07/13/2022

DRAWN BY: CHECKED BY:

Yosemite 2-Car

ELEVATION A

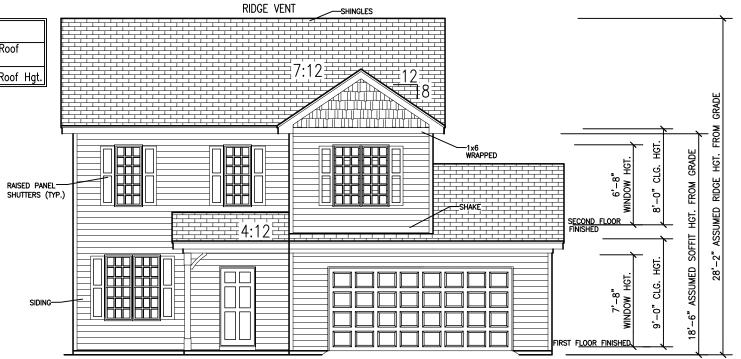
OF 9

ATTIC SPACE VENTILATION

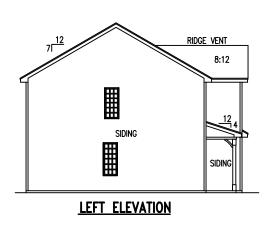
1016 SQ. FT. OF CLG. / 150 = $\frac{6.77}{}$ SQ. FT. REQUIRED

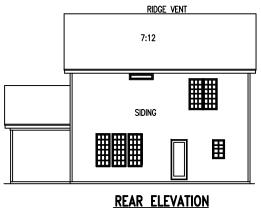
REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

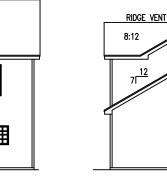
MEAN ROOF HGT.				
Soffit Hgt. From Assumed Grade	+ Highest Ridge Hgt. From Assumed Grade	÷2		= Mean Roof Hgt.
18'-6"	+ 28'-2"	÷2	= 23'-4"	Mean Roof Hg



FRONT ELEVATION "A"







SIDING

RIGHT ELEVATION

DRAWN BY: CHECKED BY:

Yosemite 2-Car

ELEVATION B

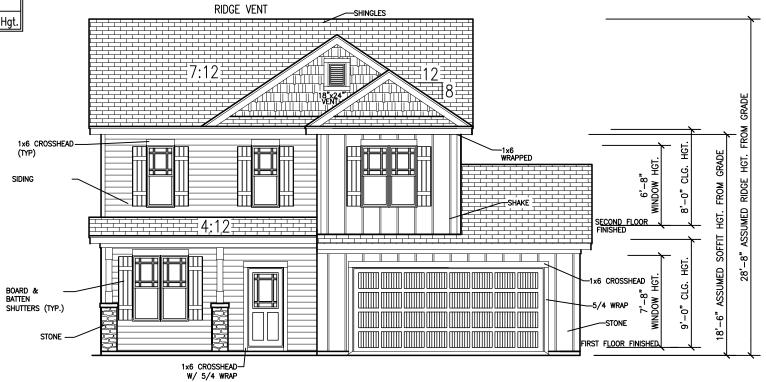
OF 9

ATTIC SPACE VENTILATION

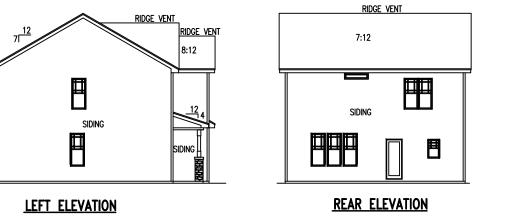
REQUIRED 1062 SQ. FT. OF CLG. / 150 = 7.08 SQ. FT. REQUIRED

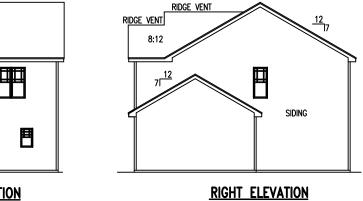
REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

MEAN ROOF HGT.				
Soffit Hgt. From Assumed Grade	+ Highest Ridge Hgt. From Assumed Grade	÷2		= Mean Roof Hgt.
18'-6"	+ 28'-2"	÷2	= 23'-4"	Mean Roof Hgt.



FRONT ELEVATION "B"



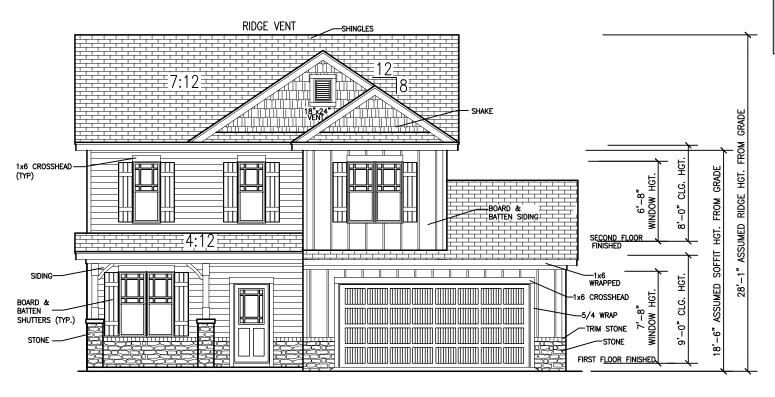


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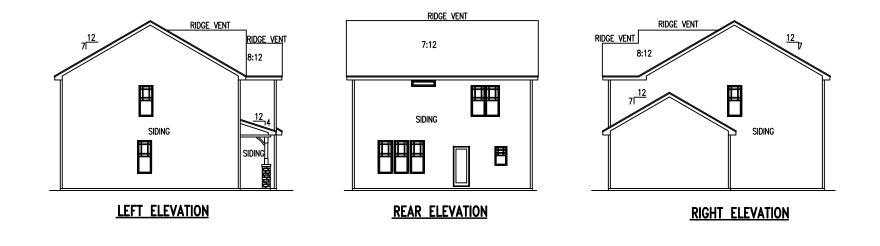
Yosemite 2-Car

ELEVATION C

 A_3



FRONT ELEVATION "C"



ATTIC SPACE VENTILATION

REQUIRED

1062 SQ. FT. 0F CLG. / 150 = 7.08 SQ. FT. REQUIRED

REFER TO SECTION R806 (ROOF VENTILATION) IN NORTH CAROLINA STATE 2018 INTERNATIONAL RESIDENTIAL BUILDING CODES.

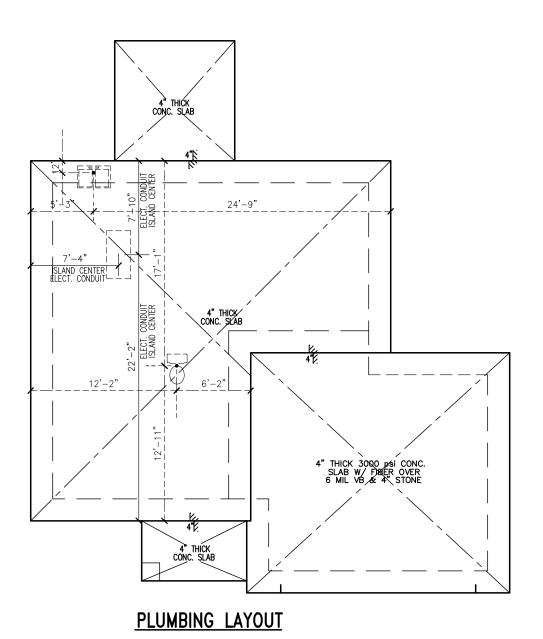
MEAN ROO	F HGT.				
Soffit Hgt. From + Highest Ridge Hgt. Assumed Grade From Assumed Grade	÷2		= Mean Hgt.	Roof	
18'-6" + 28'-2"	÷2	= 23'-4"	Mean	Roof	Hgt.

DRAWN BY:

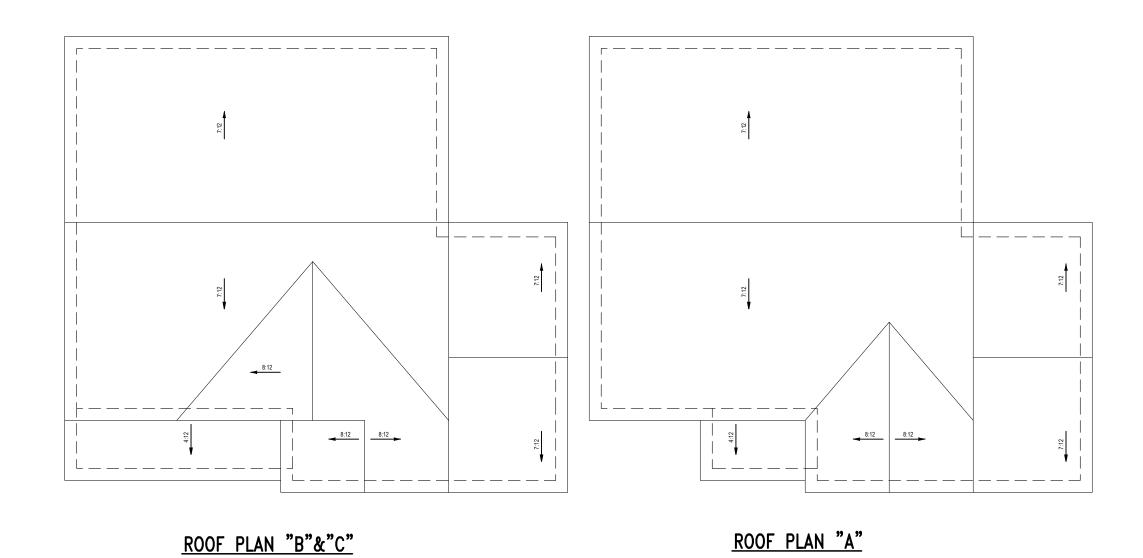
PLUMBING

CHECKED BY:

