ABBREVIATIONS INDEX Visit Andrew Visit							-
Production MODEL HAYDEN Vol Machine 1 Justice Vol Machine 1 Justice <	ABBREVI	ATIONS	IND	ΕX			
Adv Actionaries 1 Lines Model: Marcle N Adv Actionaries 0 TILE 9/EET / CoVRE 9/EET IX Root FLAN K' Adv Actionaries 0 Adv Actionaries 0 Adv Actionaries Adv Actionaries 0 Adv Actionaries 0 Adv Actionaries Adv Actionaries 10 Adv Actionaries 0 Adv Actionaries Adv Actionaries 10 Adv Actionaries 0 Adv Actionaries Adv Actionaries 10 Adv Actionaries 10 Adv Actionaries Adv Actionaries 10 Adv Actionaries 20 Adv Actionaries Adv Actionaries 10 Adv Actionaries 20 Adv Actionaries Adv Actionaries 10 Adv Actionaries 20 Adv Actionaries Adv Actionaries 20 Adv Actionaries 20 Adv Actionaries Adv Actionaries 20 Ad	ABV ABVNE						
PROJECT INFORMATION 2.2 F SIDE AND REAR ELEVATIONS 'B'- W BASEMENT 3 SW R STEM WALL PLAN 'R' ALL CONSTRUCTION TO COMPLY WITH LOCAL CODES AND ORDINANCES CURRENTLY IN USE WITH THE LOCAL JURISDICTION. 3 MS F MONOLITHIC SLAB PLAN 'F' 3 BS R BASEMENT PLAN 'R' APPLICABLE CODES: FOLLOW ALL APPLICABLE STATE AND LOCAL CODES. 2018 NORTH CAROLINA STATE SUPPLEMENTS AND AMENDMENTS 3 CS F CRAML SPACE PLAN 'R' 3 BS F BASEMENT PLAN 'F' 4 R IST FLOOR PLAN 'R' 3 BS F BASEMENT PLAN 'F' 5 R 2010 FLOOR PLAN 'R' 3 BS F BASEMENT PLAN 'F' 4 R IST FLOOR PLAN 'R' 3 BS F BASEMENT PLAN 'F' 5 R 2010 FLOOR PLAN 'R' 4 F IST FLOOR PLAN 'F' 1 A S BUILDING SECTIONS 5 F 2ND FLOOR PLAN 'F' 1.1 A S BUILDING SECTIONS 1.1.2 A S BUILDING SECTIONS 1.1.2 A S BUILDING SECTIONS 1.1.3 A S BUILDING SECTIONS 1.1.3 A S BUILDING SECTIONS	ABY ABOVE ABY ABOVE AD, ARE CANDITIONING AD, AREA DRAIN AD, ADJSTABLE AJ, ADJSTABLE ALT ALTERNATE ALT ALTERNATE BASE ALTERNATE BASE ALTERNATE BASE ALTERNATE BASE ALTERNATE ALTERN	L LENGTH LANDRY LA LANDRY LAN LANATORY LAN LANATORY LAN LANATORY LAN LANATORY LAN LANATORY MAX MAXMAM MEDI HECHANACA MAXMAM MAXMAM HECH HECHANACA MAXMAM MAXMAM HECH HECHANACA MAXMAM MAXMAM HECH HECHANACA MAXMAM PR PARALLE PR PARALE	MODEL 'I O O.I O.2 I A I.I A 2.A 3 M5 A 3 C5 A 3 B5 A 4 A 5 A I B I.I B 2.B 2.1 B 2.2 B 3 M5 B 3 C5 C 3 C5	HAYDEN' TITLE SHEET / COVER SHEET QUICK VIEW QUICK VIEW FRONT ELEVATION 'A' SIDE AND REAR ELEVATIONS 'A' SIDE AND REAR ELEVATIONS 'A' W CRAWL SPACE SIDE AND REAR ELEVATIONS 'A'- W CRAWL SPACE SIDE AND REAR ELEVATIONS 'A'- W BASEMENT MONOLITHIC SLAB PLAN 'A' STEM WALL PLAN 'A' IST FLOOR PLAN 'A' SIDE AND REAR ELEVATIONS 'B'- W CRAWL SPACE PLAN 'A' SIDE AND REAR ELEVATIONS 'B'- W CRAWL SPACE SIDE AND REAR ELEVATIONS 'B'- W GRAWL SPACE SIDE AND REAR ELEVATIONS 'B'- W GRAWL SPACE SIDE AND REAR ELEVATIONS 'B'- W DASEMENT MONOLITHIC SLAB PLAN 'B' STEM WALL PLAN 'B' STEM FACE PLAN 'B' STEM WALL PLAN 'B' STEM WAL PLAN 'B'	I.I K 2K 2.I K 2.2 K 3 SW K 3 CS K 3 SW K 3 CS K 4 K 5 K 1 P 1.I P 2.I P 2.1 P 2.2 P 3 SW P 3 SW P 5 P 1 R 1.I R 2.I R 2.I R	ROOF PLAN 'K' SIDE AND REAR ELEVATIONS 'K' W CRANL SPACE SIDE AND REAR ELEVATIONS 'K'- W GRANL SPACE SIDE AND REAR ELEVATIONS 'K'- W BASEMENT MONOLITHIC SLAB PLAN 'K' STEM WALL PLAN 'K' BASEMENT PLAN 'K' IST FLOOR PLAN 'K' 2ND FLOOR PLAN 'K' SIDE AND REAR ELEVATIONS 'P' SIDE AND REAR ELEVATIONS 'P' W GRAVL SPACE SIDE AND REAR ELEVATIONS 'P' W GRAVL SPACE SIDE AND REAR ELEVATIONS 'P' W GRAVL SPACE SIDE AND REAR ELEVATIONS 'P' W BASEMENT MONOLITHIC SLAB PLAN 'P' STEM WALL PLAN 'P' STEM WALL PLAN 'P' GRAWL SPACE SIDE AND REAR ELEVATIONS 'P' W BASEMENT MONOLITHIC SLAB PLAN 'P' STEM WALL SPACE PLAN 'R' SIDE AND REAR ELEVATIONS 'R' SIDE AND REAR ELEVATIONS 'R'- W CRAWL SPACE SIDE AND REAR ELEVATIONS 'R'- W CRAWL SPACE SIDE AND REAR ELEVATIONS 'R'-	
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PRODUCT: 6 BASEMENT UTILITY PLAN SINGLE FAMILY RESIDENCE 7 IST FLOOR UTILITY PLAN OCCUPANCY CLASSIFICATION 8 2ND FLOOR UTILITY PLAN RESIDENTIAL R-3 76 ARCHITECTURAL SHEETS	SINGLE FAMILY RESIDENCE				7 8	IST FLOOR UTILITY PLAN 2ND FLOOR UTILITY PLAN	
Residential R-3 76 ARCHITECTURAL SHEETS construction type: ALL CONSULTANT DRAWINGS ACCOMPANYING THESE ARCHITECTURAL DRAWINGS HAVE NOT BEEN TYPE VB ALL CONSULTANT DRAWINGS ACCOMPANYING THESE ARCHITECTURAL DRAWINGS HAVE NOT BEEN PREPARED BY OR UNDER THE DIRECTION OF GMD DESIGN GROUP, INC. AMD DESIGN GROUP INC. THEREFORE ASSUMES NO. LIABILITY FOR THE COMPLETENESS OR CORRECTINESS OF THESE DRAWINGS. THESE DRAWINGS.	CONSTRUCTION TYPE:		PREPARED B	Y OR UNDER THE DIRECTION OF GMD DESIGN	ITECTURAL DRAI GROUP, INC. GMI	WINGS HAVE NOT BEEN D DESIGN GROUP INC.	=

-XYKFC 40'MODEL - HAYE

Mason Ridg Lot 15 163 Fair Ch Spring Lake

JILDING CODE COMPLIANCE / ROJECT INFORMATION	2.2 F	W CRAWL SPACE SIDE AND REAR ELEVATIONS 'B'-	3 SW R	MONOLITHIC SLAB PLAN 'R' STEM WALL PLAN 'R'	PLAN	CHANGES:		
L CONSTRUCTION TO COMPLY WITH LOCAL CODES AND ORDINANCES	2 MC F	W/ BASEMENT MONOLITHIC SLAB PLAN 'F'	3 65 R	CRAWL SPACE PLAN 'R'	DATE:	DESCRIPTION:		
RRENTLY IN USE WITH THE LOCAL JURISDICTION.	3 MS F 3 SW F 3 CS F	STEM WALL PLAN 'F' CRAWL SPACE PLAN 'F'	3 BS R 4 R 5 R	BASEMENT PLAN 'R' IST FLOOR PLAN 'R' 2ND FLOOR PLAN 'R'	02.22.21 03.10.21 04.14.21	INITIAL PLAN RELEASE CLIENT REVISIONS CLIENT REVISIONS		
8 NORTH CAROLINA STATE SUPPLEMENTS AND AMENDMENTS	3 BS F 4 F	BASEMENT PLAN 'F' IST FLOOR PLAN 'F'	IAS	BUILDING SECTIONS	04.15.21 12.03.21 01.26.22	CLIENT REVISIONS CLIENT REVISIONS CLIENT REVISIONS		
NTRACTOR AND BUILDER SHALL REVIEW ENTIRE PLAN TO VERIEY (FORMANCE WITH ALL CURRENT APPLICABLE CODES IN EFFECT AT TIME OF ISTRUCTION BY USING THESE DRAWINGS FOR CONSTRUCTION IT IS RESTOOD THAT CONFORMANCE WITH ALL APPLICABLE CODES IS THE PRONSIBILITY OF THE BUILDER AND CONTRACTOR.	5 F	2ND FL <i>OO</i> R PLAN 'F'		BUILDING SECTIONS BUILDING SECTIONS BUILDING SECTIONS	04.25.22 08.08.22	ADDED LIGHT OVER TUB/S STUDY ILO BEDOOM 4 - REC		
ODUCT:			6	BASEMENT UTILITY PLAN	CON	ISULTANI	FS:	
SINGLE FAMILY RESIDENCE CUPANCY CLASSIFICATION RESIDENTIAL R-3			7 8 76	IST FLOOR UTILITY PLAN 2ND FLOOR UTILITY PLAN ARCHITECTURAL SHEETS				
NGTRUCTION TYPE: TYPE VB	PREPARED B	TANT DRAWINGS ACCOMPANYING THESE ARCHIT BY OR UNDER THE DIRECTION OF GMD DESIGN & ASSUMES NO LIABILITY FOR THE COMPLETENES	GROUP, INC. GME	DESIGN GROUP INC.				
	1							

GENERAL NOTES DESIGNER NORTH CAROLINA:

THESE DOCUMENTS ARE THE PROPERTY OF THE DESIGNER AND SHALL NOT BE COPIED, PROVIDE BLOCKING AND/OR BACKING AT ALL TOMEL BAR, TOMEL RING AND/OR DUPLICATED, ALTERED, MODIFIED OR REVISED IN ANY WAY WITHOUT THE EXPRESSED WRITTEN APPROVAL OF THE DESIGNER.

CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE AND ALL INCONSISTENCES SHALL BE BROUGHT TO THE ATTENTION OF THE DEVELOPER AND THE DESIGNER BEFORE PROCEEDING WITH WORK.

ANY ERRORS OR OMISSIONS FOUND IN THESE DRAWINGS SHALL BE BROUGHT TO

DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED

ALL DIMENSIONS ARE TO FACE OF STUD OR TO FACE OF FRAMING UNLESS

ALL TRUES DRAWINGS TO BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO ISSUANCE OF BUILDING PERMIT.

ALL OR EQUAL SUBSTITUTIONS MUST BE SUBMITTED TO AND APPROVED BY CITY BUILDING OFFICIAL PRIOR TO INSTALLATION.

ALL ANGLED PARTITIONS ARE 45 DEGREES UNLESS OTHERWISE NOTED.

PROVIDE FIREBLOCKING. (PER LOCAL CODES.)

ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY.

TOILET PAPER HOLDER LOCATIONS, AS SHOWN PER PLAN. TYPICAL AT ALL BATHROOMS AND POWDER ROOMS. VERIFY LOCATIONS AT FRAMING WALK.

ELASTOMERIC SHEET WATERPROOFING. FIRMISH AND INSTALL ALL WATERPROOFING COMPLETE. A 40 MIL. SELF-ADHERING MEMBRANE OF RUBBERIZED ASPHALT INTEGRALLY BONDED TO POLYETINEW SHEETING, OR EQUAL. INSTALL PER MANUFACTURE'S AND TRADE ASSOCIATIONS PRINTED INSTALLATION INSTRUCTIONS. 6" MINIMUM LAP AT ALL ADJACENT WALL SURFACES.

TO THE BEST OF THE DESIGNER'S KNONLEDGE THESE DOCUMENTS ARE IN CONFORMANCE WITH THE REQUIREMENTS OF THE BUILDING AUTHORITIES HAVING JURISDICTION OVER THIS TYPE OF CONSTRUCTION AND OCCUPANCY.

SHOP DRAWING REVIEW AND DISTRIBUSTION, ALONG WITH PRODUCT SUBMITTALS, REQUESTED IN THE CONSTRUCTION DOCUMENTS, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR, UNLESS DIRECTED OTHERWISE UNDER A SEPARATE AGREEMENT.

DEVIATIONS FROM THESE DOCUMENTS IN THE CONSTRUCTION PHASE SHALL BE REVIEWED BY THE DESIGNER AND THE OWNER PRIOR TO THE START OF WORK IN QUESTION. ANY DEVIATIONS FROM THESE DOCUMENTS WITHOUT PRIOR REVIEW, SHALL BE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR.

THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR ALL WORK AND MATERIALS REPRESENTED ON THESE DOCUMENTS INCLUDING T MATERIALS FURNISHED BY SUBCONTRACTORS AND VENDORS.

THE BUILDER SHALL FURNISH ANY AND ALL REPORTS RECEIVED FROM THE

GEOTECHNICAL ENGINEER (SOILS REPORT), ON THE STUDY OF THE PROPOSED SITE, TO THE DESIGNER, STRUCTURAL ENGINEER, AND GENERAL CONTRACTOR. IN THE THE DELIGIEST ON A DEPORTS DO NOT EXIST, THE SOLLS CONDITION SHALL BE ASSUMED TO BE A MINIMUM DESIGN SOL DRESSURE STATED BY THE STRUCTURAL ENGINEER OF RECORD FOR THE PURPOSE OF STRUCTURAL DESIGN. GENERAL CONTRACTOR SHALL ASSURE THE SOIL CONDITIONS MEET OR EXCEED HE CRITERIA

ALL WORK PERFORMED BY THE GENERAL CONTRACTOR SHALL COMPLY AND

CONFORM WITH LOCAL AND STATE BUILDING CODES, ORDINANCES AND REGULATIONS ALONG WITH ALL OTHER AUTHORITIES HAVING JURISDICTION. THE GENERAL CONTRACTOR IS RESPONSIBLE TO BE AVARE OF THESE REQUIREMENTS AND GOVERNING REGULATIONS

PROVIDE AN APPROVED WASHER DRAIN PAN AT SECOND FLOOR ONLY THAT DRAINS TO EXTERIOR.

WINDOW SUPPLIER TO VERIFY AT LEAST ONE WINDOW IN ALL BEDROOMS TO HAVE A CLEAR OPENABLE AREA OF 4.0 SQ FT. THE MINIMUM NET CLEAR OPENING HEIGHT SHALL BE 22" AND THE MINIMUM NET CLEAR OPENING MIDTH SHALL BE 20", GLAZING TOTAL AREA OF NOT LESS THAN 5.0 SQ FT IN THE CASE OF A GROUND WINDOW AND NOT LESS THAN 5.T SQ FT IN THE CASE OF AN UPPER STORY WINDOW. (PER NORC SECTION R3IO.1.) ALL HANDRAIL BALLUSTERS TO BE SPACED SUCH THAT A 4" SPHERE CANNOT PASS BETWEEN BALLUSTERS (PER LOCAL CODES) PROVIDE STAIR HANDRAILS AND GUARDRAILS PER LOCAL CODES.

THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BUILDER'S SET" THE SCOPE OF THIS SET OF PLANS IS TO PROVIDE A "BUILDER'S SET" OF CONSTRUCTION DOCUMENTS AND GENERAL NOTES HEREINAFTER REFERRED TO AS "PLANS". THIS SET OF PLANS IS SUFFICIENT TO OBTAIN A BUILDING PERMIT; HOMEVER, ALL MATERIALS AND METHODS OF CONSTRUCTION NECESSARY TO COMPLETE THE PROJECT ARE NOT NECESSARILY DESCRIBED. THE PLANS DELINEATE AND DESCRIBE ONLY LOCATIONS, DIMENSIONS, TYPES OF MATERIALS, AND GENERAL METHODS OF ASSEMBLING OR FASTENING. THEY ARE NOT INTENDED. TO SPECIFY PARTICULAR PRODUCTS OR OTHER METHODS OF ANY SPECIFIC MATERIALS, PRODUCT OR METHOD, THE IMPLEMENTATION OF THE PLANS REQUIRES A CLIENT / CONTRACTOR THOROUGHLY KNOWLEDGEABLE WITT THE APPLICABLE BUILDING CODES AND METHODS OF CONSTRUCTION SPECIFIC TO THIS PRODUCT TYPE AND TYPE OF CONSTRUCTION.

CONSTRUCTION REQUIREMENTS AND QUALITY: PROVIDE WORK OF THE SPECIFIC QUALITY: WHERE QUALITY LEVEL IS NOT INDICATED, PROVIDE WORK OF QUALITY CUSTOMARY IN SIMILAR TYPES OF WORK. WHERE THE PLANS AND SPECIFICATIONS, CODES, LAWS, REGULATIONS, WITH THE MOST STRINGENT REQUIREMENT; WHERE REQUIREMENTS ARE DIFFERENT BUT APPARENTLY EQUAL, AND WHERE IT IS UNCERTAIN WHICH REQUIREMENT IS MOST STRINGENT, OBTAIN CLARIFICATION FROM THE GMD DESIGN GROUP BEFORE PROCEEDING

BUILDER SET:

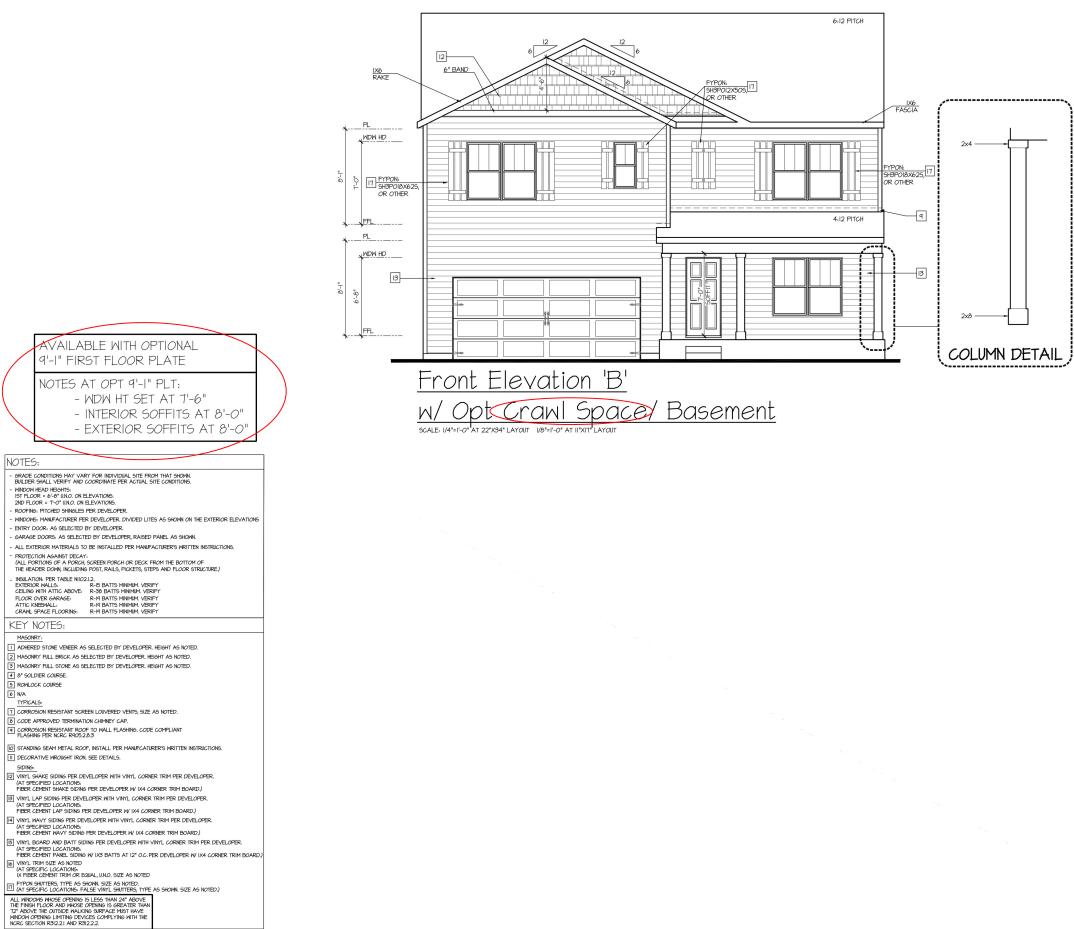
S HOMES ERIES N 4 BR - LH e	NOr DATE: REVISION: ▲ 042522
ild Road , NC 28390	PROFESSIONAL SEAL: PROJECT TITLE: 40' Series
	FOR CONSTRUCTION
MODEL 'HAYDEN' SQUARE FOOTAGES AREA ELEV 'B' Ist FLOOR Int FLOOR <	CLIENTS NAME: FROJECT NO: GMD/7049 SHEET TITLE: TITLE SHEET PRINT DATE:
	January 22, 2021 SHEET NO: O

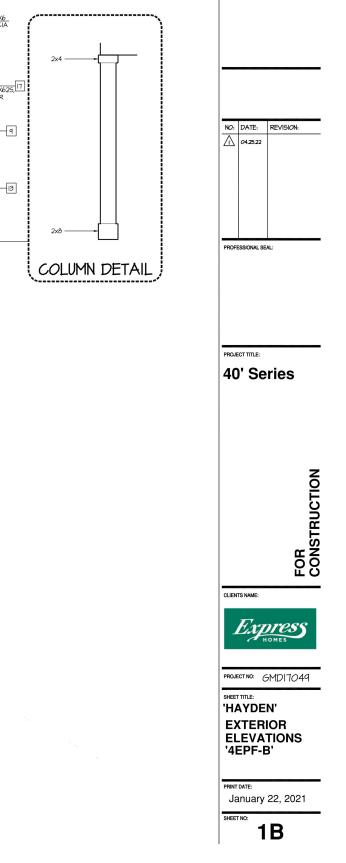




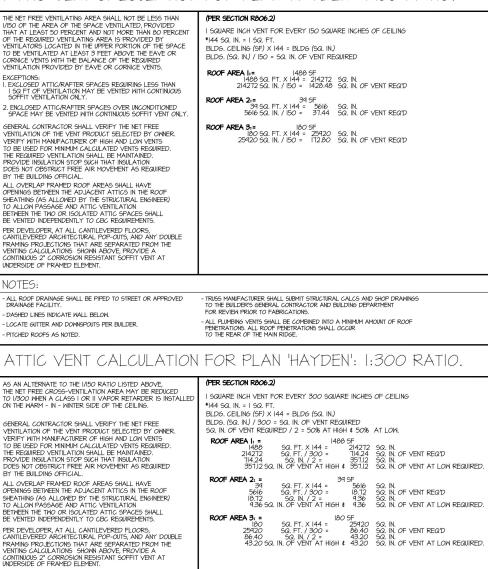


NO: DATE:	REVISION:
PROFESSIONAL SE	AL:
PROJECT TITLE: 40' Se	ries
	UCTION
CLIENTS NAME:	FOR CONSTRUCTION
	MDIT049
H	omes 5MD17049



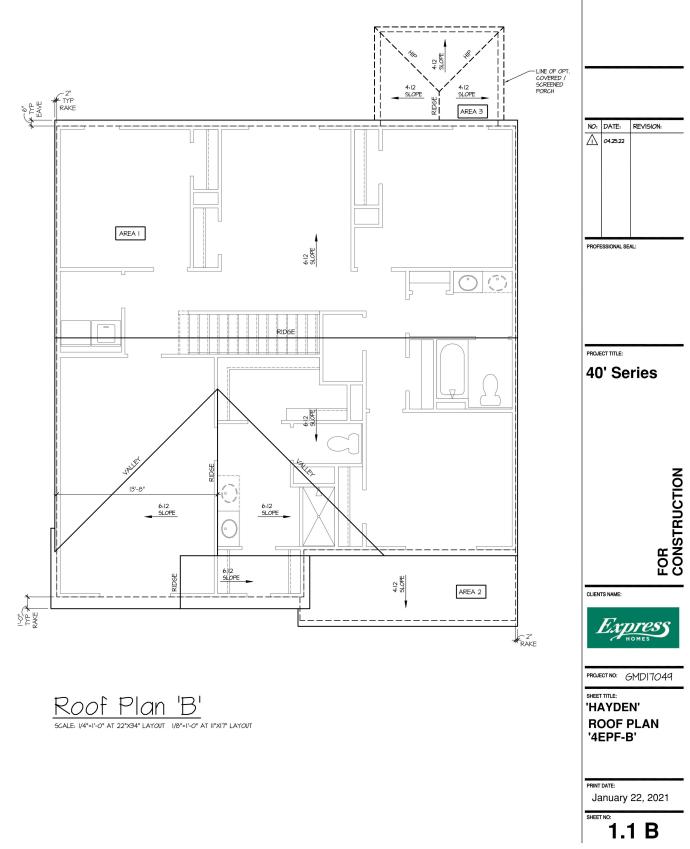


ATTIC VENT CALCULATION FOR PLAN 'HAYDEN': 1:150 RATIO.

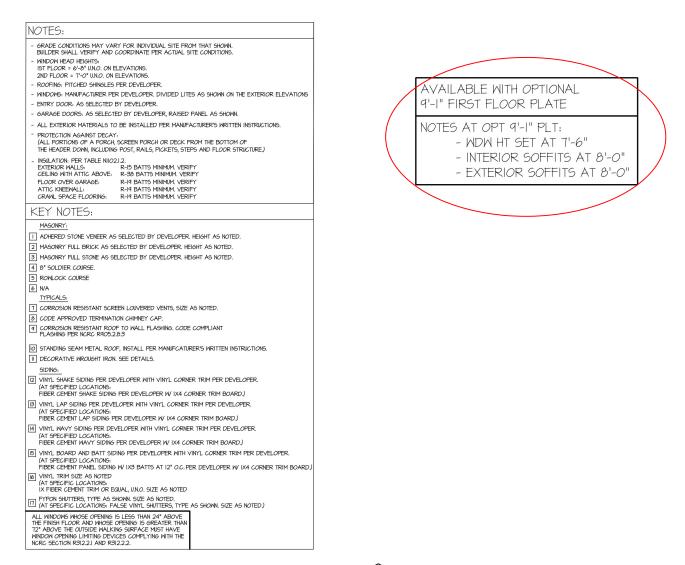


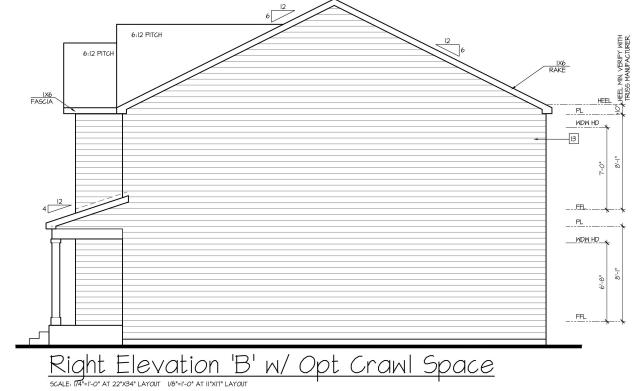
BUILDER TO PROVIDE (2) LAYERS OF UNDERLAYMENT AT ANY ROOF W/ A SLOPE FROM 2:12 TO LESS THAN 4:12

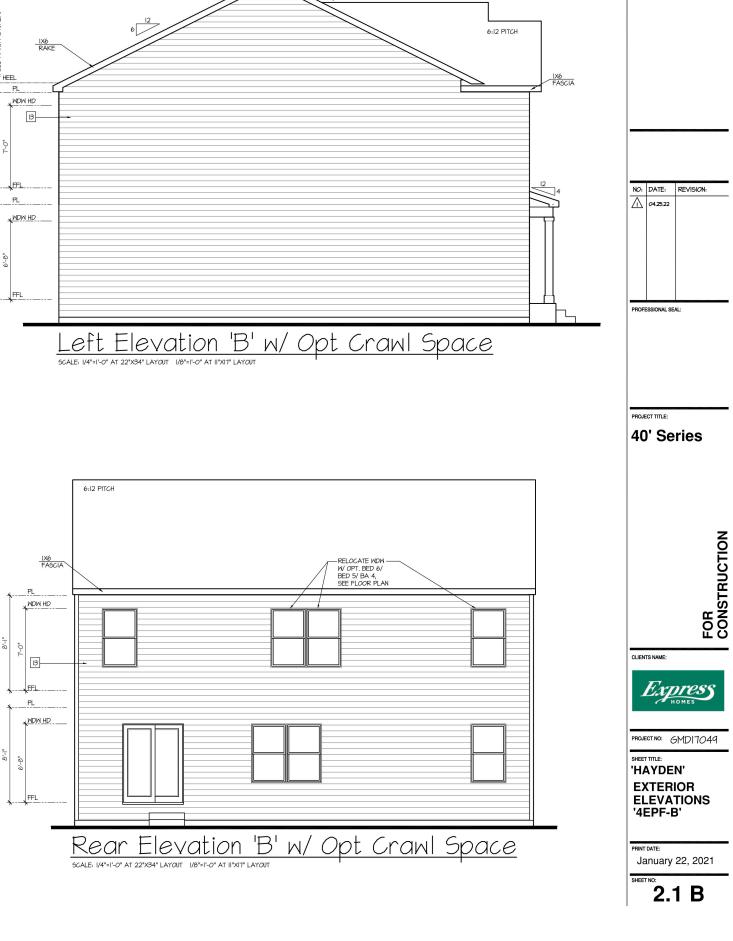
> AT SINGLE FAMILY DETACHED PLANS: PREFINISHED VENTED SOFFIT AT EAVE PER MANUFACTURER. (VERIFY FIRE SEPARATION DISTANCE FOR SOFFIT PROTECTION PER NCRC SECTION R302.1.1 AND TABLE R302.1)



