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11/14/2024
 ABCD Engineering, PLLC NC COA 0838

Site Information:	Page 1:
Customer: Structural Building Solutions LLC	Job Number: Q2411-360
Job Description: The Farm at Neills Creek	
Address: 592 Winding Creek Dr, Lillington, NC 27546	

Job Engineering Criteria:	
Design Code: IRC 2021	IntelliVIEW Version: 23.02.04A JRef #: 1Y4X98360015
Wind Standard: ASCE 7-16 Wind Speed (mph): 120	Design Loading (psf): 40.00
Building Type: Closed	

This package contains general notes pages, 38 truss drawing(s) and 7 detail(s).

Item	Drawing Number	Truss
1	319.24.0815.27817	B1A
3	319.24.0816.37740	H1
5	319.24.0816.42847	H2G
7	319.24.0818.34490	V5
9	319.24.0817.54007	PB3
11	318.24.1558.46214	C1
13	319.24.0818.25057	V3
15	319.24.0815.35220	B2
17	319.24.0815.42993	C1D
19	319.24.0817.43537	PB1G
21	319.24.0815.29433	B1G
23	319.24.0818.21383	V2
25	319.24.0817.58030	PB3G
27	318.24.1558.46229	VA3
29	319.24.0818.41620	VA1
31	319.24.0815.13590	A1
33	319.24.0816.05590	G1
35	319.24.0815.40007	B2G
37	319.24.0815.24053	A1T
39	A12015ENC160118	
41	BRCLBSUB0119	
43	GBLLETIN0118	
45	VALTN160118	

Item	Drawing Number	Truss
2	319.24.0817.45473	PB2
4	319.24.0816.39750	H2
6	319.24.0817.29243	PB1
8	319.24.0815.25817	B1
10	319.24.0818.39533	V6
12	319.24.0816.45217	P1
14	319.24.0815.37313	B2A
16	319.24.0815.16030	A1A
18	319.24.0818.16557	V1
20	319.24.0818.28813	V4
22	319.24.0815.20460	A1G
24	319.24.0816.01940	C1G
26	319.24.0818.53947	VA4
28	319.24.0818.49980	VA2
30	319.24.0819.02163	VG4
32	319.24.0816.10807	G1G
34	319.24.0816.08747	G1A
36	319.24.0816.47380	P2
38	319.24.0815.32300	B1T
40	A12030ENC160118	
42	GABRST160118	
44	PB160160118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high-quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Bearing Information:

The bearing area factor, C_b , is considered for the allowable capacity of solid sawn wood bearings supporting trusses that are located a minimum of 3" from the end of the lumber piece.

General Notes (continued)

Coated Lumber:

Coated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Coated lumber has no adjustments to lumber properties. Coated lumber may be more brittle than uncoated lumber. Special handling care must be taken to prevent breakage during all handling activities. Refer to manufacturer literature, specifications, and code evaluation reports for restrictions, details, and requirements.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

C = Coated lumber.

C-AT = AtTEK coated lumber.

C-FX = FX Lumber Guard coated lumber.

C -TE = TechWood 4400 coated lumber.

CL = Certified lumber.

Des Ld = total of TCDL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-BF = Boraflame Fire Retardant Treated lumber

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-ON = OnWood Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

General Notes (continued)

Key to Terms (continued):

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

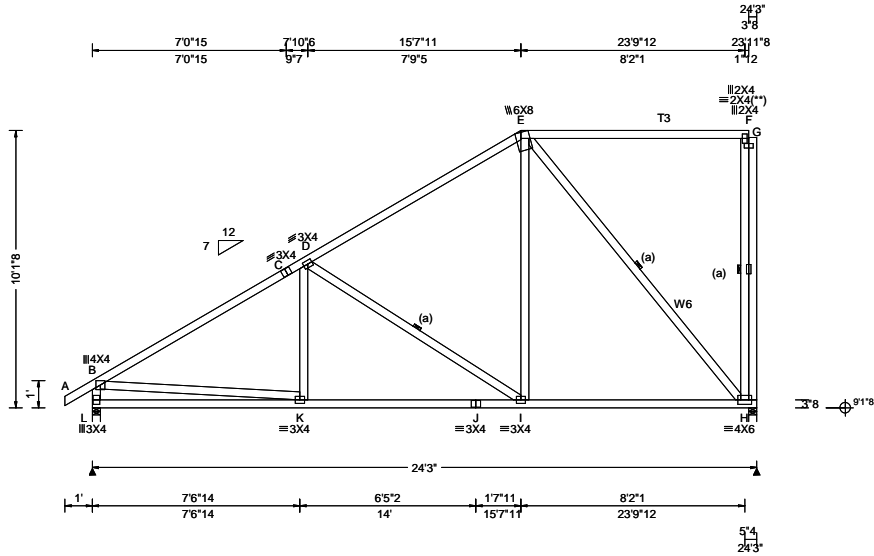
W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoclin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org.
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com



Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.042 D 999 240 VERT(CL): 0.086 D 999 240 HORZ(LL): 0.012 H - - HORZ(TL): 0.025 H - - Creep Factor: 2.0 Max TC CSI: 0.864 Max BC CSI: 0.792 Max Web CSI: 0.665 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>L</td> <td>1077</td> <td>-</td> <td>-</td> <td>/632</td> <td>-</td> <td>/269</td> </tr> <tr> <td>H</td> <td>1007</td> <td>-</td> <td>-</td> <td>/682</td> <td>-</td> <td>-</td> </tr> </tbody> </table>						Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	L	1077	-	-	/632	-	/269	H	1007	-	-	/682	-	-
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Lumber

Top chord: 2x4 SP #2; T3 2x4 SP SS;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3; W6 2x4 SP #2;
 Rt Bearing Leg: 2x4 SP #2;

Bracing

(a) Continuous lateral restraint equally spaced on member.

Plating Notes

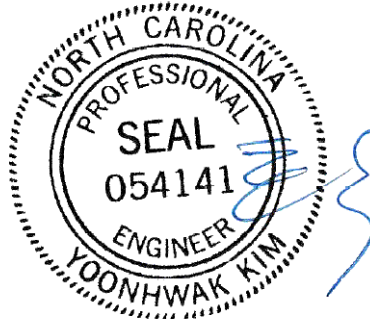
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading

Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF
 Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

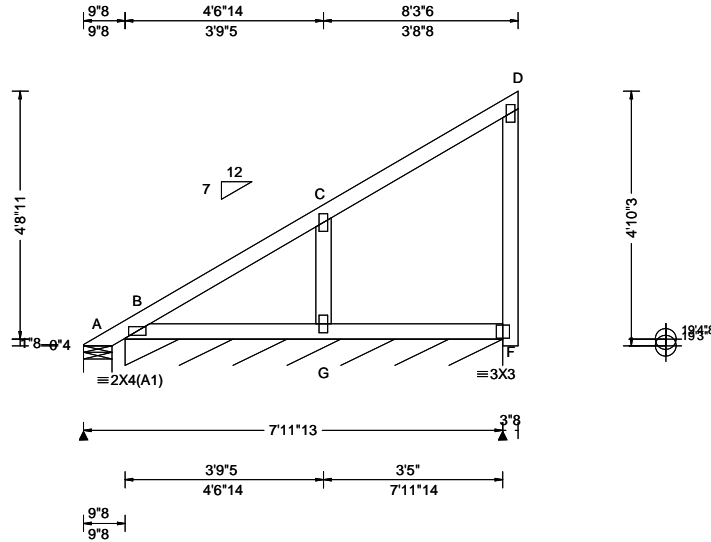


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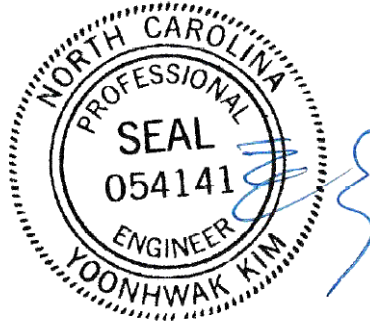
****WARNING** READ AND FOLLOW ALL NOTES ON THIS DRAWING!**
****IMPORTANT** FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS**
 Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have continuous lateral restraint (CLR), installed with diagonal bracing installed on the CLR per BCSI sections B3, B7, or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions. Refer to job's General Notes page for additional information.
 Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.
 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org



SEQN: 7732 FROM:	MONO Ply: 1 Qty: 3	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: PB2	Cust: R 9836 JRef: 1Y4X98360015 T20 DrwNo: 319.24.0817.45473 JB / YK 11/14/2024
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Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.002 D 999 240 VERT(CL): 0.003 D 999 240 HORZ(LL): -0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.241 Max BC CSI: 0.089 Max Web CSI: 0.058 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-30</td> <td>/-</td> <td>/61</td> <td>/52</td> <td>/103</td> </tr> <tr> <td>B*</td> <td>90</td> <td>/-</td> <td>/-</td> <td>/83</td> <td>/12</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 86.3 Min Req = - Bearings A & B are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-30	/-	/61	/52	/103	B*	90	/-	/-	/83	/12	/-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
A	-	/-30	/-	/61	/52	/103																									
B*	90	/-	/-	/83	/12	/-																									
Lumber Top chord: 2x4 SP #2; Bot chord: 2x4 SP #2; Webs: 2x4 SP #3; Plating Notes All plates are 2X4 except as noted. Loading Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF Bottom chord checked for 10.00 psf non-concurrent live load. Purlins In lieu of rigid ceiling use purlins to brace BC @ 24" oc. Wind Wind loads based on MWFRS with additional C&C member design. Right end vertical not exposed to wind pressure. Right cantilever is exposed to wind Wind loading based on both gable and hip roof types. Additional Notes Refer to DWG PB160160118 for piggyback details.																															

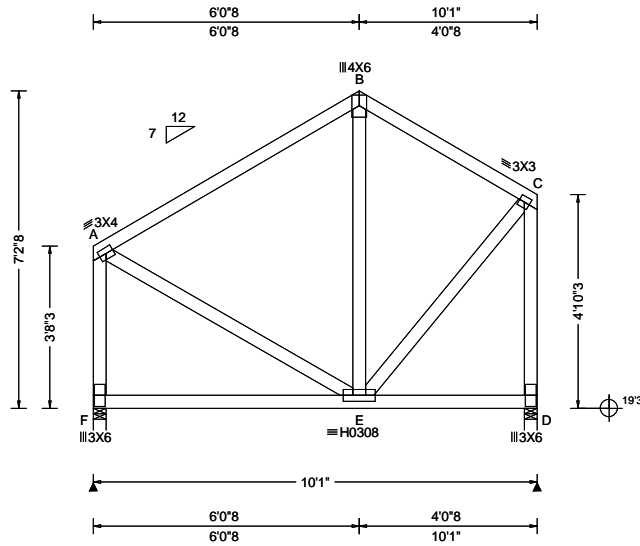


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SEQN: 7726 FROM:	COMN Ply: 1 Qty: 3	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: H1	Cust: R 9836 JRef: 1Y4X98360015 T19 DrwNo: 319.24.0816.37740 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 24.70 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 B 999 240 VERT(CL): 0.008 B 999 240 HORZ(LL): -0.002 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.693 Max BC CSI: 0.302 Max Web CSI: 0.386 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL F 438 /- /- /244 /31 /170 D 435 /- /- /254 /40 /- Wind reactions based on MWFRS F Brg Wid = 3.5 Min Req = 1.5 (Truss) D Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings F & D are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - F 187 -386 C - D 204 -410					

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

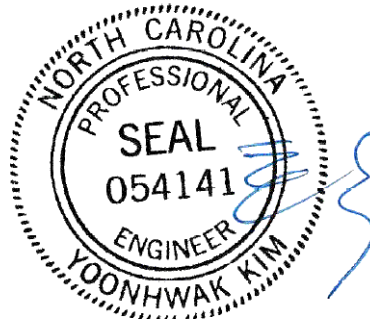
Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

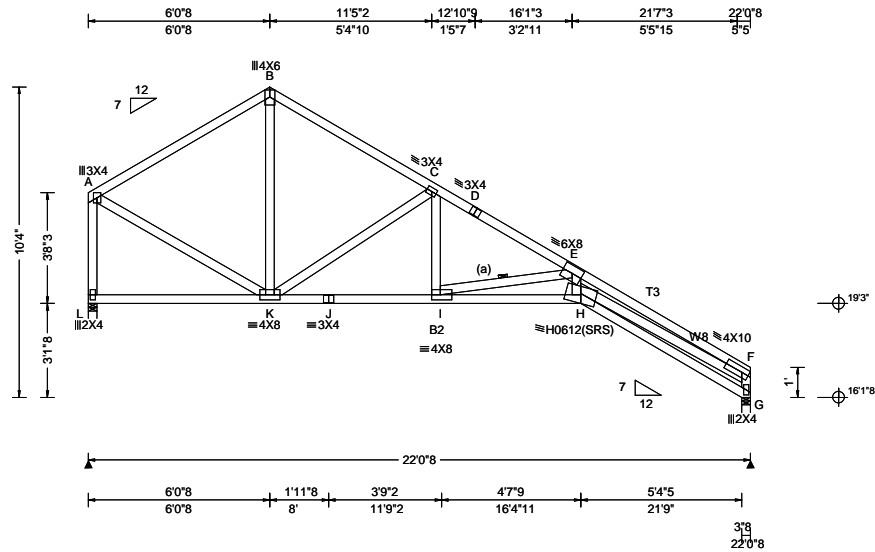


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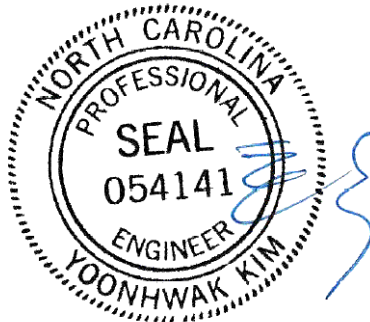
Loading Criteria (psf) TCCL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.79 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.322 H 822 240 VERT(CL): 0.668 H 395 240 HORZ(LL): 0.205 G - - HORZ(TL): 0.425 G - - Creep Factor: 2.0 Max TC CSI: 0.995 Max BC CSI: 0.523 Max Web CSI: 0.825 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+ / R-</th> <th>/ Rh</th> <th>/ Rw</th> <th>/ U</th> <th>/ RL</th> </tr> </thead> <tbody> <tr> <td>L</td> <td>916</td> <td>- / -</td> <td>/ 498</td> <td>/ 46</td> <td>/ 213</td> </tr> <tr> <td>G</td> <td>917</td> <td>- / -</td> <td>/ 535</td> <td>/ 51</td> <td>- / -</td> </tr> </tbody> </table> Wind reactions based on MWFRS L Brg Wid = 3.5 Min Req = 1.5 (Truss) G Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings L & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>154 -801</td> <td>D - E</td> <td>203 -1738</td> </tr> <tr> <td>B - C</td> <td>160 -800</td> <td>E - F</td> <td>494 -5128</td> </tr> <tr> <td>C - D</td> <td>211 -1633</td> <td></td> <td></td> </tr> </tbody> </table>	Gravity			Non-Gravity			Loc	R+ / R-	/ Rh	/ Rw	/ U	/ RL	L	916	- / -	/ 498	/ 46	/ 213	G	917	- / -	/ 535	/ 51	- / -	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	154 -801	D - E	203 -1738	B - C	160 -800	E - F	494 -5128	C - D	211 -1633		
Gravity			Non-Gravity																																									
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C - D	211 -1633																																											

Lumber
 Top chord: 2x4 SP #2; T3 2x4 SP SS;
 Bot chord: 2x4 SP #2; B2 2x4 SP SS;
 Webs: 2x4 SP #3; W8 2x4 SP SS;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Loading
 Bottom chord checked for 10.00 psf non-concurrent live load.
 Truss designed for unbalanced snow loads.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/180.
 Wind loading based on both gable and hip roof types.