 $A_{\text{OF 9}}$



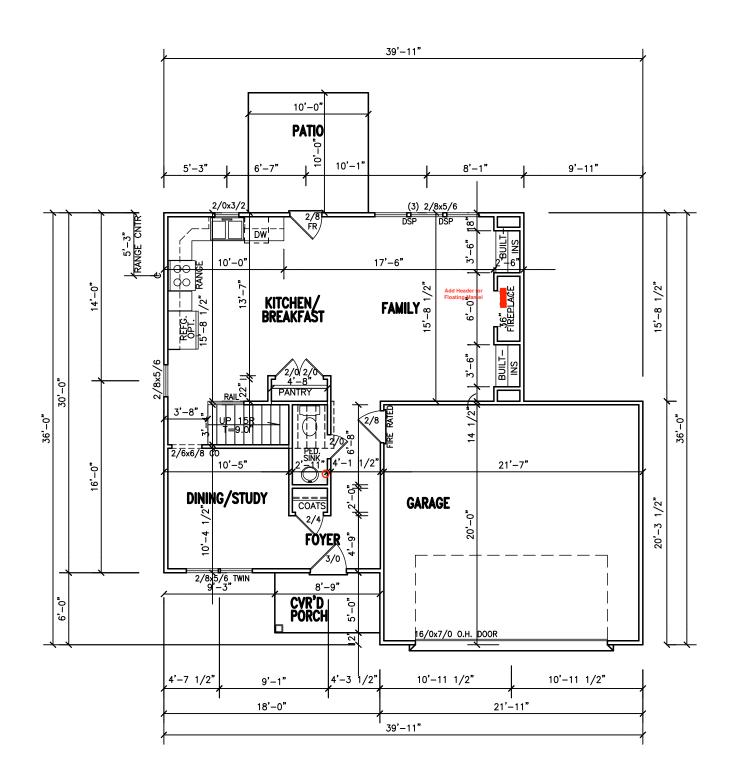
ROOF PLAN

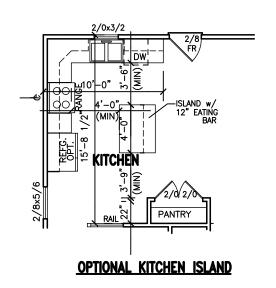


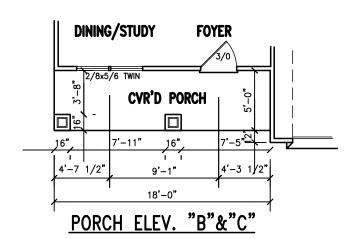
DRAWN BY:
CHECKED BY:

FLOOR PLAN









	SQUARE FOOTAGE	
	1ST FLOOR HTD. 2ND FLOOR HTD.	737 942
	FLOOR HTD.	1679
	GARAGE PATIO ELV "A" PORCH ELV "B" PORCH ELV "C" PORCH	437 100 44 90 90
ľ		
	1ST FLOOR PLAN NOTE	S.

1ST FLOOR PLAN NOTES:	
1) 9'-0" CLG. HGT. (9'-1 1/2" PLT. HGT.) UNLESS OTHERWISE NOTED.	
ALL EXTERIOR WALLS FIGURED AT 4"NOMIN WIDTHS UNLESS OTHERWISE NOTED.	AL
3) ALL INTERIOR WALLS FIGURED AT 3 1/2" WIDTHS UNLESS OTHERWISE NOTED.	
4) SET WINDOWS AT 7'-8" A.F.F. UNLESS OTHERWISE NOTED.	
5) DIMENSIONS ARE TO FRAMING UNLESS OTHERWISE NOTED.	

FIRST FLOOR PLAN

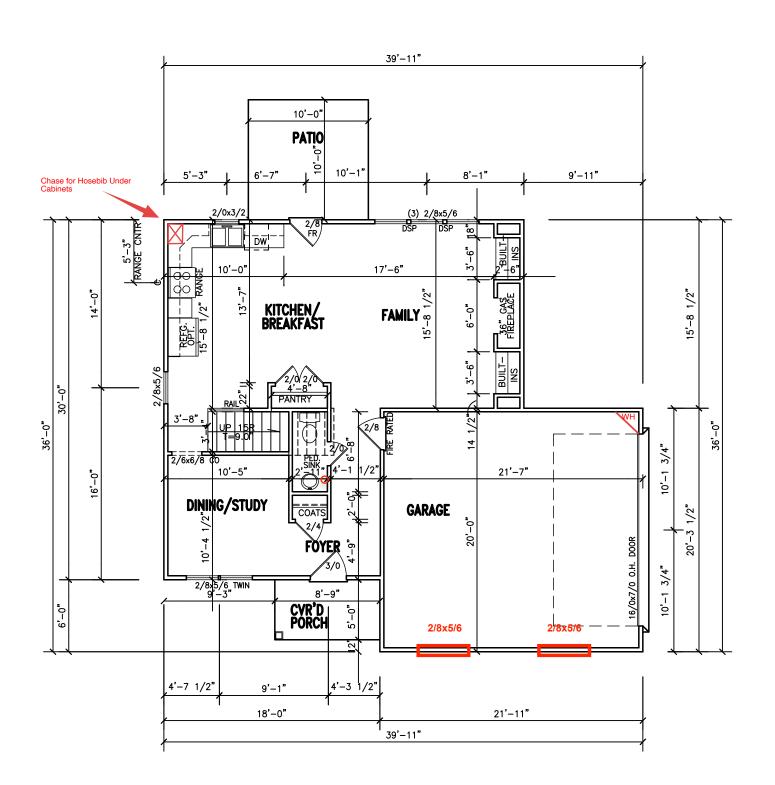


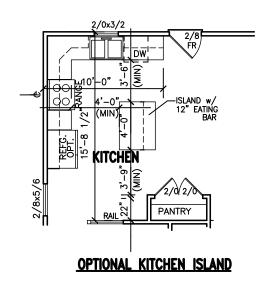


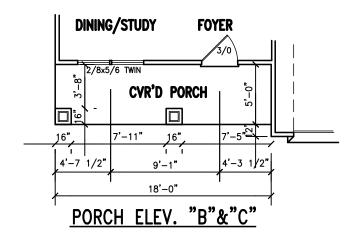
DRAWN BY:
CHECKED BY:

FLOOR PLAN









SQUARE FOOTAG	E
1ST FLOOR HTD. 2ND FLOOR HTD.	737 942
FLOOR HTD.	1679
GARAGE PATIO ELV "A" PORCH ELV "B" PORCH ELV "C" PORCH	437 100 44 90 90
1ST FLOOR PLAN NO	OTES:
9'-0' CLG. HGT. (9'-1 1/2" P UNILESS OTHERWISE NOTED. ALL EXTEROR WALLS FOURDE AT WIDTHS UNLESS OTHERWISE NOTE 3 ALL INTERIOR WALLS FICURED AT WIDTHS UNLESS OTHERWISE NOTE 4 SET WINDOWS AT 7'-8" A.F.F. UNLESS OTHERWISE NOTED. 5) DIMENSIONS ARE TO FRAMING UNLESS OTHERWISE NOTED.	4"NOMINAL D. 3 1/2"

FIRST FLOOR PLAN

SIDE LOAD GARAGE OPTION

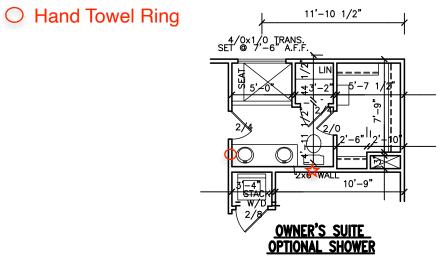


DRAWN BY: CHECKED BY:

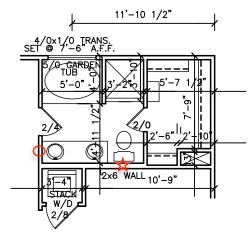
FLOOR PLAN

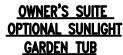
OF 9

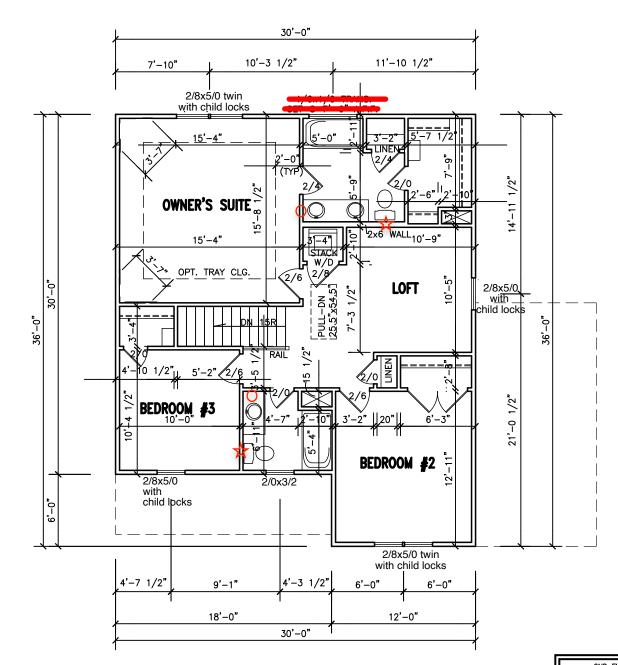
11'-10 1/2" 4/0x1/0 TRANS. SET @ 7'-6" A.F.F. 12x6 WALL10'-9" OWNER'S SUITE
OPT. GARDEN TUB/SHOWER 11'-10 1/2"



☆ Towel Bar







2ND FLOOR PLAN NOTES:

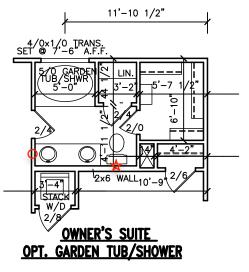
1) 8'-0" CLG. HGT. (8'-1 1/2" PLT. HGT.)
UNLESS OTHERWISE NOTED.
2) ALL EXTERIOR WALLS FIGURED AT 4"NOMINAL
WIDTHS UNLESS OTHERWISE NOTED.
3) ALL INTERIOR WALLS FIGURED AT 3 1/2"
WIDTHS UNLESS OTHERWISE NOTED.
4) SET WINDOWS AT 6'-8" A.F.F.
UNLESS OTHERWISE NOTED.
5) DIMENSIONS ARE TO FRAMING
UNLESS OTHERWISE NOTED.