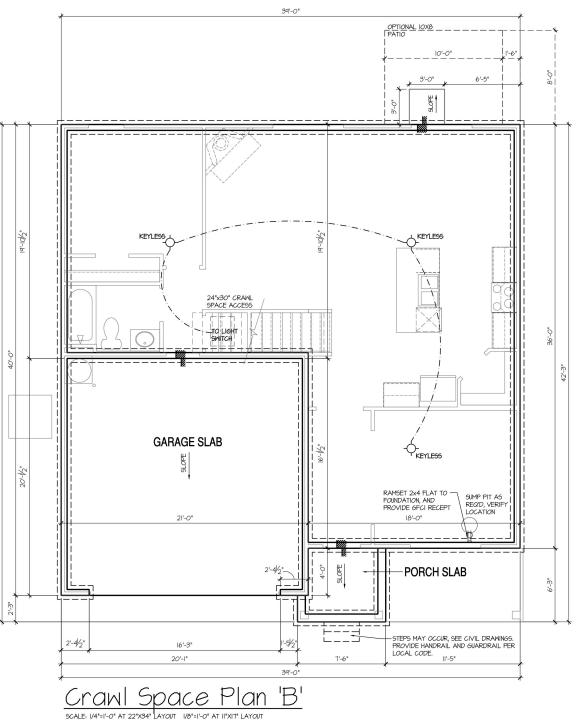








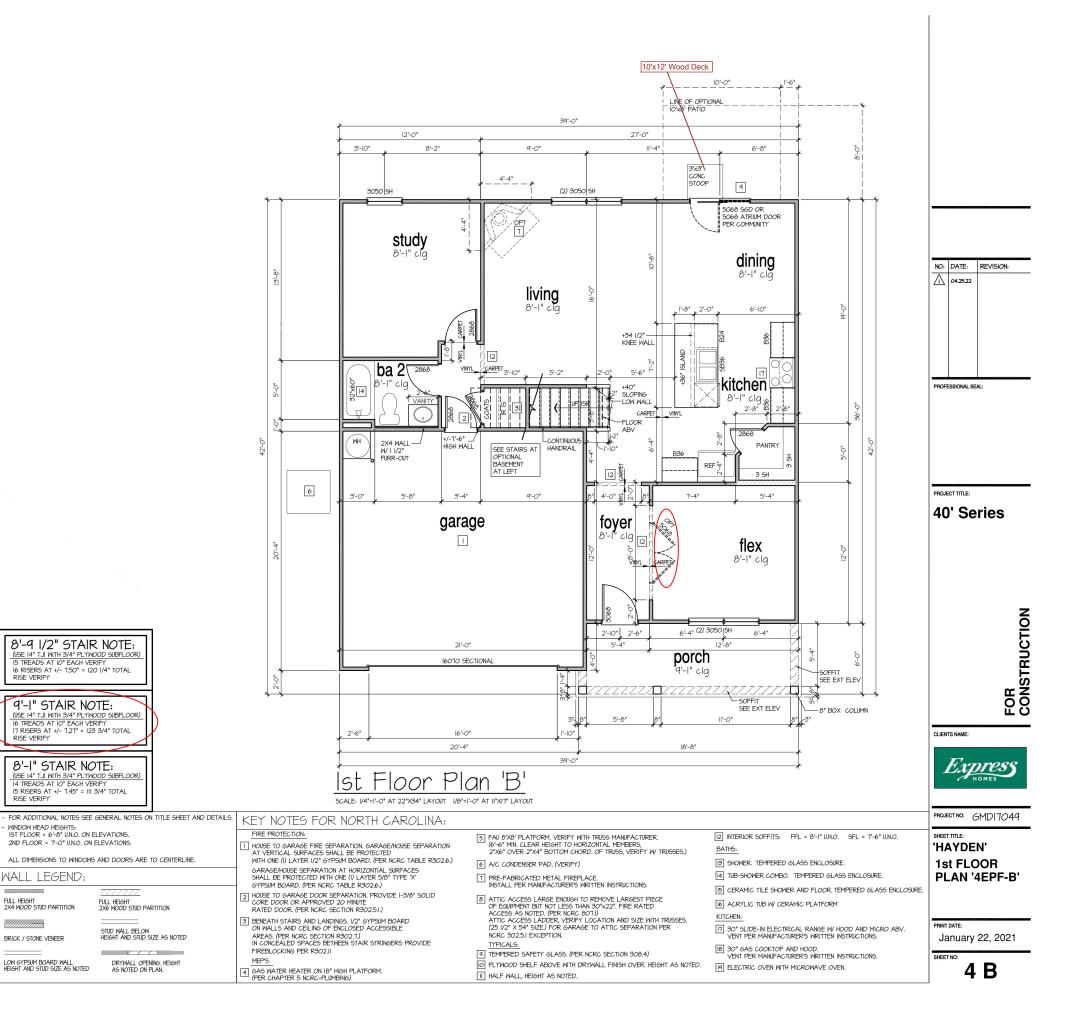


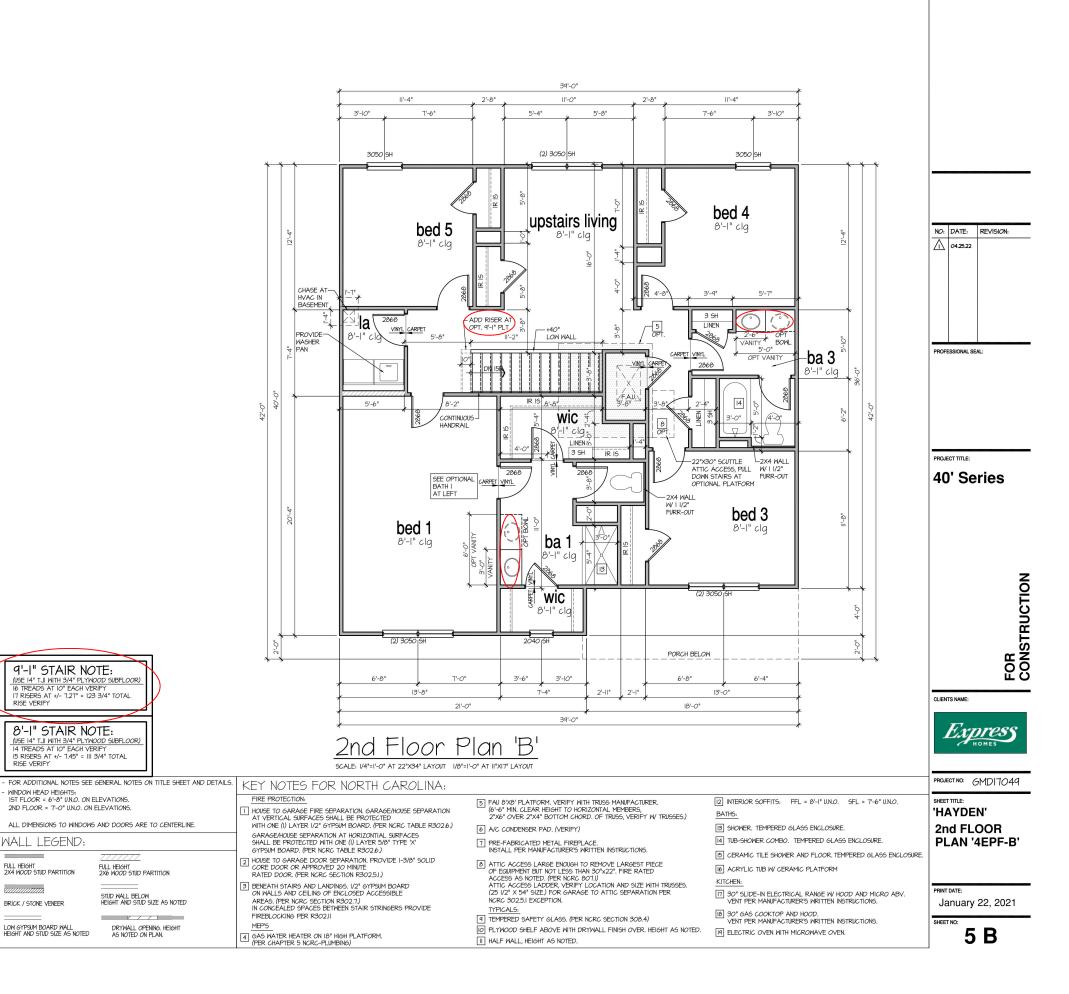


CRAWL SPACE NOTES NORTH CAROLINA:	KEY NOTES:
 REFER TO STRUCTURAL DRAWINGS FOR INFORMATION NOT SHOWN ON THIS PLAN. FOR ADDITIONAL NOTES SEATINGS FOR INFORMATION NOT SHOWN ON THIS PLAN. FOR ADDITIONAL NOTES SEATINGS (FER LOCAL CODES) ALL ELECTRICAL AND MECHANICAL EQUIPMENT AND METERS ARE SUBJECT TO RELOCATION DUE TO FIELD CONDITIONS, CONTRACTOR TO VERIFY. VERIFY ALL DOOR THRESHOLD HEIGHTS TO HARD SURFACES. J 1/4' MAX AT INSVING DOORS, (FER NCRC SECTION RSIIJ.)] SLOPE ALL STOOPS AND HARDSCAFE MATERIAL AWAY FROM BUILDING - TYPICAL. SLOPE GARAGE FLOOR 1/8' PER FOOT TO GARAGE DOOR OPENING. VERIFY CURB CUT BLOCKOUT WITH GARAGE DOOR MANUFACTURER. REFER TO CIVIL DRAWINGS FOR FINISH SURFACE ELEVATIONS. TYP STOOP AT INSVING FOR FINISH SURFACE ELEVATIONS. TYP STOOP AT INSVING FOR FINISH SURFACE ELEVATIONS. SOILS TREAMMENT DE APPLIED TO FRAMING PER PRODUCT SPECIFICATIONS. (PROVIDE CHEMICAL TREAMPENT FOR PROTECTION FROM TERMITE INVESTATION ACCORDING TO LOCAL CODES) AT VERTED CRAML SPACE: APPLY AN APPROVED VAPOR RETARDER OR EQUIVALENT, 6 MIL POLY-VINTL, GROUND COVER OVER FINISH GRADE OR CRAWL SPACE PER NCRO SECTION 4062. PROVIDE VENTS SPACED ARQUED PERIMETER TO PROMOTE CROSS VENTILATION AT A RATE OF 19 SYENT FOR EVERT 'EXOL SPACE FER NCRC SECTION ACCC. PROVIDE VENTS SPACED ARQUED PERIMETER TO PROMOTE CROSS VENTILATION AT A RATE OF 19 SYENT FOR EVERT 'EXOL SPACE FER NCRC SECTION ACAS. PROVIDE VENTS SPACED ARQUED PERIMETER TO PROMOTE CROSS VENTILATION AT A RATE OF 19 SYENT FOR EVERT 'EXOL SPACE FER NCRC SECTION ACAS. PROVIDE VANG RROSS VENTILATION. (FER NCRC SECTION ROBUL SPACE PER NCRC SECTION ACAS. ONE VENT MIST BE LOCATED WITH HIS -0'O'RE CHER OF THE BUILDING AND LOCATED TO ALLOW FOR CROSS VENTILATION. PROVIDE AN ACCESS OPENING, MINIMM SIZE OF IB'X24' FOR CRAML ACCESES COORDINATE WITH MECHANICAL CONTRACTOR FOR LARGER SIZE TEQU	I. INE OF SLAB ABOVE I. INE OF SLAB ABOVE I. INE OF FRAMED NALL ABOVE I. INE OF FRAMED NALL ABOVE I. INE OF FRAME PALL ABOVE I. INE OF STATE VENT ACCONDENSER PAD. (VERTY) TYPICAL CRANL FOUNDATION WALL SHALL BE &' CMU OR A COMBINATION OF 4" CMU WITH NOMINAL 4" BRICK. SEE STRUCTURAL DRAWINGS FOR ALL STRUCTURAL ATTACHMENTS ALL BLOCK CELLS AND SPACE BETHEETN BLOCK CHLS AND SPACE BETHEETN BLOCK AND BRICK TIE SPACING. FILL VOIDS SOLID TO TOP OF CMU WALL. (MICT COMPLY MITH NORG SECTION RE404, TABLE RE404.1.1(1) THROUGH RA40.1.1(2) AND APPLICABLE SECTIONS OF REGO, RE60, RE60, INCOMPLY WITH STRUCTURAL DRAWINGS FOR WALL FOOTING SIZE AND DEPTH.

- SEPARATION TO GRADE SHALL BE PRESSURE TREATED OR FOUNDATION GRADE REDWOOD. SET ALL EXTERIOR WALL SILLS IN MASTIC.