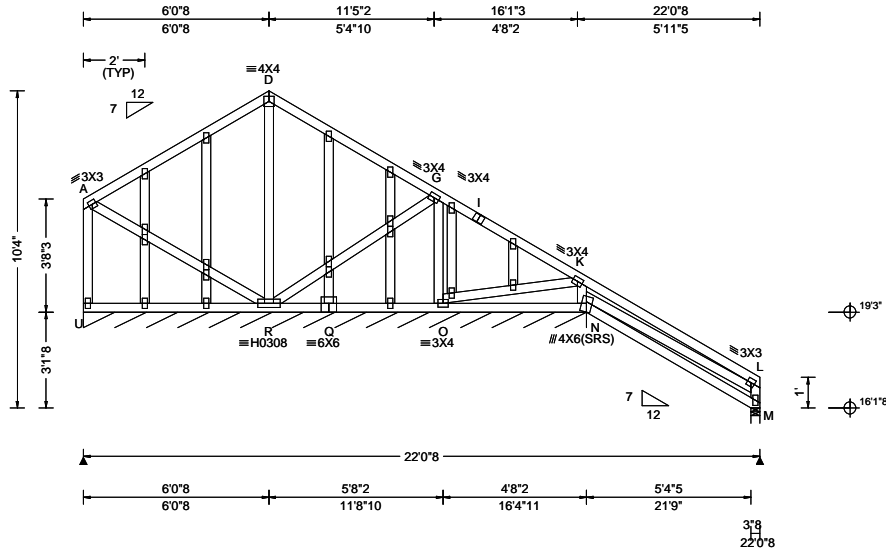


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SEQN: 8670 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: H2G	Cust: R 9836 JRef: 1Y4X98360015 T27 DrwNo: 319.24.0816.42847 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.79 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 J 999 240 VERT(CL): 0.008 J 999 240 HORZ(LL): -0.002 J - - HORZ(TL): 0.004 J - - Creep Factor: 2.0 Max TC CSI: 0.457 Max BC CSI: 0.249 Max Web CSI: 0.163 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>R-</th> <th>Rh</th> <th>Rw</th> <th>U</th> <th>RL</th> </tr> </thead> <tbody> <tr> <td>N*</td> <td>100</td> <td>-</td> <td>-</td> <td>/55</td> <td>/4</td> <td>/13</td> </tr> <tr> <td>M</td> <td>199</td> <td>-</td> <td>-</td> <td>/133</td> <td>/35</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS N Brg Wid = 196 Min Req = - M Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings U & M are a rigid surface. Members not listed have forces less than 375# Maximum Bot Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>O - N</td> <td>159 -435</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	R-	Rh	Rw	U	RL	N*	100	-	-	/55	/4	/13	M	199	-	-	/133	/35	-	Chords	Tens.Comp.	O - N	159 -435
Loc	Gravity			Non-Gravity																															
	R+	R-	Rh	Rw	U	RL																													
N*	100	-	-	/55	/4	/13																													
M	199	-	-	/133	/35	-																													
Chords	Tens.Comp.																																		
O - N	159 -435																																		

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

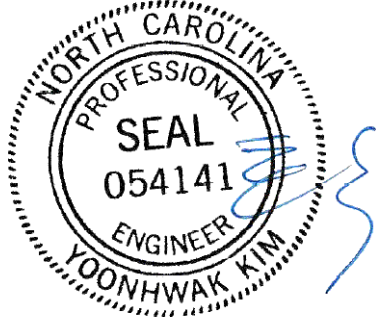
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.
 Bottom chord checked for 10.00 psf non-concurrent live load.
 Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/180.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

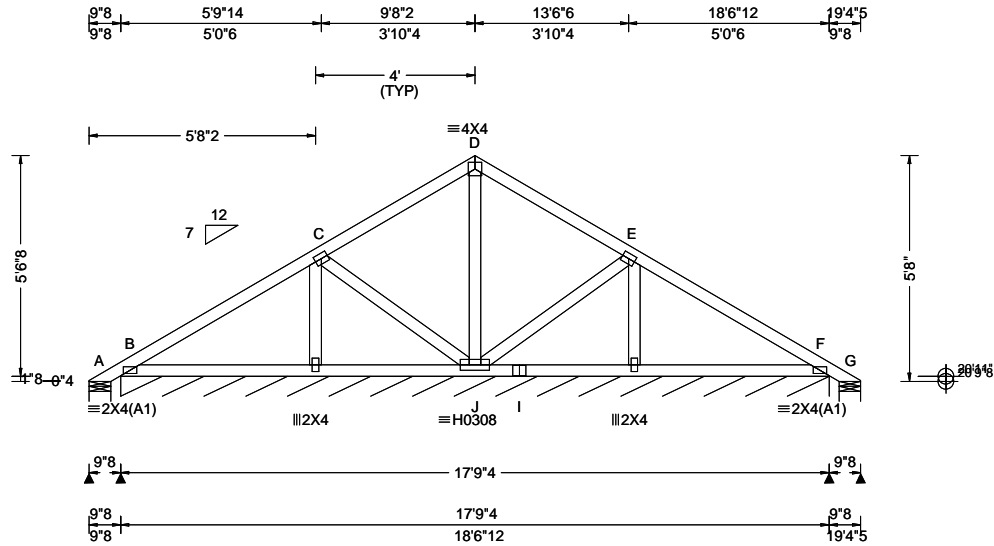


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SEQN: 8672 FROM:	GABL Ply: 1 Qty: 9	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: PB1	Cust: R 9836 JRef: 1Y4X98360015 T25 DrwNo: 319.24.0817.29243 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.85 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.002 F 999 240 VERT(CL): 0.004 F 999 240 HORZ(LL): 0.001 B - - HORZ(TL): 0.002 B - - Creep Factor: 2.0 Max TC CSI: 0.341 Max BC CSI: 0.136 Max Web CSI: 0.097 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-97</td> <td>/-</td> <td>/53</td> <td>/137</td> <td>/92</td> </tr> <tr> <td>B*</td> <td>93</td> <td>/-</td> <td>/-</td> <td>/78</td> <td>/4</td> <td>/-</td> </tr> <tr> <td>G</td> <td>-</td> <td>/-98</td> <td>/-</td> <td>/22</td> <td>/87</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 213 Min Req = - G Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & G are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-97	/-	/53	/137	/92	B*	93	/-	/-	/78	/4	/-	G	-	/-98	/-	/22	/87	/-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
A	-	/-97	/-	/53	/137	/92																																
B*	93	/-	/-	/78	/4	/-																																
G	-	/-98	/-	/22	/87	/-																																

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4 except as noted.

Loading

Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF

Gable end supports 8" max rake overhang. Top chord must not be cut or notched.

Bottom chord checked for 10.00 psf non-concurrent live load.

Truss designed for unbalanced snow loads.

Wind

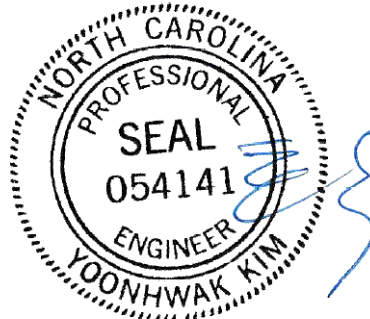
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

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Refer to DWG PB160160118 for piggyback details.



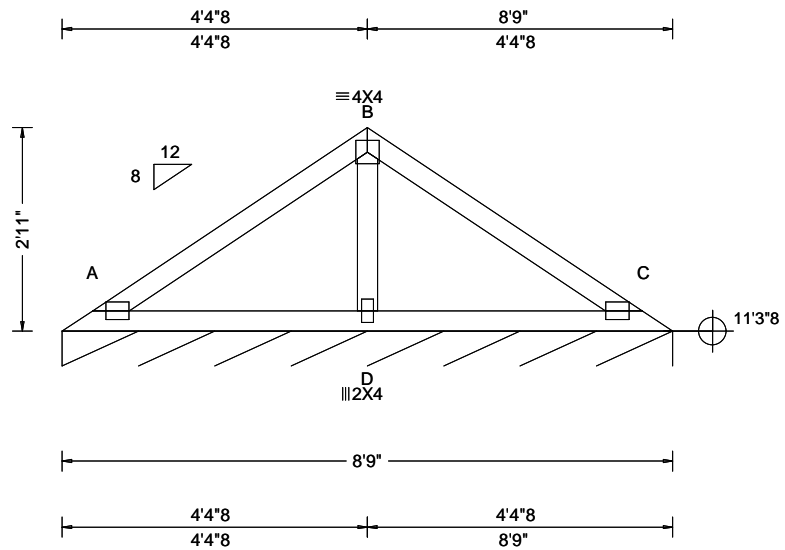
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SEQN: 7739 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: V5	Cust: R 9836 JRef: 1Y4X98360015 T16 DrwNo: 319.24.0818.34490 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.008 C 999 240 VERT(CL): 0.017 C 999 240 HORZ(LL): -0.004 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.213 Max Web CSI: 0.104 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *PLF <table border="1"> <thead> <tr> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>Loc</th> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>C*</td> <td>84</td> <td>/-</td> <td>/-</td> <td>/42</td> <td>/-</td> <td>/5</td> </tr> </tbody> </table> Wind reactions based on MWFRS C Brg Wid = 104 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Gable Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Gables</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>B - D</td> <td>188 -456</td> </tr> </tbody> </table>	Gravity			Non-Gravity			Loc	R+	/R-	/Rh	/Rw	/U	/RL	C*	84	/-	/-	/42	/-	/5	Gables	Tens.Comp.	B - D	188 -456
Gravity			Non-Gravity																									
Loc	R+	/R-	/Rh	/Rw	/U	/RL																						
C*	84	/-	/-	/42	/-	/5																						
Gables	Tens.Comp.																											
B - D	188 -456																											

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4(D1) except as noted.

Loading

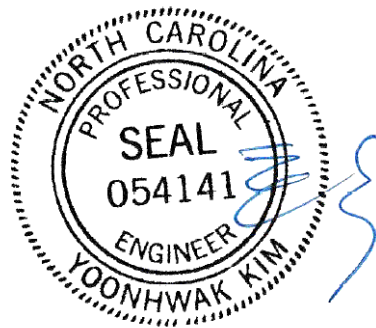
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
 See DWGS VALTN160118 and VAL180160118 for valley details.

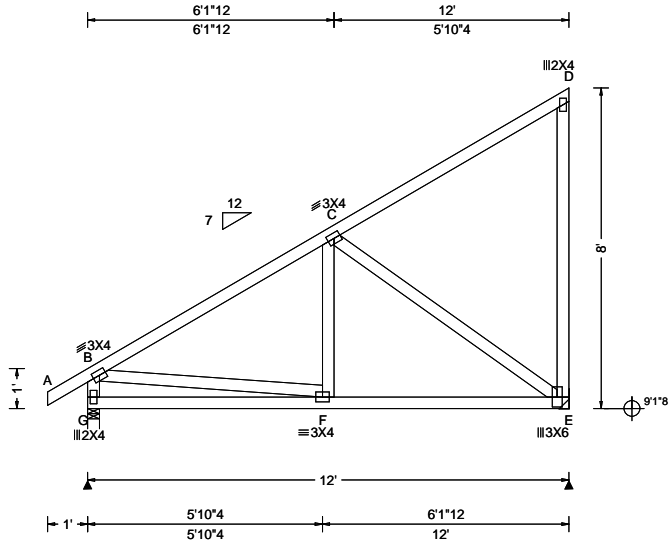


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SEQN: 7715 FROM:	MONO Ply: 1 Qty: 7	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: B1	Cust: R 9836 JRef: 1Y4X98360015 T31 DrwNo: 319.24.0815.25817 JB / YK 11/14/2024
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.008 F 999 240 VERT(CL): 0.017 F 999 240 HORZ(LL): -0.004 D - - HORZ(TL): 0.008 D - - Creep Factor: 2.0 Max TC CSI: 0.717 Max BC CSI: 0.490 Max Web CSI: 0.518 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 570 /- /- /328 /- /167 E 496 /- /- /323 /43 /- Wind reactions based on MWFRS G Brg Wid = 3.5 Min Req = 1.5 (Truss) E Brg Wid = - Min Req = - Bearing G is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. B - C 0 -581
				Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. F - E 428 -169 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. B - G 76 -523 C - E 208 -525 B - F 382 0

Lumber

Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Hangers / Ties

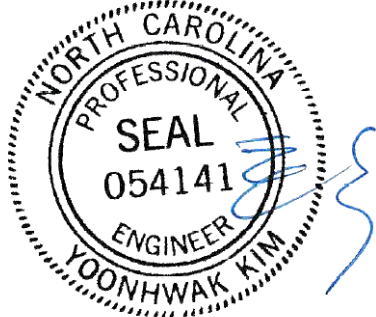
(J) Hanger Support Required, by others

Loading

Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF
 Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Left end vertical exposed to wind pressure. Deflection meets L/180.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

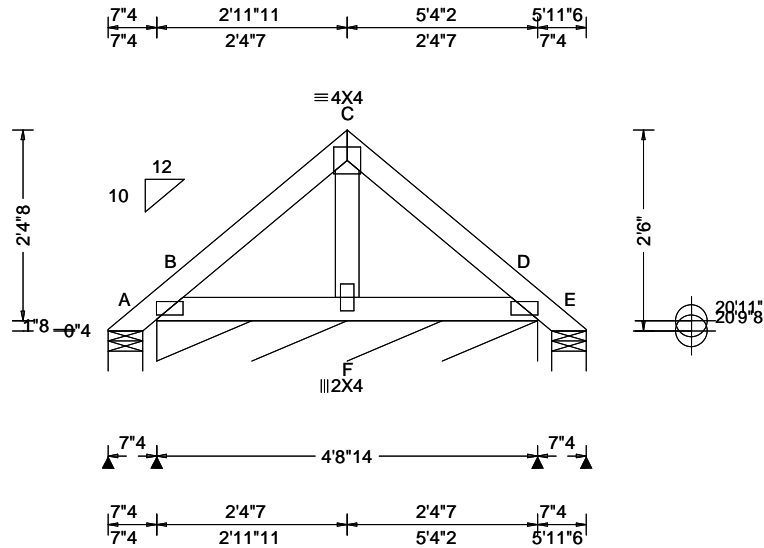


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SEQN: 7733 FROM:	COMN Ply: 1 Qty: 10	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: PB3	Cust: R 9836 JRef: 1Y4X98360015 T10 DrwNo: 319.24.0817.54007 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 22.05 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.000 D 999 240 VERT(CL): 0.000 D 999 240 HORZ(LL): 0.000 D - - HORZ(TL): 0.001 B - - Creep Factor: 2.0 Max TC CSI: 0.066 Max BC CSI: 0.048 Max Web CSI: 0.012 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A - /-20 /- /36 /45 /43 B* 105 /- /- /62 /11 /- E - /-20 /- /12 /17 /- Wind reactions based on MWFRS A Brg Wid = 5.2 Min Req = 1.5 (Truss) B Brg Wid = 56.9 Min Req = - E Brg Wid = 5.2 Min Req = 1.5 (Truss) Bearings A, B, & E are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4(A1) except as noted.

Loading

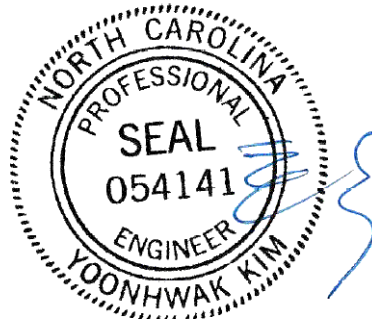
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Refer to DWG PB160160118 for piggyback details.

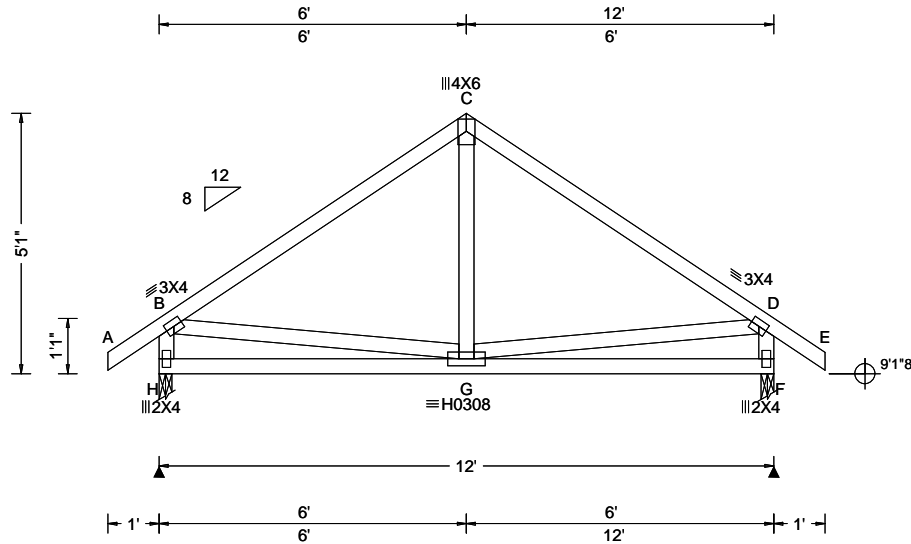


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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.006 G 999 240 VERT(CL): 0.013 G 999 240 HORZ(LL): 0.001 D - - HORZ(TL): 0.002 D - - Creep Factor: 2.0 Max TC CSI: 0.476 Max BC CSI: 0.323 Max Web CSI: 0.134 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H 573 - / - / 322 - / 110 F 573 - / - / 322 - / - Wind reactions based on MWFRS H Brg Wid = 3.0 Min Req = 1.5 (Truss) F Brg Wid = 3.0 Min Req = 1.5 (Truss) Bearings H & F are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - C 116 -541 C - D 116 -541
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

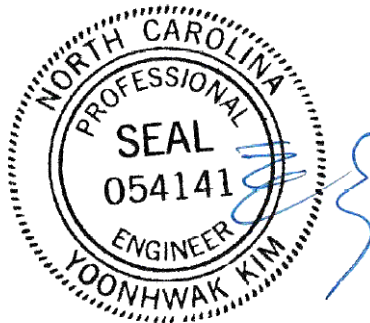
Wind loads based on MWFRS with additional C&C member design.