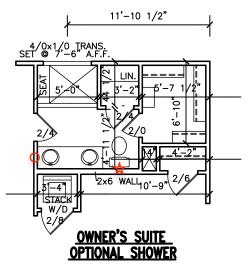
SECOND FLOOR PLAN

DRAWN BY: CHECKED BY:

FLOOR PLAN

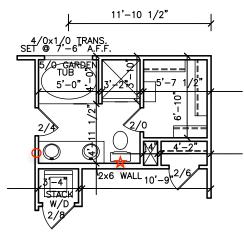




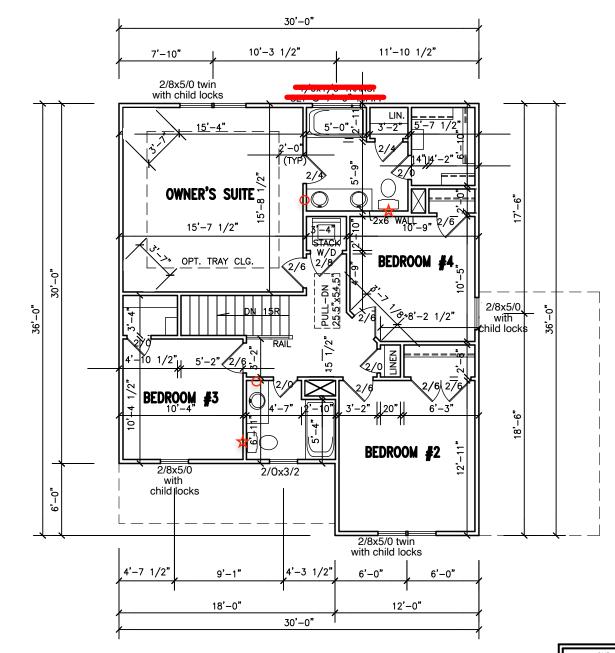


☆ Towel Bar

O Hand Towel Ring



OWNER'S SUITE
OPTIONAL SUNLIGHT **GARDEN TUB**



2ND FLOOR PLAN NOTES:

1) 8'-0" CLG. HGT. (8'-1 1/2" PLT. HGT.)
UNLESS OTHERWISE NOTED.
2) ALL EXTERIOR WALLS FIGURED AT 4"NOMINAL
WIDTHS UNLESS OTHERWISE NOTED.
3) ALL INTERIOR WALLS FIGURED AT 3 1/2"
WIDTHS UNLESS OTHERWISE NOTED.
4) SET WINDOWS AT 6"-8" A.F.F.
UNLESS OTHERWISE NOTED.
5) DIMENSIONS ARE TO FRAMING
UNLESS OTHERWISE NOTED.

SECOND FLOOR PLAN

4TH BEDROOM OPTION

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
	(/		LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:

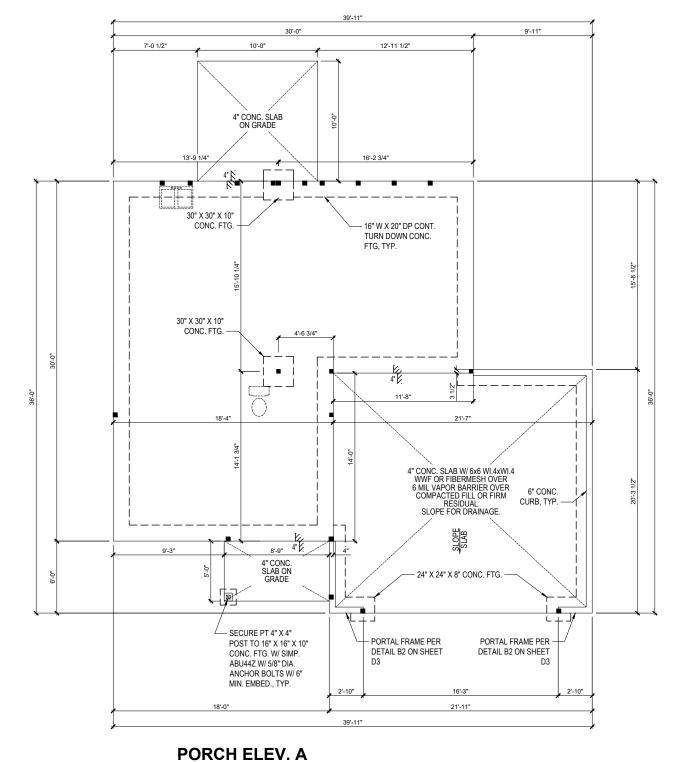
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTRACTORS RESPONSIBILITY TO VENIFF ALL DIMENSION AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (IE. ILEVEL MICROLAM)
 ALL LISL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5. AND
- (1) 2x4 JACK S I UD (N.O.) AND KING S I UDS PER I ABLE. R602.7.5, AND TOGETHER N.E. (2) 10d NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFOR TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION

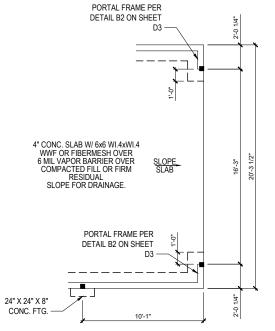
- REFER TO 2018 NO BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10.7° IN HEIGHT.

 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, fo = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/270 ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0° O.C. AND NOT MORE
 THAN 12° FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 FOR PLATE SECTION. ANCHOR BOLT SHALL EXTEND 7° INTO CONCRETE OR
 MASONRY.
- MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

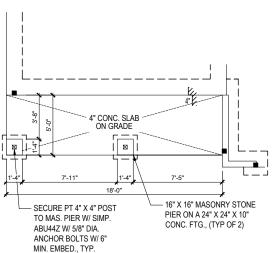
 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.





SIDE LOAD GARAGE OPTION



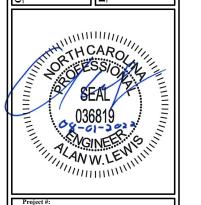
PORCH ELEV. B & C

FOUNDATION PLAN (MONO SLAB)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering





Project #: DRB2201-0065D	
Date: 08/01/22	
Engineered By: AM	
DWG. Checked By: AWL	
SEE PLAN	
REVIS	ISIONS
No. Doto.	Damaula.

S₁A

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

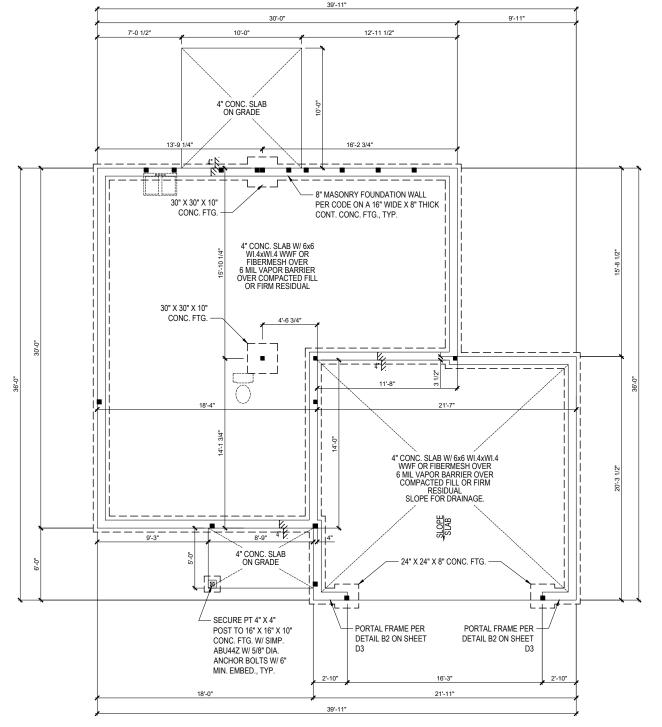
- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- Fb = 2600 PSI, E = 1.9M PSI
- (IE. ILEVEL MICROLAM)
 ALL LISL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)
 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5. AND
- (1) 2x4 JACK S I UD (N.O.) AND KING S I UDS PER I ABLE. R602.7.5, AND TOGETHER N.E. (2) 10d NAILS @ 6" O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6"5", MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1"5". OTHERWISE REFER TO TABLES R602.7(1) AND R602.7(2). ALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2x10 (U.N.O.) REFOR TO TABLES R602.7(1) AND R602.7(2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (UNO)
 REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION

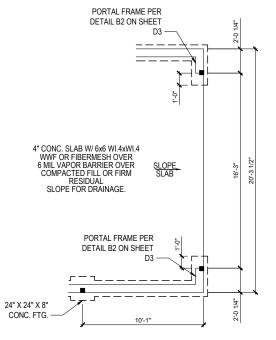
- REFER TO 2018 NO BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10.7° IN HEIGHT.

 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, fo = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/270 ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0° O.C. AND NOT MORE
 THAN 12° FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 FOR PLATE SECTION. ANCHOR BOLT SHALL EXTEND 7° INTO CONCRETE OR
 MASONRY. MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.) PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

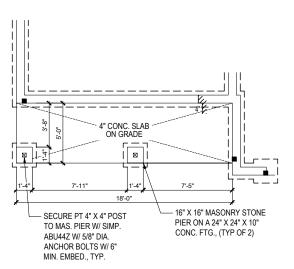
 MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.





