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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 46062 JOB: 24-1487-R01 JOB NAME: LOT 0.0130 BLAKE POND Wind Code: 37 Wind Speed: Vult= 115mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. *42 Truss Design(s)*

Trusses:

PB01, R01, R02, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, R18A, R19, R19A, R19B, R20, R21, R22, R23, R24, R25, R26, VT01, VT02, VT03, VT04, VT05, VT06, VT07, VT08, VT09, VT10, VT11, VT12



Warning !-- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling,



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

7)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 22 lb uplift at joint 2 and 26 lb uplift at joint 4.

 See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

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LOAD CASE(S) Standard
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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- Gable studs spaced at 2-0-0 oc.

6) Gable studs spaced at 2-0-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
9) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.
11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5 except (jt=lb) 2=111.
LOAD CASE(S) Standard

3/1/2024



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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2x4 =

2x4 ||

LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * DCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.35 BC 0.29 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/d -0.00 3 0.00 4 0.00 r	defl L/d n/r 180 n/r 80 n/a n/a	PLATES MT20 Weight: 15 lb	GRIP 244/190 • FT = 20%
BCDL 10.0						U U	
LUMBER-			BRACING-				
TOP CHORD 2x4 SP No 2			TOP CHORD	Structural wood	l sheathing direct	ly applied or 4-0-0 o	c purlins except
BOT CHORD 2x4 SP No 3				end verticals	i onoaannig anoor		o painio, oktoopt
WEBS 2x4 SP No.3			BOT CHORD	Rigid ceiling dire	ectly applied or 1	0-0-0 oc bracing.	
				MiTek recomm	nondo that Stabili	izere and required a	
				he installed du	ring truce creation	izers and required ci	th Stabilizer
				be installed du	anng truss erectio	on, in accordance wi	un Stabilizer
				Installation gui	iae.		

REACTIONS. (lb/size) 6=162/4-0-0 (min. 0-1-8), 2=211/4-0-0 (min. 0-1-8) Max Horz 2=51(LC 10) Max Uplift6=-28(LC 14), 2=-42(LC 10) Max Grav 6=216(LC 21), 2=289(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (12)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.
- 11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 6=154/Mechanical, 2=216/0-3-8 (min. 0-1-8) Max Horz 2=51(LC 10) Max Uplift6=-27(LC 14), 2=-46(LC 10) Max Grav 6=205(LC 21), 2=297(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads. 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 5=351/Mechanical, 2=163/0-3-8 (min. 0-1-8) Max Horz 2=72(LC 10) Max Uplift5=-93(LC 14), 2=-21(LC 10) Max Grav 5=497(LC 21), 2=189(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. TOP CHORD 3-5=-460/293

NOTES- (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit

between the bottom chord and any other members. 7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 2.

LOAD CASE(S) Standard







of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 G	GREAT SMOKY PLACE LILLINGTON, NC
24-1487-R01	R12	Common Girder	1	2	Job Reference (optional)	# 46062
		Run: 8 ID:o	430 s Feb 12 DuWOOMh	2 2021 Print _xMOj2fwo	8.430 s Feb 12 2021 MiTek Indus p2aKqzMG6w-MQJhOJ2GiT	stries, Inc. Mon Mar 4 16:31:51 2024 Page 2 3dq0clQayOgjoiCagqahzcYY2nEPzeFys

LOAD CASE(S) Standard

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 1-7=-20

Concentrated Loads (lb) Vert: 10=-1256(B) 11=-1256(B) 12=-1256(B) 13=-1256(B) 14=-1256(B) 15=-1256(B)



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Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT SM	MOKY PLACE LILLINGTON, NC
24-1487-R01	R14	Common Girder	1	2	Job Reference (optional)	# 46062
	·		Run: 8.430 s Feb 12 ID:oDuWOOMhLx	2021 Print MOj2fwcr	t: 8.430 s Feb 12 2021 MiTek Industries, Inc 52aKgzMG6w-BagyfM60IJgmYx3vnr3	. Mon Mar 4 16:31:57 2024 Page 2 pw 2dD rg LPVwTV5R3zeFym

NOTES- (13)

11) Use Simpson Strong-Tie HTU26 (20-10d Girder, 11-10dx1 1/2 Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 8-0-12 from the left end to 16-0-12 to connect truss(es) R18 (1 ply 2x4 SP), R18A (1 ply 2x4 SP), R19A (1 ply 2x4 SP) to back face of bottom chord. 12) Fill all nail holes where hanger is in contact with lumber.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf)

Vert: 1-4=-60, 4-7=-60, 1-7=-20 Concentrated Loads (lb)

Vert: 11=-708(B) 12=-708(B) 13=-818(B) 14=-1447(B) 15=-1447(B) 16=-1447(B) 17=-1447(B) 18=-1447(B)



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responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREA	T SMOKY PLACE LILLINGTON, NC
24-1487-R01	R17	PIGGYBACK BASE	1	2	Job Reference (optional)	# 46062
		Run: i	3.430 s Feb 1 ID:r9MH5r8	2 2021 Print 9tuDnH9l	: 8.430 s Feb 12 2021 MiTek Industries <etutjwyljik-uvhkineixo4litqqn< td=""><td>, Inc. Mon Mar 4 16:32:07 2024 Page 2 MyE9K5TMz0DLKsAzD1wdoUzeFyc</td></etutjwyljik-uvhkineixo4litqqn<>	, Inc. Mon Mar 4 16:32:07 2024 Page 2 MyE9K5TMz0DLKsAzD1wdoUzeFyc

NOTES- (13)

9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

10) Refer to girder(s) for truss to truss connections.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 2423 lb uplift at joint 2, 1557 lb uplift at joint 16, 1774 lb uplift at joint 14 and 2171 lb uplift at joint 10.

12) This truss has been designed for a total drag load of 200 plf. Lumber DOL=(1.33) Plate grip DOL=(1.33) Connect truss to resist drag loads along bottom chord from 0-0-0 to 15-9-8 for 468.6 plf.