End verticals exposed to wind pressure. Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

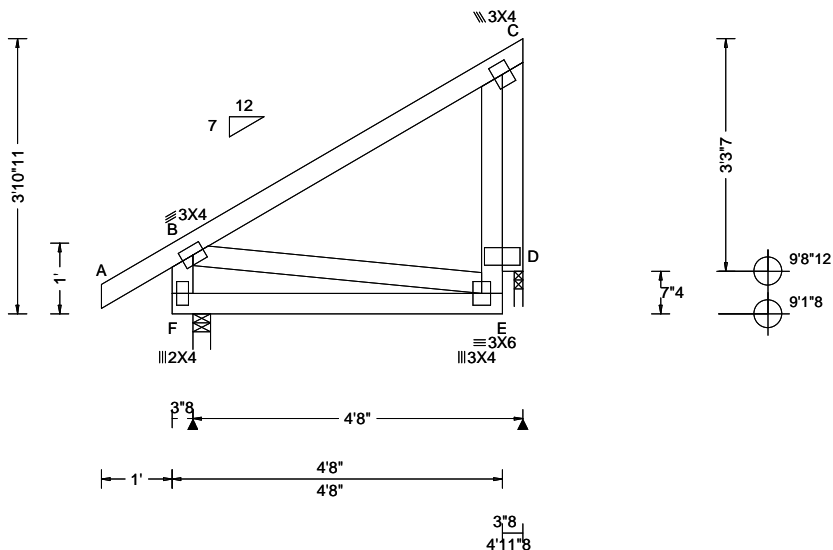
Lanai/Porch Loading : 14.7 PLF wind pressure applied to the bottom chord of the truss from 0.00 ft to 12.00 ft,



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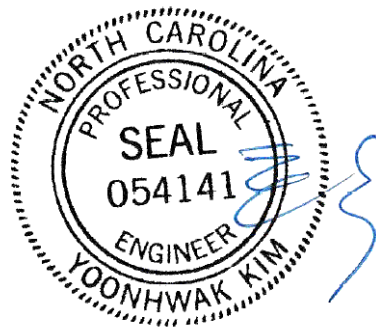


Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.005 B 913 240 VERT(CL): 0.011 B 459 240 HORZ(LL): -0.005 C - - HORZ(TL): 0.009 C - - Creep Factor: 2.0 Max TC CSI: 0.275 Max BC CSI: 0.190 Max Web CSI: 0.122 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL F 304 /- /- /168 /- /95 D 170 /- /- /122 /25 /- Wind reactions based on MWFRS F Brg Wid = 3.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = 1.5 (Support) Bearings F & D are a rigid surface. Members not listed have forces less than 375# Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. C - D 533 -470
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Left cantilever is exposed to wind
Wind loading based on both gable and hip roof types.

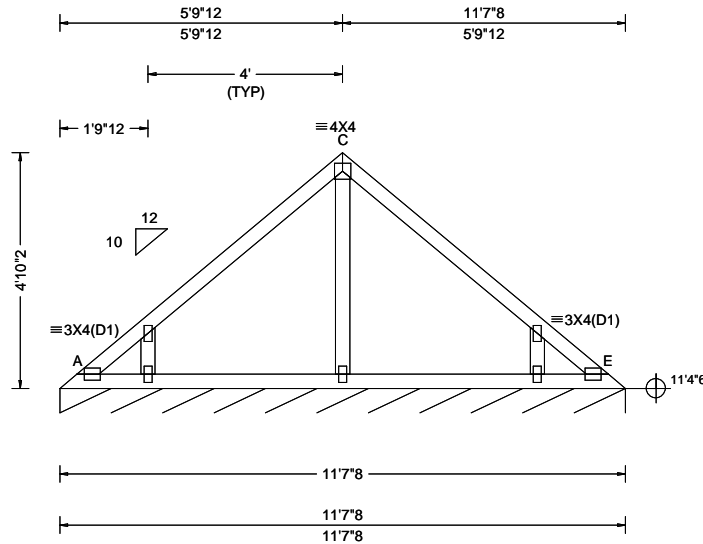


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SEQN: 7737 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: V3	Cust: R 9836 JRef: 1Y4X98360015 T34 DrwNo: 319.24.0818.25057 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 C 999 240 VERT(CL): 0.001 C 999 240 HORZ(LL): -0.001 B - - HORZ(TL): 0.001 A - - Creep Factor: 2.0 Max TC CSI: 0.239 Max BC CSI: 0.119 Max Web CSI: 0.065 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 86 /- /- /46 /- /7 Wind reactions based on MWFRS E Brg Wid = 139 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

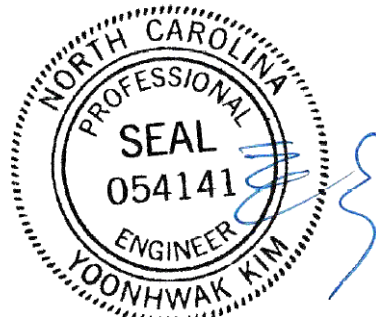
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

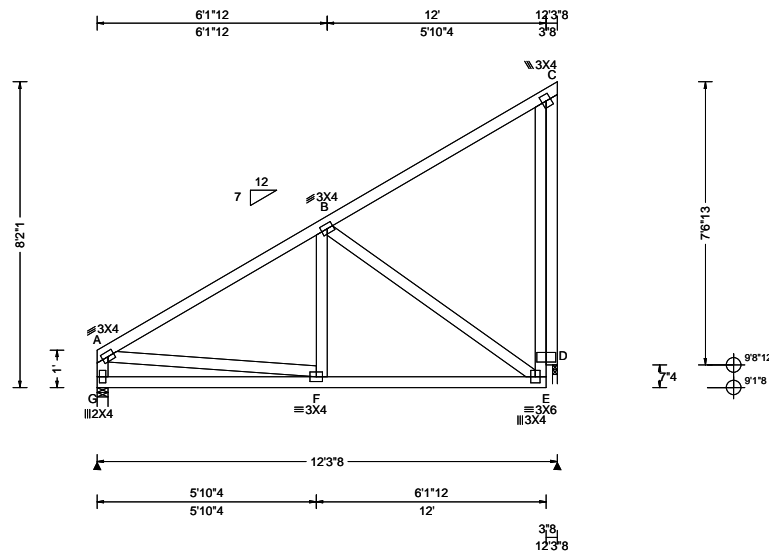
See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
See DWGS VALTN160118 and VAL180160118 for valley details.



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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.012 F 999 240 VERT(CL): 0.024 F 999 240 HORZ(LL): -0.005 C - - HORZ(TL): 0.010 C - - Creep Factor: 2.0 Max TC CSI: 0.532 Max BC CSI: 0.389 Max Web CSI: 0.506 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>G</td> <td>508</td> <td>-</td> <td>-</td> <td>/277</td> <td>-</td> <td>/200</td> </tr> <tr> <td>D</td> <td>508</td> <td>-</td> <td>-</td> <td>/341</td> <td>/28</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	G	508	-	-	/277	-	/200	D	508	-	-	/341	/28	-
				Loc		Gravity			Non-Gravity																						
R+	/R-	/Rh	/Rw		/U	/RL																									
G	508	-	-	/277	-	/200																									
D	508	-	-	/341	/28	-																									
Maximum Top Chord Forces Per Ply (lbs) <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens.Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>107</td> <td>-596</td> <td></td> </tr> </tbody> </table>				Chords	Tens.Comp.	Chords	Tens.Comp.	A - B	107	-596																					
Chords	Tens.Comp.	Chords	Tens.Comp.																												
A - B	107	-596																													

Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Rt Bearing Leg: 2x4 SP #3;

Loading
 Bottom chord checked for 10.00 psf non-concurrent live load.

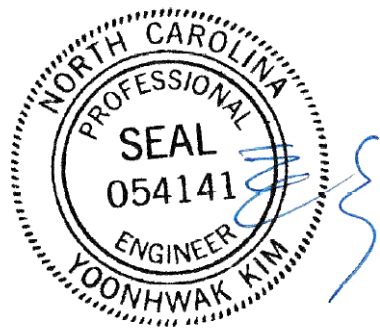
Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/180.
 Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
G - F	141 -441	F - E	442 -245

Maximum Web Forces Per Ply (lbs)

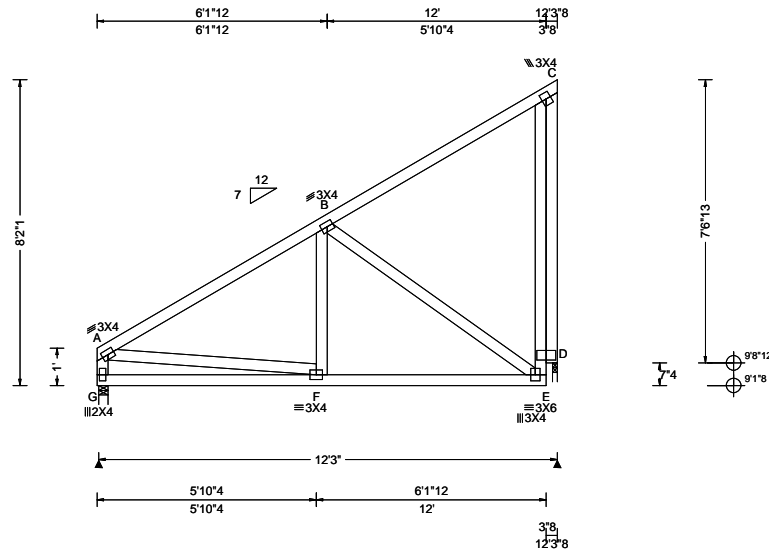
Webs	Tens.Comp.	Webs	Tens. Comp.
A - G	114 -461	B - E	200 -513
A - F	385 -7	C - D	719 -813



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.012 F 999 240 VERT(CL): 0.024 F 999 240 HORZ(LL): -0.005 C - - HORZ(TL): 0.010 C - - Creep Factor: 2.0 Max TC CSI: 0.532 Max BC CSI: 0.389 Max Web CSI: 0.506 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G 508 /- /- /277 /- /200 D 508 /- /- /341 /28 /- Wind reactions based on MWFRS G Brg Wid = 3.0 Min Req = 1.5 (Truss) D Brg Wid = 1.5 Min Req = 1.5 (Support) Bearings G & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. A - B 107 -596
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Lumber
Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;
Rt Bearing Leg: 2x4 SP #3;

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.

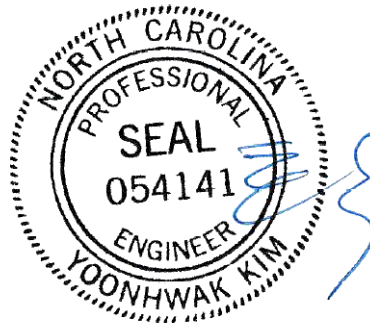
Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
G - F	141 -441	F - E	442 -245

Maximum Web Forces Per Ply (lbs)

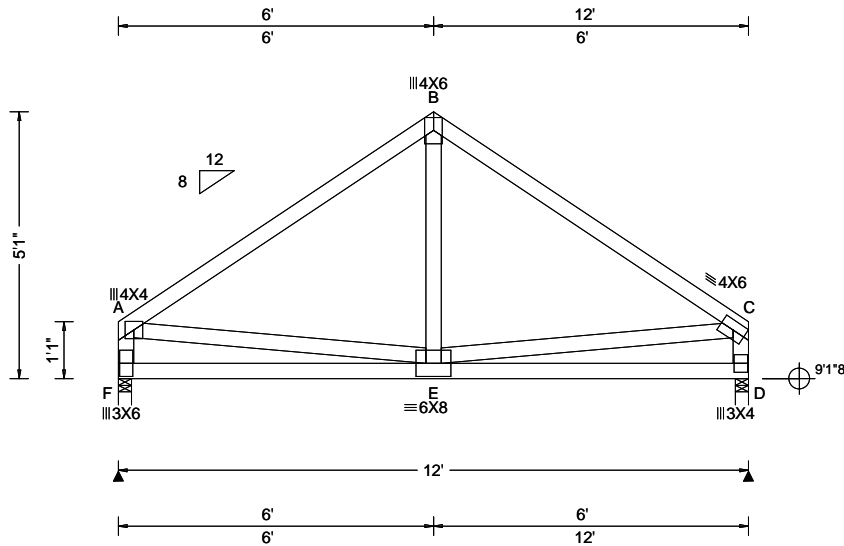
Webs	Tens.Comp.	Webs	Tens. Comp.
A - G	114 -461	B - E	200 -513
A - F	385 -7	C - D	719 -813



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCCL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.013 E 999 240 VERT(CL): 0.012 E 999 240 HORZ(LL): 0.065 B - - HORZ(TL): 0.070 B - - Creep Factor: 2.0 Max TC CSI: 0.605 Max BC CSI: 0.638 Max Web CSI: 0.757 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>F</td> <td>449</td> <td>-</td> <td>-</td> <td>/277</td> <td>/686</td> <td>/6000</td> </tr> <tr> <td>D</td> <td>449</td> <td>-</td> <td>-</td> <td>/1695</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS F Brg Wid = 3.0 Min Req = 1.5 (Truss) D Brg Wid = 3.0 Min Req = 2.0 (Truss) Bearings F & D are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - B</td> <td>1254 -486</td> <td>B - C</td> <td>120 -2354</td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>F - E</td> <td>5987 -86</td> <td>E - D</td> <td>1571 -37</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>A - F</td> <td>729 -401</td> <td>E - C</td> <td>1828 0</td> </tr> <tr> <td>A - E</td> <td>291 -1111</td> <td>C - D</td> <td>121 -1647</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	F	449	-	-	/277	/686	/6000	D	449	-	-	/1695	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	A - B	1254 -486	B - C	120 -2354	Chords	Tens.Comp.	Chords	Tens. Comp.	F - E	5987 -86	E - D	1571 -37	Webs	Tens.Comp.	Webs	Tens. Comp.	A - F	729 -401	E - C	1828 0	A - E	291 -1111	C - D	121 -1647
Loc	Gravity			Non-Gravity																																																							
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP SS;
 Webs: 2x4 SP #3;

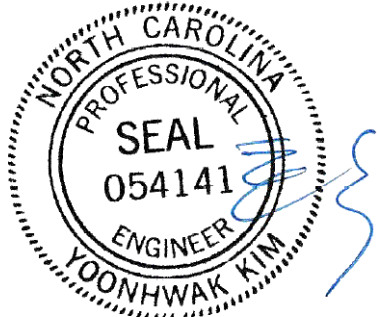
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Loading
 Truss transfers a maximum horizontal load of 3000 # (250.00 plf) along top chord, from either direction, to supports where indicated. Diaphragm and connections are to be designed by Engineer of Record.
 Drag Loads: Force(#) (PLF) Mbr Start End
 Case 1: 3000 250.00 TC 0.00 12.00
 3000 250.00 BC 0.00 12.00

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/180.
 Wind loading based on both gable and hip roof types.

Additional Notes
 Negative reaction(s) of -686# MAX. Requires uplift connection. See Maximum Reactions.
 Lanai/Porch Loading : 14.7 PLF wind pressure applied to the bottom chord of the truss from 0.00 ft to 12.00 ft,

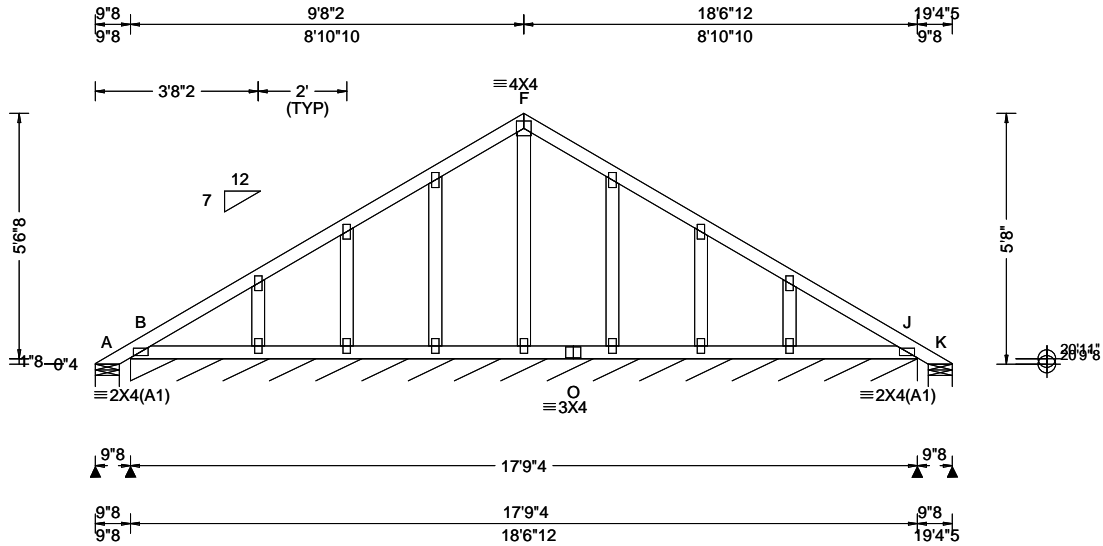


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SEQN: 8674 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: PB1G	Cust: R 9836 JRef: 1Y4X98360015 T1 DrwNo: 319.24.0817.43537 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 23.64 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.77 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 J 999 240 VERT(CL): 0.001 E 999 240 HORZ(LL): 0.001 I - - HORZ(TL): 0.002 I - - Creep Factor: 2.0 Max TC CSI: 0.073 Max BC CSI: 0.053 Max Web CSI: 0.071 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>A</td> <td>-</td> <td>/-15</td> <td>/-</td> <td>/49</td> <td>/59</td> <td>/93</td> </tr> <tr> <td>B*</td> <td>89</td> <td>/-</td> <td>/-</td> <td>/48</td> <td>/8</td> <td>/-</td> </tr> <tr> <td>K</td> <td>-</td> <td>/-15</td> <td>/-</td> <td>/5</td> <td>/13</td> <td>/-</td> </tr> </tbody> </table> Wind reactions based on MWFRS A Brg Wid = 6.5 Min Req = 1.5 (Truss) B Brg Wid = 213 Min Req = - K Brg Wid = 6.5 Min Req = 1.5 (Truss) Bearings A, B, & K are a rigid surface. Members not listed have forces less than 375#	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	A	-	/-15	/-	/49	/59	/93	B*	89	/-	/-	/48	/8	/-	K	-	/-15	/-	/5	/13	/-
Loc	Gravity			Non-Gravity																																		
	R+	/R-	/Rh	/Rw	/U	/RL																																
A	-	/-15	/-	/49	/59	/93																																
B*	89	/-	/-	/48	/8	/-																																
K	-	/-15	/-	/5	/13	/-																																

Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

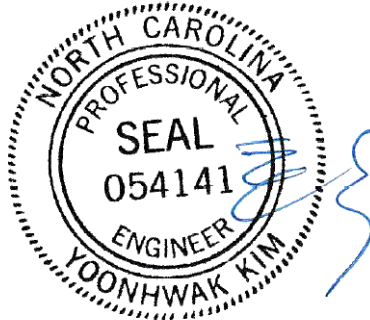
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
Refer to DWG PB160160118 for piggyback details.

