Truss A2	Truss Type Truss	Qty 6	Ply 1	S D-CRAWFORD ADG Job Reference (optional)
---------------------	-------------	---------------------	----------	----------	--

UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, clm

Run: 8.81 S Sep 13 2024 Print: 8.810 S Sep 13 2024 MiTek Industries, Inc. Mon Jan 13 16:00:12

Page: 1

ID:wOXUMlUGMehwYJ0VSf38UqzIDlr-yw7RNdNjUKB98IsK2Ae198z3wwTHO2nd_53GWTzvKOZ



Repair for a broken web as shown.

Attach 2x4 x 6' SPF No.2 scab to each face of truss as shown with 2 rows of 10d (.131" x 3") nails 4" oc.

Plate Offsets (X, Y):	[2:Edge,0-1-8], [11:Edge,0-1-8], [17:0-4-0,0-4-8]
-----------------------	---

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.21	18-20	>999	240	MT20	244/190
Snow (Pf/Pg)	20.4/20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.37	18-20	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	Horz(CT)	0.09	14	n/a	n/a		
BCLL	0.0 *	Code	IRC2021/TPI2014	Matrix-MSH							
BCDL	10.0									Weight: 445 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x6 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 3-4 cc purlins, except 2-0-0 cc purlins (5-3-4 max.); 6-7.
BOT CHORD	2x6 SP No.1 "Except" B2:2x6 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 cc bracing. Except: 6-0-0 cc bracing: 13-14, 11-13.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt
WEDGE	Left: 2x4 SP No.2 Right: 2x4 SP No.2	WEBS	2 Rows at 1/3 pts
REACTIONS	(lb/size) 2=1597/0-3-8, (min. 0-2-5), 11=149/0-3-8, (min. 0-1-8), 14=2572/0-3-8, (req. 0-3-14) Max Horiz 2=181 (LC 16) Max Uplift 2=216 (LC 12), 11=108 (LC 13), 14=157 (LC 13) Max Grav 2=1969 (LC 3), 11=290 (LC 33), 14=3266 (LC 3)		5-18, 6-17, 7-16 8-14
FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.		
TOP CHORD	2-28=4164/331, 3-28=4070/351, 3-4=3881/360, 4-5=3741/394, 5-29=2914/370, 6-29=2811/411, 6-7=1919/363, 7-30=1376/312, 8-30=1478/272, 8-9=0/1240, 9-10=9/1101, 10-31=88/382, 11-31=199/344		
BOT CHORD	2-21=407/3736, 20-21=476/3774, 20-32=299/3099, 19-32=299/3099, 19-33=299/3099, 18-33=299/3099, 18-34=58/1966, 34-35=58/1966, 17-35=58/1966, 17-36=0/1550, 36-37=0/1550, 16-37=0/1550, 16-38=0/364, 15-38=0/364, 15-39=0/364, 14-39=0/364, 13-14=666/51, 11-13=318/115		
WEBS	3-20=423/218, 5-20=437/86, 5-18=1095/312, 6-18=176/1365, 6-17=651/187, 7-17=93/1005, 7-16=965/89, 8-16=0/1474, 8-14=3157/285, 10-14=707/234, 10-13=0/544		

- NOTES (13)**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCCL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2E)-0-9-10 to 5-1-2, Interior (1) 5-1-2 to 18-1-14, Exterior(2R) 18-1-14 to 40-10-2, Interior (1) 40-10-2 to 53-10-14, Exterior(2E) 53-10-14 to 59-9-10 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Pf=20.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=0-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4. This truss has been checked for uniform snow load only, except as noted.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are 5x5 MT20 unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - WARNING: Required bearing size at joint(s) 14 greater than input bearing size.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 216 lb uplift at joint 2, 108 lb uplift at joint 11 and 157 lb uplift at joint 14.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - This repair has been prepared based on information and use conditions supplied by client. Designer has made a good faith effort to outline damage and repair conditions as reported by client. When actual field conditions do not approximate those indicated on this drawing, client shall immediately inform the engineer and refrain from applying the repair.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