LOAD CASE(S) Standard



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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



LOAD CASE(S)

aor	Iruss	Iruss Type	Qty	Ply	LOI 0.0130 BLAKE POND 95 GREAT SMOKY PLACE LI	LLINGTON, NC
24-1487-R01	R19	PIGGYBACK BASE	2	· ·	Lich Reference (optional) # 4	6062
			Run: 8.430 s Feb	12 2021 Pri	JOB Reference (optional) t: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 4 16:	32:14 2024 Page 2
			ID:oDuWOOMhL	.xMOj2fwcj	v2aKqzMG6w-BrCNDAJhIXzM5YsAHwso6aFXKrVpT2	2y?qd7VYazeFy
LOAD CASE(S) Sta	indard					
1) Dead + Snow (ba Uniform Loads (r	llanced): Lumber Increas If)	e=1.15, Plate Increase=1.15				
Vert: 1-5	=-60, 5-6=-60, 6-10=-60,	2-9=-20				
3) Dead + 0.75 Roo	f Live (balanced) + 0.75	Uninhab. Attic Storage: Lumber Increa	se=1.15, Plate Increas	se=1.15		
Vert: 1-5	=-50. 5-6=-50. 6-10=-50.	2-23=-20. 23-24=-65. 24-25=-20. 25-2	6=-65. 9-26=-20. 27-2	8=-45(F)		
4) Dead + 0.75 Sno	w (balanced) + 0.75 Unin	hab. Attic Storage: Lumber Increase=	1.15, Plate Increase=1	1.15		
Uniform Loads (p	lf) =-50 5-6=-50 6-10=-50	2-23=-20 23-24=-65 24-25=-20 25-2	6=-65 9-26=-20 27-2	8=-45(F)		
5) Dead + Uninhabi	table Attic Without Stora	ge: Lumber Increase=1.25, Plate Incre	ase=1.25	040(1)		
Uniform Loads (p	lf)	2 0- 40 27 28- 60(5)				
19) Dead + Uninhat	=-20, 5-6=-20, 6-10=-20, bitable Attic Storage: Lum	2-9=-40, 27-28=-60(F) ber Increase=1.25. Plate Increase=1.2	25			
Uniform Loads (plf)					
Vert: 1-	5=-20, 5-6=-20, 6-10=-20	, 2-23=-20, 23-24=-80, 24-25=-20, 25-	26=-80, 9-26=-20, 27-	28=-60(F)	and 1 60 Plate Increase 1 60	
Uniform Loads (ow (bal.) + 0.75 Uninnab olf)	. Allic Storage + 0.75(0.6 MWFRS Wir	ia (Neg. Int) Leit): Lun	nder Incre	ase=1.60, Plate increase=1.60	
Vert: 1-2	2=-55, 2-5=-58, 5-6=-34,	6-9=-44, 9-10=-40, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F)	
Horz: 1-	2=5, 2-5=8, 6-9=6, 9-10=	:10 Attic Storago + 0.75(0.6 MM/EBS M/ir	d (Nog. Int) Dight): Lu	umbor Inor	anno-1.60. Blata Ingranga-1.60	
Uniform Loads (plf)	. Allic Slorage + 0.75(0.0 MWFRS Wil			ease-1.00, Flate Increase-1.00	
Vert: 1-2	2=-40, 2-5=-44, 5-6=-34,	6-9=-58, 9-10=-55, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F)	
-Horz: 1 22) Dead + 0 75 Sn	2=-10, 2-5=-6, 6-9=-8, 9- ow (bal) + 0 75 Uninbab	10=-5 Attic Storage + 0.75(0.6 MWERS Wir	nd (Neg. Int) 1st Parall	el): Lumb	er Increase=1.60. Plate Increase=1.60	
Uniform Loads (plf)					
Vert: 1-2	2=-30, 2-5=-34, 5-6=-44,	6-9=-44, 9-10=-40, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F)	
	2=-20, 2-5=-16, 6-9=6, 9 ow (bal.) + 0.75 Uninhab	. Attic Storage + 0.75(0.6 MWFRS Wir	nd (Neg. Int) 2nd Paral	llel): Lumb	er Increase=1.60. Plate Increase=1.60	
Uniform Loads (plf)		··· (···· g · ···) _···· · ···			
Vert: 1-2	2=-40, 2-5=-44, 5-6=-44, 2= 10, 2, 5= 6, 6, 0=16, 0	6-9=-34, 9-10=-30, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F)	
-1012. 1- 24) Dead + 0.75 Ro	of Live (bal.) + 0.75 Unin	hab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. Int) Left):	Lumber II	crease=1.60, Plate Increase=1.60	
Uniform Loads (plf)					
Vert: 1-2 Horz: 1-	2=-55, 2-5=-58, 5-6=-34, 2=5_2-5=8_6-9=6_9-10=	6-9=-44, 9-10=-40, 2-23=-20, 23-24=-6 :10	55, 24-25=-20, 25-26=	-65, 9-26=	=20, 27-28=-45(F)	
25) Dead + 0.75 Ro	of Live (bal.) + 0.75 Unin	hab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. Int) Right): Lumber	Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf)			0.000		
Horz: 1-	2=-40, 2-5=-44, 5-6=-34, 2=-10, 2-5=-6, 6-9=-8, 9-	0-9=-58, 9-10=-55, 2-23=-20, 23-24=-0 10=-5	55, 24-25=-20, 25-26=	-05, 9-20-	-20, 27-28=-45(F)	
26) Dead + 0.75 Ro	of Live (bal.) + 0.75 Unin	hab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. Int) 1st Pa	arallel): Lu	mber Increase=1.60, Plate Increase=1.60	
Uniform Loads (plf) 2=-30_2-5=-34_5-6=-44	6-9=-11 9-10=-10 2-23=-20 23-21=-6	35 24-25=-20 25-26=	-65 9-26-	-20 27-28=-45(F)	
Horz: 1-	2=-20, 2-5=-16, 6-9=6, 9-	-10=10	55, 24-25-20, 25-20-	-05, 9-20-	-20, 27-2043(1)	
27) Dead + 0.75 Ro	of Live (bal.) + 0.75 Unin	hab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. Int) 2nd F	Parallel): L	umber Increase=1.60, Plate Increase=1.60	
Uniform Loads (Vert: 1-2	pit) 2=-40 2-5=-44 5-6=-44	6-9=-34 9-10=-30 2-23=-20 23-24=-6	5 24-25=-20 25-26=	-65 9-26=	-20 27-28=-45(F)	
Horz: 1-	2=-10, 2-5=-6, 6-9=16, 9	-10=20	20, 20 20, 20 20	00,0 20	20, 21 20 10(1)	
32) 3rd Dead + 0.75	Roof Live (unbalanced)	+ 0.75 Uninhab. Attic Storage: Lumbe	r Increase=1.15, Plate	Increase	=1.15	
Vert: 1-	5=-50, 5-6=-50, 6-10=-20	, 2-23=-20, 23-24=-65, 24-25=-20, 25-	26=-65, 9-26=-20, 27-	28=-45(F)		
33) 4th Dead + 0.75	Roof Live (unbalanced)	+ 0.75 Uninhab. Attic Storage: Lumbe	r Increase=1.15, Plate	Increase	=1.15	
Uniform Loads (Vert: 1-	pit) 5=-20 5-6=-50 6-10=-50	2-23=-20 23-24=-65 24-25=-20 25-	26=-65 9-26=-20 27-	28=-45(F)		
46) Reversal: Dead	+ 0.75 Snow (bal.) + 0.7	5 Uninhab. Attic Storage + 0.75(0.6 M	WFRS Wind (Neg. Int)) Left): Lur	nber Increase=1.60,	
Plate Increase=	1.60					
Vert: 1-2	2=-55, 2-5=-58, 5-6=-34,	6-9=-44, 9-10=-40, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F)	
Horz: 1-	2=5, 2-5=8, 6-9=6, 9-10=	10				
47) Reversal: Dead Plate Increase=	+ 0.75 Snow (bal.) + 0.75 1.60	5 Uninhab. Attic Storage + 0.75(0.6 M)	WERS Wind (Neg. Int)) Right): Li	Imber Increase=1.60,	
Uniform Loads (plf)					
Vert: 1-2	2=-40, 2-5=-44, 5-6=-34,	6-9=-58, 9-10=-55, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F)	
-1 :Horz 48) Reversal: Dead	2=10, 2-5=-6, 6-9=-8, 9- + 0.75 Snow (bal.) + 0.7	5 Uninhab. Attic Storage + 0.75(0.6 M)	WFRS Wind (Neg. Int)) 1st Paral	el): Lumber	
Increase=1.60,	Plate Increase=1.60	· · · · · · · · · · · · · · · · · ·		,	NONULINI CA STATU	
Uniform Loads (plf) 2-30-25-34-56-44	6 0- 44 0 10- 40 2 23- 20 23 24- 6	5 24 25- 20 25 26-	65 0 26-	20 27 28- 45(E)	e
Horz: 1-	2=-20, 2-5=-16, 6-9=6, 9	-10=10	JU, Z T -2U-2U, 2U-2U-	-00, 9-20-	LO, LI-LU-TU(I)	Inen
49) Reversal: Dead	+ 0.75 Snow (bal.) + 0.7	5 Uninhab. Attic Storage + 0.75(0.6 M	WFRS Wind (Neg. Int)) 2nd Para	llel): Lumber	1111
Increase=1.60,	Plate Increase=1.60				SEAL	
Vert: 1-2	2=-40, 2-5=-44, 5-6=-44,	6-9=-34, 9-10=-30, 2-23=-20, 23-24=-6	65, 24-25=-20, 25-26=	-65, 9-26=	-20, 27-28=-45(F) 28147	1111
Horz: 1-	2=-10, 2-5=-6, 6-9=16, 9	-10=20		1		In
DU) Reversal: Dead Plate Increase=	+ 0.75 KOOT LIVE (bal.) + 1.60	U.15 UNINNAD. Attic Storage + 0.75(0.0	o WINTERS Wind (Neg.	. Int) Left):	Lumber Increase=1.60, A NoinEER 6	in.
					MARK & MORRAUM	
					All the second s	