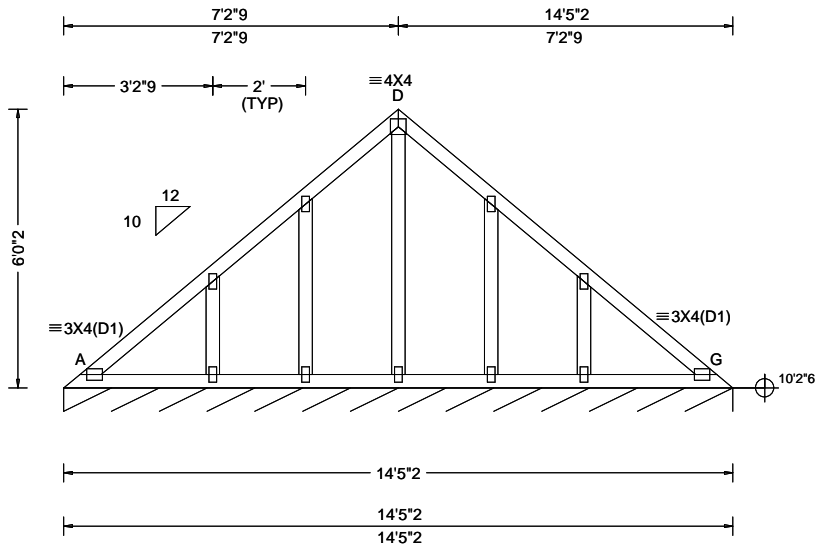


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SEQN: 8676 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: V4	Cust: R 9836 JRef: 1Y4X98360015 T33 DrwNo: 319.24.0818.28813 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.003 A 999 240 VERT(CL): 0.006 A 999 240 HORZ(LL): -0.002 G - - HORZ(TL): 0.003 G - - Creep Factor: 2.0 Max TC CSI: 0.098 Max BC CSI: 0.077 Max Web CSI: 0.096 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL G* 86 /- /- /46 /- /7 Wind reactions based on MWFRS G Brg Wid = 173 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

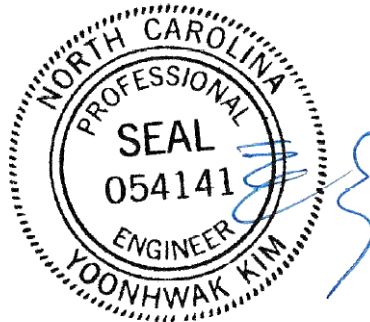
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
See DWGS VALTN160118 and VAL180160118 for valley details.

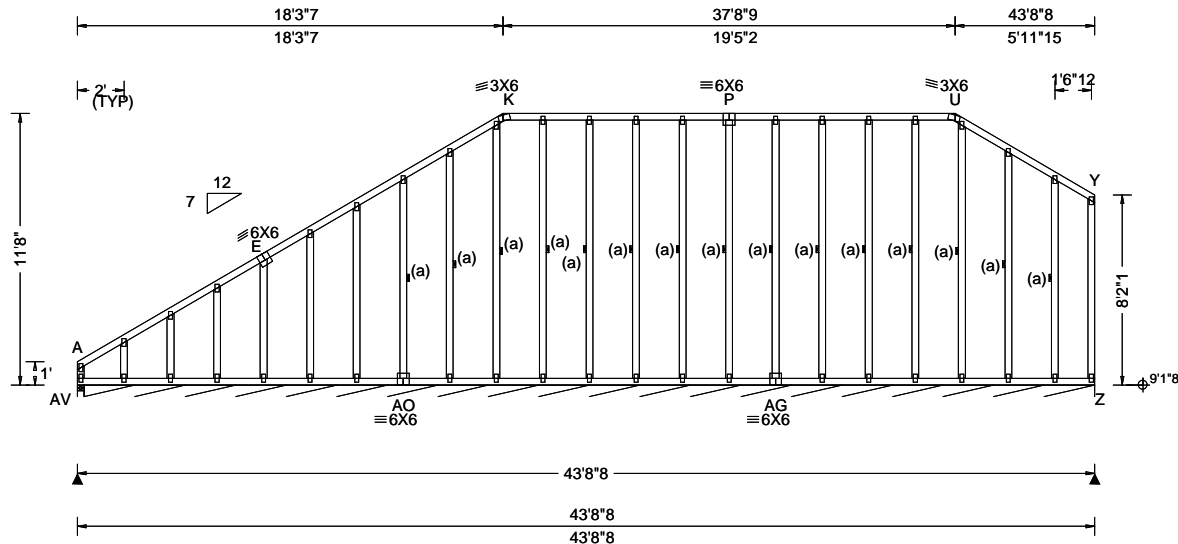


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SEQN: 8647 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: A1G	Cust: R 9836 JRef: 1Y4X98360015 T30 DrwNo: 319.24.0815.20460 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.46 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.37 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.002 B 999 240 VERT(CL): 0.004 K 999 240 HORZ(LL): -0.051 Y - - HORZ(TL): 0.086 Y - - Creep Factor: 2.0 Max TC CSI: 0.140 Max BC CSI: 0.140 Max Web CSI: 0.617 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL AV 80 - / - /232 /113 /263 Z* 82 - / - /47 /1 - Wind reactions based on MWFRS AV Brg Wid = 3.5 Min Req = 1.5 (Truss) Z Brg Wid = 521 Min Req = - Bearings AV & AV are a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Bracing

(a) Continuous lateral restraint equally spaced on member.
Fasten rated sheathing to one face of this frame.

Plating Notes

All plates are 2X4 except as noted.

Loading

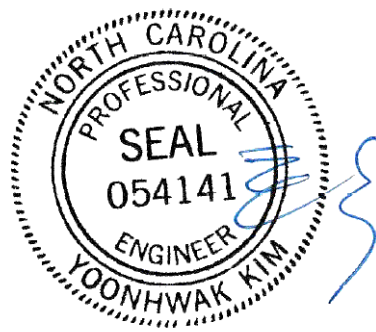
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced snow loads.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

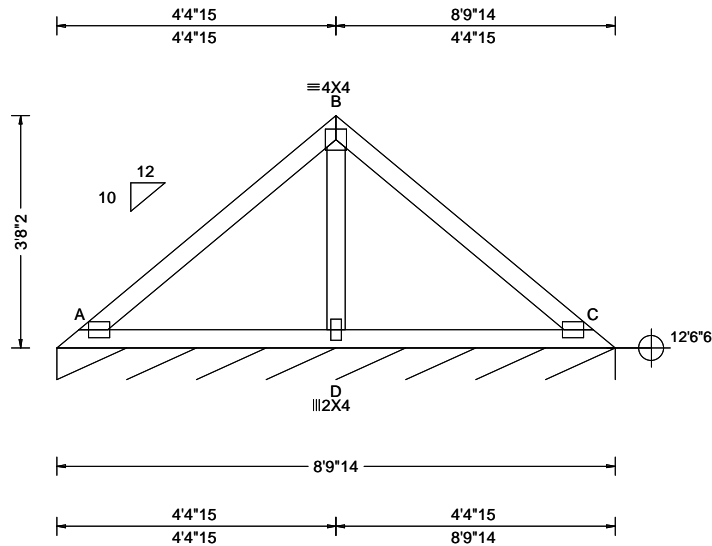


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SEQN: 7736 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: V2	Cust: R 9836 JRef: 1Y4X98360015 T35 DrwNo: 319.24.0818.21383 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.007 A 999 240 VERT(CL): 0.016 A 999 240 HORZ(LL): -0.004 C - - HORZ(TL): 0.010 C - - Creep Factor: 2.0 Max TC CSI: 0.296 Max BC CSI: 0.228 Max Web CSI: 0.127 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL C* 86 /- /- /45 /- /6 Wind reactions based on MWFRS C Brg Wid = 105 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375# Maximum Gable Forces Per Ply (lbs) Gables Tens.Comp. B - D 208 -466
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 3X4(D1) except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

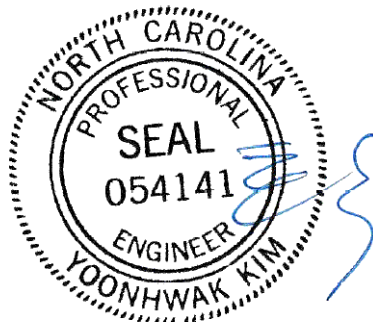
Wind loads based on MWFRS with additional C&C member design.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

See DWGS VALTN160118 and VAL180160118 for valley details.



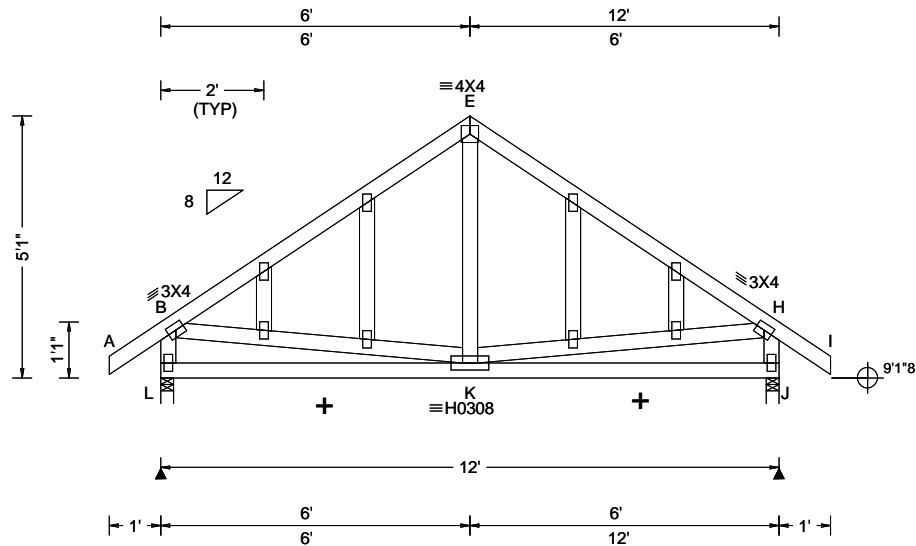
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SEQN: 7775 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: C1G	Cust: R 9836 JRef: 1Y4X98360015 T11 DrwNo: 319.24.0816.01940 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.038 F 999 240 VERT(CL): 0.078 F 999 240 HORZ(LL): 0.023 C - - HORZ(TL): 0.047 C - - Creep Factor: 2.0 Max TC CSI: 0.325 Max BC CSI: 0.322 Max Web CSI: 0.627 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL L 573 /- /- /322 /- /110 J 573 /- /- /322 /- /- Wind reactions based on MWFRS L Brg Wid = 3.0 Min Req = 1.5 (Truss) J Brg Wid = 3.0 Min Req = 1.5 (Truss) Bearings L & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. B - E 121 -505 E - H 121 -505
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

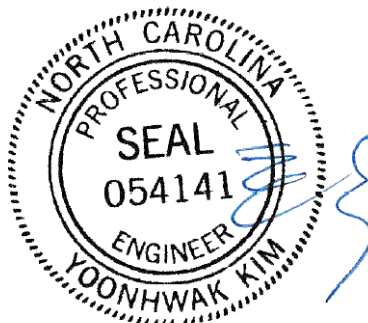
Gable end supports 8" max rake overhang. Top chord must not be cut or notched.
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
End verticals exposed to wind pressure. Deflection meets L/180.
Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
Lanai/Porch Loading : 14.7 PLF wind pressure applied to the bottom chord of the truss from 0.00 ft to 12.00 ft,
+ MEMBER TO BE Laterally Braced For HORIZONTAL WIND LOADS.

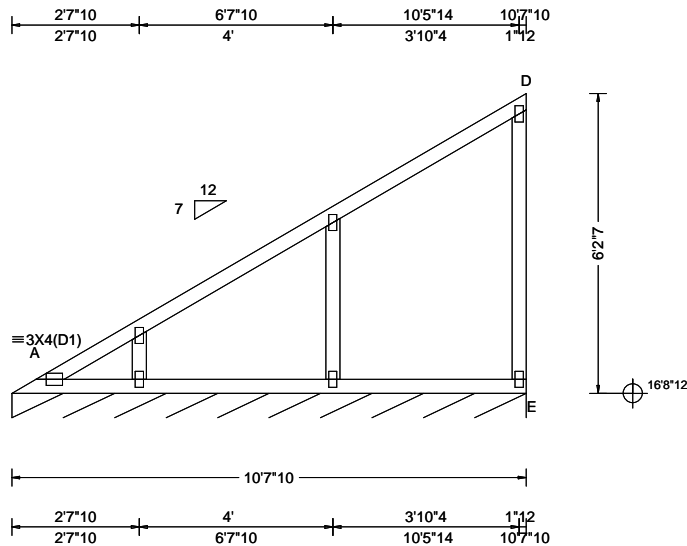


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SEQN: 7743 / FROM:	GABL Ply: 1 Qty: 2	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: VA3	Cust: R 9836 JRef: 1Y4X98360015 T32 DrwNo: 318.24.1558.46229 / YK 11/13/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.98 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.001 A 999 240 VERT(CL): 0.002 A 999 240 HORZ(LL): -0.003 D - - HORZ(TL): 0.004 D - - Creep Factor: 2.0 Max TC CSI: 0.226 Max BC CSI: 0.130 Max Web CSI: 0.478 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E* 83 /- /- /50 /2 /16 Wind reactions based on MWFRS E Brg Wid = 127 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

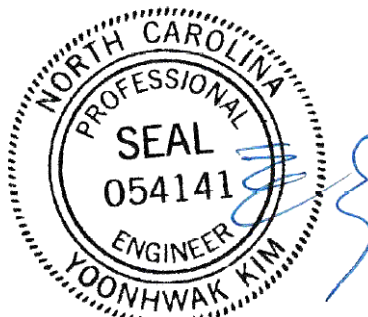
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



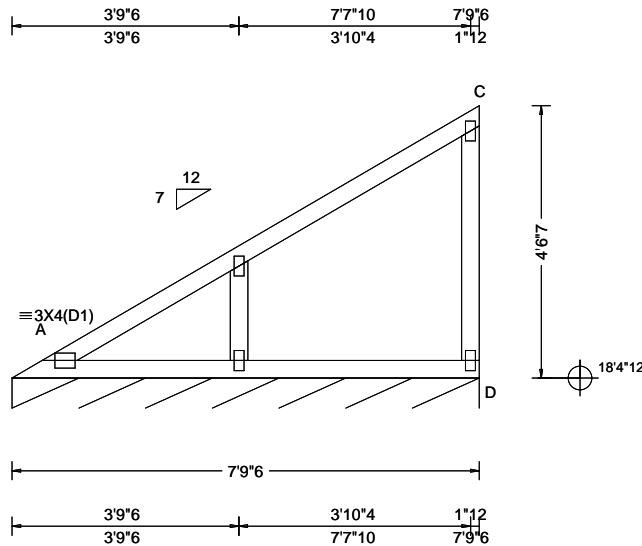
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SEQN: 7742 FROM:	GABL Qty: 2	Ply: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: VA2	Cust: R 9836 JRef: 1Y4X98360015 T39 DrwNo: 319.24.0818.49980 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 20.81 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.009 A 999 240 HORZ(LL): -0.002 C - - HORZ(TL): 0.002 A - - Creep Factor: 2.0 Max TC CSI: 0.284 Max BC CSI: 0.169 Max Web CSI: 0.250 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL D* 83 /- /- /49 /3 /16 Wind reactions based on MWFRS D Brg Wid = 93.4 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

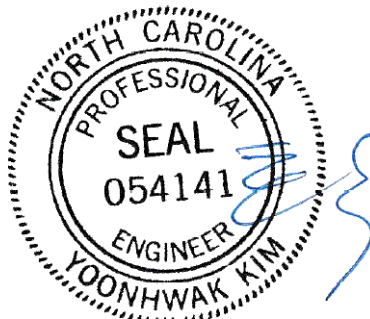
Right end vertical exposed to wind pressure.
Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.

See DWGS VALTN160118 and VAL180160118 for valley details.