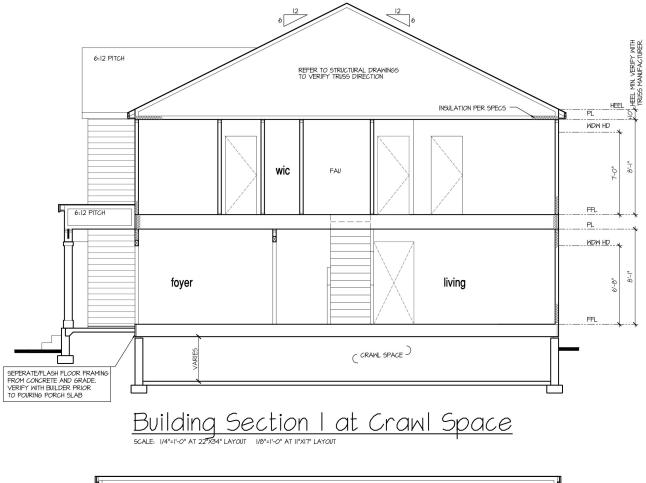


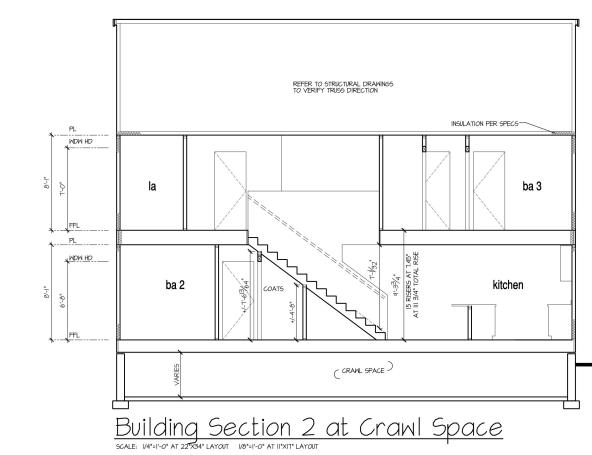




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BRICK / STONE VENEER

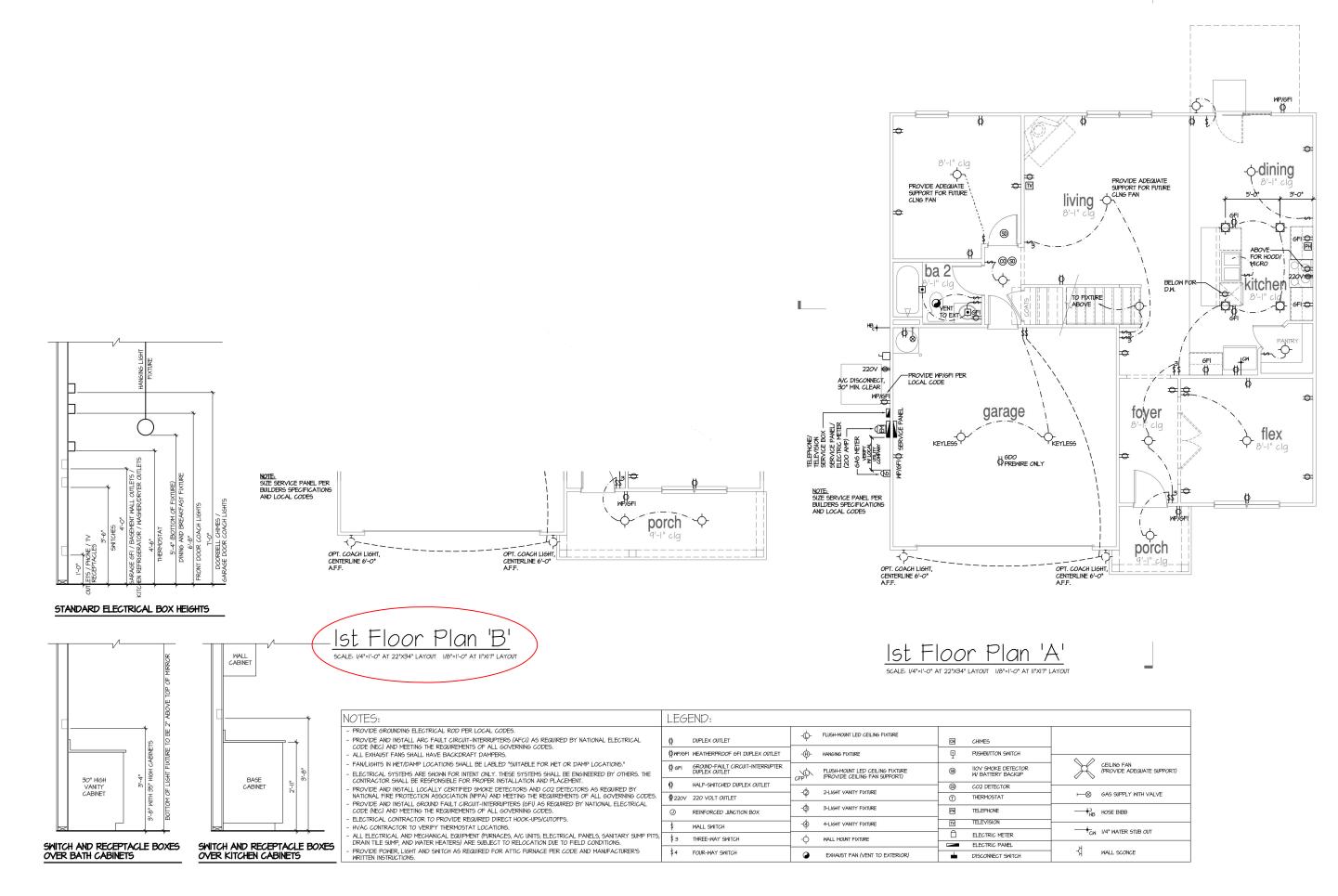




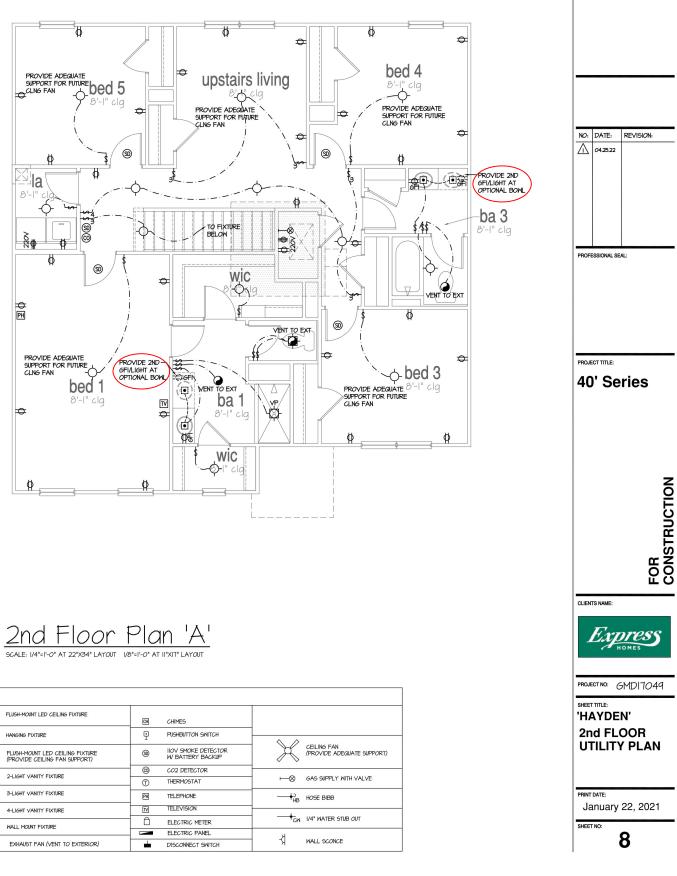
9'-1" STAIR NOTE: (USE 14" TJI WITH 3/4" PLYWOOD SUBFLOOR) 16 TREADS AT 10" EACH VERIFY 17 RISERS AT 1/- 7.27" = 123 3/4" TOTAL RISE VERIFY	
8'-1" STAIR NOTE: USE 14" TJI WITH 34" PLYWOOD SUBFLOOR) 14 TREADS AT 10" EACH VERIPY 15 RISERS AT 1/- 7.45" = 111 3/4" TOTAL RISE VERIPY	

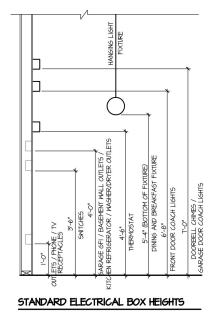
- REFER TO FLOOR PLAN NO	TES FOR TYPICAL FIRE PROTECTION NOTES AND LOG	CATIONS.				
 THESE BUILDING SECTIONS MAY VARY AT ALTERNATE ELEVATION STYLES AND AT "PLAN OPTION" CONDITIONS, REFER TO MAIN FLOOR PLAN AND ALTERNATE FLOOR PLANG FOR INFORMATION NOT SHOWN HERE. 						
 BUILDING SECTIONS SHOWN HERE DEPICT VOLUMN SPACES WITHIN THE STRUCTURE. REFER TO STRUCTURAL DRAWINGS, TRUSS DRAWINGS, STRUCTURAL DETAILS AND CALCULATIONS BY OTHER FOR ALL STRUCTURAL INFO. 						
- ROOFING: PITCHED SHINGLE	ROOF. REFER TO ROOF PLAN FOR TYPICALS.					
- WOOD FLOORS: FLOOR SHE REFER TO STRUCTURAL AND	ATHING OVER FLOOR JOIST. 7 TRUSS DRAWINGS BY OTHERS.					
 VERIFY STAIRS MINIMUM AN WITH LOCAL CODES. INSULATION: 	D MAXIMUM REQUIREMENTS FOR CONSTRUCTION CLE	ARANCES				
	R-13 BATTS MINIMUM, VERIFY R-15 BATTS MINIMUM, VERIFY					
CEILING WITH ATTIC ABOVE						
CEILING WITH ATTIC ABOVE	R-38 BATTS MINIMUM. VERIFY UNCOMPRESSED INSULATION (HEELS IN TRUSSES): R-30 BATTS MINIMUM. VERIFY	PER STATE RESIDENTIAL COL COMPLIANCE METHOD TO BE DETERMINED BY BUILDER.				
FLOOR OVER GARAGE:	R-19 BATTS MINIMUM. VERIFY					
ATTIC KNEEWALL:	R-19 BATTS MINIMUM. VERIFY					
CRAWL SPACE FLOORING:	R-19 BATTS MINIMUM, VERIFY					

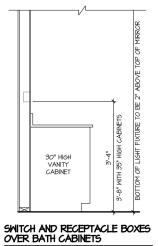


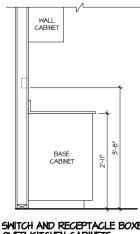




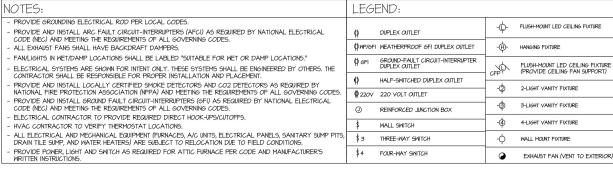








SWITCH AND RECEPTACLE BOXES OVER KITCHEN CABINETS



					SHEET LIS	ът.		
	DESIGN SPECIFICATIONS:							Description
	Construction Type: Commerical 🗌 Residential 🛛				Sheet		С	over Sheet, Specifications, Revisions
	Applicable Building Codes:				S1.4			Monolithic Slab Foundation
	 2018 North Carolina Residential Building Code with All Loca ASCE 1-10: Minimum Design Loads for Buildings and Other St 				91.0 51.0	-		Stem Wall Foundation Crawl Space Foundation
	2 2				51.2			Basement Foundation
	Design Loads: 1. Roof Live Loads				5 2.	-		Basement Framing Plan
	I.I. Conventional 2x 20 PSF I.2. Truss 20 PSF		SUN	ΙΜΙΤ	63. 54	-		First Floor Framing Plan Second Floor Framing Plan
	1.2.1. Attic Truss 60 PSF				54.	-		Roof Framing Plan
	2. Roof Dead Loads 2.1. Conventional 2x 10 PSF		ENGINEERING LA	BORATORT TESTING	56			Basement Bracing Plan
	2.2. Truss 20 PSF 3. Snow 15 PSF				61.	-		First Floor Bracing Plan
	3.1. Importance Factor				58	0		Second Floor Bracing Plan
	4. Floor Live Loads 4.1. Typ. Dwelling			DEN LH				
	4.2. Sleeping Areas							
	4.3. Decks 40 PSF 4.4. Passenger Garage 50 PSF		PROJECT ADDRESS:	OUNER:	REVISION	LIST:		
	5. Floor Dead Loads 5.1. Conventional 2x 10 P6F		TBD	DR Horton, Inc. 8001 Arrowridge Blvd.				
	5.2. I-Joist			Charlotte, NC 28273	Revision No.	Date	Project No.	Description
	5.3. Floor Truss		DESIGNER;		1	4,19,21	TØ177	Updated elevation names
	6.1. Exposure		GMD Design Group					Added Stem Wall, Crawlspace, and Basement
	6.2. Importance Factor		102 Fountain Brook Circle Suite C		2	6,14,21	TØ177	Foundations Added OX-15 option and table for framing
	6.3.1. Vx = 6.3.2. Vy =		Cary, NC 27511		3	11.23.21	TØ177	Updated framing in the first floor
	7. Component and Cladding (in PSF)			d with the architectural, mechanical, plumbing,				
	MEAN ROOF UP TO 30' 30'1"-35' 35'1"-40' 44	0'1"-45'	electrical, and civil drawings. This co	ordination is not the responsibility of the		-		
	BG	8,-202	structural engineering of record (SER apparent, the contractor shall notify S). Should any discrepancies become UMMIT Engineering, Laboratory 4 Testing,				
		8,-23.6	P.C. before construction begins.					
		8,-23.6						
		0.4,-21.3 14,-26.9	PLAN ABBREVIATIONS:					
	ZONE 5 162,-24,0 152,-252 153,-262 20	.4,-20.3	AB ANCHOR BOLT	PT PRESSURE TREATED				
	8. Seismic		CJ CEILING JOIST	3C STUD COLUMN		-		
	8.1. Site Class E 82. Design Category C		CLR CLEAR	SJ SINGLE JOIST				
	83. Importance Factor	2	DJ DOUBLE JOIST	SPF SPRUCE PINE FIR				
	8.5. Spectral Response Acceleration		DSP DOUBLE STUD POCKET EE EACH END	SGT SIMPSON STRONG-TIE SYP SOUTHERN YELLOW PINE				
	8.5.1. Sms = %g 8.5.2. Sml = %g			TJ TRIPLE JOIST				
	8.6. Seismic Base Shear		NTS NOT TO SCALE	TSP TRIPLE STUD POCKET				
	8.6.1. Vx = 8.6.2. Vy =		OC ON CENTER					
	8.7. Basic Structural System (check one)		PSF POUNDS PER SQUARE FOOT PSI POUNDS PER SQUARE INCH	UNO UNLESS NOTED OTHERWISE				
	⊠ Bearing Wall □ Building Frame							
	□ Moment Frame □ Dual w/ Special Moment Frame			d their corresponding loading details,				
	Dual w/ Intermediate R/C or Special Stee	el		ering, Laboratory & Testing, P.C. (SUMMIT) truss and joist directions were assumed				
	□ Inverted Pendulum 8.8. Arch/Mech Components Anchored	0	based on the information provided b	y <u>DR Horton, Inc</u> , Subsequent plan				
	8.9. Lateral Design Control: Seismic 🗌 Wind 🖂 9. Assumed Soil Bearing Capacity		revisions based on roof truss and flo revision list, indicating the date the l	ayouts were provided. Should any				
		000000	discrepancies become apparent, the	contractor shall notify SUMMIT immediately.				
]				
GENERAL STRUCTURAL NOTES: 1. The design professional whose seal appears on these drawings	The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However,		-grade shall be constructed in accordance : "Guide for Concrete Slab and Slab	 Where reinforcing dowels are required , the in size and spacing to the vertical reinforce 			WOOD TRUS	3 <u>3553:</u> bod truss manufacturer/fabricator is responsible for the
is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements	the bottom of all footings shall be a minimum of 12" below grade.	Construction".		shall extend 48 bar diameters vertically an into the footing			desigr	n of the wood trusses. Submit sealed shop drawings and
and the performance of this structure. No other party may revise,	 Any fill shall be placed under the direction or recommendation of a licensed professional engineer. 		on-grade has been designed using a of k=250 pci and a design loading of 200	 Into the footing, IO. Where reinforcing steel is required vertical 	illy, dowels shall	be		rting calculations to the SER for review prior to attain at the series of the series o
alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering.	 The resulting soil shall be compacted to a minimum of 95% maximum dry density. 		t responsible for differential settlement, slab uture defects resulting from unreported	provided unless otherwise noted.	•			. The review by the SER shall review for overall ance with the design documents. The SER shall assume no
Laboratory & Testing, P.C. (SUMMIT) or the SER. For the	Excavations of footings shall be lined temporarily with a 6 mil	conditions not in a	ccordance with the above assumptions.	WOOD FRAMING:			respor	nsibility for the correctness for the structural design for
purposes of these construction documents the SER and SUMMIT shall be considered the same entity,	polyethylene membrane if placement of concrete does not occur within 24 hours of excavation,		joints shall be spaced in interior a maximum of 15'-0" O.C. and in exterior	 Solid sawn wood framing members shall con specifications listed in the latest edition of 				nod trusses. Tood trusses shall be designed for all required loadings
 The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction 	 No concrete shall be placed against any subgrade containing water, ice, frost, or loose material. 		a maximum of 10'-0" unless otherwise noted. joints shall be produced using conventional	Design Specification for Wood Construction otherwise noted, all wood framing members				ecified in the local building code, the ASCE Standard m Design Loads for Buildings and Other Structures."
to stabilize the structure.		process within 4 to	12 hours after the slab has been finished	Southern-Yellow-Pine (SYP) #2.	2		(ASCE	: 7-10), and the loading requirements shown on these
 The SER is not responsible for construction sequences, methods, or techniques in connection with the construction of this 	<u>STRUCTURAL STEEL:</u> 1. Structural steel shall be fabricated and erected in accordance		nay not extend through a control joint. nay extend through a saw cut joint.	 LVL or PSL engineered wood shall have t design values: 	he following min	imum		ications. The truss drawings shall be coordinated with all construction documents and provisions provided for
structure. The SER will not be held responsible for the	with the American Institute of Steel Construction "Code of	10. All welded wire fab	pric (WWF.) for concrete slabs-on-grade shall	2.1. E = 1,900,000 psi			loads	shown on these drawings including but not limited to
contractor's failure to conform to the contract documents, should any non-conformities occur.	Standard Practice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design"	supported during t	depth of slab. The W.W.F. shall be securely the concrete pour.	2.2. Fb = 2600 psi 2.3. Fv = 285 psi			HVAC the tru	equipment, piping, and architectural fixtures attached to usses.
 Any structural elements or details not fully developed on the construction drawings shall be completed under the direction of 	latest editions. 2. Structural steel shall receive one coat of shop applied	CONCRETE REINFORCED	MENIT	2.4.Fc = 700 psi 3. Wood in contact with concrete, masonry, or	مما الدماء ماندم			usses shall be designed, fabricated, and erected in dance with the latest edition of the "National Design
a licensed professional engineer. These shop drawings shall be	rust-inhibitive paint.	1. Fibrous concrete re	einforcement, or fibermesh, specified in	pressure treated in accordance with AWPA	standard C-15		Specif	ication for Wood Construction." (NDS) and "Design"
submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it	 All steel shall have a minimum yield stress (Fy) of 36 ksi unless otherwise noted. 		grade may be used for control of cracking nd thermal expansion/contraction, lowered	other moisture exposed wood shall be tre with AWPA standard C-2	ated in accord	ance		"ication for Metal Plate Connected Wood Trusses." uss manufacturer shall provide adequate bracing
relates to the structural design of this project. Verification of	4. Welding shall conform to the latest edition of the American	water migration, an	increase in impact capacity, increased	4. Nails shall be common wire nails unless othe		a1	ínforma	ation in accordance with "Commentary and
the shop drawings for dimensions, or for actual field conditions, is not the responsibility of the SER or SUMMIT.	Welding Society's Structural Welding Code AWS DI.I. Electrodes for shop and field welding shall be class ETØXX. All welding		e, and residual strength. ng to be 100% virgin polypropylene fibers	 Lag screws shall conform to ANSI/ASME st. Lead holes for lag screws shall be in according to the state of the screws shall be in according to the state of the st				mendations for Handling, Installing, and Bracing Metal Connected Wood Trusses" (HIB-91). This bracing, both
 Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for 	shall be performed by a certified welder per the above standards.	containing no repro	ocessed olefin materials and specifically se as concrete secondary reinforcement.	specifications. 6. All beams shall have full bearing on suppo			tempor	rary and permanent, shall be shown on the shop drawings. he shop drawings shall show the required attachments for
accuracy and report any discrepancies to SUMMIT before		3. Application of fibe	ermesh per cubic yard of concrete shall equal	unless otherwise noted.	· ·		the tru	16565.
construction begins. 6. The SER is not responsible for any secondary structural elements	<u>CONCRETE:</u> 1. Concrete shall have a normal weight aggregate and a minimum		y volume (1.5 pounds per cubic yard) ply with ASTM CIII6, any local building code	 Exterior and load bearing stud walls are t O.C. unless otherwise noted. Studs shall be 				pords or truss webs shown on these drawings have been as a reference only. The final design of the trusses shall
or non-structural elements, except for the elements specifically	compressive strength (f'c) at 28 days of 3000 psi, unless	requirements, and s	hall meet or exceed the current industry	sole plate to the double top plate. Stud	s shall only be			r the manufacturer.
noted on the structural drawings. 7. This structure and all construction shall conform to all	otherwise noted on the plan. 2. Concrete shall be proportioned, mixed, and placed in		ars shall be new billet steel conforming to	discontinuous at headers for window/door of one king stud shall be placed at each			EXTERIOR	WOOD FRAMED DECKS:
applicable sections of the international residential code. 8. This structure and all construction shall conform to all	accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301:	ASTM A615, grade		King studs shall be continuous. 8. Individual studs forming a column shall be			1. Decks	are to be framed in accordance with local building and as referenced on the structural plans, either through
applicable sections of local building codes.	"Specifications for Structural Concrete for Buildings".	be in accordance u	uith the latest edition of ACI 315: "Manual of	nail @ 6" O.C. staggered. The stud column	shall be continu			references or construction details.
 All structural assemblies are to meet or exceed to requirements of the current local building code. 	 Air entrained concrete must be used for all structural elements exposed to freeze/thaw cycles and deicing chemicals. Air 		for Detailing Concrete Structures" and wall reinforcement shall be continuous	to the foundation or beam. The column shall blocked at all floor levels to ensure prop		.	WOOD STRL	ICTURAL PANELS:
FOUNDATIONS:	entrainment amounts (in percent) shall be within -1% to +2% of target values as follows:	and shall have 90°	bends, or corner bars with the same a horizontal reinforcement with a class B	 Multi-ply beams shall have each ply attach 24" OC. 			1. Fabrica	ation and placement of structural wood sheathing shall be ordance with the APA Design/Construction Guide
I. The structural engineer has not performed a subsurface	3.1. Footings: 5%	tension splice.		10. Four and five ply beams shall be bolted to			"Resid	lential and Commercial," and all other applicable APA
investigation. Verification of this assumed value is the	3.2. Exterior Slabs: 5%	1 8. Lap reinforcement	as required, a minimum of 40 bar diameters	of 1/2" diameter through bolts staggered	≈ 16" O.C. unless	· 11	standa	Iras.