Continuing by paging 3design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

3/1/2024

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT SMOKY	PLACE LILLINGTON, NC
24-1487-R01	R19	PIGGYBACK BASE	2	1	Job Reference (optional)	# 46062

Run: 8.430 s Feb 12 2021 Print: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon Mar 4 16:32:14 2024 Page 3 ID:oDuWOOMhLxMOj2fwcp2aKqzMG6w-BrCNDAJhIXzM5YsAHwso6aFXKrVpT2y?qd7VYazeFyV

LOAD CASE(S) Standard

Uniform Loads (plf)

Vert: 1-2=-55, 2-5=-58, 5-6=-34, 6-9=-44, 9-10=-40, 2-23=-20, 23-24=-65, 24-25=-20, 25-26=-65, 9-26=-20, 27-28=-45(F)

Horz: 1-2=5, 2-5=8, 6-9=6, 9-10=10

51) Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) Right): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-40, 2-5=-44, 5-6=-34, 6-9=-58, 9-10=-55, 2-23=-20, 23-24=-65, 24-25=-20, 25-26=-65, 9-26=-20, 27-28=-45(F)

Horz: 1-2=-10, 2-5=-6, 6-9=-8, 9-10=-5

52) Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 1st Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-30, 2-5=-34, 5-6=-44, 6-9=-44, 9-10=-40, 2-23=-20, 23-24=-65, 24-25=-20, 25-26=-65, 9-26=-20, 27-28=-45(F)

Horz: 1-2=-20, 2-5=-16, 6-9=6, 9-10=10

53) Reversal: Dead + 0.75 Roof Live (bal.) + 0.75 Uninhab. Attic Storage + 0.75(0.6 MWFRS Wind (Neg. Int) 2nd Parallel): Lumber Increase=1.60, Plate Increase=1.60 Uniform Loads (plf)

Vert: 1-2=-40, 2-5=-44, 5-6=-44, 6-9=-34, 9-10=-30, 2-23=-20, 23-24=-65, 24-25=-20, 25-26=-65, 9-26=-20, 27-28=-45(F)

