

Trenco 818 Soundside Rd Edenton, NC 27932

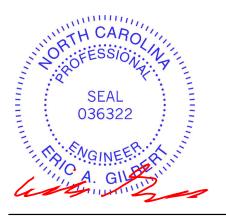
Re: 24110087-A 8 Mason Ridge-Wilmington C

The truss drawing(s) referenced below have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Pages or sheets covered by this seal: I74189936 thru I74189936

My license renewal date for the state of North Carolina is December 31, 2025.

North Carolina COA: C-0844



June 16,2025

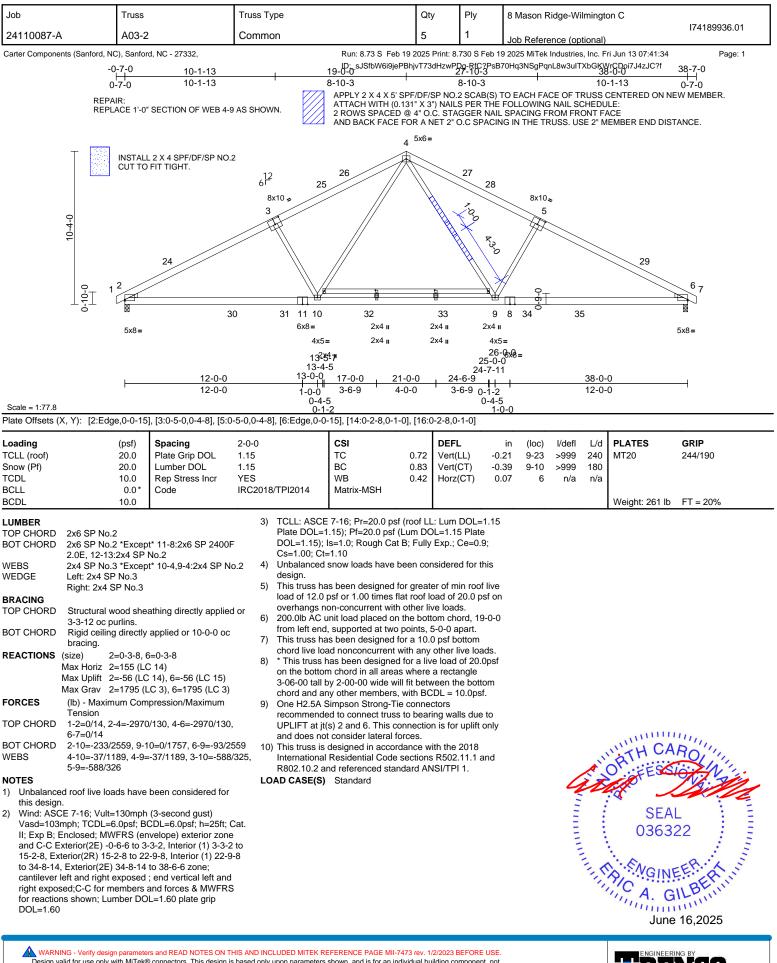
Gilbert, Eric

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job Tru 24110087-A A0		Truss Type		Qty P			- 3				
////	3	Common			5 1			8 Mason Ridge-Wilmington C Job Reference (optional)			
Carter Components (Sanford, NC), Sa			Run: 8.73 S Feb 19 20		S Feb 19				ri Jun 13 07:41:34		Page: 1
-0-7-0	10-1-13	1	ID: sJSfbW6i9jePBhjv 9-0-0							38-7-0	
0-7-0 REPAIR: CLEAN BREAK IN WEB 4-9	10-1-13		-10-3 APPLY 2 X 4 ATTACH WI 2 ROWS SP AND BACK	8-1 4 X 4' SPF/DF/ TH (0.131" X 3 ACED @ 4" O FACE FOR A I	0-3 /SP NO.2 3") NAILS 9.C. STAG	SCAB(S) TO PER THE F GER NAIL S	O EACH I OLLOWI SPACING	10- FACE C NG NAI G FROM	-1-13 DF TRUSS CENTE L SCHEDULE: I FRONT FACE S. USE 2" MEMBE	0-7-0 RED ON D	
		6 ¹² 25 8x10 = 3 0 31 11 10 6x8= 4x5= 13 ² 5 ⁴ : 13-4-5 13-0-0 1-0-0		27 33 2x4 II 2x4 II	28 9 2x4 = 4 25-0-0 24-7-11 9 9 9 0-1-2	- 	3	55 <u>38-0-</u> 12-0-		67 5x8=	
Scale = 1:77.8 Plate Offsets (X, Y): [2:Edge,0-0		0-4-5 0-1-2			0-4-5	-0					
Loading (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 TCDL 10.0 BCLL 0.0 BCDL 10.0 LUMBER 10.0 TOP CHORD 2x6 SP No.2 BOT CHORD 2x6 SP No.2 *Exi 2.0E, 12-13:2x4 \$ 2.0E, 12-13:2x4 \$ WEBS 2x4 SP No.3 *Exi WEDGE Left: 2x4 SP No.3 *Exi BOT CHORD Structural wood \$ 3-3-12 oc purlins BOT CHORD BOT CHORD Rigid ceiling directival bracing. REACTIONS (size) 2=0-3- Max Horiz 2=155 Max Horiz 2=155 Max Horiz 2=155 Max Grav 2=179 FORCES (lb) - Maximum C Tension 1-2=0/14, 2-4=-2 6-7=0/14 BOT CHORD 2-10=-233/2559, <td>Spacing Plate Grip DOL Lumber DOL Rep Stress Incr Code Code Code Code Code Plate Grip DOL Lumber DOL Rep Stress Incr Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code Code 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Ct=1 0.2 4) Unbalanced si design. 