Ine structural engineer has not performed a subsurace investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse solic condition be encountered the SER must be contacted before proceeding.

3.1. Footings: 5% 3.2. Exterior Slabs: 5% 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masorry shall be a minimum of 40 bar diameters.
- row and rive piy beams shall be bolted together with (2) ro of 1/2" diameter through bolts staggered & 16" O.C. unless noted otherwise.

- Association and commercial, and all over applicable Ar A standards. All structurally required wood sheathing shall bear the mark of the APA.

Manager	
	Signature
Operations	
Operations System	
Operations	

	3.	Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information. Sheathing shall be applied with the long direction
	4.	perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure I or 2. Roof sheathing shall be continuous over two supports and attached to its supporting roof framing with (1)-8d CC nail at 6°/0/c at perie edges and at 12°/0/c inpart field unless
		cheruise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of plyucod clips or lumber blocking unless otherwise noted. Panel end joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
		Wood floor sheathing shall be APA rated sheathing exposure I or 2. Attach sheathing to its supporting framing with (1)-8d CC ringshark hail at 6°0/c at panel edges and at 12°0/c in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing, sheathing shall have a span rating consistent with the framing spacing. Use suitable edge support by use of 14G plywood or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code.
	6.	Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.
	STR	JCTURAL FIBERBOARD PANELS:
	1.	Fabrication and placement of structural fiberboard sheathing
		shall be in accordance with the applicable AFA standards.
	2.	All structurally required fiberboard sheathing shall bear the mark of the AFA.
	3.	Teberbard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
1	1.	

Sheathing shall have a 1/8" gap at panel ends and edges are recommended in accordance with the AFA.



ENGLOCEMENTAL MEMOLING OTHER ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

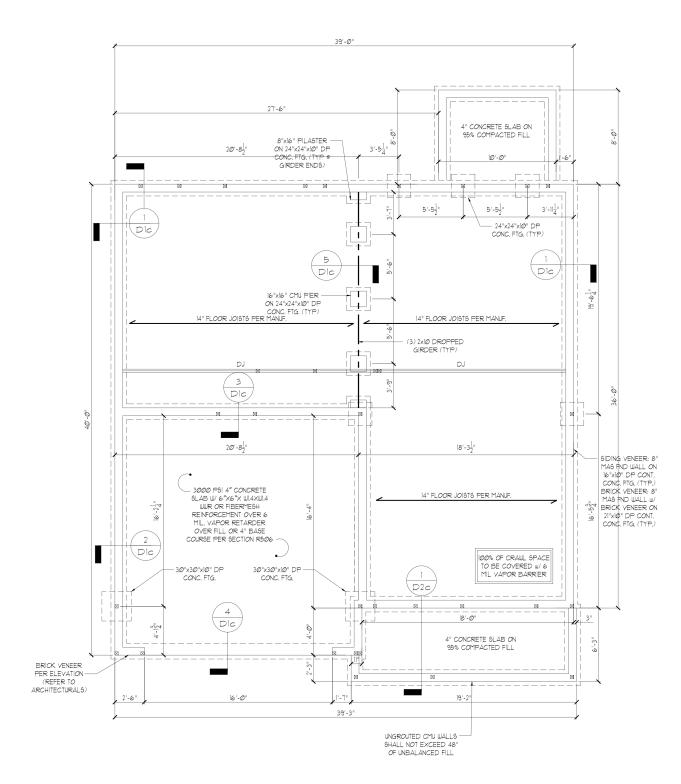
STRUCTURAL MEMBERS ONLY

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

 CRAWL
 SPACE
 FOUNDATION
 PLAN

 SCALE:
 14":61"-0"
 ON 12":54" OR 12":51"-0"
 ON 11":51"

<u>ELEVATION B.F.K.</u>





		REQUIRED	BRACED W	ALL PANEL CONNEC	CTIONS			
				REQUIRED CONNECTION				
	METHOD	MATERIAL	MIN, THICKNESS	© PANEL EDGES	© INTERMEDIATE SUPPORTS			
	CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS © 6" O.C.	6d COMMON NAILS @ 12" O.C.			
	GB	GYPSUM BOARD	1/2=	5d COOLER NAILS** @ 1ª O.C.	5d COOLER NAILS** @ 7" O.C.			
	WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS @ 12" O.C.			
	PF	WOOD STRUCTURAL PANEL 1/16"		PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4			
- [#OR EQUIVALENT PER TABLE R10235							

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS. WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- 130 MPH REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES 3
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED 04 FEET FOR ISOLATED PANEL METHOD AND 12 FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. 5. MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1
- THE INTERIOR SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON
- 8 ALL SHEATHABLE SUFFACTING THEO, EACHONG WALLD SHEALD BEACED WALL ALL SHEATHABLE SUFFACES INCLUDING INFILL AREAS BETUEEN BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS. FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- 9. OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A
- BRACED WALL LINE. THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A
- MASCHRY OK CONCRETE 5 IEM WALLS W/A LENGTH OF 48" OK LESS 50FPORTING A BRACED WALL PAREL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3 OF THE 2018 NCRC.
 BRACED WALL PAREL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4
 BRACED WALL PAREL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5

- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R60210.4.6
- PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.00.1 (UNO) 17. ABBREVIATIONS:

. PANEL

GB = GYPSUM BOARD	WSP = WOOD STRUCTURAL PANEL
CS-XXX = CONT. SHEATHED	ENG = ENGINEERED SOLUTION
PF = PORTAL FRAME	PF-ENG = ENG. PORTAL FRAME

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS. CONTRACTOR SHALL VERITY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC
- PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM THIS PLAN.
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO REGIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOUG: MICROLLAM (LVL): F_p = 12600 PS), F₂ = 255 PS), E = 1921/0⁶ PSI PARALLAM (PSL): F₂ = 12600 PS), F₂ = 230 PS], E = 1251/0⁶ PSI ALL WOOD MEMBERS SHALL BE ⁹ SYP UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE ⁹ SYP (UNO). ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 ⁹ SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERUISE. ALL REINFORCING STEEL SHALL BE GRADE 6/0 BARS CONFORMING TO ASTM AGE AND GUILUI UNGE A NUMBER COURD CO 3¹¹

- ASTM A615 AND SHALL HAVE A MINIMUM COVER OF 3". 8. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- PERFENDICULAR 10 RAFLERS. FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH I/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL I/D3f. 9. MINE EDGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED
 MINIMUM 6" FROM EACH END OF THE BEAM.
 ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP ¹2,
- DROPPED FOR NON-LOAD BEARING HEADERS SCHEED & 3'-0' IN WIDTH AND/OR WITH MORE THAN 2'-0'' OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP 12, DROPPED. (UNLESS NOTED OTHERWISE)

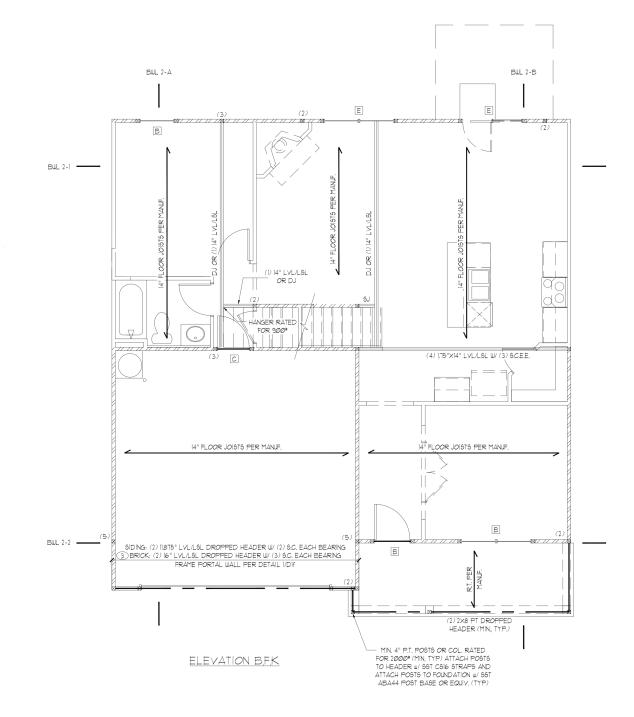
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANG PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>4/15/21</u>, IT 16 THE RESPONSIBILITY OF THE CLIENT TO NOTIEY SUMMIT ENGINEERING LABORATORY & TESTING P.C. IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY & TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

FIRST FLOOR FRAMING PLAN SCALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"



FIRST FLOOR BRACING (FT)				
CONTINUOUS SHEATHING METHOD				
	REQUIRED	PROVIDED		
BWL 1-1	11.6	24.8		
BWL 1-2	11.6	15 <i>.0</i>		
BWL I-A	11.3	40.0		
BWL 1-B	11.3	36 <i>.</i> Ø		

1	EADER SCHED	V – –
TAG	SIZE	JACKS (EACH END)
Д	(2) 2x6	(1)
в	(2) 2x8	(2)
С	(2) 2x1Ø	(2)
D E	(2) 2x12	(2)
	(2) 9-1/4" LSL/LVL	(3)
F	(3) 2x6	(1)
G	(3) 2x8	(2)
Н	(3) 2x1Ø	(2)
1	(3) 2x12	(2)

HEADER SIZES SHOWN ON PLANS ARE MINIMUMS. GREATER HEADER SIZES MAY BE USED FOR EASE OF CONSTRUCTION. 2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD COLUMNS LISTED ABOVE (U.N.O.),

KING STUD SCHEDULE MAXIMUM HEADER SPAN MINIMUM KING STUDS E.E. 4'-Ø 8'-0" 10'-0" 12'-Ø" 4'-0 16'-0 18'-0"

WALL STUD SCHEDULE (10 FT HEIGHT) ATUD AITE ATUD ARACING (OC

STUD SIZE		STUD SPACING (D.C.)				
	ROOF ONLY	R00F & 1 FL00R	ROOF & 2 FLOORS	NON-LOAD BEARING		
2×4	24"	16"	12"	24"		
2×6	24"	24"	6"	24"		
NOTES:						

1. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3. TWO STORY WALLS SHALL BE FRAMED W/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED W/ HORIZ. BLOCKING @ 6'-Ø" O.C. VERTICALLY.

LINTEL SCHEDULE				
TAG: SIZE Image: Description of the state of t		OPENING SIZE		
		LESS THAN 6'-0"		
2	L5x3x1/4"	6'-0" TO 10'-0"		
3 L5x3-1/2x5/16"		GREATER THAN 10'-0"		
4	ALL ARCHED OPENINGS			
SECURE LINTEL TO HEADER W/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED @ 16" O.C. (TYP FOR) ALL HEADERS WHERE BRICK 15 USED, TO BE: (UNO)				

SHADED WALLS INDICATED LOAD BEARING WALLS

NOTE: REDUCE JOIST SPACING UNDER TILE FLOORS, GRANITE COUNTERTOPS AND/OR ISLANDS.

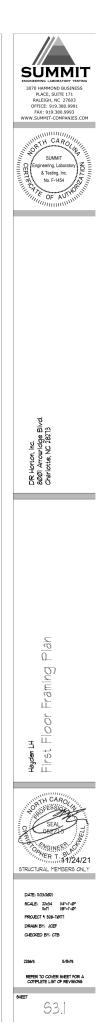
JOIST & BEAM SIZES SHOLN ARE MINIMUMS BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: ____ DESIGNATES JOIST SUPPORTED LOAD BEARING WALL ABOVE. PROVIDE BLOCKING UNDER JOIST SUPPORTED LOAD BEARING WALL.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE! THE ENTIRETY OF THE MEMBER 15 WRAPPED TO PREVENT MOISTURE INTRUSION.

INSTALL HOLD-DOWNS FOR BRACED WALL END CONDITIONS PER SECTION R602.0.8 # FIG. R602.0.1 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R602.3.5 OF THE 2018 NCRC.