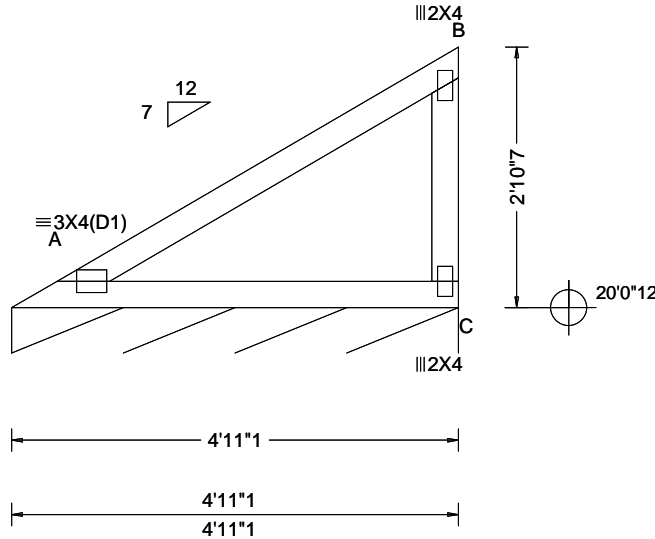
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SEQN: 7741 FROM:	VAL Qty: 2	Ply: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: VA1	Cust: R 9836 JRef: 1Y4X98360015 T40 DrwNo: 319.24.0818.41620 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.64 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): NA VERT(CL): NA HORZ(LL): 0.006 A - - HORZ(TL): 0.012 A - - Creep Factor: 2.0 Max TC CSI: 0.323 Max BC CSI: 0.294 Max Web CSI: 0.126 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL C* 83 /- /- /48 /3 /15 Wind reactions based on MWFRS C Brg Wid = 59.1 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

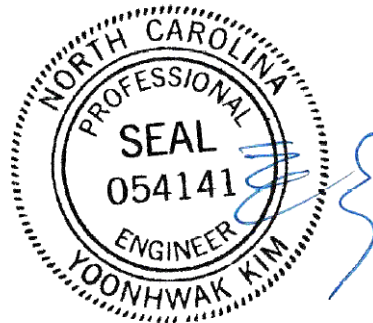
Wind loads based on MWFRS with additional C&C member design.

Right end vertical exposed to wind pressure.
Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

See DWG VALTN160118 for valley details.



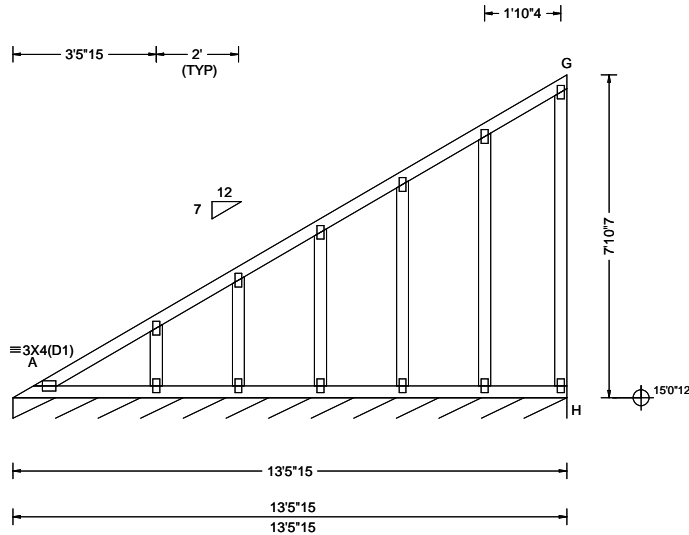
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SEQN: 7745 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: VG4	Cust: R 9836 JRef: 1Y4X98360015 T3 DrwNo: 319.24.0819.02163 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.14 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.004 A 999 240 VERT(CL): 0.009 A 999 240 HORZ(LL): -0.004 G - - HORZ(TL): 0.006 G - - Creep Factor: 2.0 Max TC CSI: 0.124 Max BC CSI: 0.097 Max Web CSI: 0.777 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *=PLF Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL H* 83 /- /- /50 /2 /16 Wind reactions based on MWFRS H Brg Wid = 161 Min Req = - Bearing A is a rigid surface. Members not listed have forces less than 375#
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Lumber

Top chord: 2x4 SP #2;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3;

Plating Notes

All plates are 2X4 except as noted.

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

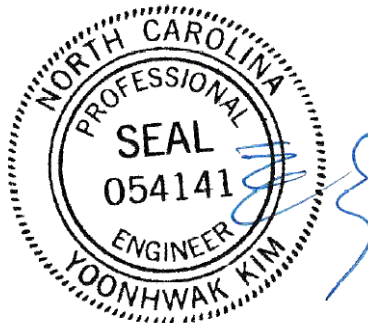
Right end vertical exposed to wind pressure.
Deflection meets L/180.

Wind loading based on both gable and hip roof types.

Additional Notes

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See DWGS VALTN160118 and VAL180160118 for valley details.



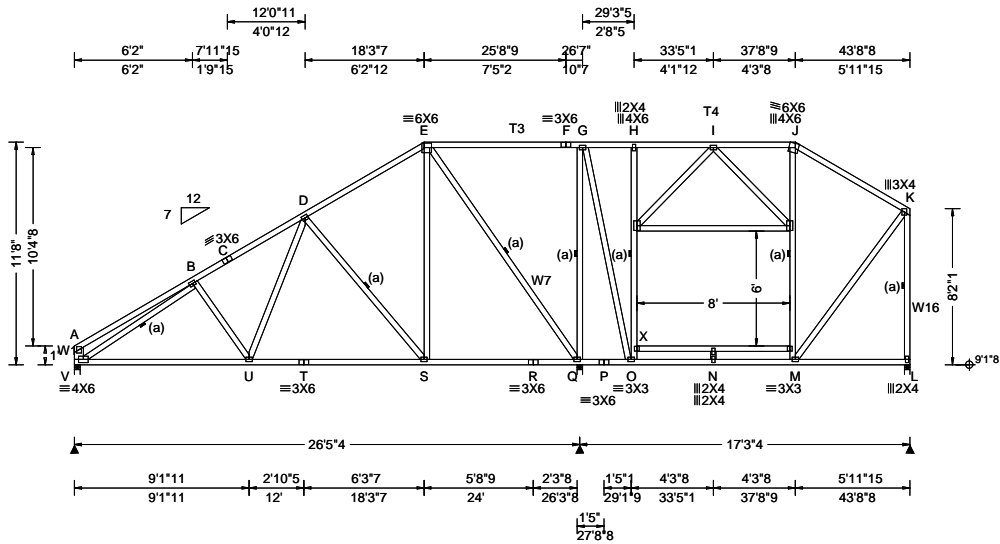
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SEQN: 8641 FROM:	COMN Ply: 1 Qty: 6	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: A1	Cust: R 9836 JRef: 1Y4X98360015 T24 DrwNo: 319.24.0815.13590 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 4.37 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.200 Y 999 240 VERT(CL): 0.390 Y 536 240 HORZ(LL): 0.028 M - - HORZ(TL): 0.055 M - - Creep Factor: 2.0 Max TC CSI: 0.698 Max BC CSI: 0.846 Max Web CSI: 0.752 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity V 1206 /- /- /693 /- /269 Q 1774 /- /- /1161 /- /- L 1058 /- /- /605 /- /- Wind reactions based on MWFRS V Brg Wid = 3.5 Min Req = 1.5 (Truss) Q Brg Wid = 3.5 Min Req = 1.7 (Truss) L Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings V, Q, & L are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp.
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Lumber
Top chord: 2x4 SP #2; T3,T4 2x4 SP SS;
Bot chord: 2x4 SP #2;
Webs: 2x4 SP #3; W1 2x6 SP #2; W7,W16 2x4 SP #2;

Collar-tie braced with continuous lateral bracing at 24" OC. or rigid ceiling.
It is the responsibility of the Building Designer and Truss Fabricator to review this drawing prior to cutting lumber to verify that all data, including dimensions and loads, conform to the architectural plans/specifications and fabricators truss layout.

Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
All plates are 3X4 except as noted.

Loading
Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF
Bottom chord checked for 10.00 psf non-concurrent live load.
Truss designed for unbalanced snow loads.
Truss supports 250# mech unit; unit centered at 33-2-12; supported by TC; unit width 4-0-0; supported by 6 trusses.

Wind
Wind loads based on MWFRS with additional C&C member design.
End verticals not exposed to wind pressure.
Wind loading based on both gable and hip roof types.

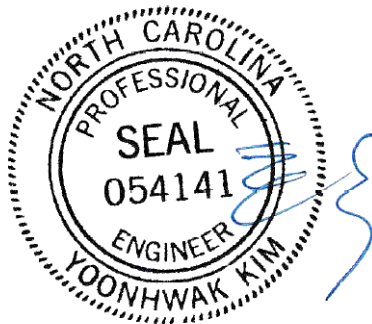
Special loads
----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
TC: From 63 plf at 0.00 to 63 plf at 43.71
PLB: From 80 plf at 29.42 to 80 plf at 37.42
BC: From 20 plf at 0.00 to 20 plf at 43.71
PLT: 42 lb Conc. Load at (31.23,10.05), (35.23,10.05)

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
V - U	1397 -276	R - Q	728 -84
U - T	1131 -186	O - N	447 -40
T - S	1131 -186	N - M	447 -40
S - R	728 -84		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.	Webs	Tens. Comp.
V - B	30 -1323	Q - G	0 -1121
U - D	457 -22	G - O	673 0
D - S	161 -635	O - X	0 -435
E - S	686 -11	M - K	759 0
E - Q	41 -753	K - L	0 -1033

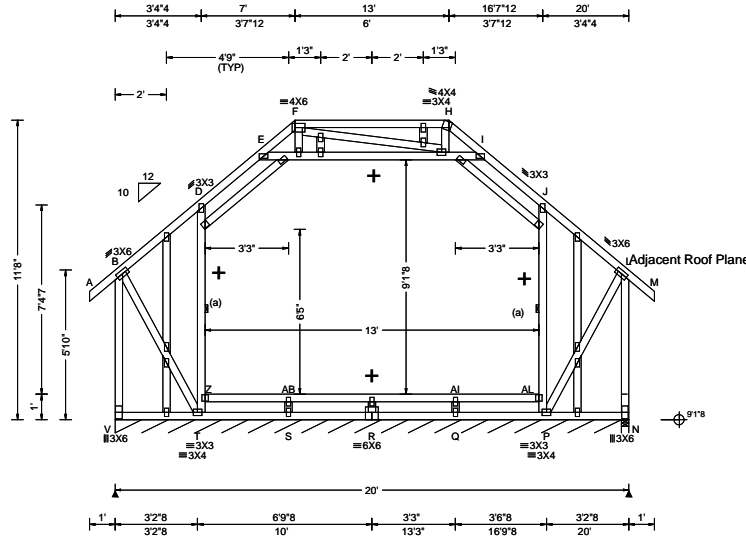


11/14/2024
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SEQN: 7758 FROM:	GABL Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: G1G	Cust: R 9836 JRef: 1Y4X98360015 T17 DrwNo: 319.24.0816.10807 JB / YK 11/14/2024
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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.46 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: varies Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.045 G 999 240 VERT(CL): 0.118 G 999 240 HORZ(LL): 0.026 K - - HORZ(TL): 0.066 K - - Creep Factor: 2.0 Max TC CSI: 0.358 Max BC CSI: 0.169 Max Web CSI: 0.856 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs), or *PLF <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>V*</td> <td>109</td> <td>-</td> <td>-</td> <td>/40</td> <td>/4</td> <td>/14</td> </tr> <tr> <td>N</td> <td>1363</td> <td>-</td> <td>-</td> <td>/528</td> <td>/77</td> <td>-</td> </tr> <tr> <td>T</td> <td></td> <td colspan="2">/-245</td> <td></td> <td></td> <td></td> </tr> <tr> <td>P</td> <td></td> <td colspan="2">/-241</td> <td></td> <td></td> <td></td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS V Brg Wid = 236 Min Req = - N Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings V & N are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - D</td> <td>169 -663</td> <td>H - I</td> <td>220 -596</td> </tr> <tr> <td>D - E</td> <td>233 -687</td> <td>I - J</td> <td>235 -685</td> </tr> <tr> <td>E - F</td> <td>246 -640</td> <td>J - L</td> <td>167 -663</td> </tr> <tr> <td>F - H</td> <td>212 -533</td> <td></td> <td></td> </tr> </tbody> </table> <p>Maximum Bot Chord Forces Per Ply (lbs)</p> <table border="1"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>T - S</td> <td>425 -153</td> <td>R - Q</td> <td>425 -153</td> </tr> <tr> <td>S - R</td> <td>425 -153</td> <td>Q - P</td> <td>425 -153</td> </tr> </tbody> </table> <p>Maximum Web Forces Per Ply (lbs)</p> <table border="1"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - V</td> <td>279 -1345</td> <td>AI - Q</td> <td>0 -389</td> </tr> <tr> <td>B - T</td> <td>949 -196</td> <td>AL - P</td> <td>91 -392</td> </tr> <tr> <td>T - Z</td> <td>92 -389</td> <td>P - L</td> <td>948 -185</td> </tr> <tr> <td>AB - S</td> <td>0 -389</td> <td>L - N</td> <td>265 -1344</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	V*	109	-	-	/40	/4	/14	N	1363	-	-	/528	/77	-	T		/-245					P		/-241					Chords	Tens.Comp.	Chords	Tens. Comp.	B - D	169 -663	H - I	220 -596	D - E	233 -687	I - J	235 -685	E - F	246 -640	J - L	167 -663	F - H	212 -533			Chords	Tens.Comp.	Chords	Tens. Comp.	T - S	425 -153	R - Q	425 -153	S - R	425 -153	Q - P	425 -153	Webs	Tens.Comp.	Webs	Tens. Comp.	B - V	279 -1345	AI - Q	0 -389	B - T	949 -196	AL - P	91 -392	T - Z	92 -389	P - L	948 -185	AB - S	0 -389	L - N	265 -1344
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Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

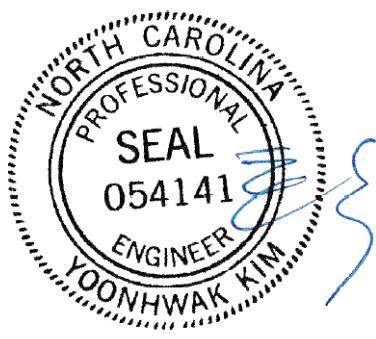
Plating Notes
 All plates are 2X4 except as noted.

Loading
 Bottom chord checked for 10.00 psf non-concurrent live load.
 Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.
 Attic room loading from 3-6-0 to 16-6-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF
 Truss designed for unbalanced snow loads.

Purlins
 Collar-tie braced with continuous lateral bracing at 24" oc. or rigid ceiling.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals exposed to wind pressure. Deflection meets L/180.
 Wind loading based on both gable and hip roof types.

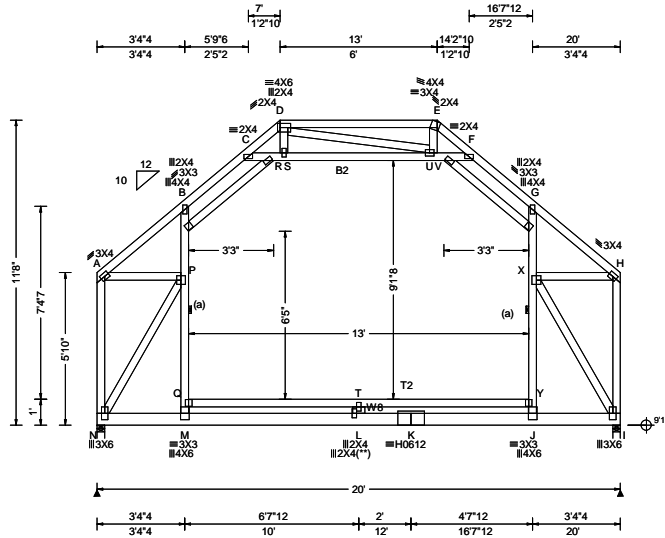
Additional Notes
 See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
 + MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.



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Loading Criteria (psf) TCLL: 20.00 TCDL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.46 ft TCDL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: varies Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE, HS	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.358 T 669 240 VERT(CL): 0.976 T 245 240 HORZ(LL): -0.093 G - - HORZ(TL): 0.237 G - - Creep Factor: 2.0 Max TC CSI: 0.936 Max BC CSI: 0.809 Max Web CSI: 0.982 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) Gravity Non-Gravity Loc R+ /R- /Rh /Rw /U /RL N 1585 -/- /- /477 -/ /235 I 1585 -/- /- /477 -/ -/ Wind reactions based on MWFRS N Brg Wid = 3.5 Min Req = 1.6 (Truss) I Brg Wid = 3.5 Min Req = 1.6 (Truss) Bearings N & I are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 157 -998 E - F 279 -648 B - C 327 -913 F - G 316 -907 C - D 295 -630 G - H 153 -996 D - E 261 -554 Maximum Bot Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. N - M 694 -147 K - J 752 -141 M - L 752 -141 J - I 694 -150 L - K 752 -141 Maximum Web Forces Per Ply (lbs) Webs Tens.Comp. Webs Tens. Comp. A - N 112 -736 T - L 28 -734 A - P 703 -51 U - V 71 -613 N - P 185 -1319 V - F 101 -702 P - Q 1322 0 X - Y 1320 0 Q - M 1040 0 X - H 701 -54 C - R 95 -681 X - I 182 -1319 R - S 88 -585 Y - J 1038 0 S - U 104 -560 H - I 108 -734
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Lumber
Top chord: 2x4 SP #2; T2 2x4 SP SS;
Bot chord: 2x6 SP SS Dense; B2 2x4 SP #2;
Webs: 2x4 SP #3; W8 2x6 SP #2;

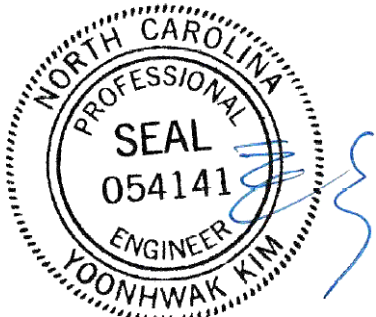
Bracing
(a) Continuous lateral restraint equally spaced on member.