Horz: 1-2=-10, 2-5=-6, 6-9=16, 9-10=20



3/1/2024



responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT SMOKY	PLACE LILLINGTON, NC
24-1487-R01	R19A	PIGGYBACK BASE	3	1		# 46062
		Run: 8	.430 s Feb 1	12 2021 Prin	Job Reference (optional) nt: 8.430 s Feb 12 2021 MiTek Industries, Inc. Mon	Mar 4 16:32:18 2024 Page 2
NOTES- (12)		ID:oDu	WOOMhLx	(MOj2fwcp	2aKqzMG6w-4cRu3XNCMmTna9AxWlxkG0	2QCvSt7Pp1alF5ihLzeFyR
10) Load case(s) 3, 4, 5,	19, 20, 21, 22, 23, 24, 25, 2	6, 27, 32, 33, 46, 47, 48, 49, 50, 51, 52, 53	has/have	been mo	dified. Building designer must review loa	ads to verify that they
11) In the LOAD CASE(S) section, loads applied to the	he face of the truss are noted as front (F) o	r back (B)			
LOAD CASE(S) Standard	1					
1) Dead + Snow (balance	d): Lumber Increase=1.15,	Plate Increase=1.15				
Vert: 1-8=-60, 8	8-9=-60, 9-12=-60, 2-21=-2	0, 18-20=-20, 12-17=-20				
3) Dead + 0.75 Roof Live Uniform Loads (plf)	(balanced) + 0.75 Uninhab	. Attic Storage: Lumber Increase=1.15, Pla	te Increas	e=1.15		
Vert: 1-8=-50, 8	8-9=-50, 9-12=-50, 2-21=-20	0, 18-20=-20, 17-27=-20, 27-28=-65, 12-28 ic Storage: Lumber Increase=1 15, Plate In	=-20, 29-3	30=-45(F)		
Uniform Loads (plf)						
5) Dead + Uninhabitable	8-9=-50, 9-12=-50, 2-21=-20 Attic Without Storage: Lum	0, 18-20=-20, 17-27=-20, 27-28=-65, 12-28 per Increase=1.25, Plate Increase=1.25	=-20, 29-3	30=-45(F)		
Uniform Loads (plf) Vert: 1-8=-20	8-9=-20 9-12=-20 2-21=-4	0 18-20=-40 12-17=-40 29-30=-60(F)				
19) Dead + Uninhabitable	Attic Storage: Lumber Incr	ease=1.25, Plate Increase=1.25				
Vert: 1-8=-20	, 8-9=-20, 9-12=-20, 2-21=-2	20, 18-20=-20, 17-27=-20, 27-28=-80, 12-2	8=-20, 29	-30=-60(F	-)	
20) Dead + 0.75 Snow (b Uniform Loads (plf)	al.) + 0.75 Uninhab. Attic St	corage + 0.75(0.6 MWFRS Wind (Neg. Int)	Left): Lum	ber Increa	ase=1.60, Plate Increase=1.60	
Vert: 1-2=-55	, 2-8=-58, 8-9=-34, 9-12=-4	4, 2-21=-20, 18-20=-20, 17-27=-20, 27-28=	-65, 12-28	3=-20, 29-	-30=-45(F)	
Drag: 8-9=0	2-0-0, 9-12-0					
21) Dead + 0.75 Snow (b Uniform Loads (plf)	al.) + 0.75 Uninhab. Attic St	orage + 0.75(0.6 MWFRS Wind (Neg. Int)	Right): Lui	mber Incr	ease=1.60, Plate Increase=1.60	
Vert: 1-2=-40, Horz: 1-2=-10	, 2-8=-44, 8-9=-34, 9-12=-56 2-8=-6 9-12=-8	8, 2-21=-20, 18-20=-20, 17-27=-20, 27-28=	-65, 12-28	3=-20, 29-	-30=-45(F)	
Drag: 8-9=0	-1) + 0.75 Huishah Attis 01		4 - 4 D II -	N. I		
Uniform Loads (plf)	al.) + 0.75 Uninnab. Attic St	orage + 0.75(0.6 MWFRS Wind (Neg. Int)	ist Paralle	ei): Lumbe	Fincrease=1.60, Plate Increase=1.60	
Vert: 1-2=-30, Horz: 1-2=-20	, 2-8=-34, 8-9=-44, 9-12=-4), 2-8=-16, 9-12=6	4, 2-21=-20, 18-20=-20, 17-27=-20, 27-28=	-65, 12-28	3=-20, 29-	·30=-45(F)	
Drag: 8-9=0	al) + 0.75 Uninhab Attia St	erose + 0.75/0.6 MW/ERS Wind (Neg. Int)	and Doroll		per Increase 1.60 Plate Increase 1.60	
Uniform Loads (plf)				ei). Luinb	el inclease 1.00, Flate inclease 1.00	
Vert: 1-2=-40, Horz: 1-2=-10	, 2-8=-44, 8-9=-44, 9-12=-34), 2-8=-6, 9-12=16	4, 2-21=-20, 18-20=-20, 17-27=-20, 27-28=	-65, 12-28	3=-20, 29-	·30=-45(F)	
Drag: 8-9=0 24) Dead + 0 75 Roof Liv	e (bal.) + 0.75 Uninbab. Atti	c Storage + 0.75(0.6 MWERS Wind (Neg	Int) eft):	l umber In	crease=1.60 Plate Increase=1.60	
Uniform Loads (plf)						
Horz: 1-2=-55, 2	, 2-8=-58, 8-9=-34, 9-12=-4 2-8=8, 9-12=6	4, 2-21=-20, 18-20=-20, 17-27=-20, 27-28=	-05, 12-28	3=-20, 29-	-30=-45(F)	
Drag: 8-9=0 25) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg.	Int) Riaht)	: Lumber	Increase=1.60. Plate Increase=1.60	
Uniform Loads (plf)	2 8- 11 8 0- 31 0 12- 5	8 2 21- 20 18 20- 20 17 27- 20 27 28-	65 12 29	2-20.20	30- 45(E)	
Horz: 1-2=-10), 2-8=-6, 9-12=-8	5, 2-2120, 10-2020, 11-2120, 21-20-	-00, 12-20	520, 25-		
Drag: 8-9=0 26) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg.	Int) 1st Pa	arallel): Lu	ımber Increase=1.60,	
Plate Increase=1.60						
Vert: 1-2=-30	, 2-8=-34, 8-9=-44, 9-12=-4	4, 2-21=-20, 18-20=-20, 17-27=-20, 27-28=	-65, 12-28	3=-20, 29-	-30=-45(F)	
Drag: 8-9=0	9, 2-8=-16, 9-12=6					
27) Dead + 0.75 Roof Liv Plate Increase=1.60	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg.	Int) 2nd Pa	arallel): Li	umber Increase=1.60,	
Uniform Loads (plf)	2-8=-11 8-9=-11 9-12=-3	4 2-21=-20 18-20=-20 17-27=-20 27-28=	-65 12-28	8=_20 20_	-30=-45(E)	
Horz: 1-2=-10), 2-8=-6, 9-12=16	+, 2-2120, 10-2020, 11-2120, 21-20-	-00, 12-20)20, 23-		
Drag: 8-9=0 32) 3rd Dead + 0.75 Roof	Live (unbalanced) + 0.75 L	Jninhab. Attic Storage: Lumber Increase=1	.15, Plate	Increase=	=1.15	
Uniform Loads (plf) Vert: 1-8=-50	. 8-9=-50. 9-12=-20. 2-21=-:	20. 18-20=-20. 17-27=-20. 27-28=-65. 12-2	8=-20, 29	-30=-45(F	=)	
33) 4th Dead + 0.75 Roof	Live (unbalanced) + 0.75 L	Jninhab. Attic Storage: Lumber Increase=1	.15, Plate	Increase=	=1.15	1141111
Vert: 1-8=-20	8-9=-50, 9-12=-50, 2-21=-2	20, 18-20=-20, 17-27=-20, 27-28=-65, 12-2	8=-20, 29	-30=-45(F) FESSIA	KING
46) Reversal: Dead + 0.7 Plate Increase=1.60	5 Snow (bal.) + 0.75 Uninha	ab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int)	Left): Lun	nber Increase=1.60,	A DE LA DE L
Uniform Loads (plf) Vert [.] 1-2=-55	2-8=-58 8-9=-34 9-12=-4	4. 2-21=-20. 18-20=-20 17-27=-20 27-28=	-65, 12-28	3=-20 29-	-30=-45(F)	
Horz: 1-2=5, 2	2-8=8, 9-12=6	.,	, i <i>z</i> -20	,	28147	/ <u>Ē</u>
Drag: ۵-9=0 47) Reversal: Dead + 0.7	5 Snow (bal.) + 0.75 Uninha	ab. Attic Storage + 0.75(0.6 MWFRS Wind	(Neg. Int)	Right): Lu	umber Increase=1.60	R. M
Plate Increase=1.60					THARK Y M	ORAL
					141111111111	aller