REQUIRED BRACED WALL PANEL CONNECTIONS				CTIONS	
				REQUIRED (CONNECTION
	METHOD	MATERIAL	MIN. THICKNESS	© PANEL EDGES	© INTERMEDIATE SUPPORTS
	CS-WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS © 6" O.C.	6d COMMON NAILS © 12" O.C.
	GB	GYPSUM BOARD	1/2"	5d COOLER NAILS** ⊕ 1≣ O.C.	5d COOLER NAILS** @ 7" O.C.
	WSP	WOOD STRUCTURAL PANEL	3/8"	6d COMMON NAILS @ 6" O.C.	6d COMMON NAILS # 12" O.C.
	PF	WOOD STRUCTURAL PANEL	7/16"	PER FIGURE R602.10.6.4	PER FIGURE R602.10.6.4
"OR EQUIVALENT PER TABLE R10235					

BRACED WALL NOTES:

- WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R602.10 FROM THE 2018 NORTH CAROLINA RESIDENTIAL CODE WITH ALL LOCAL AND STATE AMENDMENTS WALLS ARE DESIGNED FOR SEISMIC ZONES A-C AND MAXIMUM WIND SPEEDS UP TO
- 130 MPH REFER TO ARCHITECTURAL PLAN FOR DOOR/WINDOW OPENING SIZES 3
- BRACING MATERIALS, METHODS AND FASTENERS SHALL BE IN ACCORDANCE WITH TABLE R602.10.1
- 5. ALL BRACED WALL PANELS SHALL BE FULL WALL HEIGHT AND SHALL NOT EXCEED WE FEET FOR ISOLATED PANEL METHOD AND IS FEET FOR CONTINUOUS SHEATHING METHOD WITHOUT ADDITIONAL ENGINEERING CALCULATIONS.
- MINIMUM PANEL LENGTH SHALL BE PER TABLE R602.10.1
- THE INTERCE LEATE OFFICE DE LET LET AND BOLL ROBAND. THE INTERCE SIDE OF EXTERIOR WALLS AND BOTH SIDES OF INTERIOR WALLS SHALL BE SHEATHED CONTINUOUSLY WITH MINIMUM 1/2" GYPSUM BOARD (UNO). FOR CONTINUOUS SHEATHING METHOD, EXTERIOR WALLS SHALL BE SHEATHED ON 8 ALL SHEATHABLE SUFFACES NCLLDNG INTERIOR WALLS SHEATHED ON ALL SHEATHABLE SUFFACES NCLLDNG INFILL AREAS BETLERB BRACED WALL PANELS, ABOVE AND BELOW WALL OPENINGS, AND ON GABLE END WALLS.
 FLOORS SHALL NOT BE CANTILEVERED MORE THAN 24" BEYOND THE FOUNDATION
- OR BEARING WALL BELOW WITHOUT ADDITIONAL ENGINEERING CALCULATIONS. 10. A BRACED WALL PANEL SHALL BE LOCATED WITHIN 12 FEET OF EACH END OF A
- BRACED WALL LINE.
- THE MAXIMUM EDGE DISTANCE BETWEEN BRACED WALL PANELS SHALL NOT EXCEED 21 FEET.
- 12. MASONRY OR CONCRETE STEM WALLS W/ A LENGTH OF 48" OR LESS SUPPORTING A MASCHRY OK CONCRETE SIEM WALLS W/A LENGTH OF 48° CK LESS SUPPORTING A BRACED WALL PAREL SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.4.3 OF THE 2018 NCRC.
 BRACED WALL PAREL CONNECTIONS TO FLOOR/CEILING SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.4
 BRACED WALL PAREL CONNECTIONS TO ROOF SHALL BE CONSTRUCTED IN ACCORDANCE WITH SECTION R602.10.4.5

- CRIPPLE WALLS AND WALK OUT BASEMENT WALLS SHALL BE DESIGNED IN ACCORDANCE WITH SECTION R60210.4.6
- 16. PORTAL WALLS SHALL BE DESIGNED IN ACCORDANCE WITH FIGURE R602.10.1 (UNO) 17. ABBREVIATIONS:

PANEL TION

GB = GYPSUM BOARD	WSP = WOOD STRUCTURAL PANEL
CS-XXX = CONT, SHEATHED	ENG = ENGINEERED SOLUTION
PF = PORTAL FRAME	PF-ENG = ENG. PORTAL FRAME

GENERAL STRUCTURAL NOTES:

- CONSTRUCTION SHALL CONFORM TO 2018 NORTH CAROLINA RESIDENTIAL BUILDING CODE WITH ALL LOCAL AND STATE AMENDMENTS. CONTRACTOR SHALL VERITY ALL DIMENSIONS. CONTRACTOR SHALL COMPLY WITH THE CONTENTS OF THE DRAWING FOR THIS SPECIFIC
- PROJECT, ENGINEER IS NOT RESPONSIBLE FOR ANY DEVIATIONS FROM HIS PLAN
- 3 CONTRACTOR IS RESPONSIBLE FOR PROVIDING TEMPORARY BRACING REQUIRED TO REGIST ALL FORCES ENCOUNTERED DURING ERECTION. PROPERTIES USED IN THE DESIGN ARE AS FOLLOUS:
- PROPERTIES USED IN THE DESIGN ARE AS FOLLOUG: MICROLLAM (LVL): F₀ = 26000 PS), F₂ = 285 PS), E = 1926/⁶ PSI PARALLAM (PSL): F₂ = 3200 PSI, F₂ = 230 PSI, E = 1257/⁶ PSI ALL WOOD MEMBERS SHALL BE *2 SYP UNLESS NOTED ON PLAN. ALL STUD COLUMNS AND JOISTS SHALL BE *2 SYP (UNO). ALL BEAMS SHALL BE SUPPORTED WITH A (2) 2x4 *2 SYP STUD COLUMN AT EACH END UNLESS NOTED OTHERUISE. ALL REINFORCING STELL SHALL BE GRADE 60 BARS CONFORMING TO ATT ACH END BUILD UNCE A SIMUMA COURD COL

- ASTM AGIS AND SHALL HAVE A MINIMUM COVER OF 3". 8. CONTRACTOR TO PROVIDED LOOKOUTS WHEN CEILING JOISTS SPAN PERPENDICULAR TO RAFTERS.
- FLITCH BEAMS, 4-PLY LVLS AND 3-PLY SIDE LOADED LVLS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. THRU BOLTS SPACED AT 24" O.C. (MAX) STAGGERED OR EQUIVALENT CONNECTIONS PER DETAIL 1/D3f. MINE DEGE DISTANCE SHALL BE 2" AND (2) BOLTS SHALL BE LOCATED MINIMUM 6" FROM EACH END OF THE BEAM. 10. ALL NON-LOAD BEARING HEADERS SHALL BE (1) FLAT 2x4 SYP ¹²,
- DROPPED. FOR NON-LOAD BEARING HEADERS OLECEDING 3'-0" IN WIDTH AND/OR WITH MORE THAN 2'-0" OF CRIPPLE WALL ABOVE, SHALL BE (2) FLAT 2x4 SYP 12, DROPPED. (UNLESS NOTED OTHERWISE)

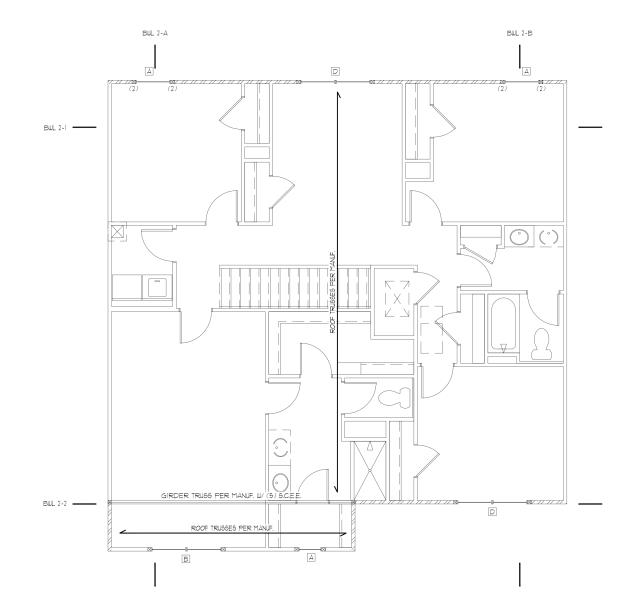
THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL, PLANG PROVIDED BY <u>DR HORTON</u> COMPLETED/REVISED ON <u>4/15/21</u>, IT 16 THE RESPONSIBILITY OF THE CLIENT TO NOTIEY SUMMIT ENGINEERING LABORATORY & TESTING PC, IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, P.C. CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

STRUCTURAL MEMBERS ONLY

ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGINEERING, LABORATORY & TESTING, P.C FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

SECOND FLOOR FRAMING PLAN SCALE: 1/4"=1'-Ø" ON 22"x34" OR 1/8"=1'-Ø" ON 11"x17"



ELEVATION B.F.K.

SECOND FLOOR BRACING (FT)				
CONTINUOUS SHEATHING METHOD				
	REQUIRED	PROVIDED		
BWL 2-1	6.0	27.0		
BWL 2-2	6.0	25.Ø		
BWL 2-A	5.8	40.0		
BWL 2-B	5.8	36.0		

TAG	SIZE	JACKS (EACH END)
Д	(2) 2x6	(1)
в	(2) 2x8	(2)
С	(2) 2x1Ø	(2)
D	(2) 2x12	(2)
E	(2) 9-1/4" L6L/LVL	(3)
F	(3) 2x6	(1)
G	(3) 2x8	(2)
Н	(3) 2x1Ø	(2)
1	(3) 2xl2	(2)

2. ALL HEADERS TO BE DROPPED (UN.O.). 3. STUD COLUMNS NOTED ON PLAN OVERRIDE STUD

COLUMNS LISTED ABOVE (UN.O.)

KING STUD SCHEDULE			
MAXIMUM HEADER SPAN	MINIMUM KING STUDS E.E.		
4'-Ø"	(D		
6'-0"	(2)		
8'-Ø"	(2)		
10'-0"	(3)		
12'-Ø"	(3)		
14'-Ø"	(3)		
16'-Ø"	(4)		
18'-0"	(4)		

WALL STUD SCHEDULE (10 FT HEIGHT)

STUD SIZE		STUD SPA	CING (0,C,)	
	ROOF ONLY	ROOF & I FLOOR	ROOF \$ 2 FLOORS	NON-LOAD BEARING
2×4	24"	16 "	12"	24"
2x6	24"	24"	16 "	24"
NOTEC				

I. BRACED WALLS STUDS SHALL BE A MAX. OF 16" O.C. 2. STUDS SUPPORTS OPTIONAL WALK-UP ATTIC SHALL BE SPACED A MAX OF 16" OC

3. TWO STORY WALLS SHALL BE FRAMED w/ 2x4 STUDS @ 12" O.C. OR 2x6 STUDS @ 16" O.C. BALLOON FRAMED w/ HORIZ. BLOCKING @ 6'-0" O.C. VERTICALLY,

LINTEL SCHEDULE				
TAG SIZE I L3x3x1/4"		OPENING SIZE		
		LESS THAN 6'-Ø"		
2	L5x3x1/4"	6'-0" TO 10'-0" GREATER THAN 10'-0"		
3	L5x3-1/2x5/16"			
4	L5x3-1/2x5/16" ROLLED OR EQUIV.	ALL ARCHED OPENINGS		
SECURE LINTEL TO HEADER w/ (2) 1/2" DIAMETER LAG SCREWS STAGGERED & 16" O.C. (TYP FOR (3))				
ALL HEADERS WHERE BRICK IS USED, TO BE: (UNO)				

SHADED WALLS INDICATED LOAD BEARING WALLS

JOIST & BEAM SIZES SHOWN ARE MINIMUMS, BUILDER MAY INCREASE DEPTH FOR EASE OF CONSTRUCTION.

NOTE: MEMBERS NOTED AS PRESSURE TREATED MAY BE FRAMED WITH NON-PRESSURE TREATED LUMBER PROVIDE THE ENTIRETY OF THE MEMBER 15 WRAPPED TO PREVENT MOISTURE INTRUSION

INSTALL HOLD-DOUNS FOR BRACED WALL END CONDITIONS PER SECTION R602.10.8 4 FIG. R602.10.7 OF THE 2018 NCRC.

NOTE: WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE CONTINUOUS WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION R60235 OF THE 2018 NCRC.



ENGLOCEMENTAL MEMOLIND OTHER ENGINEERING SEAL APPLIES ONLY TO STRUCTURAL COMPONENTS ON THIS DOCUMENT, SEAL DOES NOT INCLUDE CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR SAFETY PRECAUTIONS. ANY DEVIATIONS OR DISCREPANCIES ON PLANS ARE TO BE BROUGHT TO THE IMMEDIATE ATTENTION OF SUMMIT ENGLINEERING, LABORATORY & TESTING, P.C. FAILURE TO DO SO WILL VOID SUMMIT LIABILITY.

STRUCTURAL ANALYSIS BASED ON 2018 NCRC.

STRUCTURAL MEMBERS ONLY

ROOF FRAMING: PLAN

<u>ELEVATION B,F,K</u>

THESE PLANS ARE DESIGNED IN ACCORDANCE WITH ARCHITECTURAL PLANS PROVIDED BY <u>DR.HORTON</u> COMPLETED/REVISED ON <u>4/15/21</u> IT IS THE RESPONSIBILITY OF THE CLIENT TO NOTIFY SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC, IF ANY CHANGES ARE MADE TO THE ARCHITECTURAL PLANS PRIOR TO CONSTRUCTION. SUMMIT ENGINEERING, LABORATORY 4 TESTING, PC, CANNOT GUARANTEE THE ADEQUACY OF THESE STRUCTURAL PLANS WHEN USED WITH ARCHITECTURAL PLANS DATED DIFFERENTLY THAN THE DATE LISTED ABOVE.

NOTE: TRUSS UPLIFT LOADS SHALL BE DETERMINED PER TRUSS MANUFACTURER IN ACCORDANCE WITH SECTION REGULUI, WALL SHEATHING AND FASTENERS HAVE BEEN DESIGNED TO RESIST THE WIND UPLIFT LOAD PATH IN ACCORDANCE WITH METHOD 3 OF SECTION REG0135 OF THE 2018 NORC, REFER TO BRACED WALL PLANS FOR SHEATHING AND FASTENER REQUIREMENTS.

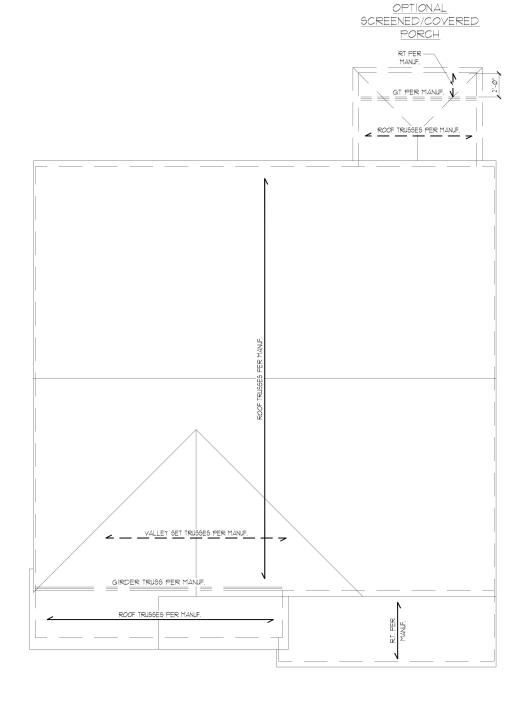
REFER TO DETAIL 5/D3F FOR EYEBROW, RETURN OR SHED ROOF FRAMING REQUIREMENTS, (TYP FOR ROOFS PROTRUDING MAXIMUM 24" FROM STRUCTURE)

NOTE: ROOF TRUSSES SHALL BE SPACED TO SUPPORT FALSE FRAMED DORMER WALLS (TYP, UNO)

NOTE: 19T PLY OF ALL SHOWN GIRDER TRUSSES TO ALIGN WITH INSIDE FACE OF WALL (TYP, UNO)

MAX, UPLIFT	ROOF TO WALL	FLOOR TO FLOOR	FLOOR TO FND	
6 <i>00</i> LBS	H2.5A	PER WALL SHEATHIN	G & FASTENERS	
12 <i>00</i> LBS	(2) H2.5A	CSI6 (END = 11")	DTT2Z	
1450 LBS	HTS2Ø	CSI6 (END = 11")	DTT2Z	
2 <i>000</i> LBS	(2) MT92Ø	(2) CSI6 (END = 11")	DTT2Z	
2900 LBS	(2) HTS2Ø	(2) CSI6 (END = 11")	HTT4	
3685 LBS	LGT3-6D62.5	MSTC52	HTT4	
 ALL PRODUCTS LIGTED ARE GIMPSON STRONG-TIE. EQUIVALENT PRODUCTS MAY BE USED FER MANUFACTURER'S SPECIFICATIONS. UPLIFT VALUES LISTED ARE FOR SYPT ⁹ GRADE MEMBERS. REFER TO TRUSS LAYOUT FER MANUF.FOR UPLIFT VALUES AND TRUSS TO TRUSS CONNECTOR'S OPECIFIED BY TRUSS MANUFACTURER OVERRIDE THOSE LISTED ABOVE. CONTACT SUMMIT FOR REQUIRED CONNECTOR'S UHEN LOADS EXCEED THOSE LISTED ABOVE. 				