SIDE LOAD GARAGE OPTION



PORCH ELEV. B & C

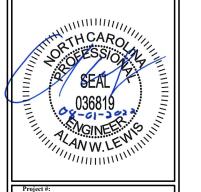
FOUNDATION PLAN (STEM WALL)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

PORCH ELEV. A

Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engineering





Project #: DRB2201-0065D
Date: 08/01/22
Engineered By: AM
DWG. Checked By: AWL
SEE PLAN
REVISIONS

S₁B

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION	
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	BASED ON SEISMIC ZONES A, B & C			

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS. IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND

- ALL LVI. LUMBER 10 BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2800 PSI, E = 1 9M PSI

 (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

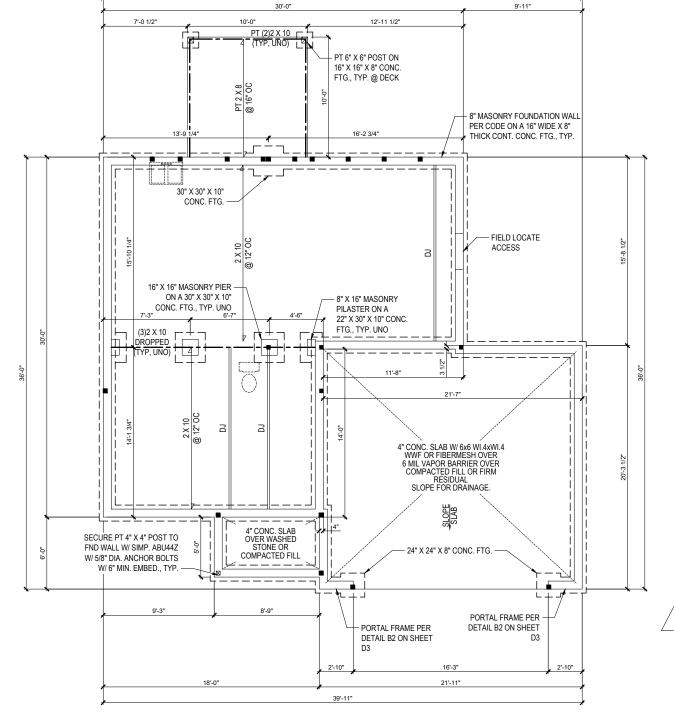
 ALL LODB BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w

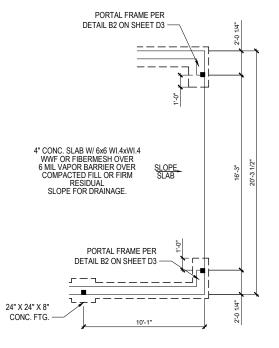
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
- (1) 243 JACKS 100 (JANUC) AND KINDS 21 IOSP SEY 1802 ER 1902 J.S., AND TOGETHER W. (2) 100 MAILS & 8° O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 10°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 10°5', OTHERWISE REFER TO TABLES RE02 (71) AND RE02 (72) CALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2470 (U.N.O.) REFOR TO TABLES RE02 (71) AND RE02 (72) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (INNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.

- OF ALL WALLS OVER 10-0" IN HEIGHT.
 ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (LINO)
 ALL EXTERIOR LUMBER TO BE #2 SVP PT
 ALL CONCRETE, 10 = 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6'-0" O.C. AND NOT MORE
 THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS
 PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL BE STACED AT 3"-0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
 MASONRY. MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- INC.

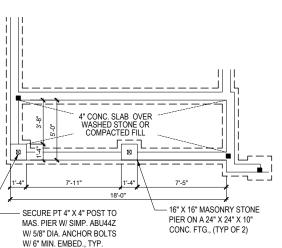
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 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY
- ANCHORED TO THE FOUNDATION. METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.





SIDE LOAD GARAGE OPTION



PORCH ELEV. B & C

FOUNDATION PLAN (CRAWL SPACE)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

4.4 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 5 VENTS REQ'D (BASED ON 8" X 16" VENTS)

660 SQ. FT. OF CRAWL SPACE / 1500 = 0.44 SQ. FT. OF REQ'D VENTILATION WITH CROSS VENTILATION 0.44 SQ. FT. OF VENTILATION REQ'D / 0.88 SQ.FT. PER VENT = 1 VENTS REQ'D (BASED ON 8" X 16" VENTS)

660 SOLETIOF CRAWL SPACE / 150 = 4.4 SOLETIOF RED'D VENTILATION WITHOUT CROSS VENTILATION

- THE TOTAL AREA OF VENTILATION OPENINGS MAY BE REDUCED TO 11500 OF THE CRAWL SPACE. GROUND AREA WHERE THE REQUIRED OPENINGS ME PLACED SO AS TO REVOICE CROSS VENTILATE OF THE COMES. SHOULD THE RESTALLATION OF OPENINGEL CONTRES SHALL NOT BE REPORTED TO THE COMES. SHOULD THE STATE OF OPENING SHALL TO SHALL THE SHALL THE PARKING THE RITHER WHEN THE CRAWL SHALL SHALL TO MAIS LOPED SHE THE LIPINEL FOUNDATION WALLS MAY BE COST RECTED WITHOUT WALL SHALL TO SHALL SHALL THE PROPERTY WALLS MAY BE COST RECTED WITHOUT WALL SHALL TO SHALL SHALL THE SHALL SHALL THE PROPERTY WALLS MAY BE COST RECTED WITHOUT WALL SHALL THE SHALL SHALL THE PROPERTY WALLS MAY BE COST RECTED WITHOUT WALL SHALL THE SHALL SH



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	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLECTION		
	(- /	(- /	LL	TL	
FLOOR (primary)	40	10	L/360	L/240	
FLOOR (secondary)	40	10	L/360	L/240	
ATTIC (w/ storage)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD	BASED ON 120 MPH (EXPOSURE B)				
SEISMIC	BASED ON SEISMIC ZONES A, B & C				

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE". IN
- ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS IT IS THE CONTINCE TO THE STORY OF THE CONTINUE OF THE CONTIN
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND

- ALL LVI. LUMBER 10 BE 1.75' WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2800 PSI, E = 1 9M PSI

 (I.E. ILEVEL MICROLAM)
 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

 ALL LODB BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w

 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R602.7.5, AND
- (1) 243 JACKS 100 (JANUC) AND KINDS 21 IOSP SEY 1802 ER 1902 J.S., AND TOGETHER W. (2) 100 MAILS & 8° O.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 10°5', MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 10°5', OTHERWISE REFER TO TABLES RE02 (71) AND RE02 (72) CALL INTERIOR LOAD BEARING HEADERS TO BE (2) 2470 (U.N.O.) REFOR TO TABLES RE02 (71) AND RE02 (72) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS (INNO)
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT
 ALL CONCRETE, f6 = 3000 PSI MIN.

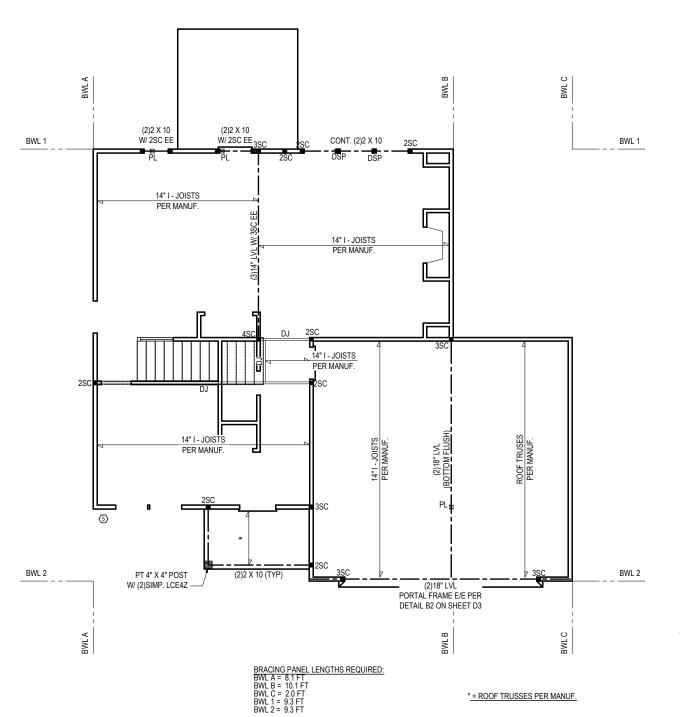
- ALL CONCRETE, 16 3000 PSI MIN.
 PRESUMPTIVE BEARING CAPACITY = 2000 PSF
 1/2"9 ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0" O.C. AND NOT MORE
 THAN 12" FROM THE CORNET. THERE SHALL BE A MINIMUM OF (2) BOLTS
 PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3"0" O.C.
 FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR
 MASCANIDY. MASONRY.
 PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9'-0" (UNO)
 PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP
- AND BOTTOM OF PORCH COLLIMNS (U.N.O.)
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018
- MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

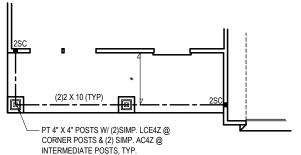
 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL.
- ANCHORED TO THE FOUNDATION

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC. 3) BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3. REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING
- CONNECTIONS & SUPPORT OF BRACED WALL PANELS. 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0" (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING). SECURE W 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5) SPACEO @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- 3 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION BOOM AS A WIND OF THE PROPERTY OF THE PROPERT
- R602-10.3 (UNO)

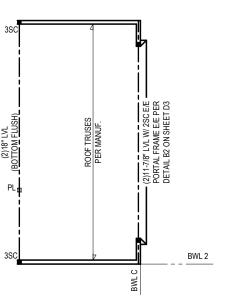
 ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS
 ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE
 CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP)
 SHEATHING WITH A MINIMUM THICKNESS OF 36". SHEATHING SHALL BE
 SECURED WITH MINIMUM 60 COMMON NAILS SPACED AT 6" O.C. AT
 PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

 MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
 BE ASTEDIOLOGY.
- BE AS FOLLOWS:
 - S FOLLOWS:
 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 30" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.
 48" FOR OPENINGS GREATER THAN 85% OF
 WALL HEIGHT.
- 4 SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIFU OF A CORNER RETURN. ACCORDANCE WITH FIGURE R802.10 3.(4). IN LIEU OF A CORNER RETURE EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 8009 SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE





PORCH (ELEV. B & C)



SIDE LOAD GARAGE OPTION

BRACING PANEL LENGTHS REQUIRED: BWL C = 2.0 FT BWL 2 = 9.3 FT

BRACING PANEL LENGTHS PROVIDED: BWL C = 6.44 FT PF BWL 2 = 33.5 FT CS-WSP

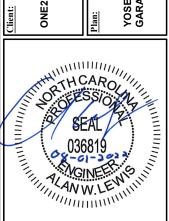
FIRST FLOOR PLAN

BRACING PANEL LENGTHS PROVIDED: BWL A = 27.33 FT CS-WSP BWL B = 15.71 FT CS-WSP BWL C = 20.29 FT CS-WSP BWL 1 = 16.67 FT CS-WSP BWL 2 = 17.5 FT CS-WSP / PF

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36") Engineers seal does not include cor procedures or safety precau

- Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engin & Design, P.A. liability. Please review these doc
- Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendetc. presented in these documents were





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(2)2 X 10 (2)5-1/2" LVL BWL A BWL A PER MANUF ROOF TRUSSES PER MANUF. BWL B BWL B BRACING PANEL LENGTHS REQUIRED: BWL A = 3.2 FT BWL B = 3.2 FT BWL 1 = 3.8 FT BWL 2 = 3.8 FT

BRACING PANEL LENGTHS PROVIDED: BWL A = 30.0 FT BWL B = 33.33 FT BWL 1 = 20.5 FT BWL 2 = 20.5 FT

SECOND FLOOR PLAN

1/8" = 1'-0" (11"X17")

1/4" = 1'-0" (24"X36")

DESIGN LOADS

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION
	(/	(/	LL	TL
FLOOR (primary)	40	10	L/360	L/240
FLOOR (secondary)	40	10	L/360	L/240
ATTIC (w/ storage)	20	10	L/240	L/180
ATTIC (no access)	10	5	L/240	L/180
EXTERNAL BALCONY	40	10	L/360	L/240
ROOF	20	10	L/240	L/180
ROOF TRUSS	20	20	L/240	L/180
WIND LOAD	BAS	SED ON 120 MPH (E	XPOSURE B)	
SEISMIC	BAS	ED ON SEISMIC ZO	NES A, B & C	

- STRUCTURAL NOTES:
 1) ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS
 OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE", IN
- OF YOKEN CAROLINA SEATE 2018 RESIDENTIAL BUILDING CODE", IN ADDITION TO ALL LOCAL CODES AND REGULATIONS.
 IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION. TYMPALL BUILDING TO THE PRIOR TO THE PROPOSIBLE FOR DIMENSIONS AND SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.
 ALL LIMBER COLLAD RESPONSE ONCE CONSTRUCTION BEGINS.
- ALL LUMBER SHALL BE SYP #2 (UNO)
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND
- ALL LVL LUMBER 10 be 1.7.6" WIDE NOMINAL EACH SINGLE MEMBER AND FD = 2800 PS, E = 1.9M PS!

 (I.E. ILEVEL MICROLAM)

 ALL LSL LUMBER IS TO BE 1.55E (Fb = 2325 PSI)

 ALL LOAD BEARING EXTERIOR WINDOW HEADERS ARE TO BE (2) 2x10 w/
 (1) 2x4 JACK STUD (U.N.O.) AND KING STUDS PER TABLE R802.7.5, AND
- (1) 244 JACAS TO IQ UNIO, YAND KING STIDES PER TABLE ROBLE? A, AND TOGETHER WILZ (2) 100 MAILS & 8° C.C., PROVIDED THAT THE TOP OF THE WINDOW HEIGHT IS 6°.5°, MINIMUM BOTTOM OF THE WINDOW HEIGHT IS 1-6°. OTHERWISE REFER TO TABLES REGOZ! (7) AND ROBLE SEGOZ! (7) AND ROBLE Z (7) AND ROBL
- REFER TO 2018 NC BUILDING CODE SECTION R602 FOR CONSTRUCTION OF ALL WALLS OVER 10'-0" IN HEIGHT.
- ALL STRUCTURAL STEEL SHALL BE ASTM A992 GRADE 50
 Fy = 50 KSI MIN. (UNO)
 ALL EXTERIOR LUMBER TO BE #2 SYP PT

- ALL CONCRETE, fc = 3000 PSI MIN.

 PRESUMPTIVE BEARING CAPACITY = 2000 PSF

 1/2*Ø ANCHOR BOLTS SPACED AT MAXIMUM OF 6-0* O.C. AND NOT MORE
- 1/2"O ANCHOR BOLTS SPACED AT MAXIMUM OF 6:0" O.C. AND NOT MORE THAN 12" FROM THE CORNER. THERE SHALL BE A MINIMUM OF (2) BOLTS PER PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3:0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. PSL COLUMNS DESIGNED WITH MAX. HEIGHT OF 9:0" (UNO) PROVIDE A MINIMUM OF 500H UPILIT'S A LATERAL CONNECTION AT TOP AND ROTTOM OF EMPIREY COLUMNS. (MIN.)
- AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- AND BOTTOM OF PORCH COLUMNS, (U.N.O.)

 PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.4 OF THE 2018 IRC.

 15) MAXIMUM MASONRY PIER HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.

 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY

- ANCHORED TO THE FOUNDATION. 17) METAL HANGERS SHALL BE SIMPSON OR APPROVED EQUAL

STRUCTURAL SHEATHING NOTES

- DESIGNED FOR SEISMIC ZONE A-C AND WIND SPEEDS OF 120 MPH OR LESS.
 WALLS SHALL BE BRACED IN ACCORDANCE WITH SECTION R602.10 OF THE 2018 NCRC.
- BRACING REQUIREMENTS SHALL BE PER TABLE R602.10.3.

 REFER TO SECTION R602.10.4 FOR LOAD PATH DETAILS INCLUDING CONNECTIONS & SUPPORT OF BRACED WALL PANELS.
- 1 REFERENCE FIGURE R602.10.4.3 OF THE 2018 NCRC.
- 4) INTERIOR BRACED WALL PANELS (BWP) INDICATED SHALL BE SHEATHED IN ACCORDANCE WITH THE GB METHOD OR WSP METHOD AS PRESCRIBED IN SECTION R602.10.1 (UNO)
- 2) 1/2" GYPSUM BOARD (GB) MINIMUM LENGTH OF 8"-0" (ISOLATED PANELS) OR 4"-0" (CONTINUOUS SHEATHING).
 SECURE W 5d COOLER NAILS (OR EQUAL PER TABLE R702.3.5)
 SPACED @ 7" O.C. AT PANEL EDGES, INCLUDING TOP AND BOTTOM PLATES & 7" O.C. AT INTERMEDIATE SUPPORTS
- (3) 3/8" WOOD STRUCTURAL PANEL (WSP) SECURE w/ 6d COMMON NAILS SPACED AT 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS
- 5) EXTERIOR BRACED WALL PANELS (BWP) SHALL BE CONSTRUCTED IN ACCORDANCE WITH CS-WSP METHOD AS PRESCRIBED IN SECTION R602.10.3 (UNO)
- R602-10.3 (UNO)

 ALL SHEATHABLE SURFACES OF EXTERIOR WALLS (INCLUDING AREAS
 ABOVE AND BELOW OPENINGS AND GABLE END WALLS) SHALL BE
 CONTINUOUSLY SHEATHED WITH WOOD STRUCTURAL PANEL (WSP)
 SHEATHING WITH A MININUM THICKNESS OF 3/6". SHEATHING SHALL BE
 SECURED WITH MINIMUM 64 COMMON NAILS SPACED AT 6" O.C. AT
 PANEL EDGES AND SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS.

 MINIMUM BRACED WALL PANEL LENGTHS WITH CS-WSP METHOD SHALL
 BE AS EQUI CIUS.
- MINIMOM BRACED WALL PANEL LENGTHS WITH CS-WSP
 BE AS FOLLOWS:
 24" ADJACENT TO OPENINGS NOT MORE THAN
 67% OF WALL HEIGHT
 3" ADJACENT TO OPENINGS GREATER THAN
 67% AND LESS THAN 85% OF WALL HEIGHT.
 48" FOR OPENINGS GREATER THAN 85% OF
 WALL HEIGHT
- 4 SHEATH INTERIOR & EXTERIOR
- 8) FOR CS-WSP METHOD, A MINIMUM 24" BRACED WALL PANEL CORNER RETURN SHALL BE PROVIDED AT BOTH ENDS OF A BRACED WALL LINE IN ACCORDANCE WITH FIGURE R602.10.3(4). IN LIEU OF A CORNER RETURN, ACCORDANCE WITH FIGURE R802.10 3.(4). IN LIEU OF A CORNER RETURE EITHER A MIN. 48° BRACED WALL PANEL SHALL BE PROVIDED AT THE CORNER OR A HOLD-DOWN DEVICE WITH A MINIMUM UPLIFT DESIGN VALUE OF 800% SHALL BE FASTENED TO THE EDGE OF THE BRACED WALL PANEL CLOSEST TO THE CORNER AND TO THE FOUNDATION OR FRAMING BELOW.
- 5 MINIMUM 800# HOLD-DOWN DEVICE

Engineers seal does not include cor means, methods, techniques, seque procedures or safety precaution

- Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Fallure to do so will void Tyndall Engineering
- **Please review these documents carefully Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommender, presented in these documents were depend executable accounts when the presented in these documents were depend executable accounts.



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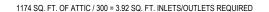


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S3

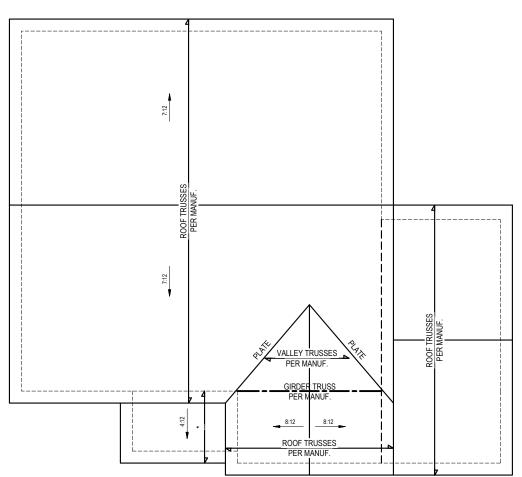


- CALCULATION BASED ON VENTILATORS USED AT LEAST 3'-0" ABOVE THE COMICE VENTS WITH THE BALANCE OF VENTILATION PROVIDED BY EAVE VENTS.
- CATHEDRAL CEILINGS SHALL HAVE A 1" MINIMUM CLEARANCE BETWEEN THE BOTTOM OF THE ROOF DECK AND THE INSULATION.



* ATTIC VENTILATION CALCULATION

VALLEY TRUSSES
PER MANUF. 8:12 ROOF TRUSSES PER MANUF.