Continuing by Sacisfy Saesign parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

3/1/2024

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT	SMOKY PLACE LILLINGTON, NC
24-1487-R01	R19A	PIGGYBACK BASE	3		Job Reference (optional)	# 46062
	L		Run: 8.430 s Feb 1 ID:oDuWOOMhLx	12 2021 Prii MOj2fwcp	nt: 8.430 s Feb 12 2021 MiTek Industries, Ir D2aKqzMG6w-4cRu3XNCMmTna9Ax	nc. Mon Mar 4 16:32:18 2024 Page 3 WIxkGQQCvSt7Pp1alF5ihLzeFyF
LOAD CASE(S)	Standard					
Uniform Load Vert	s (plf) 1-2=-40_2-8=-44_8-9=-34	L 9-12=-58 2-21=-20 18-20=-20 17-27=	-20 27-28=-65 12-28	3=-20 29	-30=-45(F)	
Horz:	1-2=-10, 2-8=-6, 9-12=-8	., 0 12 00, 2 21 20, 10 20 20, 11 21	20, 21 20 00, 12 20	20,20		
Drag:	8-9=0	75 Lininghach Attin Changers 1 0 75/0 C MMA				In and a sec = 1, CO
48) Reversal: Dea	ad + 0.75 Snow (bal.) + 0. s (blf)	75 Uninnab. Allic Storage + 0.75(0.6 MW	FRS Wind (Neg. Int)	ist Para	lier): Lumber Increase=1.60, Plate	Increase=1.60
Vert:	1-2=-30, 2-8=-34, 8-9=-44	l, 9-12=-44, 2-21=-20, 18-20=-20, 17-27=	-20, 27-28=-65, 12-28	3=-20, 29	-30=-45(F)	
Horz:	1-2=-20, 2-8=-16, 9-12=6	5				
19) Reversal: De	8-9=0 ad + 0.75 Snow (bal.) + 0.	75 Uninhab Attic Storage + 0 75(0.6 MM	(FRS Wind (Neg. Int)	2nd Para	allel): Lumber Increase=1.60. Plate	hcrease=1.60
Uniform Load	s (plf)	10 Ommas. Alle Glorage + 0.75(0.0 MM	into wind (Neg. int)			
Vert:	1-2=-40, 2-8=-44, 8-9=-44	l, 9-12=-34, 2-21=-20, 18-20=-20, 17-27=	-20, 27-28=-65, 12-28	3=-20, 29	-30=-45(F)	
Horz:	1-2=-10, 2-8=-6, 9-12=16					
50) Reversal: Dea	ad + 0.75 Roof Live (bal.)	+ 0.75 Uninhab. Attic Storage + 0.75(0.6	MWFRS Wind (Neg.	Int) Left):	: Lumber Increase=1.60. Plate Inc	rease=1.60
Uniform Load	s (plf)	5 - (11	()	, ,	,	
Vert:	1-2=-55, 2-8=-58, 8-9=-34	4, 9-12=-44, 2-21=-20, 18-20=-20, 17-27=	-20, 27-28=-65, 12-28	3=-20, 29	-30=-45(F)	
Drag	1-2=5, 2-8=8, 9-12=6 8-9=0					
51) Reversal: Dea	ad + 0.75 Roof Live (bal.)	+ 0.75 Uninhab. Attic Storage + 0.75(0.6	MWFRS Wind (Neg.	Int) Right	t): Lumber Increase=1.60, Plate In	crease=1.60
Uniform Load	s (plf)		00 07 00 05 40 00		00 45(5)	
Vert: Horz	1-2=-40, 2-8=-44, 8-9=-34 1-2=-10 2-8=-6 9-12=-8	4, 9-12=-58, 2-21=-20, 18-20=-20, 17-27=	-20, 27-28=-65, 12-28	3=-20, 29	-30=-45(F)	
Drag:	8-9=0					
52) Reversal: Dea	ad + 0.75 Roof Live (bal.)	+ 0.75 Uninhab. Attic Storage + 0.75(0.6	MWFRS Wind (Neg.	Int) 1st P	Parallel): Lumber Increase=1.60, P	late Increase=1.60
Uniform Load	s (plt) 1 2- 30 2 8- 34 8 0- 4/		20 27 28- 65 12 28	8-20.20	30- 45(E)	
Horz:	1-2=-20, 2-8=-16, 9-12=6	;, 9 -1244, 2-2120, 10-2020, 1 <i>1-21-</i>	-20, 27-2005, 12-20	520, 29	-3043(1)	
Drag:	8-9=0					
53) Reversal: Dea	ad + 0.75 Roof Live (bal.)	+ 0.75 Uninhab. Attic Storage + 0.75(0.6	MWFRS Wind (Neg.	Int) 2nd I	Parallel): Lumber Increase=1.60, F	Plate Increase=1.60
Uniform Load Vert	s (pii) 1-2=-40, 2-8=-44, 8-9=-44	l. 9-12=-34, 2-21=-20, 18-20=-20, 17-27=	-20. 27-28=-65. 12-28	3=-20, 29	-30=-45(F)	
Horz:	1-2=-10, 2-8=-6, 9-12=16))	20, 2. 20 00, 12 20			