TRUSS UPLIFT CONNECTOR SCHEDULE





DESIGN SPECIFICATIONS:

81. Site Class 82. Design Category ... 83. Importance Factor .

Seisnic Use Group . 8.5. Spectral Response Acceleration 85. Seismic Base Shear 861 VX = 8**6**2.Vy =

8.1. Basic Structural System (check one) ⊠ Bearing Wall □ Building Frame □ Moment Frame

8.8. Arch/Mech Components Anchored ... 8.9. Lateral Design Control: Seismic
9. Assumed Soil Bearing Capacity

 Dual w/ Special Moment Frame Dual w/ Intermediate R/C or Special Steel

> Wind 🖂 200005

8 Seismic

Construction Tupe: Commerical 🔲 Residential 🛛

Applicable Building Codes:	
 2018 North Carolina Residential 	

°• 2Ø	le Building Codes: 18 North Carolina Residential Building Code with CE 7-10: Minimum Design Loads for Buildings an	
Design L	oads:	
	Roof Live Loads	
	I.I. Conventional 2x	
	1.2. Truse	20 PSF
	12.1. Attic Truss	_ 60 PSF
2.	Roof Dead Loads	
	2.1. Conventional 2x	
	2.2. Truse	
3.	5now	15 PSF
	3.1. Importance Factor	1.0
4.	Floor Live Loads	
	4.1. Typ. Dwelling	40 PSF
	4.2. Sleeping Areas	
	4.3. Decks	
	4.4. Passenger Garage	
5.	Floor Dead Loads	
	5.1. Conventional 2x	
	52. I-Joist	15 PSF
	5.3. Floor Truss	15 PSF
6.	Ultimate Wind Speed (3 sec. gust)	, PER PL A N
	6.1. Exposure	
	6.2. Importance Factor	
	6.3. Wind Base Shear	
	63.1. Vx =	
	632.Vy =	
٦.	Component and Cladding (in PSF)	

J					
				PSF	
				K I"LAN	
		in (PSF)			
	~				
	UP TO 30'	301"-35'	351"-40	4011-45	
ZONE 1	16.1, - 18.Ø	17.5,-18.9	18.2,-19.6	18.7,-20.2	
ZONE 2	16.7,-21.Ø	17.5,-22.1	18.2,-22.9	18.1,-23.5	
ZONE 3	16.7,-21Ø	17.5,-22.1	18.2, -22.9	18.1,-23.5	
ZONE 4	18.2, - 19.Ø	19.2,-20.0	19.9,-2 0 .1	20.4,-21.3	
ZONE 5	182,-24Ø	192,-252	19.9,-26.1	20.426.9	
	Floor Live LC 41. Typ. Du 42. Sleepin 43. Decks 44. Passen Floor Dead L 51. Conver 53. Floor T Uttimate Und 63. Und 63	Floor Live Loads 41. Typ. Duelling	Floor Live Loads 41. Typ. Duelling	41. Typ. Duelling 40 42. Sleeping Areas 30 43. Decks 40 44. Passenger Garage 50 Floor Dead Loads 50 51. Conventional 2x 10 F 52. I-Jolat 15 F 53. Floor Truss 15 F 61. Exposure 10 F 63. Wind Base Shear 63. Wind Base Shear 63. Wind Base Shear 63. Vy = 63. Wind Ease Shear 63. Vy = Component and Cladding (in PSF) MEAN ROOF MEAN ROOF 115.78.8 182.78.6 ZONE I 16.1.79.00 11522.1 182.22.3 ZONE 3 16.1.200 11522.1 182.22.3	Floor Live Loads 40 PSF 41. Typ. Duelling 40 PSF 30 PSF 42. Sleeping Areas 30 PSF 30 PSF 43. Decks 40 PSF 40 PSF 44. Passenger Garage 50 PSF Floor Dead Loads 10 PSF 51. Conventional 2x 15 PSF 15 PSF 51 F-Joint 15 PSF 52. I-Joints 15 PSF 15 PSF 15 PSF 51. Loads 18 PSF 14 Passence 15 PSF 52. Indoats Speed (3 sec. gust) 18 PSF 140 16 16 6.3 100 Base Shear 6.3 100 6.3 100 6.3 100 6.3 140 115 16 15 16 17.202 17.40 40'I''-45' 17.202 20NE 1 16.1, -180

SUMMIT

SHEET LIST: Description Sheet No. CGI Cover Sheet Specifications Revision Dim Monolithic Slab Foundation Details Dla Stem Wall Foundation Details Dic Craul Space Foundation Details DЬ Basement Foundation Details DIF Framing Details

STANDARD DETAILS OUNER: DR Horton Carolinas Division

8001 Arrowridge Blvd Charlotte, NC 28213

STRUCTURAL PLANS PREPARED FOR

ARCHITECT/DESIGNER GMD Design Group 1845 Satellite Blvd Duluth GA 3009

PROJECT ADDRESS:

TBD

These drawings are to be coordinated with the architectural, mechanical, plumbing, electrical, and civil drawings. This coordination is not the responsibility of the structural engineering of record (SER). Should any discrepancies become apparent, the contractor shall notify SUMMIT Engineering, Laboratory 4 Testing, P.C. before construction begins.

PLAN ABBREVIATIONS:

AВ	ANCHOR BOLT	PŤ	PRESSURE TREATED
AFF	ABOVE FINISHED FLOOR	RS	ROOF SUPPORT
CJ	CEILING JOIST	SC	STUD COLUMN
CLR	CLEAR	5J	SINGLE JOIST
ÐJ	DOUBLE JOIST	SPF	SPRUCE PINE FIR
D5P	DOUBLE STUD POCKET	SST	SIMPSON STRONG-TIE
Ē	EACH END	S YP	SOUTHERN YELLOW PINE
EΨ	EACH WAY	ŤJ	TRIPLE JOIST
NT9	NOT TO SCALE	TSP	TRIPLE STUD POCKET
8	ON CENTER	TYP	TYPICAL
P#F	POUNDS PER SQUARE FOOT	UNO	UNLESS NOTED OTHERWISE
PSI	POUNDS PER SQUARE INCH	WWF	WELDED WIRE FABRIC

Roof truss and floor joist layouts, and their corresponding loading details, were not provided to SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) prior to the initial design. Therefore, truss and joist directions were assumed based on the information provided by <u>DR Horton. Inc.</u> Subsequent plan revisions based on roof truss and floor Joist layouts shall be noted in the revision list, indicating the date the layouts were provided. Should any discrepancies become apparent, the contractor shall notify $\$U^{HHI}$ immediately.

REVISION LIST: **Re**vision Project No. Date Description No. E IIIT Added box bay detail (2/D2f). Added deck options with basement. Revised deck options with stem wall and crawl space foundations 2 7,12,17 Revised stem wall insulation note 3 2.15.18 Revised garage door detail, NC only 4 2.28.18 Added high-wind foundation details 5 12.19.18 Revised per 2018 NCRC 6 2.19.19 Revised per Mecklenburg County Comments Revised stem wall deck attachment and i sheathing on wall sections. 8 3.6.19 Corrected dimensions at perimeter footings 9 3220 Added tall turndown detail 10 3.18.20 Added balloon framing detai Added alternate two-pour detail for slab and 102020 added note for crawl girder above grade 3121 12 Added OX-19 Standard Details 13 5.18.21 Updated OX-15 Standard Details 14 @2.14.23 Added 4/D2m - Tall Slab Detail w/ Siding

GENERAL STRUCTURAL NOTES:

- The design professional whose seal appears on these drawings is the structural engineer of record (SER) for this project. The SER bears the responsibility of the primary structural elements and the performance of this structure. No other party may revise, alter, or delete any structural aspects of these construction documents without written permission of SUMMIT Engineering, Laboratory & Testing, P.C. (SUMMIT) or the SER. For t purposes of these construction documents the SER and SUMMIT
- shall be considered the same entity. The structure is only stable in its completed form. The contractor shall provide all required temporary bracing during construction 2 to stabilize the structure.
- The SER is not responsible for construction sequences, methods, or techniques in comection with the construction of this structure. The SER will not be held responsible for the solutions in our on the contract documents, should any non-conformities occur. Any structural elements or details not fully developed on the
- construction drawings shall be completed under the direction of a licensed professional engineer. These shop drawings shall be submitted to SUMMIT for review before any construction begins. The shop drawings will be reviewed for overall compliance as it relates to the structural design of this project. Verification of the shop drawings for dimensions, or for actual field conditions,
- is stop crawings to comprise or or summarized controller, is not the responsibility of the SER or SUMMIT. Verification of assumed field conditions is not the responsibility of the SER. The contractor shall verify the field conditions for accuracy and report any discrepancies to SUMMIT before construction begins. The SER is not responsible for any secondary structural elements
- or non-structural elements, except for the elements specifically noted on the structural drawings. This structure and all construction shall conform to all
- applicable sections of the international residential code.
- This structure and all construction shall conform to all applicable sections of local building codes.
 All structural assemblies are to meet or exceed to requirements
- of the current local building code

FOUNDATIONS:

The structural engineer has not performed a subsurface investigation. Verification of this assumed value is the responsibility of the owner or the contractor. Should any adverse soil condition be encountered the SER must be contacted before proceeding.

- The bottom of all footings shall extend below the frost line for the region in which the structure is to be constructed. However, the bottom of all footings shall be a minimum of 12" below grade. 6.
- Any fill shall be placed under the direction or recommendation
- of a licensed professional engineer. The resulting soil shall be compacted to a minimum of 95%
- maximum div density. Excavations of footings shall be lined temporarily with a 6 mil polysthylene memorane if placement of concrete does not occur within 24 hours of excavation.
- No concrete shall be placed against any subgrade containing water, ice, frost, or loose material.

- <u>STRUCTURAL STEEL:</u>
 1. Structural steel shall be fabricated and erected in accordance with the American Institute of Steel Construction "Code of Standard Fractice for Steel Buildings and Bridges" and the manual of Steel Construction "Load Resistance Factor Design latest editions.
- Structural steel shall receive one coat of shop applied rust-inhibitive paint. 3. All steel shall have a minimum yield stress (F_{μ}) of 36 ksi unless
- otherwise noted. Welding shall conform to the latest edition of the American
- Weiding shall conform to the latest edition of the American Weiding Society's Structural Weiding Code AWS D.I., Electrodes for shop and field weiding shall be class ETØXX. All weiding shall be performed by a certified weider per the above

CONCRETE:

- NUMBER: Concrete shall have a normal weight aggregate and a minimum compressive strength (Fp) at 28 days of 3000 psi, unless otherwise noted on the plan. Concrete shall be proportioned, mixed, and placed in
- accordance with the latest editions of ACI 318: "Building Code Requirements for Reinforced Concrete" and ACI 301: "Specifications for Structural Concrete for Buildings".
- Air entrained concrete must be used for all structural elements exposed to freeze/thau cycles and deicing chemicals. Air entrainment amounts (in percent) shall be within -1% to +2% of
- target values as follows: 3.1. Footings: 5% 3.2. Exterior Slabs: 5%
- 4. No admixtures shall be added to any structural concrete without written permission of the SER.

- Concrete slabs-on-grade shall be constructed in accordance 5 uith ACI 302.IR-96: "Guide for Concrete Slab and Slab Construction"
- The concrete slab-on-arade has been designed using a subgrade modulus of k=250 pci and a design loading of 200 psf. The SER is not responsible for differential settlement, slab cracking or other future defects resulting from unreported
- conditions not in accordance with the above assumptions. Control or saw cut joints shall be spaced in interior slabs-on-orade at a maximum of 15'-0" O.C. and in exterior
- slabs-on-grade at a maximum of 10'-0" unless otherwise noted. 8 Control or saw cut joints shall be produced using conventional process within 4 to 12 hours after the slab has been finished
- process winin + to 1/ nous after the slap has been initiated.
 Reinforcing steel may not extend through a control joint.
 Reinforcing steel may extend through a saw cut joint.
 I/I welded wire fabric (UWE) for concrete slabs-on-grade shall be placed at mid-depth of slab. The UWE, shall be securely supported during the concrete pour.

- <u>CONCRETE REINFORCEMENT:</u> I. Fibrous concrete reinforcement, or fibermesh, specified in concrete slabs-on-grade may be used for control of cracking due to shrinkage and thermal expansion/contraction lowered water migration, an increase in impact capacity, increased abrasion resistance, and residual strength.
- Thermosh reinforcing to be 100% virgin polypropylene fibers containing no reprocessed olefin materials and specifically manufactured for use as concrete secondary reinforcement.
- Application of fibermesh per cubic yard of concrete shall equal a minimum of 20% by volume (15 pounds per cubic yard) Fibermesh shall comply with ASTM CIII6, any local building code requirements, and shall meet or exceed the current industry 4
- standard. Stæel reinforcing bars shall be new billet steel conforming to
- 6
- ASTM A65, grade 60. ASTM A65, grade 60. Detailing, fabrication, and placement of reinforcing steel shall be in accordance with the latest edition of ACI 315: "Nanual of Standard Practice for Detailing Concrete Structures" Horizontal footing and wall reinforcement shall be continuous and shall have 30° bends, or comer bars with the same are for acless as the borgent at the class B size/spacing as the horizontal reinforcement with a class B
- tension splice. Lap reinforcement as required, a minimum of 40 bar diameters for tension or compression unless otherwise noted. Splices in masonry shall be a minimum of 48 bar diameters.

- Where reinforcing dowels are required , they shall be equivalent in size and spacing to the vertical reinforcement. The doule shall extend 48 bar diameters vertically and 20 bar diameters
- into the footing. 10. Where reinforcing steel is required vertically, dowels shall be provided unless otherwise noted.
- Solid saun wood framing members shall conform to the specifications listed in the latest edition of the "National
- Design Specification for Wood Construction" (NDS), Unless otherwise noted, all wood framing members are designed to be
- Spruce-Yellow-Pine (SYP) 2. LVL or PSL engineered wood shall have the following minimum

- 2.4.Fc = 100 psi
- Wood in contact with concrete, masony, or earth shall be pressure treated in accordance with AWPA standard C-15. All other moisture exposed wood shall be treated in accordance
- Nails shall be common wire nails unless otherwise noted.
- specifications. All beams shall have full bearing on supporting framing members
- unless otherwise noted. Exterior and load bearing stud walls are to be 2x4 SYP #2 # 16" O.C. unless otherwise noted. Studs shall be continuous from the sole plate to the double top plate. Studs shall only be discontinuous at headers for window/door openings. A minimum of one king stud shall be placed at each end of the header. King studs shall be continuous.
- King studs shall be continuous. Individual studs forming a column shall be attached with one l&d nall # 6" O.C. staggered. The stud column shall be continuous to the foundation or beam. The column shall be properly blocked at all filor levels to ensure proper load transfer.
- Multi-ply beams shall have each ply attached with (3) 10d nails @ 24" O.C.
- 10. Flitch beams, 4-ply beams and 3-ply side loaded beams shall be Inter beams, + py beams and ppg side back to be the bolted together with (2) rous of 12^n diameter through bolts staggered = 16" OC. unless noted otherwise. Min. edge distance shall be 2" and (2) bolts shall be located a min. 6" from each end of the beam

WOOD TRUSSES:

- <u>QOD TRUSES</u>. The wood truss manufacturer/fabricator is responsible for the design of the wood trusses. Submit sealed shop drawings and supporting calculations to the SER for review prior to fabrication. The SER shall have a minimum of five (5) days for review. The review by the SER shall review for overail compliance with the design documents. The SER shall assume no responsibility for the correctness for the structural design for the wood trusses shall be designed for all required loadings a specification to the design document of the design for the wood trusses.
- 2 Ine wood trusses shall be designed for all required loadings as specified in the local building code, the ASCE Standard "Minimum Design Loads for Buildings and Other Structures." (ASCE 1-05), and the loading requirements shown on these specifications. The truss drawings shall be coordinated with all other construction documents and provisions provided for loads shown on these drawings including but not limited to HVAC equipment, piping, and architectural fixtures attached to
- Hrve expension, provide a statement of the trusses. The trusses shall be designed, fabricated, and erected in accordance with the latest edition of the "National Design Specification for Wood Construction" (NDS) and "Design Specification for Metal Plate Connected Wood Trusses." 3.
- 4. The truss manufacturer shall provide adequate bracing Instruiss manufacturer sharp provide adequate cracing information in accordance with "Commentary and Recommendations for Handling, installing, and Bracing Metal Plate Connected Wood Trusses" (HIB-91). This bracing, both temporary and permanent, shall be shown on the shop drawings. Also, the shop drawings shall show the required attachments fo
- the trusses.
 Any chords or truss webs shown on these drawings have been shown as a reference only. The final design of the trusses shall be per the manufacture

EXTERIOR WOOD FRAMED DECKS:

Decks are to be framed in accordance with local building codes and as referenced on the structural plans, either through code references or construction details.