Plating Notes
(**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
Bottom chord checked for 10.00 psf non-concurrent live load.
Live loads applied in combination per ASCE 7 sec. 2.4.1 use 0.75 factor for multiple live loads.
Attic room loading from 3-6-0 to 16-6-0: Live Load: 40 PSF. Dead Load: 10 PSF Ceiling: 10 PSF, Kneewalls: 10 PSF
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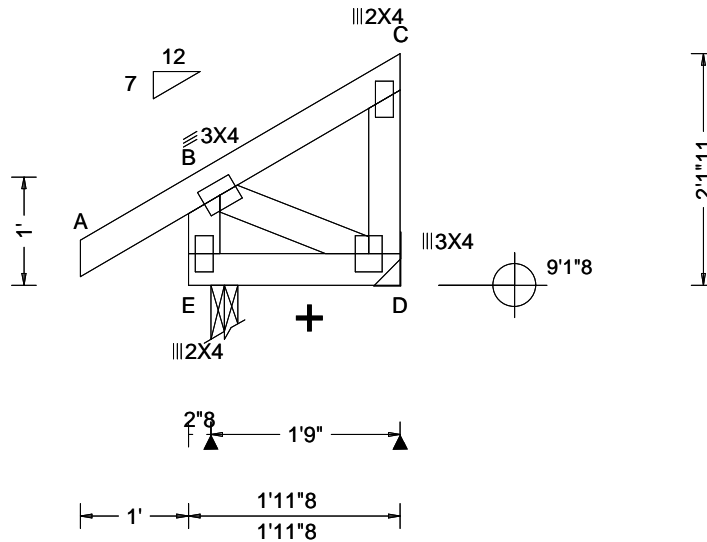


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SEQN: 7792 FROM:	MONO Ply: 1 Qty: 1	Job Number: Q2411-360 The Farm at Neills Creek Truss Label: P2	Cust: R 9836 JRef: 1Y4X98360015 T26 DrwNo: 319.24.0816.47380 JB / YK 11/14/2024
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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BC LL: 0.00 BC DL: 10.00 Des Ld: 40.00 NCBC LL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BC DL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.002 B 999 240 VERT(CL): 0.004 B 946 240 HORZ(LL): -0.002 C - - HORZ(TL): 0.004 C - - Creep Factor: 2.0 Max TC CSI: 0.078 Max BC CSI: 0.099 Max Web CSI: 0.031 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table border="1"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>E</td> <td>201</td> <td>-</td> <td>-</td> <td>/113</td> <td>-</td> <td>/48</td> </tr> <tr> <td>D</td> <td>35</td> <td>-</td> <td>-</td> <td>/31</td> <td>/21</td> <td>-</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	E	201	-	-	/113	-	/48	D	35	-	-	/31	/21	-
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Lumber

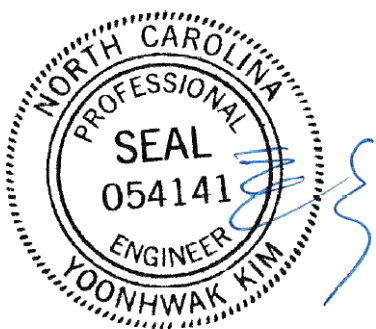
Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

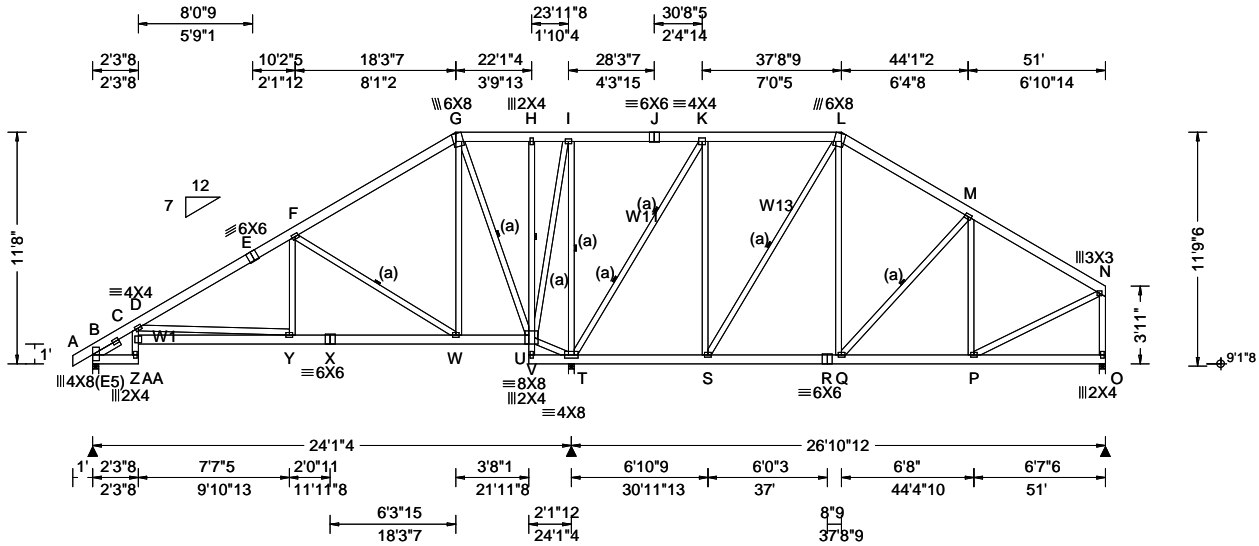
Wind loads based on MWFRS with additional C&C member design.
 Left end vertical exposed to wind pressure. Deflection meets L/180.
 Right end vertical not exposed to wind pressure.
 Left cantilever is exposed to wind
 Wind loading based on both gable and hip roof types.
 See DWGS A12015ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.
 + MEMBER TO BE LATERALLY BRACED FOR HORIZONTAL WIND LOADS.



11/14/2024
 ABCD Engineering, PLLC NC COA 0838

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Loading Criteria (psf) TCCL: 20.00 TCCL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TCCL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 5.10 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Snow Criteria (Pg,Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT:20(0)/10(0) Plate Type(s): WAVE	Defl/CSI Criteria PP Deflection in loc L/def L/# VERT(LL): 0.064 D 999 240 VERT(CL): 0.133 D 999 240 HORZ(LL): 0.048 O - - HORZ(TL): 0.100 O - - Creep Factor: 2.0 Max TC CSI: 0.548 Max BC CSI: 0.419 Max Web CSI: 0.849 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>B</td> <td>578</td> <td>-</td> <td>-</td> <td>/324</td> <td>-</td> <td>/228</td> </tr> <tr> <td>T</td> <td>3055</td> <td>-</td> <td>-</td> <td>/2077</td> <td>-</td> <td>-</td> </tr> <tr> <td>O</td> <td>766</td> <td>-</td> <td>-</td> <td>/596</td> <td>-</td> <td>-</td> </tr> </tbody> </table> <p>Wind reactions based on MWFRS B Brg Wid = 3.5 Min Req = 1.5 (Truss) T Brg Wid = 3.5 Min Req = 3.2 (Truss) O Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings B, T, & O are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs)</p> <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>95 -646</td> <td>H - I</td> <td>902 -46</td> </tr> <tr> <td>C - D</td> <td>99 -602</td> <td>I - J</td> <td>1028 -37</td> </tr> <tr> <td>D - E</td> <td>60 -422</td> <td>J - K</td> <td>1028 -37</td> </tr> <tr> <td>F - G</td> <td>611 -107</td> <td>L - M</td> <td>102 -496</td> </tr> <tr> <td>G - H</td> <td>901 -46</td> <td>M - N</td> <td>38 -668</td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	B	578	-	-	/324	-	/228	T	3055	-	-	/2077	-	-	O	766	-	-	/596	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	95 -646	H - I	902 -46	C - D	99 -602	I - J	1028 -37	D - E	60 -422	J - K	1028 -37	F - G	611 -107	L - M	102 -496	G - H	901 -46	M - N	38 -668
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Lumber
 Top chord: 2x6 SP #2;
 Bot chord: 2x6 SP #2;
 Webs: 2x4 SP #3; W1 2x4 SP SS; W11, W13 2x4 SP #2;
 Lt Slider: 2x4 SP #3; block length = 1.500'

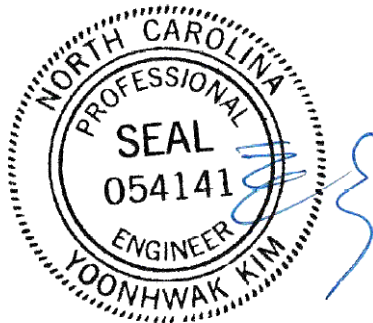
Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 All plates are 3X4 except as noted.

Loading
 Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF
 Bottom chord checked for 10.00 psf non-concurrent live load.
 Truss designed for unbalanced snow loads.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 Right end vertical not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.

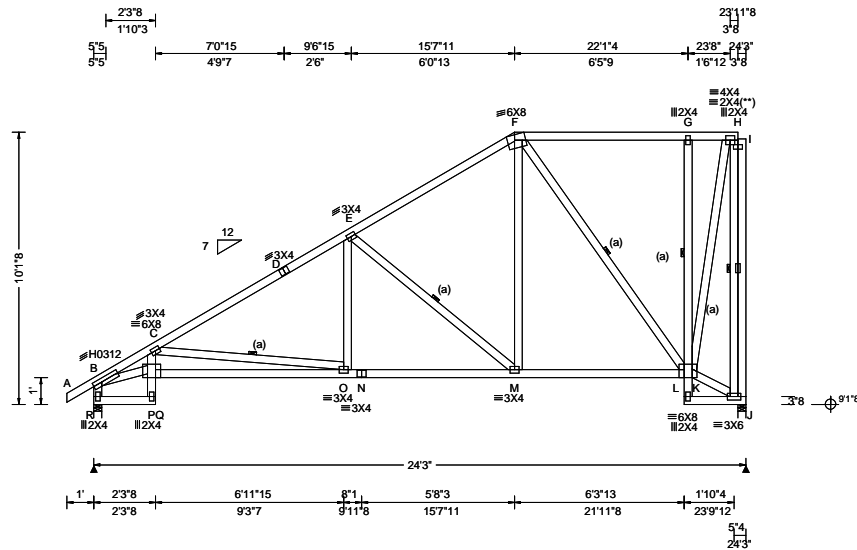
Additional Notes
 WARNING: Furnish a copy of this DWG to the installation contractor. Special care must be taken during handling, shipping and installation of trusses. See "WARNING" note below.



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Loading Criteria (psf) TCLL: 20.00 TC DL: 10.00 BCLL: 0.00 BCDL: 10.00 Des Ld: 40.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Criteria Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 15.00 ft TC DL: 5.0 psf BCDL: 5.0 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCp: 0.18 Wind Duration: 1.60	Snow Criteria (Pg, Pf in PSF) Pg: 20.0 Ct: 1.1 CAT: II Pf: 15.4 Ce: 1.0 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IRC 2021 TPI Std: 2014 Rep Fac: Yes FT/RT: 20(0)/10(0) Plate Type(s): HS, WAVE	Defl/CSI Criteria PP Deflection in loc L/defl L/# VERT(LL): 0.078 O 999 240 VERT(CL): 0.161 O 999 240 HORZ(LL): 0.062 J - - HORZ(TL): 0.127 J - - Creep Factor: 2.0 Max TC CSI: 0.666 Max BC CSI: 0.777 Max Web CSI: 0.981 VIEW Ver: 23.02.04A.0207.13	▲ Maximum Reactions (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th rowspan="2">Loc</th> <th colspan="3">Gravity</th> <th colspan="3">Non-Gravity</th> </tr> <tr> <th>R+</th> <th>/R-</th> <th>/Rh</th> <th>/Rw</th> <th>/U</th> <th>/RL</th> </tr> </thead> <tbody> <tr> <td>J</td> <td>1077</td> <td>-</td> <td>-</td> <td>/636</td> <td>-</td> <td>/269</td> </tr> <tr> <td>R</td> <td>1007</td> <td>-</td> <td>-</td> <td>/687</td> <td>-</td> <td>-</td> </tr> </tbody> </table> Wind reactions based on MWFRS R Brg Wid = 3.5 Min Req = 1.5 (Truss) J Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings R & J are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - C</td> <td>80 -2707</td> <td>D - E</td> <td>27 -1330</td> </tr> <tr> <td>C - D</td> <td>13 -1484</td> <td>E - F</td> <td>53 -873</td> </tr> </tbody> </table> Maximum Bot Chord Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Chords</th> <th>Tens.Comp.</th> <th>Chords</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>P - O</td> <td>2357 -787</td> <td>N - M</td> <td>1179 -288</td> </tr> <tr> <td>O - N</td> <td>1179 -288</td> <td>M - K</td> <td>662 -160</td> </tr> </tbody> </table> Maximum Web Forces Per Ply (lbs) <table border="1" style="width:100%; border-collapse: collapse;"> <thead> <tr> <th>Webs</th> <th>Tens.Comp.</th> <th>Webs</th> <th>Tens. Comp.</th> </tr> </thead> <tbody> <tr> <td>B - R</td> <td>63 -1045</td> <td>F - M</td> <td>590 -13</td> </tr> <tr> <td>B - P</td> <td>2324 -129</td> <td>F - K</td> <td>102 -690</td> </tr> <tr> <td>P - C</td> <td>502 -12</td> <td>K - H</td> <td>1041 -2</td> </tr> <tr> <td>C - O</td> <td>503 -1176</td> <td>H - I</td> <td>17 -982</td> </tr> <tr> <td>O - E</td> <td>391 0</td> <td>I - J</td> <td>97 -1071</td> </tr> <tr> <td>E - M</td> <td>170 -681</td> <td></td> <td></td> </tr> </tbody> </table>	Loc	Gravity			Non-Gravity			R+	/R-	/Rh	/Rw	/U	/RL	J	1077	-	-	/636	-	/269	R	1007	-	-	/687	-	-	Chords	Tens.Comp.	Chords	Tens. Comp.	B - C	80 -2707	D - E	27 -1330	C - D	13 -1484	E - F	53 -873	Chords	Tens.Comp.	Chords	Tens. Comp.	P - O	2357 -787	N - M	1179 -288	O - N	1179 -288	M - K	662 -160	Webs	Tens.Comp.	Webs	Tens. Comp.	B - R	63 -1045	F - M	590 -13	B - P	2324 -129	F - K	102 -690	P - C	502 -12	K - H	1041 -2	C - O	503 -1176	H - I	17 -982	O - E	391 0	I - J	97 -1071	E - M	170 -681		
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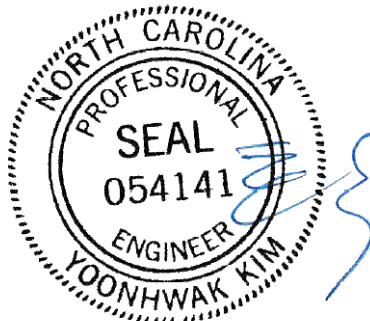
Lumber
 Top chord: 2x4 SP #2;
 Bot chord: 2x4 SP #2;
 Webs: 2x4 SP #3;
 Rt Bearing Leg: 2x4 SP SS;

Bracing
 (a) Continuous lateral restraint equally spaced on member.

Plating Notes
 (**) 1 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

Loading
 Design Dead Loads based on material weight adjusted for slope: BC: 7.00 PSF
 Bottom chord checked for 10.00 psf non-concurrent live load.

Wind
 Wind loads based on MWFRS with additional C&C member design.
 End verticals not exposed to wind pressure.
 Wind loading based on both gable and hip roof types.