Drag: 8-9=0



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vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 G	REAT SMOKY PLACE	E LILLINGTON, NC
24-1487-R01	R19B	PIGGYBACK BASE	1	1		#	46062
		Run: 8.4 ID:oDuWC	30 s Feb 12 OMhLxMC	 2 2021 Print)i2fwcp2al	Job Reference (optional) : 8.430 s Feb 12 2021 MiTek Indus (gzMG6w-UB71hZP4ehrMRdu	tries, Inc. Mon Mar 4	16:32:21 2024 Page 2 cA01RCJNHgzeFvO
NOTES- (11)					fad Daildian daalaa aa aa	•	
9) Load case(s) 3, 4, 5, 1 correct for the intended	9, 20, 21, 22, 23, 24, 25, 26 I use of this truss.	, 27, 32, 33, 46, 47, 48, 49, 50, 51, 52, 53 na	as/nave b	een moai	lied. Building designer musi	I review loads to v	erity that they are
10) In the LOAD CASE(S) section, loads applied to tl	ne face of the truss are noted as front (F) or	back (B).				
LOAD CASE(S) Standard	I						
1) Dead + Snow (balance	d): Lumber Increase=1.15,	Plate Increase=1.15					
Vert: 1-8=-60, 8	8-9=-60, 9-13=-60, 2-22=-20	0, 19-21=-20, 12-18=-20					
3) Dead + 0.75 Roof Live Uniform Loads (plf)	(balanced) + 0.75 Uninhab	. Attic Storage: Lumber Increase=1.15, Plate	Increase	=1.15			
Vert: 1-8=-50, 8 4) Dead + 0.75 Snow (ba	8-9=-50, 9-13=-50, 2-22=-20 anced) + 0.75 Uninhab. Att	0, 19-21=-20, 18-28=-20, 28-29=-65, 12-29= ic Storage: Lumber Increase=1.15, Plate Inc	-20, 30-3 [.] rease=1. [.]	1=-45(F) 15			
Uniform Loads (plf)	3-9=-50 9-13=-50 2-22=-20	19-21=-20 18-28=-20 28-29=-65 12-29=	-20 30-3	1=-45(F)			
5) Dead + Uninhabitable	Attic Without Storage: Lumb	ber Increase=1.25, Plate Increase=1.25	20,000				
Vert: 1-8=-20, 8	3-9=-20, 9-13=-20, 2-22=-40	D, 19-21=-40, 12-18=-40, 30-31=-60(F)					
19) Dead + Uninhabitable Uniform Loads (plf)	Attic Storage: Lumber Incr	ease=1.25, Plate Increase=1.25					
Vert: 1-8=-20, 20) Dead + 0.75 Spow (b)	8-9=-20, 9-13=-20, 2-22=-2	20, 19-21=-20, 18-28=-20, 28-29=-80, 12-29	=-20, 30-3	31=-60(F)	se=1.60. Plate Increase=1.6	60	
Uniform Loads (plf)					20 - 20, 20, 24 - 45(5)	50	
Horz: 1-2=5, 2	2-8=8, 9-12=6, 12-13=10	+, 12-1340, 2-2220, 19-2120, 16-262	20, 20-29-	00, 12-2	29–-20, 30-31–-45(F)		
Drag: 8-9=0 21) Dead + 0.75 Snow (b	al.) + 0.75 Uninhab. Attic St	orage + 0.75(0.6 MWFRS Wind (Neg. Int) R	ight): Lun	ber Incre	ase=1.60, Plate Increase=1	1.60	
Uniform Loads (plf) Vert: 1-2=-40	2-8=-44, 8-9=-34, 9-12=-58	8, 12-13=-55, 2-22=-20, 19-21=-20, 18-28=-2	20, 28-29:	=-65, 12-2	29=-20, 30-31=-45(F)		
Horz: 1-2=-10 Drag: 8-9=0	, 2-8=-6, 9-12=-8, 12-13=-5						
22) Dead + 0.75 Snow (b	al.) + 0.75 Uninhab. Attic St	orage + 0.75(0.6 MWFRS Wind (Neg. Int) 1	st Parallel): Lumbe	r Increase=1.60, Plate Incre	ase=1.60	
Vert: 1-2=-30	2-8=-34, 8-9=-44, 9-12=-44	4, 12-13=-40, 2-22=-20, 19-21=-20, 18-28=-2	20, 28-29:	=-65, 12-2	29=-20, 30-31=-45(F)		
Horz: 1-2=-20 Drag: 8-9=0	, 2-8=-16, 9-12=6, 12-13=1	0					
23) Dead + 0.75 Snow (b Uniform Loads (plf)	al.) + 0.75 Uninhab. Attic St	orage + 0.75(0.6 MWFRS Wind (Neg. Int) 2	nd Paralle	el): Lumbe	er Increase=1.60, Plate Incre	ease=1.60	
Vert: 1-2=-40, Horz: 1-2=-10 Drag: 8,9=0	2-8=-44, 8-9=-44, 9-12=-34 , 2-8=-6, 9-12=16, 12-13=2	4, 12-13=-30, 2-22=-20, 19-21=-20, 18-28=-2 0	20, 28-29=	=-65, 12-2	29=-20, 30-31=-45(F)		
24) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg. Ir	it) Left): L	umber In	crease=1.60, Plate Increase	e=1.60	
Vert: 1-2=-55	2-8=-58, 8-9=-34, 9-12=-44	4, 12-13=-40, 2-22=-20, 19-21=-20, 18-28=-2	20, 28-29	=-65, 12-2	29=-20, 30-31=-45(F)		
Drag: 8-9=0	-6-6, 9-12-0, 12-13-10	- Changes I & ZE/O & MM/EDO M/rd (New In	4) Dischat).				
25) Dead + 0.75 Roof Liv Uniform Loads (plf)	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg. Ir	it) Right):	Lumber I	ncrease=1.60, Plate Increas	se=1.60	
Vert: 1-2=-40, Horz: 1-2=-10	, 2-8=-44, 8-9=-34, 9-12=-58), 2-8=-6, 9-12=-8, 12-13=-5	3, 12-13=-55, 2-22=-20, 19-21=-20, 18-28=-2	20, 28-29=	=-65, 12-2	29=-20, 30-31=-45(F)		
Drag: 8-9=0 26) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg. Ir	it) 1st Par	allel): Lur	nber Increase=1.60,		
Plate Increase=1.60			,	,			
Vert: 1-2=-30, Horz: 1-2=-20	2-8=-34, 8-9=-44, 9-12=-44, 2-8=-16, 9-12=6, 12-13=1	4, 12-13=-40, 2-22=-20, 19-21=-20, 18-28=-2 0	20, 28-29=	-65, 12-2	29=-20, 30-31=-45(F)		
Drag: 8-9=0 27) Dead + 0.75 Roof Liv	e (bal.) + 0.75 Uninhab. Atti	c Storage + 0.75(0.6 MWFRS Wind (Neg. Ir	it) 2nd Pa	rallel): Lu	mber Increase=1.60,		
Plate Increase=1.60 Uniform Loads (plf)							
Vert: 1-2=-40 Horz: 1-2=-10	2-8=-44, 8-9=-44, 9-12=-34 , 2-8=-6, 9-12=16, 12-13=2	4, 12-13=-30, 2-22=-20, 19-21=-20, 18-28=-2 0	20, 28-29=	=-65, 12-2	29=-20, 30-31=-45(F)		
Drag: 8-9=0 32) 3rd Dead + 0.75 Roof	Live (unbalanced) + 0.75 L	Jninhab. Attic Storage: Lumber Increase=1.1	5, Plate I	ncrease=	1.15		
Vert: 1-8=-50	8-9=-50, 9-13=-20, 2-22=-2	20, 19-21=-20, 18-28=-20, 28-29=-65, 12-29	=-20, 30-3	31=-45(F)			
Uniform Loads (plf)	Live (unbalanced) + 0.75 C	ininnab. Allic Storage: Lumber increase= 1.1	o, Plate II	ncrease=	1.15	ATH CARO	1111
Vert: 1-8=-20 46) Reversal: Dead + 0.7	8-9=-50, 9-13=-50, 2-22=-2 5 Snow (bal.) + 0.75 Uninha	20, 19-21=-20, 18-28=-20, 28-29=-65, 12-29 ab. Attic Storage + 0.75(0.6 MWFRS Wind (I	=-20, 30-3 Neg. Int) L	31=-45(F) .eft): Lum	ber Increase=1.60,	ROFESSION	Aller
Plate Increase=1.60 Uniform Loads (plf)					THE STATE	SEAL	
Vert: 1-2=-55 Horz: 1-2=5	2-8=-58, 8-9=-34, 9-12=-4 2-8=8, 9-12=6, 12-13=10	4, 12-13=-40, 2-22=-20, 19-21=-20, 18-28=-2	20, 28-29=	=-65, 12-2	29=-20, 30-31=-45(F)	28147	11111
1012: + 2-0, 2 Drag: 8-9=0 47) Reversal: Dead ± 0.7	5 Snow(hal) + 0.75 Uninfra	ab Attic Storage + 0.75/0.6 MW/EPS Wind /	lea Int) E	Right). Lu	mber Increase=1.60	. Sa. a /	inner 1
Plate Increase=1.60		ab. Allo oloraye + 0.73(0.0 MMTAS MINU (1	veg. III.) r	igni). Lu	TIDET ITICIE ase - 1.00, The	RECORD	annus .
					(11)	Manager Multim	