* = ROOF TRUSSES PER MANUF

ROOF PLAN (ELEV. B & C)

1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36")

ROOF PLAN (ELEV. A)

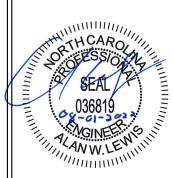
1/8" = 1'-0" (11"X17") 1/4" = 1'-0" (24"X36") * Engineers seal does not include constructi means, methods, techniques, sequences,

- means, methods, techniques, sequences, procedures or safety precaution.

 * Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Faliure to do so will void Tyndall Engineering & Design, P.A. liability.

 * Please review these documents carefully. Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommendations, etc. presented in these documents were deemed acceptable once construction begins.





Project #: DRB2201-0065D
Date: 08/01/22
Engineered By: AM
DWG. Checked By: AWL
Scale: SEE DLAN

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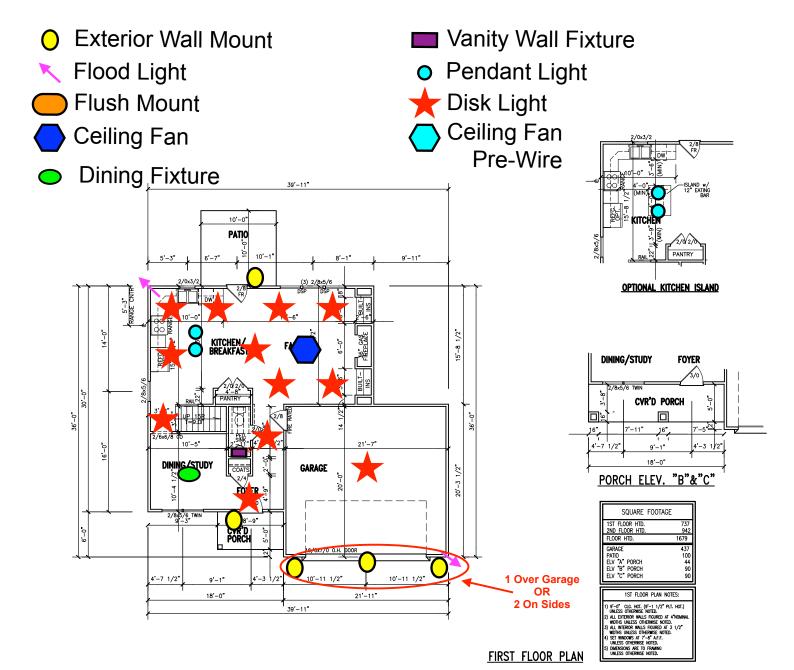
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FLOOR PLAN

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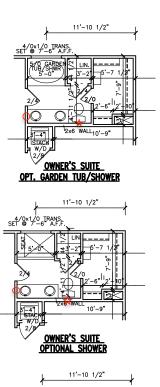
PROJECT #: DRB2201-0065B

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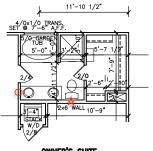
FLOOR PLAN

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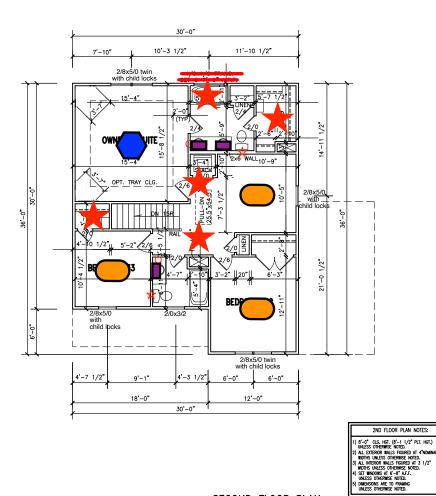


☆ Towel Bar

O Hand Towel Ring







SECOND FLOOR PLAN



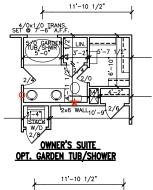
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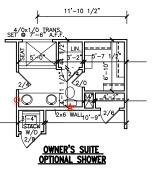
FLOOR PLAN

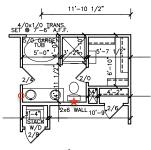




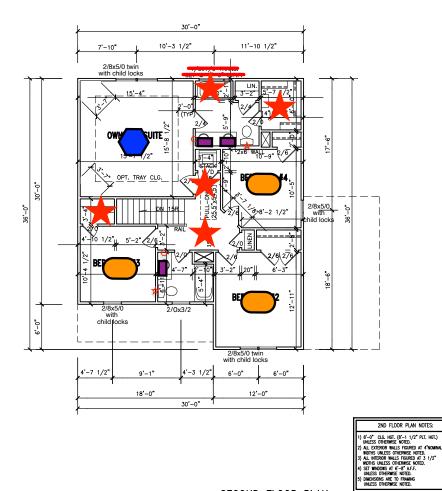
Towel Bar

O Hand Towel Ring





OWNER'S SUITE OPTIONAL SUNLIGHT GARDEN TUB



SECOND FLOOR PLAN

4TH BEDROOM OPTION

STRUCTURAL NOTES

ALL CONSTRUCTION SHALL CONFORM TO THE LATEST REQUIREMENTS OF "NORTH CAROLINA STATE 2018 RESIDENTIAL BUILDING CODE". IN ADDITION TO ALL LOCAL CODES AND REGULATIONS

DESIGN LOADS:

	LIVE LOAD (PSF)	DEAD LOAD (PSF)	DEFLE	CTION	
	(, 0,)	(1.01)	LL	TL	
ALL FLOORS	40	10	L/360	L/240	
ATTIC (w/ walk up stairs)	30	10	L/360	L/240	
ATTIC (pull down access)	20	10	L/240	L/180	
ATTIC (no access)	10	5	L/240	L/180	
EXTERNAL BALCONY	40	10	L/360	L/240	
ROOF	20	10	L/240	L/180	
ROOF TRUSS	20	20	L/240	L/180	
WIND LOAD		BASED ON 120 MPH (EXPOSURE B)			
SEISMIC	SEISMIC ZONES A, B & C				

- MINIMUM ALLOWABLE SOIL BEARING PRESSURE = 2000 PSF
- CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI AND A MAXIMUM SLUMP OF FIVE INCHES UNLESS NOTED OTHERWISE, (U.N.O.)
- MAXIMUM DEPTH OF UNBALANCED FILL AGAINST FOUNDATION WALLS TO BE LESS THAN 4'-0" WITHOUT USING SUFFICIENT WALL BRACING. REFER TO SECTION R404 OF 2018 NC BUILDING CODE FOR BACKFILL LIMITATIONS BASED ON WALL HEIGHT. WALL THICKNESS, SOIL TYPE, AND UNBALANCED BACKFILL HEIGHT.
- ALL FRAMING LUMBER SHALL BE SYP #2 (Fb = 800 PSI, BASED ON 2x10) UNO.
 ALL FRAMING LUMBER EXPOSED TO THE ELEMENTS SHALL BE TREATED MATERIAL.
 ALL LVL LUMBER TO BE 1.75" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2600 PSI, E = 1.9M PSI (U.N.O.)
 ALL SL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2235 PSI, E = 1.6M PSI (U.N.O.)
 ALL PSL LUMBER TO BE 3.5" WIDE NOMINAL EACH SINGLE MEMBER AND Fb = 2400 PSI, E = 1.8M PSI (U.N.O.)
- ALL LOAD BEARING EXTERIOR HEADERS SHALL BE AT (2) 2x10. (U.N.O.) REFER TO TABLE R602.7(1) & (2) FOR JACK STUD REQUIREMENTS FOR HEADER SPANS FOR INTERIOR AND EXTERIOR LOAD CONDITIONS UNLESS SPECIFICALLY NOTED ON PLANS.
- ALL STRUCTURAL STEEL W-SHAPES (I-BEAMS) SHALL BE ASTM A992 GRADE 50.
- ALL STEEL ANGLES, PLATES, AND C-CHANNELS SHALL BE ASTM A36. ALL STEEL PIPE SHALL BE ASTM A53 GRADE B.
- STEEL BEAMS SHALL BE SUPPORTED AT EACH END WITH A MINIMUM BEARING LENGTH OF 3-1/2" AND FULL FLANGE WIDTH. PROVIDE SOLID BEARING FROM BEAM SUPPORT TO FOUNDATION. BEAMS SHALL BE ATTACHED TO EACH SUPPORT WITH TWO (2) LAG SCREWS (1/2") at **LONG, LATERAL SUPPORT IS CONSIDERED ADEQUATE PROVIDED THE JOISTS ARE TOE NAILED TO THE SOLE PLATES, AND THE SOLE PLATES ARE NAILED OR BOLTED TO THE BEAM FLANGES @ 48" O.C.
- PROVIDE ANCHOR BOLT PLACEMENT PER SECTION 403.1.6: 1/2"Ø ANCHOR BOLTS SPACED AT 6'-0" O.C. AND PLACED 12" FROM THE END OF EACH PLATE SECTION. ANCHOR BOLTS SHALL BE SPACED AT 3'-0" O.C. FOR BASEMENTS. ANCHOR BOLT SHALL EXTEND 7" INTO CONCRETE OR MASONRY. THE BOLTS SHALL BE LOCATED IN THE MIDDLE THIRD OF THE WIDTH OF THE PLATE. THERE SHALL BE A MINIMUM TWO ANCHOR BOLTS PER PLATE SECTION.
- FOUNDATION DRAINAGE-DAMP PROOFING OR WATERPROOFING PER SECTION 405 AND 406 OF NC BUILDING CODE.