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Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 15' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 mph Wind Speed, 15' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 15' Mean Height, Enclosed, Exposure D, Kzt = 1.00

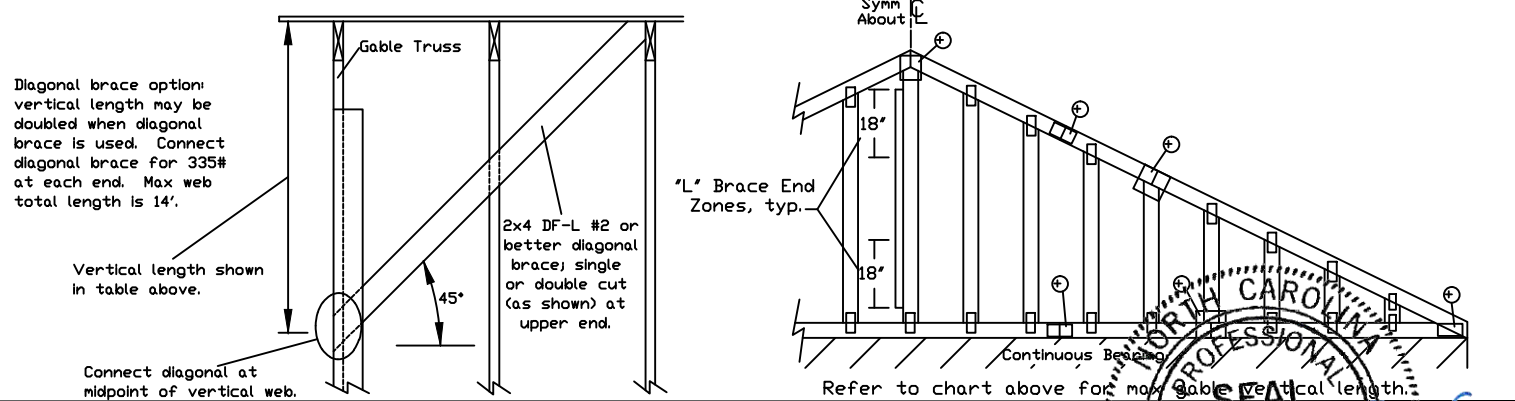
Max Gable Vertical Length	2x4 Gable Vertical		Brace No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **			
	Spacing	Species		Grade	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
	24" o.c.	SPF	#1 / #2	#1	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"
#3				4' 7"	7' 9"	8' 3"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
Stud				4' 7"	7' 8"	8' 2"	9' 7"	9' 11"	11' 5"	11' 10"	14' 0"	14' 0"	14' 0"	14' 0"	
HF			#1	4' 7"	6' 7"	7' 0"	8' 10"	9' 5"	11' 5"	11' 10"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"
			#2	4' 10"	8' 4"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
			#3	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
SP		DFL	#1	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"	
			#2	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"	
			Standard	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"	
		SPF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	
			#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
			Stud	5' 3"	9' 3"	9' 7"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"	
16" o.c.	SPF	#1	5' 0"	8' 4"	8' 7"	9' 10"	10' 2"	11' 8"	12' 1"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 10"	8' 2"	8' 6"	9' 8"	10' 1"	11' 6"	12' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"		
	SP	#1	4' 8"	7' 0"	7' 5"	9' 3"	9' 11"	11' 5"	11' 11"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"		
		Standard	4' 7"	6' 2"	6' 7"	8' 2"	8' 9"	11' 1"	11' 10"	12' 10"	13' 9"	14' 0"	14' 0"		
12" o.c.	SPF	#1 / #2	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 3"	9' 3"	9' 9"	10' 11"	11' 4"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"		
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		#3	5' 6"	9' 5"	9' 9"	11' 1"	11' 6"	13' 2"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"		
SP	DFL	#1	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	5' 5"	8' 6"	9' 1"	11' 0"	11' 5"	13' 1"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	5' 3"	7' 6"	8' 0"	10' 0"	10' 9"	13' 0"	13' 7"	14' 0"	14' 0"	14' 0"	14' 0"		
	SPF	#1 / #2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 9"	10' 2"	10' 7"	12' 0"	12' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
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12" o.c.	SP	#1	6' 4"	10' 6"	10' 10"	12' 4"	12' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	6' 1"	10' 4"	10' 8"	12' 2"	12' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#3	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
	DFL	#1	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		#2	5' 11"	9' 10"	10' 6"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
		Standard	5' 9"	8' 8"	9' 3"	11' 7"	12' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3		#3	
Stud		Stud	
Standard		Standard	
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1		#1	
#2		#2	

1x4 Braces shall be SRB (Stress-Rated Board).
***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.
Provide uplift connections for 35 plf over continuous bearing (5 psf TC Dead Load).
Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
'L' bracing must be a minimum of 80% of web member length.

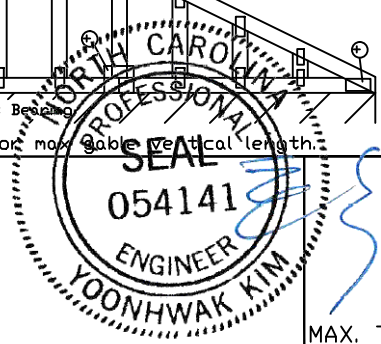
Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

ALPINE
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Glenview, IL 60025

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ABCD Engineering, PLLC NC COA 0838
MAX. TOT. LD. 60 PSF
MAX. SPACING 24'0"

REF	ASCE7-16-GAB12015
DATE	01/26/2018
DRWG	A12015ENC160118

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or: 100 Mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00
Or: 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

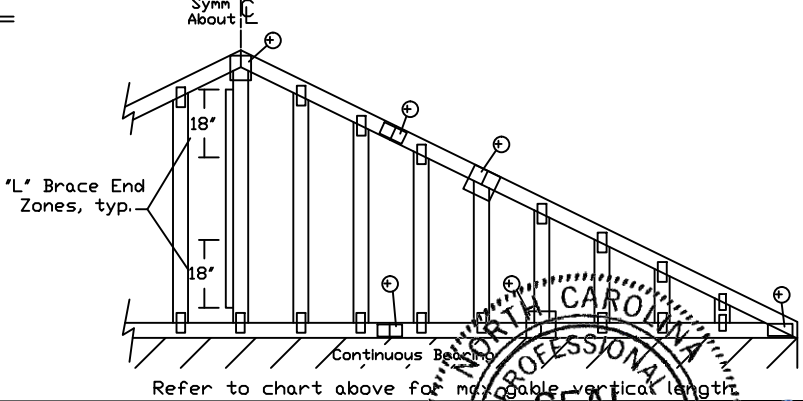
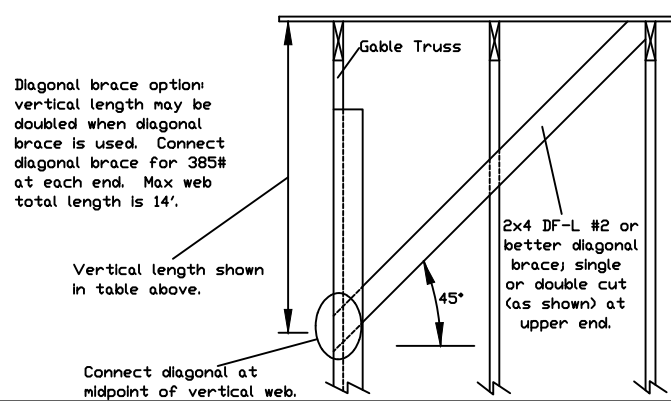
Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade	No Braces	(1) 1x4 'L' Brace *		(1) 2x4 'L' Brace *		(2) 2x4 'L' Brace **		(1) 2x6 'L' Brace *		(2) 2x6 'L' Brace **		
	Species	Grade			Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	
	24" o.c.	SPF	#1 / #2	HF	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
4' 4"					7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	
Stud					4' 4"	7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"
Standard			DFL	4' 4"	6' 2"	6' 7"	8' 2"	8' 9"	10' 10"	11' 4"	12' 10"	13' 9"	14' 0"	14' 0"	
				#1	4' 10"	7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"
				#2	4' 7"	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"
16" o.c.		SPF	#1 / #2	HF	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
					5' 0"	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
					Stud	5' 0"	8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"
		Standard	DFL	#1	5' 0"	7' 6"	8' 0"	10' 1"	10' 9"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"
				#2	5' 3"	8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"
				#3	5' 1"	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"
12" o.c.	SPF	#1 / #2	HF	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	12' 7"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	
				Stud	5' 6"	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"
	Standard	DFL	#1	6' 0"	10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"		
			#2	5' 9"	9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"		
			#3	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"		
Stud	5' 8"	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"				
Standard	5' 6"	8' 1"	8' 7"	10' 9"	11' 6"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"				

Bracing Group Species and Grades:

Group A:			
Spruce-Pine-Fir		Hem-Fir	
#1 / #2	Standard	#2	Stud
#3	Stud	#3	Standard
Douglas Fir-Larch		Southern Pine***	
#3	Stud	#3	Stud
Standard		Standard	
Group B:			
Hem-Fir			
#1 & Btr			
#1			
Douglas Fir-Larch		Southern Pine***	
#1	Stud	#1	Stud
Standard		Standard	

1x4 Braces shall be SRB (Stress-Rated Board).
***For 1x4 So. Pine use only Industrial 55 or Industrial 45 Stress-Rated Boards. Group B values may be used with these grades.

Gable Truss Detail Notes:
Wind Load deflection criterion is L/240.
Provide uplift connections for 70 plf over continuous bearing (5 psf TC Dead Load).
Gable end supports load from 4' 0" outlookers with 2' 0" overhang, or 12' plywood overhang.



Attach 'L' braces with 10d (0.128"x3.0" min) nails.
* For (1) 'L' brace: space nails at 2' o.c. in 18' end zones and 4' o.c. between zones.
** For (2) 'L' braces: space nails at 3' o.c. in 18' end zones and 6' o.c. between zones.
'L' bracing must be a minimum of 80% of web member length.

Gable Vertical Plate Sizes	
Vertical Length	No Splice
Less than 4' 0"	1X4 or 2X3
Greater than 4' 0", but less than 11' 6"	2X4
Greater than 11' 6"	3X4

+ Refer to common truss design for peak, splice, and heel plates.

Refer to the Building Designer for conditions not addressed by this detail.

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MAX. TOT. LD. 60 PSF

ABCD Engineering, PLLC

REF	ASCE7-16-GAB12030
DATE	01/26/2018
DRWG	A12030ENC160118
MAX. SPACING	24'0"

CLR Reinforcing Member Substitution

This detail is to be used when a Continuous Lateral Restraint (CLR) is specified on a truss design but an alternative web reinforcement method is desired.

Notes:

This detail is only applicable for changing the specified CLR shown on single ply sealed designs to T-reinforcement or L-reinforcement or scab reinforcement.

Alternative reinforcement specified in chart below may be conservative. For minimum alternative reinforcement, re-run design with appropriate reinforcement type.

Use scabs instead of L- or T- reinforcement on webs with intersecting truss joints, such as K-web joints, that may interfere with proper application along the narrow face of the web.

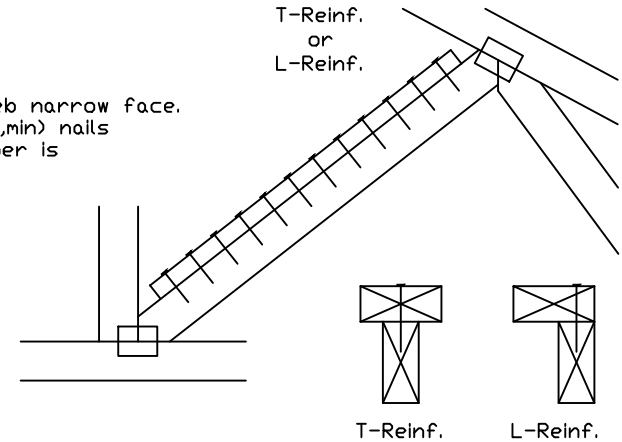
Web Member Size	Specified CLR Restraint	Alternative Reinforcement T- or L- Reinf.	Scab Reinf.
2x3 or 2x4	1 row	2x4	1-2x4
2x3 or 2x4	2 rows	2x6 or 2x4	2-2x4
2x6	1 row	2x4	1-2x6
2x6	2 rows	2x6	2-2x4(*)
2x8	1 row	2x6	1-2x8
2x8	2 rows	2x6	2-2x6(*)

T-reinforcement, L-reinforcement, or scab reinforcement to be same species and grade or better than web member unless specified otherwise on Engineer's sealed design.

(*) Center scab on wide face of web. Apply (1) scab to each face of web.

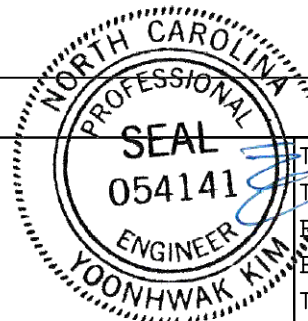
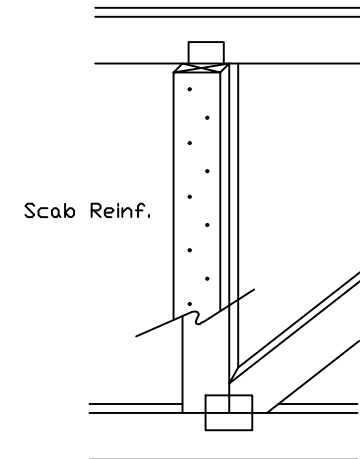
T-Reinforcement or L-Reinforcement:

Apply to either side of web narrow face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



Scab Reinforcement:

Apply scab(s) to wide face of web. No more than (1) scab per face. Attach with 10d (0.128"x3.0",min) nails at 6" o.c. Reinforcing member is a minimum 80% of web member length.



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ABCD Engineering, PLLC NC COA 0838

11/14/2024

TC LL	PSF	REF CLR Subst.
TG DL	PSF	DATE 01/02/19
BC DL	PSF	DRWG BRCLBSUB0119
BC LL	PSF	
TOT. LD.	PSF	
DUR. FAC.		
SPACING		

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

See Engineer's sealed design referencing this detail for lumber, plates, and other information not shown on this detail.

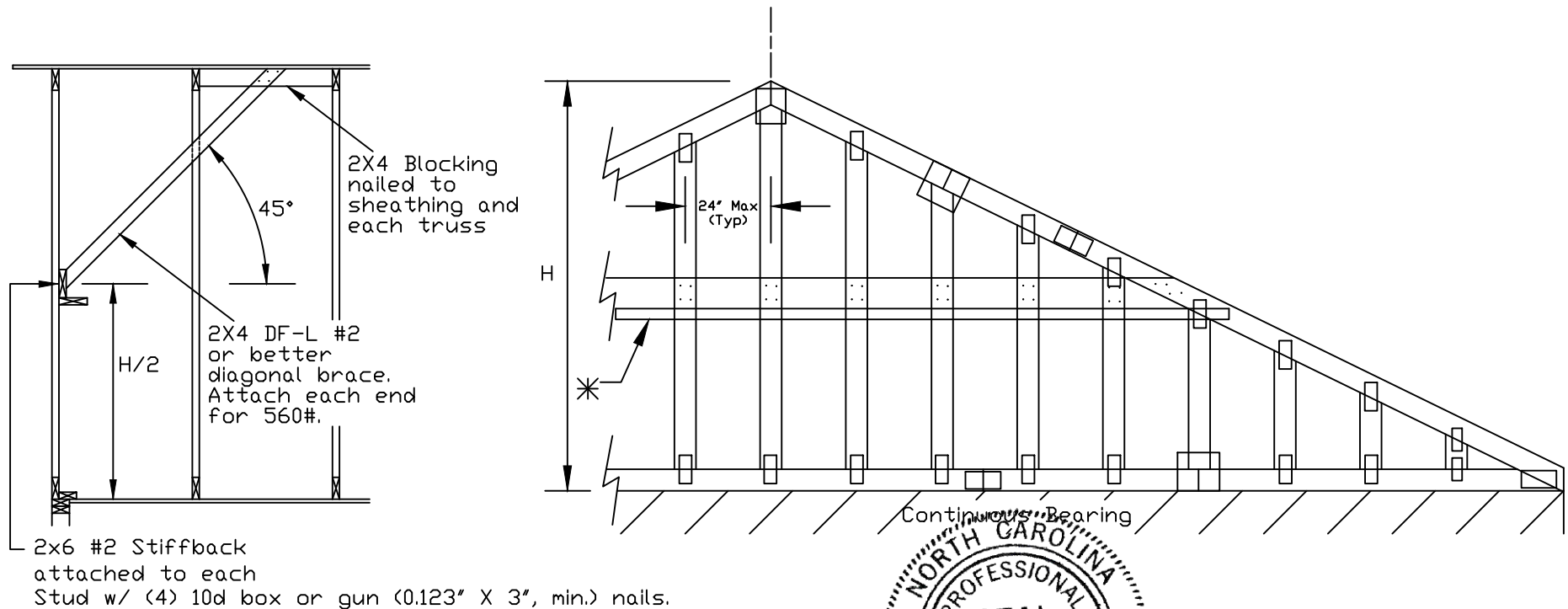
Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required

H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace stiffback to roof diaphragm every 6'0" (see detail below or refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace to roof diaphragm every 4'0" (see detail below or refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached to stiffback with 10d box or gun (0.128" x 3", min.) nails @ 6" o.c.



2x6 #2 Stiffback attached to each Stud w/ (4) 10d box or gun (0.123" X 3", min.) nails.

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MAX. TOT. LD. 60 PSF

MAX. SPACING

ABCD Engineering, PLLC

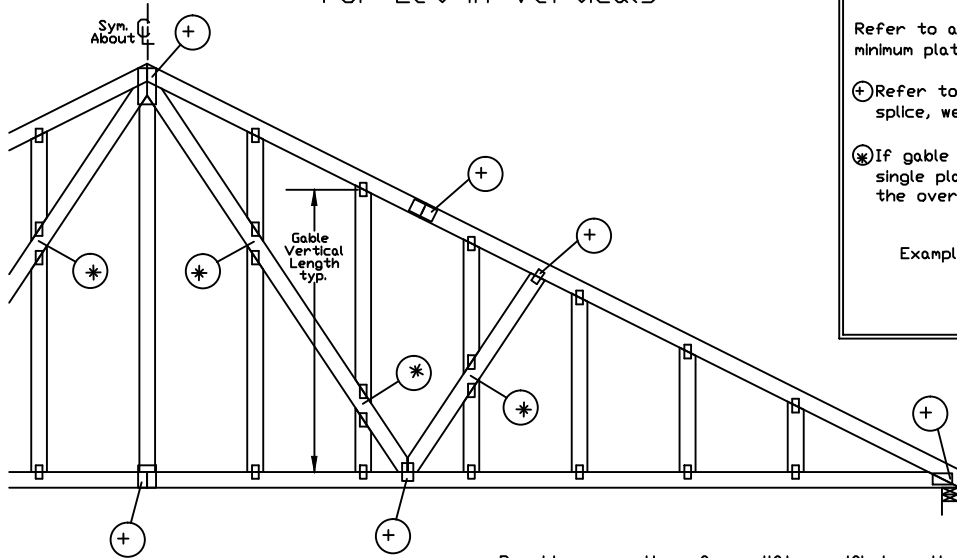
NC COA 0838

REF GE WHALER
DATE 01/02/2018
DRWG GABRST160118



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Glenview, IL 60025

Gable Detail For Let-in Verticals

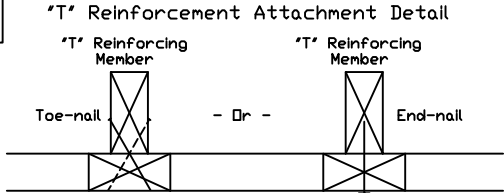


Gable Truss Plate Sizes

Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

- ⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.
- * If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.