3/1/2024

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND	95 GREAT SMOKY PLACE LILLINGTON, NC
24-1487-R01	R19B	PIGGYBACK BASE	1	1	Job Reference (optional)	# 46062
		Rui ID:oDi	n: 8.430 s Feb 1: IWOOMhLxMC	2 2021 Print: Dj2fwcp2ak	8.430 s Feb 12 2021 MiTek I (qzMG6w-UB71hZP4ehr)	ndustries, Inc. Mon Mar 4 16:32:21 2024 Pag MRduWBuURu22jJgwHcA01RCJNHgzeF
LOAD CASE(S) Standard	i					
Vert: 1-2=-40	2-8=-44, 8-9=-34, 9-12=-5	8, 12-13=-55, 2-22=-20, 19-21=-20, 18-2	8=-20, 28-29	=-65, 12-2	9=-20, 30-31=-45(F)	
Horz: 1-2=-10 Drag: 8-9=0	9, 2-8=-6, 9-12=-8, 12-13=-5					
48) Reversal: Dead + 0.7	5 Snow (bal.) + 0.75 Uninha	ab. Attic Storage + 0.75(0.6 MWFRS Wir	nd (Neg. Int)	Ist Paralle	el): Lumber Increase=1.	60, Plate Increase=1.60
Uniform Loads (plf)	. ,		,			
Vert: 1-2=-30	, 2-8=-34, 8-9=-44, 9-12=-4	4, 12-13=-40, 2-22=-20, 19-21=-20, 18-2 0	8=-20, 28-29	=-65, 12-2	29=-20, 30-31=-45(F)	
Drag: 8-9=0	, z-o10, 9-1z-0, 1z-13-1	0				
49) Reversal: Dead + 0.7	5 Snow (bal.) + 0.75 Uninha	ab. Attic Storage + 0.75(0.6 MWFRS Wir	nd (Neg. Int) 2	2nd Parall	el): Lumber Increase=1	.60, Plate Increase=1.60
Uniform Loads (plf)		4 40 40 00 0 00 00 40 04 00 40 0		05 40 6	0 00 00 04 45(5)	
Vert: 1-2=-40, Horz: 1-2=-10	, 2-8=-44, 8-9=-44, 9-12=-3) 2-8=-6 9-12=16 12-13=2	4, 12-13=-30, 2-22=-20, 19-21=-20, 18-2 0	8=-20, 28-29	=-65, 12-2	29=-20, 30-31=-45(F)	
Drag: 8-9=0	, 2 0 - 0, 5 12 10, 12 10 - 2	0				
50) Reversal: Dead + 0.7	5 Roof Live (bal.) + 0.75 Ur	inhab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. I	nt) Left): l	umber Increase=1.60,	Plate Increase=1.60
Uniform Loads (plf)	2 9 - 59 9 0 - 24 0 12 - 4	4 12 12- 40 2 22- 20 10 21- 20 18 2	o- 20 20 20	- 65 10 0	0- 20 20 21- 45(E)	
Horz: 1-2=5.2	2-8=8. 9-12=6. 12-13=10	4, 12-1340, 2-2220, 19-2120, 16-2	020, 20-29	05, 12-2	2920, 30-3143(F)	
Drag: 8-9=0	,,					
51) Reversal: Dead + 0.7	5 Roof Live (bal.) + 0.75 Ur	inhab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. I	nt) Right):	Lumber Increase=1.60), Plate Increase=1.60
Uniform Loads (pif) Vert: 1-2=-40	2-8=-44 8-9=-34 9-12=-5	8 12-13=-55 2-22=-20 19-21=-20 18-2	8=-20 28-29	=-65 12-2	9=-20 30-31=-45(F)	
Horz: 1-2=-10	, 2-8=-6, 9-12=-8, 12-13=-5	i	0-20,2020	00, 12 2	20, 00 01 - 40(1)	
Drag: 8-9=0						
52) Reversal: Dead + 0.7	5 Roof Live (bal.) + 0.75 Ur	inhab. Attic Storage + 0.75(0.6 MWFRS	Wind (Neg. I	nt) 1st Pa	rallel): Lumber Increase	∋=1.60, Plate Increase=1.60
Vert: 1-2=-30.	2-8=-34, 8-9=-44, 9-12=-4	4. 12-13=-40. 2-22=-20. 19-21=-20. 18-2	8=-20, 28-29	=-65. 12-2	29=-20, 30-31=-45(F)	
Horz: 1-2=-20	, 2-8=-16, 9-12=6, 12-13=1	0	,			
Drag: 8-9=0						4.00 Plate la marca 4.00
Juliform Loads (plf)	5 ROOI LIVE (Dal.) + 0.75 Ur	innab. Auic Storage + 0.75(0.6 MWFRS	wina (Neg. I	ni) 2na Pa	araliei): Lumber Increas	
Vert: 1-2=-40.	, 2-8=-44, 8-9=-44, 9-12=-3	4, 12-13=-30, 2-22=-20, 19-21=-20, 18-2	8=-20, 28-29	=-65, 12-2	29=-20, 30-31=-45(F)	
Horz: 1-2=-10	, 2-8=-6, 9-12=16, 12-13=2	0	-,	,	,	