5) This truss has load of 12.0 ps overhangs nor 6) 200.0lb AC un from left end, s 7) This truss has chord live load 8) * This truss has chord live load 6) UPLIFT at jt(s) and does not c 559 10) This truss is d International R R802.10.2 and LOAD CASE(S) Cat.</td> <td>CSI TC 0.7 BC 0.7 BC 0.6 WB 0.4 Matrix-MSH 7-16; Pr=20.0 psf (roo 15); Pf=20.0 psf (Lum =1.0; Rough Cat B; Fi 1.10 now loads have been been designed for grundle for the concurrent with other it load placed on the b supported at two point been designed for a chord in all areas wher co-concurrent with a subseen designed for a chord in all areas wher co-concurrent with a subseen designed for a chord in all areas wher co-concurrent with a subseen designed for a chord in all areas wher co-concurrent with a subseen designed for a chord in all areas wher co-concurrent with a subseen designed for a chord in all areas wher consider lateral forces consider lateral forces consider lateral forces consider lateral forces consider lateral forces consider lateral forces consider lateral forces</td> <td>DEFL Vert(LL) Vert(CT) Horz(CT) Horz(CT) Horz(CT) Horz(CT) Considered for Stater of min r of load of 20.0 r live loads. potter of more hord, s, 5-0-0 apar No of botto ny other live live load of 20.0 r rectangle etween the b BCDL = 10.00 nectors earing walls of with the 201 ons R502.11.</td> <td>-0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -1.: -0.: -0.: -0.: -0.: -1.: -0.: -1.: -0.: -1.: -0.: -1.: -0.: -1.: -0.: -1.: -0.: -1.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: -0.: 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TRENCED AMITEK Affiliate 818 Soundside Road Edenton, NC 27932

A WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE. Design valid for use only with MITEk® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANS/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)



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818 Soundside Road Edenton, NC 27932