WALL AND ROOF CLADDING VALUES:
WALL CADDING SHALL BE DESIGNED FOR 28.0 POUNDS PER SQUARE FOOT (LBS/SQFT) OR GREATER POSITIVE AND NEGATIVE PRESSURE.
ROOF VALUES BOTH POSITIVE AND NEGATIVE SHALL BE AS FOLLOWS:

39.0 LBS/SQFT FOR ROOF PITCHES 0/12 TO 1.5/12

36.0 LBS/SQFT FOR ROOF PITCHES 1.5/12 TO 6/12 18.0 LBS/SQFT FOR ROOF PITCHES 6/12 TO 12/12

**MEAN ROOF HEIGHT 30'-0" OR LESS

- 13) FOR ROOF SLOPES FROM 2/12 THROUGH 4/12, BUILDER TO INSTALL 2 LAYERS OF 15# FELT PAPER.
- REFER TO SECTION R602.3 FOR FRAMING OF ALL WALLS OVER 10'-0" IN HEIGHT.
- PROVIDE CONTINUOUS SHEATHING PER SECTION 602.10.3 OF THE 2018 NCRC.
- 16) UPLIFT LOADS GREATER THAN 500# SHALL BE CONTINUOUSLY ANCHORED TO THE FOUNDATION.
- 17) REFER TO TABLE N1102.1 FOR PRESCRIPTIVE BUILDING ENVELOPE THERMAL COMPONENT CRITERIA.
- 18) PSI COLUMNS DESIGNED WITH MAXIMUM HEIGHT OF 9'-0" (U.N.O.)
- PROVIDE A MINIMUM OF 500# UPLIFT & LATERAL CONNECTION AT TOP AND BOTTOM OF PORCH COLUMNS. (U.N.O.)
- MAXIMUM MASONRY PEIR HEIGHT SHALL NOT EXCEED FOUR TIMES ITS LEAST HORIZONTAL DIMENSION.
- IT IS THE CONTRACTORS RESPONSIBILITY TO VERIFY ALL DIMENSIONS AND SQUARE FOOTAGE PRIOR TO CONSTRUCTION.
 TYNDALL ENGINEERING & DESIGN, PA IS NOT RESPONSIBLE FOR DIMENSION OR SQUARE FOOTAGE ERRORS ONCE CONSTRUCTION BEGINS.

CLIMATE ZONES	FENESTRATION U-FACTOR	SKYLIGHT ^b U-FACTOR	GLAZED FENESTRATION SHGC ^{b,k}	CEILING ^m R-VALUE	WOOD FRAMED WALL R-VALUE	MASS WALL R-VALUE	FLOOR R-VALUE	BASEMENT ^{c,g} WALL R-VALUE	SLAB ^d R-VALUE AND DEPTH	CRAWL SPACE C WALL R-VALUE
3	0.35	0.55	0.30	38 or 30 cont	15 or 13 + 2.5 h	5/13 or 5/10 cont	19	<u>5/13</u> f	0	5/13
4	0.35	0.55	0.30	38 or 30 cont	15 or 13 + <u>2.5</u> h	5/13 or 5/10 cont	19	10/15	10	10/15
5	0.35	0.55	NR	38 or 30	n 19, or 13 + 5 or 15 + 3	13/17 <u>or</u> 13/12.5 cont	30 ^g	10/15	10	10/19

TABLE N1102.1 CLIMATE ZONES 3-5

R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS
OF THE INSULATION, THE INSTALLED R-VALUE OF THE INSULATION SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE.

ASSIGNATION OF THE HOUSE SHARE OF REASTATON.

"UNIS MEANS AND CONTINUOUS BUILD AND READMEND OF THE WITEROO OF EXTERIOR OF THE HOME OF THE CHARLES AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF DOMINIMAND TO THE BOTTOM OF THE POST OF AND AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF DESTROYAND AND AND ASSIGNATION SHALL BE APPLIED FROM THE INSPECTION OF LISTS FOR THE CHARLES AND AND AND ASSIGNATION OF THE CHARLES AND ASSIGNATION OF THE POST OF THE POST

e. <u>Deleted</u> f. Basement wall insulation is not required in Warm-Humid Locations as defined by <u>Figure N1101.7</u> and <u>Table N1101.7</u>.

a OR INSULATION SUFFICIENT TO FILL THE FRAMING CAVITY IR-19 MINIMUM

5. OR INSTALLAND SHY-EXH I DUTL THE PROMINE CANTE. YES WINDOW.

THE FRIST YALKE COANTY ROULDING THE SECOND VALUE SO CONTINUOUS RISULATION, SO "13-5" MEANS R-11 CANTY NIGULATION PULS R-8 INSLATED
SHEARING. "15-7" MEANS R-15 CANTY ROULDING THE STATE OF THE CATEGORY OF THE COTTEROR SHAPE OF THE COTTEROR SHAPE

OR MASS WALLS, THE SECOND R-VALUE APPLIES WHEN MORE THAN HALF THE INSULATION IS ON THE INTERIOR MASS WA

I FOR MASS WALLS, THE SECTION WALLE APPLES WHEN LINKE HINDRED HER DESIGNATION OF THE INTERIOR MASS WALLS.

IN ADDITION TO THE EXPERTION AS EXCENT WITH 23.3 ALXIVAGIAN OF TWO GLAZES THE STRUND APPLICATION PRODUCT ASSESSMESS HAVE A DEFACTOR TO GREATER THAN 0.55 SHALL BE PREMITTED TO BE SIGNATION TO THE TOTAL THE TOTAL THAN 1.55 SHALL BE PREMITTED TO BE SIGNATION TO THE TOTAL THAT 23.4 ALXIVAGIAN OF TWO GLAZES THE STRUNDED RESONANTION PRODUCT ASSESSMESS WHINCH PROBALTY.

IN ADDITION TO THE EXPERTITION TO SET OF THE TOTAL THAT THAT THE TOTAL THAT THE TOTAL THAT THAT THAT THE TOTAL THAT THAT THAT THAT THE TOTAL THAT THAT THE TOTAL THAT THE TOTAL THAT THAT THAT THE TOTAL THAT THAT THAT THE TOTAL THAT THAT THE TOTAL THAT THAT THE TOTAL THAT THAT THE TOTAL THAT THAT THE TOTAL TH

OF THE ATTO-ROOF DEEX.

FINE THE REPORT OF THE PROPERTY OF ROOF EDGE WHERE THE SPACE BILMHED BY THE PITCH OF THE ROOF. THERE THE INSLITATION MUST FILL THE SPACE UP TO THE ARE MAYIE.

8. THE REPORT ASS BATTS COMPRESSED AND INSTILLED IN A MOUNTAL 2 - 6 FRANING CANTY IS DEEMED TO COMPLY. PREPORTASS BATTS RATTO R-19 OR HIGHER COMPRESSED

AND INSTILLED AND A 20 MUST LIST OF DEEMED TO COMPLY. M MASS WALL SPECIFIC HEAT CONTENT REQUIREMENT MAY USE THE MASS WALL R-VALUE AS THE MINIMUM REQUIREMENT

DEFINITIONS FOR COMMON ABBREVIATIONS

ALT	=	ALTERNATE	MAX	=	MAXIMUM
CANT	=	CANTILEVER	MIN	=	MINIMUM
CJ	=	CEILING JOIST	NOM	=	NOMINAL
CMU	=	CONCRETE MASONRY UNIT	O.C.	=	ON CENTER
COL	=	COLUMN	PL	=	POINT LOAD
CONC	=	CONCRETE	PT	=	PRESSURE TREATED
CONT	=	CONTINUOUS	REINF	=	REINFORCED
CT	=	COLLAR TIE	REQD	=	REQUIRED
DBL	=	DOUBLE	RJ	=	ROOF JOIST
DIA	=	DIAMETER	RS	=	ROOF SUPPORT
DJ	=	DOUBLE JOIST	SC	=	STUD COLUMN
DR	=	DOUBLE RAFTER	SCH	=	SCHEDULE
EA	=	EACH	SPEC	=	SPECIFIED
EE	=	EACH END	THK	=	THICK
FJ	=	FLOOR JOIST	TJ	=	TRIPLE JOIST
FND	=	FOUNDATION	TRTD	=	TREATED
FTG	=	FOOTING	TYP	=	TYPICAL
GALV	=	GALVANIZED	UNO	=	UNLESS NOTED OTHERWISE
HORIZ	=	HORIZONTAL	W	=	WIDE FLANGE BEAM
HT	=	HEIGHT	WWF	=	WELDED WIRE FABRIC
MANUF	=	MANUFACTURER	XJ	=	EXTRA JOIST

MAXIMUM HEIGHT OF DECK SUPPORT POSTS AS FOLLOWS:

POST SIZE	MAX. POST HEIGHT**
4 x 4	8'-0"
6 x 6	20'-0"
***	OVER 20'-0"

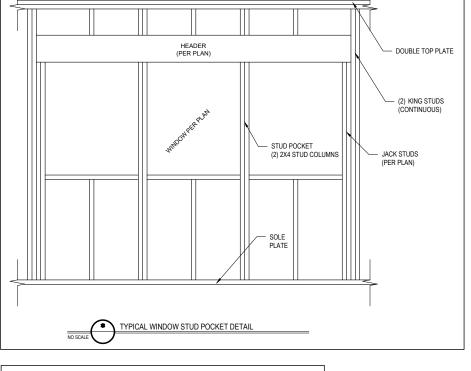
- THIS TABLE IS BASED ON NO. 2 TREATED SOUTHERN PINE POSTS.

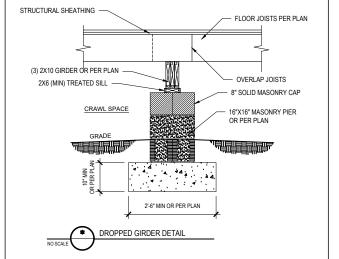
 MAXIMUM TRIBUTARY AREA IS BASED ON 128 TOTAL SQUARE FEET
 WHICH MAY BE LOCATED AT DIFFERENT LEVELS.
- ROM TOP OF FOOTING TO BOTTOM OF GIRDER DECKS WITH POST HEIGHTS OVER 20'-0" SHALL BE DESIGNED AND
- SEALED BY A PROFESSIONAL ENGINEER OR REGISTERED ARCHITECT.
- DECKS SHALL BE BRACED TO PROVIDE LATERAL STABILITY BY ONE OF
- A. THE DECK FLOOR HEIGHT IS LESS THAN 4-0" AND THE DECK IS
 ATTACHED TO THE STRUCTURE IN ACCOPDANCE WITH SECTION (4)
 ABOVE LATERAL BRACING IS NOT REQUIRED.