Drag: 8-9=0



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Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT SM	OKY PLACE LILLINGTON, NC
24-1487-R01	R21	GABLE	1	1	Job Reference (optional)	# 46062
		Run: 8	430 s Feb 1	2 2021 Print	t: 8.430 s Feb 12 2021 MiTek Industries, Inc.	Mon Mar 4 16:32:24 2024 Page 2

ID:r9MH5r89tuDnH9IxEtUtJwyLJIK-umo9KbRyxcExI4d5s028WhgJmt?8pYjT7AY1u?zeFyL 12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard



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of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT SMC	OKY PLACE LILLINGTON, NC
24-1487-R01	R22	Piggyback Base Structural Gable COMMON I I Gable I G	able	1	Job Reference (optional)	# 46062
		Run: 8.43 ID:oDuW	0 s Feb 12 /OOMhLx	2021 Print	t: 8.430 s Feb 12 2021 MiTek Industries, Inc. 1 2aKqzMG6w-nX2q9yUT?rkNmixs5s64q	Mon Mar 4 16:32:28 2024 Page 2 Xqv1UHXITe32oWE1mzeFyH

NOTES- (12)

10) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 15 except (jt=lb) 2=111, 14=267, 12=188.

LOAD CASE(S) Standard



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responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.





vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0130 BLAKE POND 95 GREAT SM	NOKY PLACE LILLINGTON, NC
24-1487-R01	R26	Monopitch	5	1	Job Reference (optional)	# 46062
Run: 8 /30 s Feb 12 2021 Print: 8 /30 s Feb 12 2021 MiTek Indus:					8 430 c Eob 12 2021 MiTok Industrios Inc.	Mon Mar. 4 16:32:36 2024, Page 2

ID:r9MH5r89tuDnH9lxEtUtJwyLJIK-Y4XhrhbU6lkEkwYPZXFy?DANkj5ydzqEu2SfJlzeFy9

LOAD CASE(S) Standard



3/1/2024



NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-8-8, Exterior(2E) 6-8-8 to 11-6-2 zone; C-C for

members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=160, 6=160.

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LOAD CASE(S) Standard
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NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. WEBS 2-9=-316/237, 4-6=-316/237

NOTES- (8)

- NOTES- (8)1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 12-2-10, Exterior(2E) 12-2-10 to 17-0-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 9=210, 6=210.

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LOAD CASE(S) Standard
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- (8)
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 9-6-10, Exterior(2E) 9-6-10 to 14-4-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=176, 6=176.

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LOAD CASE(S) Standard
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NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-10-10, Exterior(2E) 6-10-10 to 11-8-3 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=159, 6=159.

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LOAD CASE(S) Standard
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NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3, 4.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





- (1) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
 9) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates the member must be braced.
 10) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
 11) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing.
 12) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDEM MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.
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Warning !--Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

3/1/2024



LOADING (psf) SPACING-GRIP DEFL. PLATES 2-0-0 CSI. l/defl L/d in (loc) TCLL (roof) 20.0 244/190 Plate Grip DOL 1.15 тс 0 4 1 Vert(LL) n/a n/a 999 MT20 Snow (Pf) 20.0 BC Lumber DOL 1.15 0.33 Vert(CT) n/a n/a 999 TCDL 10.0 Rep Stress Incr YES WB 0.00 Horz(CT) 0.00 n/a n/a BCLL 0.0 Code IRC2021/TPI2014 Weight: 16 lb FT = 20% Matrix-P BCDL 10.0 LUMBER-BRACING-TOP CHORD 2x4 SP No.2 TOP CHORD Structural wood sheathing directly applied or 5-7-4 oc purlins, except BOT CHORD 2x4 SP No.3 end verticals. WFBS 2x4 SP No.3 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide

REACTIONS. (lb/size) 1=162/5-7-4 (min. 0-1-8), 3=162/5-7-4 (min. 0-1-8) Max Horz 1=24(LC 10) Max Uplift1=-14(LC 10), 3=-19(LC 10) Max Grav 1=201(LC 20), 3=201(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; Gable Roof; End Jack Truss; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.
- 8) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced.
- 9) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.
- 10) Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling,
- he member must be braced. Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated. Web bracing shown is for lateral support of individual web members only. Refer to BCSI Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. ISEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS. AD CASE(S) Standard 11) SEE BČŠI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR ŘECOMMENDED

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3/1/2024