Example:



Provide connections for uplift specified on the engineered truss design.
 Attach each 'T' reinforcing member with
 End Driven Nails:
 10d Common (0.148"x 3",min) Nails at 4' o.c. plus
 (4) nails in the top and bottom chords.

Toenailed Nails:
 10d Common (0.148"x 3",min) Toenails at 4' o.c. plus
 (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

- ASCE 7-05 Gable Detail Drawings
 A13015051014, A12015051014, A11015051014, A10015051014, A14015051014,
 A13030051014, A12030051014, A11030051014, A10030051014, A14030051014
- ASCE 7-10 & ASCE 7-16 Gable Detail Drawings
 A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118,
 A18015ENC100118, A20015ENC100118, A20015END100118, A20015PED100118,
 A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118,
 A18030ENC100118, A20030ENC100118, A20030END100118, A20030PED100118,
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 S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118,
 S18030ENC100118, S20030ENC100118, S20030END100118, S20030PED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.

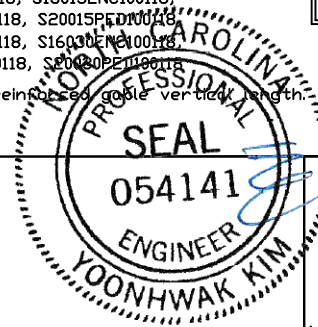
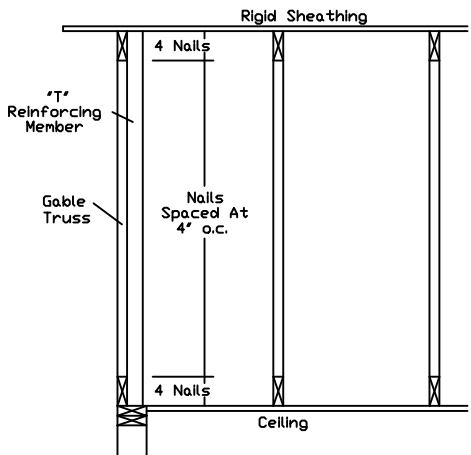
To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord.
 'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf. Mbr. Size	'T' Increase
2x4	30 %
2x6	20 %

Example:
 ASCE 7-10 Wind Speed = 120 mph
 Mean Roof Height = 30 ft, Kzt = 1.00
 Gable Vertical = 24' o.c. SP #3
 'T' Reinforcing Member Size = 2x4
 'T' Brace Increase (From Above) = 30% = 1.30
 (1) 2x4 'L' Brace Length = 8' 7"
 Maximum 'T' Reinforced Gable Vertical Length
 1.30 x 8' 7" = 11' 2"



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ABCD Engineering, PLLC 11/14/2024	REF LET-IN VERT
	DATE 01/02/2018
	DRWG GBLLETIN0118
MAX. TOT. LD. 60 PSF	MAX. SPACING 24.0"

Piggyback Detail - ASCE 7-16: 160 mph, 30' Mean Height, Enclosed, Exposure C, Kzt=1.00

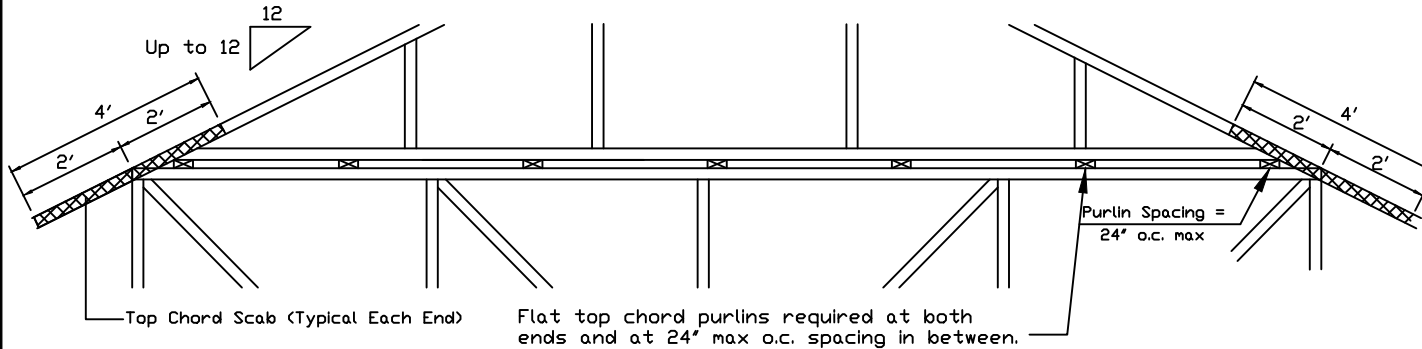
160 mph Wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp C, Wind DL= 5.0 psf (min), Kzt=1.0.
 Or 140 mph wind, 30.00 ft Mean Hgt, ASCE 7-16, Enclosed Bldg. located anywhere in roof, Exp D, wind DL= 5.0 psf (min), Kzt=1.0.

Note: Top chords of trusses supporting piggyback cap trusses must be adequately braced by sheathing or purlins. The building Engineer of Record shall provide diagonal bracing or any other suitable anchorage to permanently restrain purlins, and lateral bracing for out of plane loads over gable ends.

Maximum truss spacing is 24' o.c. detail is not applicable if cap supports additional loads such as cupola, steeple, chimney or drag strut loads.

** Refer to Engineer's sealed truss design drawing for piggyback and base truss specifications.

Detail A : Purlin Spacing = 24" o.c. or less

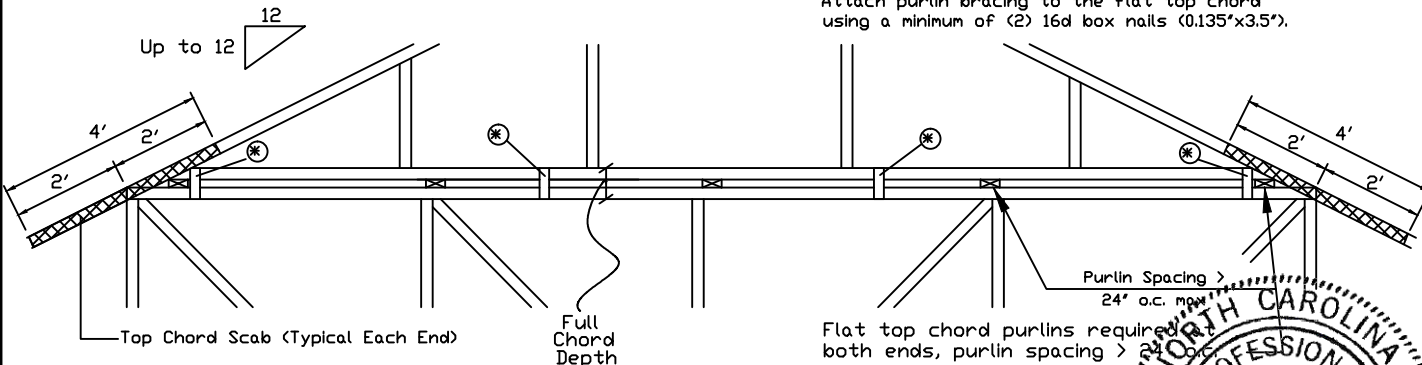


Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using (2) 16d box nails (0.135"x3.5").

The top chord #3 grade 2x4 scab may be replaced with either of the following: (1) 3X8 Trulox plate attached with (8) 0.120"x1.375" nails, (4) into cap TC & (4) into base truss TC or (1) 28PB wave piggyback plate plated to the piggyback truss TC and attached to the base truss TC with (4) 0.120"x1.375" nails. Note: Nailing thru holes of wave plate is acceptable.

Detail B : Purlin Spacing > 24" o.c.



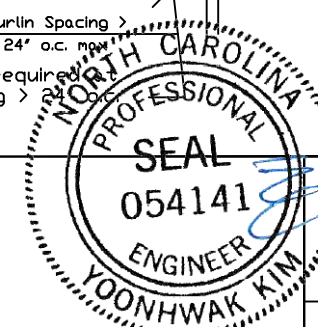
Piggyback cap truss slant nailed to all top chord purlin bracing with (2) 16d box nails (0.135"x3.5") and secure top chord with 2x4 #3 grade scab (1 side only at each end) attached with 2 rows of 10d box nails (0.128"x3") at 4' o.c.

Attach purlin bracing to the flat top chord using a minimum of (2) 16d box nails (0.135"x3.5").

* In addition, provide connection with one of the following methods:

- Trulox**
Use 3X8 Trulox plates for 2x4 chord member, and 3X10 Trulox plates for 2x6 and larger chord members. Attach to each face @ 8' o.c. with (4) 0.120"x1.375" nails into cap bottom chord and (4) in base truss top chord. Trulox plates may be staggered 4' o.c. front to back faces.
- APA Rated Gusset**
8"x8"x7/16" (min) APA rated sheathing gussets (each face). Attach @ 8' o.c. with (8) 6d common (0.113"x2") nails per gusset, (4) in cap bottom chord and (4) in base truss top chord. Gussets may be staggered 4' o.c. front to back faces.
- 2x4 Vertical Scabs**
2x4 SPF #2, full chord depth scabs (each face). Attach @ 8' o.c. with (6) 10d box nails (0.128"x3") per scab, (3) in cap bottom chord and (3) in base truss top chord. Scabs may be staggered 4' o.c. front to back faces.
- 28PB Wave Piggyback Plate**
One 28PB wave piggyback plate to each face @ 8' o.c. Attach teeth to piggyback at time of fabrication. Attach to supporting truss with (4) 0.120"x1.375" nails per face per ply. Piggyback plates may be staggered 4' o.c. front to back faces.

Note: If purlins or sheathing are not specified on the flat top of the base truss, purlins must be installed at 24' o.c. max. and use Detail A.



155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

WARNING: READ AND FOLLOW ALL NOTES ON THIS DRAWING. IMPORTANT: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.

Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI (Building Component Safety Information, by TPI and SBCA) for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs shall have bracing installed per BCSI sections B3, B7 or B10, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 160A-Z for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.

A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see this Job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ABCD Engineering, PLLC NC COA 0838
 SPACING 24.0"

REF PIGGYBACK
 DATE 01/02/2018
 DRWG PB160160118

Valley Detail - ASCE 7-16: 30' Mean Height, Enclosed, Exp. C, Kzt=1.00

Top Chord 2x4 SP #2N, SPF #1/#2, DF-L #2 or better.
 Bot Chord 2x4 SP #2N or SPF #1/#2 or better.
 Webs 2x4 SP #3, SPF #1/#2, DF-L #2 or better.

** Attach each valley to every supporting truss with:
 (2) 16d box (0.135" x 3.5") nails toe-nailed for
 ASCE 7-16, 30' Mean Height, Enclosed Building, Exp. C,
 Wind TC DL=5 psf, Kzt = 1.00, Max. Wind Speed based on
 supporting truss material at connection location:
 170 mph for SP (G = 0.55, min.),
 155 mph for DF-L (G = 0.50, min.), or
 120 mph for HF & SPF (G = 0.42, min.).

Maximum top chord pitch is 10/12 for supporting trusses
 below valley trusses.

Bottom chord of valley trusses may be square or
 pitched cut as shown.

Valleys short enough to be cut as solid triangular
 members from a single 2x6, or larger as required,
 shall be permitted in lieu of fabricating from
 separate 2x4 members.

All plates shown are Alpine Wave Plates.

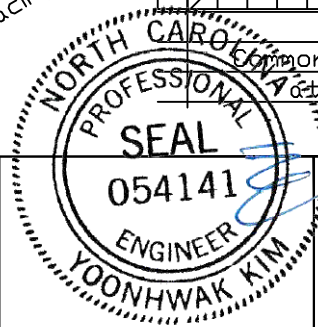
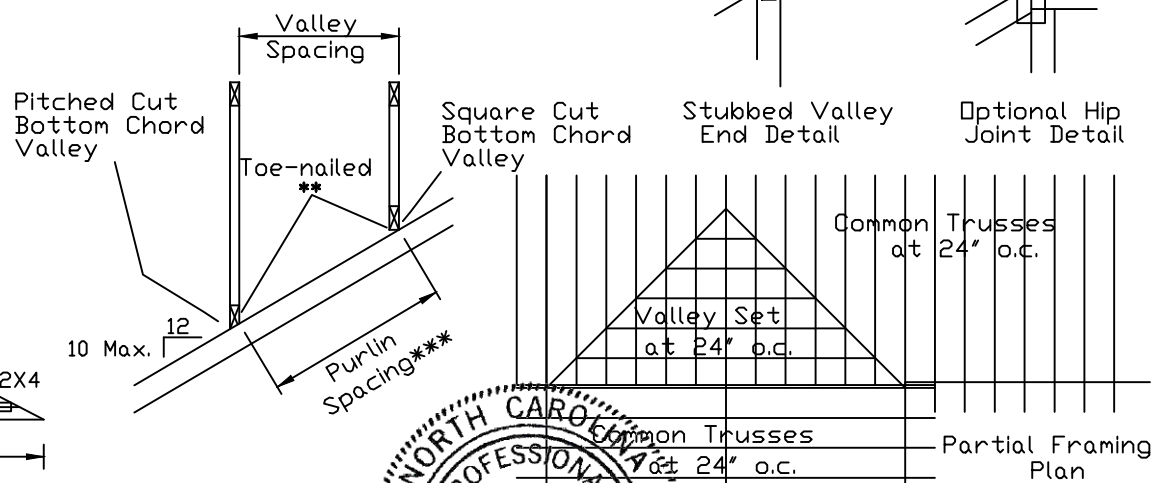
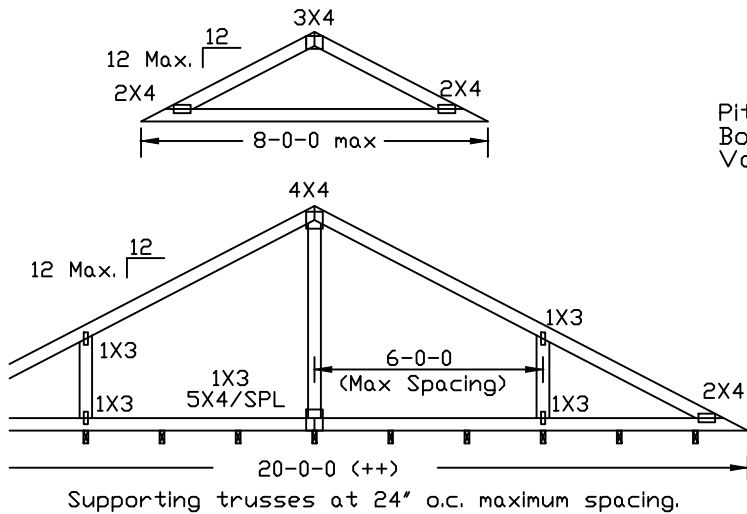
Unless specified otherwise on engineer's sealed design, for vertical
 valley webs taller than 7-9" apply 2x4 "T" reinforcement, 80% length of
 web, same species and grade or better, attached with 10d box
 (0.128" x 3.0") nails at 6" o.c. In lieu of "T" reinforcement, 2x4 Continuous
 Lateral Restraint applied at mid-length of web is permitted with diagonal
 bracing as shown in DRWG BRCLBANC1014.

Top chord of truss beneath valley set must be braced with:
 properly attached, rated sheathing applied prior to valley truss
 installation.

- Or
- Purlins at 24" o.c. or as otherwise specified on engineer's sealed design
- Or
- By valley trusses used in lieu of purlin spacing as specified on
 Engineer's sealed design.

*** Note that the purlin spacing for bracing the top chord of the truss
 beneath the valley is measured along the slope of the top chord.

++ Larger spans may be built as long as the vertical height does
 not exceed 14'-0".



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 For more information see this job's general notes page and these web sites:
 ALPINE: www.alpineitw.com; TPI: www.tpinst.org; SBCA: www.sbcacomponents.com; ICC: www.iccsafe.org

ABCD Engineering, PLLC
 11/14/2024

TC LL	30	30	40PSF
TC DL	20	15	7 PSF
BC DL	10	10	10 PSF
BC LL	0	0	0 PSF
TOT. LD.	60	55	57PSF
NURFOA	1854.33	1.15	1.15
SPACING	24.0"		

REF	VALLEY DETAIL
DATE	01/26/2018
DRWG	VALTN160118