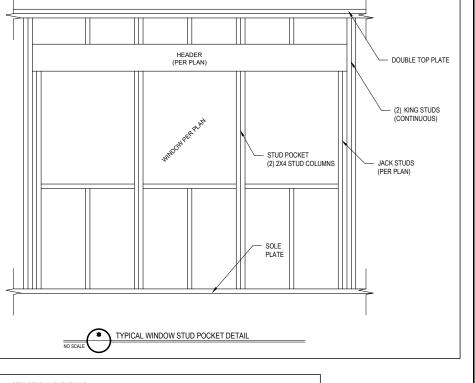
 B. 4-4 WOOD NIKE BRACES MAY BE PROVIDED ON EACH COLUMN IN
 BOTH DIRECTIONS. THE NIKEE BRACES SHALL ATTACH TO EACH POST
 AT A POINT NOT LESS THAN 13 OF THE POST LENGTH FROM THE
 TOP OF THE POST, AND THE BRACES SHALL BE ANGLED BETWEEN 45° AND 60° FROM THE HORIZONTAL, KNEE BRACES SHALL BE BOLTED TO THE POST AND GIRDER WITH ONE 5/8"Ø HOT DIPPED GALVANIZED
- TO THE POST AND SIGNER WITH OWNERS OF HOT DIFFED GALVANIZED BOLT AT EACH END OF THE BRACE.
 REESTANDING DECKS WITHOUT KNEE BRACES OR DIAGONAL BRACING, LATERAL STABILITY MAY BE PROVIDED BY EMBEDDING THE POSTS IN ACCORDANCE WITH THE FOLLOWING:

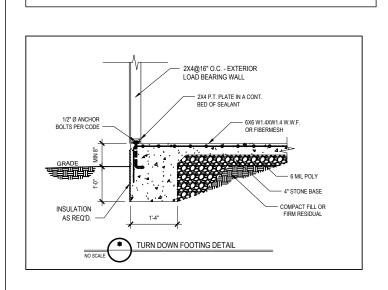
POST SIZE	MAX. TRIBUTARY AREA	MAX. POST HEIGHT	EMBEDMENT DEPTH	CONCRETE DIAMETER	
4 x 4	48 SQ. FT.	4'-0"	2'-6"	1'-0"	
6 x 6	120 SQ. FT.	6'-0"	3'-6"	1'-8"	

D. 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO 2 x 6 DIAGONAL VERTICAL CROSS BRACING MAY BE PROVIDED IN TWO
(2) PERPENDICULAR DIBECTIONS FOR FREESTANDING DECKS OR PARALLEL
TO THE STRUCTURE AT THE EXTERIOR COLUMN LINE FOR ATTACHED DECKS.
THE 2 x 6s SHALL BE ATTACHED TO THE POSTS WITH ONE 50°0 HOT
DIPPED GALVANIZED BOLT AT EACH END OF FACH BRACING MEMBER.
FOR EMBEDMENT OF PILES IN COASTAL REGIONS, SEE CHAPTER 46.







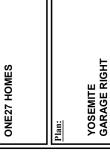


Engineers seal does not include construc procedures or safety precaution Any deviations or discrepancies on plans are to be brought to the immediate attention of Tyndall Engineering & Design, P.A. Failure to do so will void Tyndall Engi

& Design, P.A. liability. Please review these docu Tyndall Engineering & Design, P.A. will interpret that all dimensions, recommen etc. presented in these documents were



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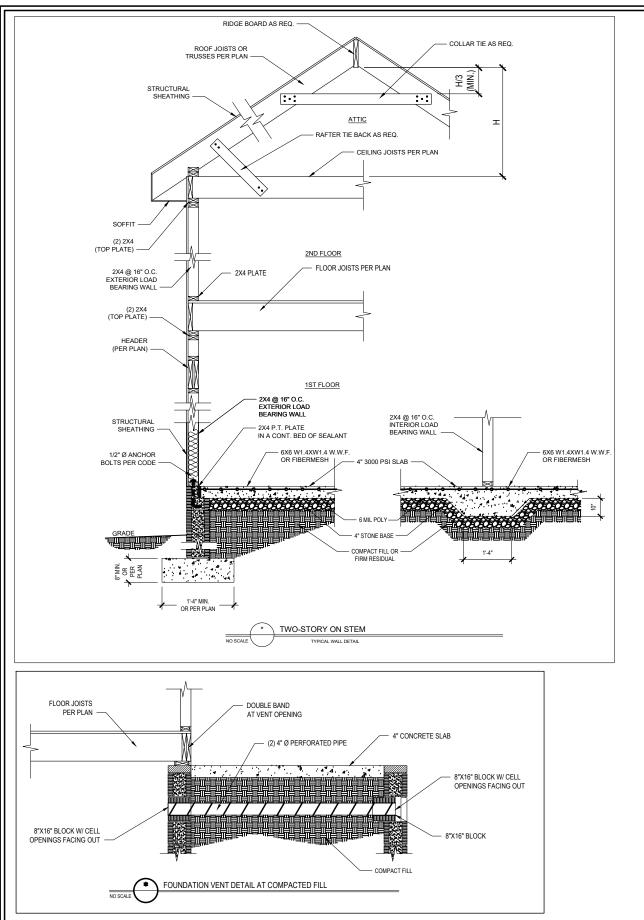


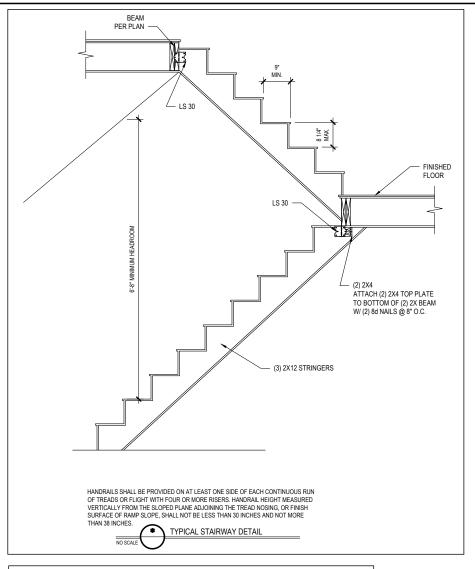


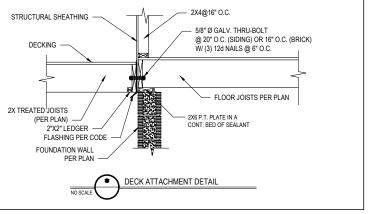
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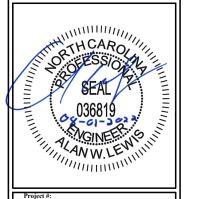
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> TYNDALL ENGINEERING & DESIGN, PA



Dr.

Plan: YOSEMITE GARAGE RIGHT

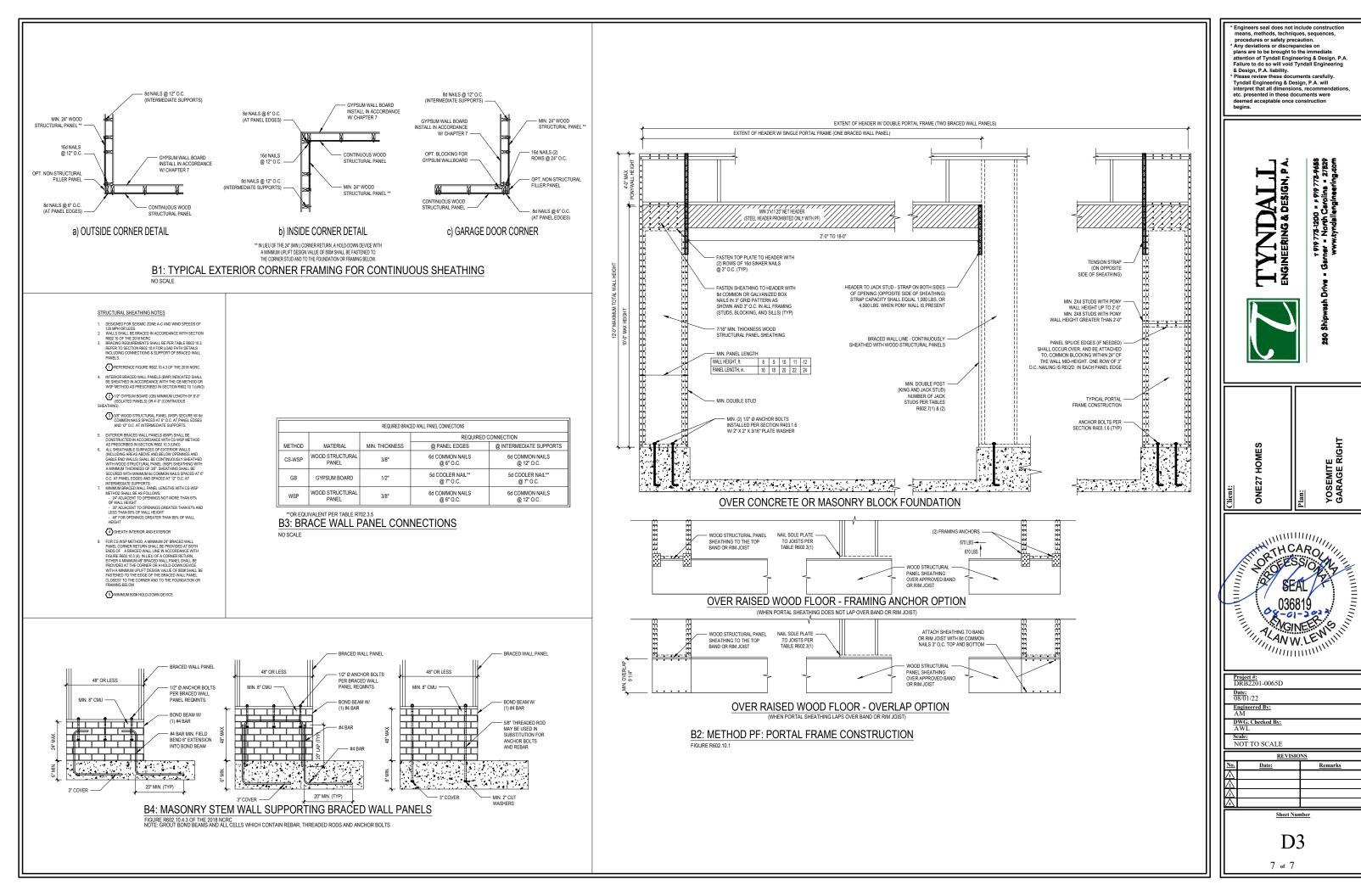


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Project #:
DRB2201-0065D
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08/01/22
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