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Job	Truss		Truss Type		Qty	Ply	8 Mason F	lidge-Wiln	ningto	on C	174400000 00
24110087-A	A03-3	3	Common		5	1	Job Refere	ence (opti	onal)		174189936.02
Carter Componen	ts (Sanford, NC), Sanfor	rd, NC - 27332,		Run: 8.73 S Feb 19	2025 Print: 8	.730 S Feb 1				ri Jun 13 07:41:34	Page: 1
	-0-7-0	10-1-13	1	ID: sJSfbW6i9jePBI	hjvT73dHzwP	Dg-RfC?PsB 27-10-3	70Hq3NSgPqı	nL8w3uITX	bGKW 38	/rCDoi7J4zJC?f -0-0	38-7-0
	0-7-0	10-1-13	1	8-10-3		8-10-3				-1-13	0-7-0
	REPAIR:		+ + + -	ATTACH 1/2" PLY TO EACH FACE O	F TRUSS W	TH (0.131" X	2.5" MIN.) N	AILS PER	THE F	OLLOWING NAIL	
	REPLACE 4'-0" \$	SECTION OF WEB 4-9	· + +	2 X 3'S - 2 ROWS, NAILS TO BE DRIV							
			+ + -	FOR A NET 2" O.C	. SPACING						
		L 2 X 4 SPF/DF/SP NO.2 FIT TIGHT.	2	4	5x6=						
			10	26	h > - h	27					
			6 ¹² 2	5	\parallel $>$	28					
			8x10 =				8x10.	*			
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	10-4-0	/	- H		Ŕ	λ	- T	\searrow			
					\times	H.			\geq		
		24		11 214			+			29	
	Q 1 ²			2x4 II	2x4 II						⁶ 7
			<u> </u>	¢	<u> </u>	× ¥		-			
	○ ⊠ 5x8=	•	0 31 11 10 6x8=	4x5=	33	9	8 34	35			₩
APPL)	(2 X 4 X 4' SPF/DF/SF	P NO.2 SCAB(S) TO EA	CH FACE OF TRUSS C	ENTERED ON SPLICE.	2x4 🛛	2x4 II	_				5x8=
2 ROV	VS SPACED @ 4" O.C	. STAGGER NAIL SPAC	OWING NAIL SCHEDUL	CE		4x5 26-	· @				
	BACK FACE FOR A NE " MEMBER END DIST.	T 2" O.C SPACING IN ANCE.	13-			25-0- 24-7-1	0				
	F	<u> 12-0-0</u> 12-0-0	13-0-(4-6-9 🔐	1		<u>38-0-</u> 12-0-		—
Casla 4:77.0		12-0-0	1-0-0 0-4	-5	-0 3	-6-9 0-1-2 0-4-5	5		12-0-	0	
Scale = 1:77.8 Plate Offsets (X	Y): [2:Edge.0-0-15	5]. [3:0-5-0.0-4-8]. [5:0		<u>1-2</u> 0-15], [14:0-2-8,0-1-0],	. [16:0-2-8.0		0-0				
										_	
Loading TCLL (roof)	(psf) 20.0	Spacing Plate Grip DOL	2-0-0 1.15	CSI TC	0.72 Vert		in (loc) 21 9-23	l/defl >999	L/d 240	PLATES MT20	GRIP 244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.83 Vert	. ,	39 9-10		180		211/100
TCDL BCLL	10.0 0.0*	Rep Stress Incr Code	YES IRC2018/TPI2014	WB Matrix-MSH	0.42 Horz	2(CT) 0.	07 6	n/a	n/a		
BCDL	10.0	Code	162010/1112014	Wattix-WiSt 1						Weight: 261 lb	FT = 20%
LUMBER			3) TCLL ASC	E 7-16; Pr=20.0 psf (r	oofll·lum	DOI =1 15					
TOP CHORD	2x6 SP No.2		Plate DOL:	=1.15); Pf=20.0 psf (Lu	Im DOL=1.1	5 Plate					
BOT CHORD	2x6 SP No.2 *Excep 2.0E, 12-13:2x4 SP	ot* 11-8:2x6 SP 2400F No.2	= DOL=1.15) Cs=1.00; C	; Is=1.0; Rough Cat B; tt=1.10	Fully Exp.;	Ce=0.9;					
WEBS	2x4 SP No.3 *Excep	ot* 10-4,9-4:2x4 SP N	, ,	d snow loads have bee	en consider	ed for this					
WEDGE	Left: 2x4 SP No.3 Right: 2x4 SP No.3			has been designed for							
BRACING			overbonge) psf or 1.00 times flat non-concurrent with ot			l				
TOP CHORD	Structural wood she 3-3-12 oc purlins.	athing directly applied		unit load placed on th							
BOT CHORD	Rigid ceiling directly	applied or 10-0-0 oc		d, supported at two po has been designed for							
REACTIONS (bracing. size) 2=0-3-8, (6=0-3-8	chord live l	oad nonconcurrent with	h any other	live loads.					
Ň	Max Horiz 2=155 (LO	C 14)		has been designed fo om chord in all areas w							
	/lax Uplift 2=-56 (LC /lax Grav 2=1795 (I	C 14), 6=-56 (LC 15) LC 3), 6=1795 (LC 3)	3-06-00 tal	by 2-00-00 wide will fi	it between t	he bottom					
FORCES	(lb) - Maximum Com			any other members, wi Simpson Strong-Tie c		iu.upst.					11.5
TOP CHORD	Tension 1-2=0/14 2-4=-297(0/130, 4-6=-2970/130	recommen	ded to connect truss to	bearing wa					UNUL CA	Partie
	6-7=0/14		and does r	it(s) 2 and 6. This conr ot consider lateral forc		i apint only			NI'	RTHUR	Cliff
BOT CHORD WEBS	,	10=0/1757, 6-9=-93/2 =-37/1189, 3-10=-588		s designed in accordar al Residential Code se				1		OFES	Dist
	5-9=-588/326			and referenced standa				3	P	ier /	12/1
NOTES	I roof live leade have	been considered for	LOAD CASE(S	Standard						SEA	
, this design.								Ξ		0363	• –
	E 7-16; Vult=130mph nph: TCDI =6 0psf: B	n (3-second gust) CDL=6.0psf; h=25ft; 0	Cat					11	3		- j E
II; Exp B; Er	nclosed; MWFRS (er	nvelope) exterior zone)					1		N.E.	Riki
		-3-2, Interior (1) 3-3-2 2-9-8, Interior (1) 22-9							15	A GIN	EF. PAN
to 34-8-14,	Exterior(2E) 34-8-14	to 38-6-6 zone;						the state of the s	1	1, A. C	ALBEIN
		; end vertical left and and forces & MWFRS									11111.
for reactions	s shown; Lumber DC									Jun	e 16,2025
DOL=1.60											
				REFERENCE PAGE MII-747 rs shown, and is for an indivi						ENGINEE	
a truss sys	stem. Before use, the build	ling designer must verify the	applicability of design para	neters and properly incorpor I members only. Additional to	ate this design	into the overal	1			i ke	NLU
is always r	equired for stability and to	prevent collapse with poss n and bracing of trusses an	ible personal injury and prop	erty damage. For general gu	uidance regard	ing the	-	www.thingt.o	(ra)		A MiTek Affiliate

a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent oulzes with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria and DSB-22** available from Truss Plate Institute (www.tpinst.org) and **BCSI Building Component Safety Information** available from the Structural Building Component Association (www.sbcacomponents.com)

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