2

- UCOD STRUCTURAL PANELS: I. Fabrication and placement of structural wood sheathing shall be in accordance with the APA Design/Construction Guide "Residential and Commercial," and all other applicable APA
- All structurally required wood sheathing shall bear the mark of the APA

WOOD FRAMING:

- ign values: 2.1. E = 1,900,000 psi
 - - 2.2. F_b = 2600 psi 2.3. F_v = 285 psi
 - with AWPA standard C-2
 - Lag screws shall conform to ANSI/ASME standard B182.1-1381. Lead holes for lag screws shall be in accordance with NDS

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Development

	DR HORTON PROJECT	<u>1 Sign-077:</u>
I	Manager	Signature
	Operations	
	Operations System	
I	Operations Product	

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culent: Dr. Hercin Carolina Division 2009 Arramidge Bivd. Charolus, NC 2001 3
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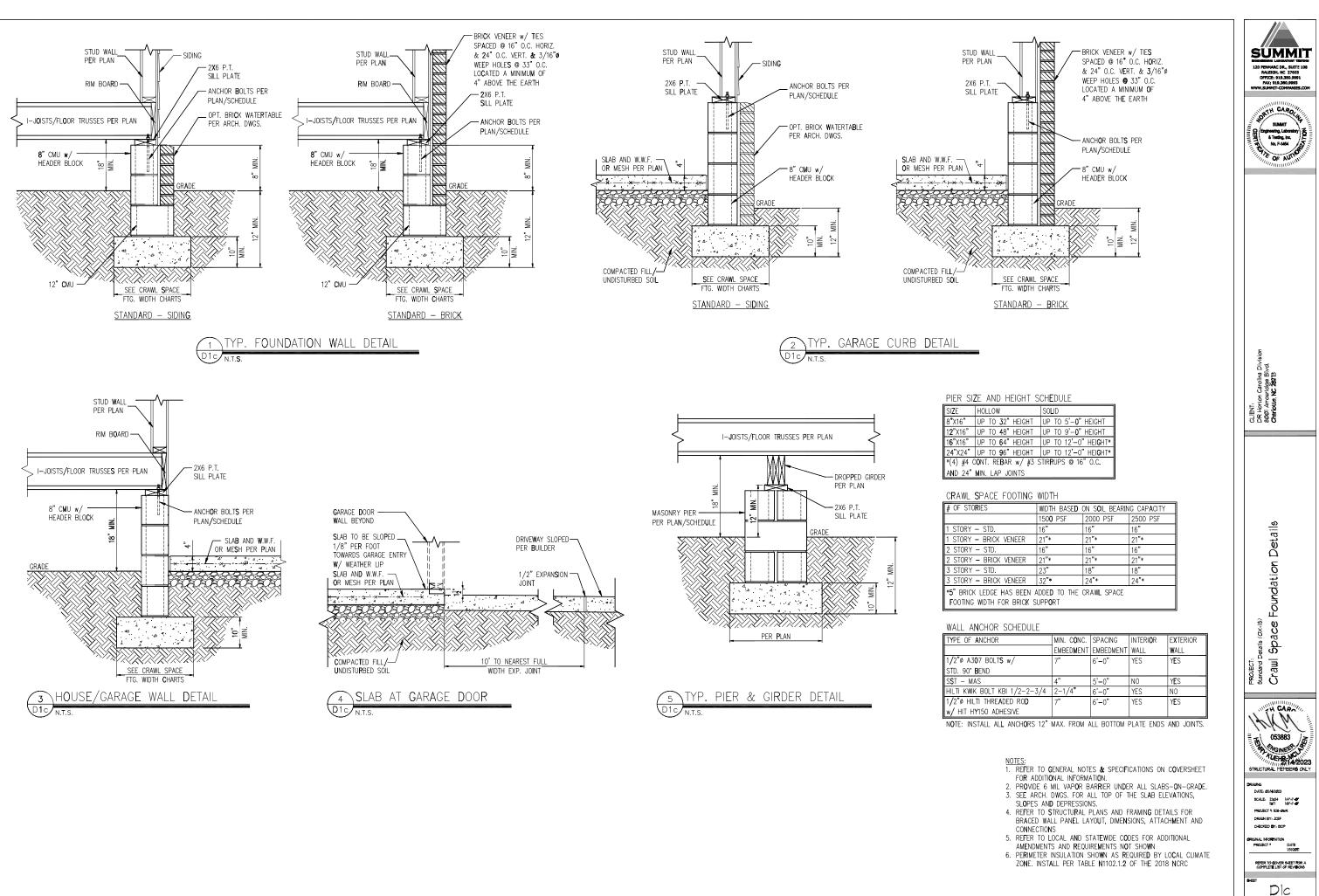
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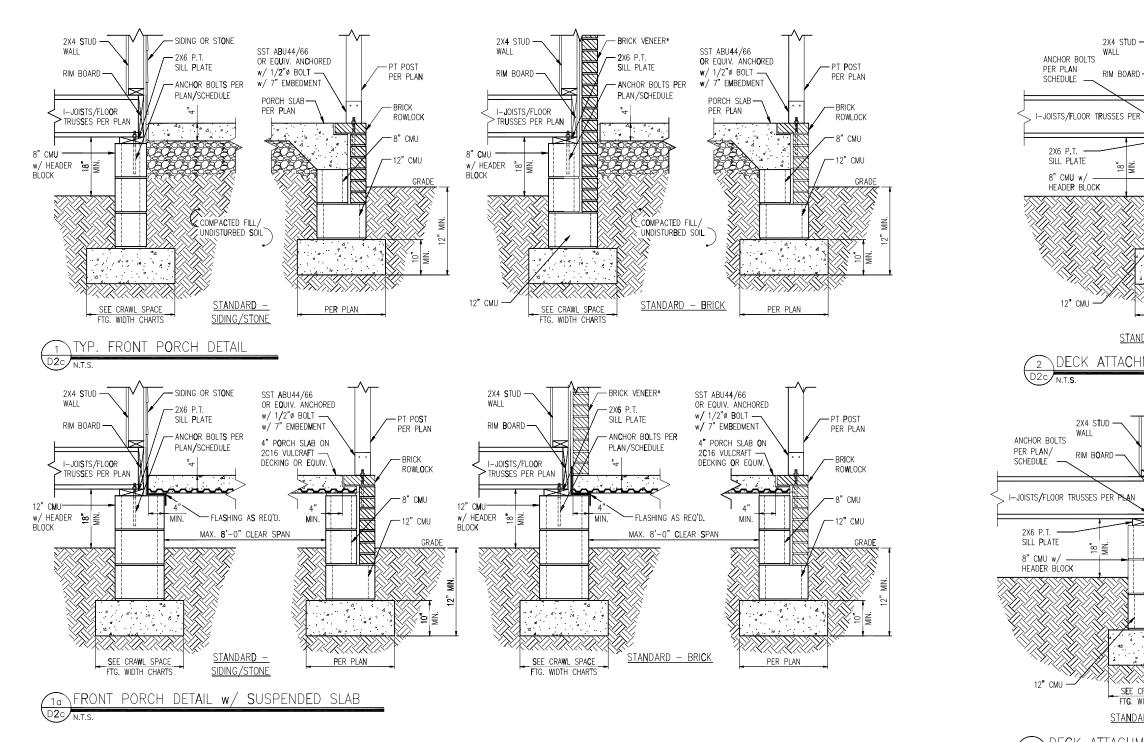
- Wood wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information, theathing shall be applied with the long direction perpendicular to framing, unless noted otherwise. Roof sheathing shall be APA rated sheathing exposure 1 or 2.
- Roof sheathing shall be continuous over two subports and attached to its supporting roof framing with (1)-8d CC nail at 6"o/c at panel edges and at 12"o/c in panel field unless otherwise noted on the plans. Sheathing shall be applied with the long direction perpendicular to framing. Sheathing shall have a span rating consistent with the framing spacing. Use
- support by use of T4C plywood or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the
- She building Code. Sheathing shall have a 1/8" gap at panel ends and edges as recommended in accordance with the APA.

<u>STRUCTURAL FIBERBOARD PANELS:</u> I. Fabrication and placement of structural fiberboard sheathing

- shall be in accordance with the applicable AFA standards All structurally required fiberboard sheathing shall bear the mark of the AFA.
- Fiberboard wall sheathing shall comply with the requirements of local building codes for the appropriate state as indicated on these drawings. Refer to wall bracing notes in plan set for more information.
- Sheathing shall have a 1/8" gap at panel ends and edges are

have a span rating consistent with the framing spacing, Use suitable edge support by use of plyucod clips or lumber blocking unless otherwise noted. Panel and joints shall occur over framing. Apply building paper over the sheathing as required by the state Building Code. Wood floor sheathing to its supporting framing with (1)-8d CC ringshank nail at 6°/or at panel edges and at 2°/or in panel field unless otherwise noted on the plans. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing. Sheathing shall be applied perpendicular to framing sheathing shall be applied perpendicular to framing sheathing shall be applied perpendicular to framing. Sheathing and the edge support buse of TK of bluocod or lumber blocking unless





	DECK ATTACHMENT	SCHEDULE (A	ALL STRUCTURES	FXCEPT BRICK)	
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MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
SPAN	SPAN
(1) @ 3'-6" 0.C.	(1) @ 1'-8" O.C.
AND	AND
(2) @ 8" 0. C .	(3) @ 6" O.C.
	SPAN (1) @ 3'-6" O.C. AND

a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.

b. MINIMUM EDGE DISTANCE FOR BOLTS IS 21".

c. NAILS MUST PENETRATE THE SUPPORTING STRUCTURE BAND A MINIMUM OF $1\frac{1}{2}$ "

DECK ATTACHMENT SCHEDULE (BRICK STRUCTURES)

FASTENERS	MAX. 8'-0" JOIST	MAX. 16'-0" JOIST
	SPAN	SPAN
5/8" GALV. BOLTS w/ NUT & WASHER ^D	(1) @ 2'-4" 0.C.	(1) @ 1'-4" O.C.

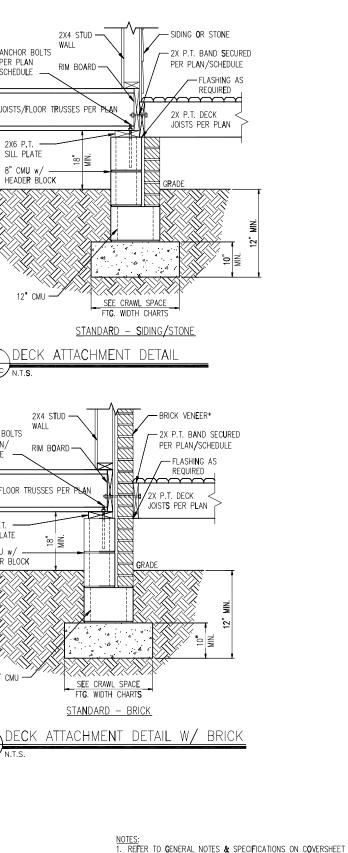
a. ATTACHMENT INTERPOLATION BETWEEN 8' AND 16' JOIST SPANS IS ALLOWED.

b. MINIMUM EDGE DISTANCE FOR BOLTS IS 21".

CRAWL SPACE FOOTING WIDTH

# OF STORIES	WIDTH BASED	ON SOI L BEARIN	ig capa c ity		
	1500 PSF	2000 PSF	2500 P S F		
1 STORY - STD.	16"	16"	16"		
1 STORY - BRICK VENEER	21"*	21"*	21"*		
2 STORY - STD.	16"	16"	16"		
2 STORY - BRICK VENEER	21"*	21"*	21"*		
3 STORY - STD.	23"	18"	18"		
3 STORY - BRICK VENEER	32"*	24"*	24"*		
*5" BRICK LEDGE HAS BEEN ADDED TO THE CRAWL SPACE FOOTING WIDTH FOR BRICK SUPPORT					

*BRICK TIES SPACED @ 16" O.C. HORIZ. & 24" O.C. VERT. AND 3/16"Ø WEEP HOLES @ 33" O.C. LOCATED A MINIMUM OF 4" ABOVE THE EARTH

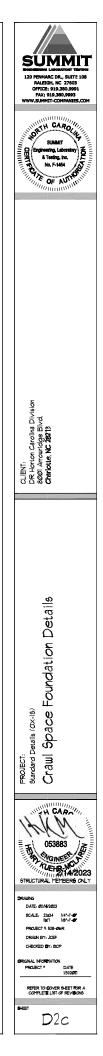


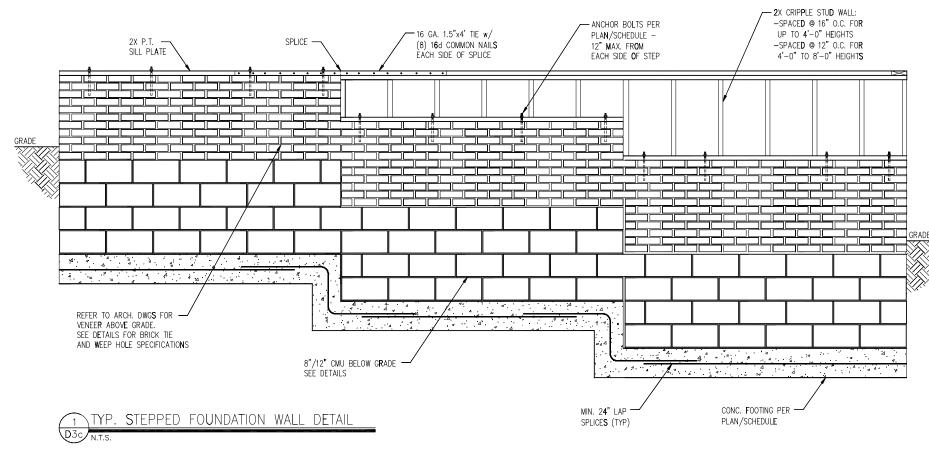
FOR ADDITIONAL INFORMATION.

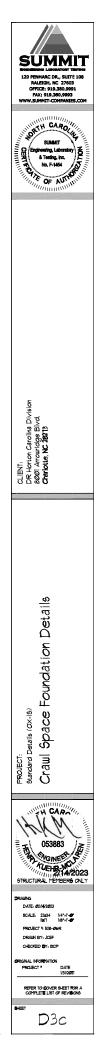
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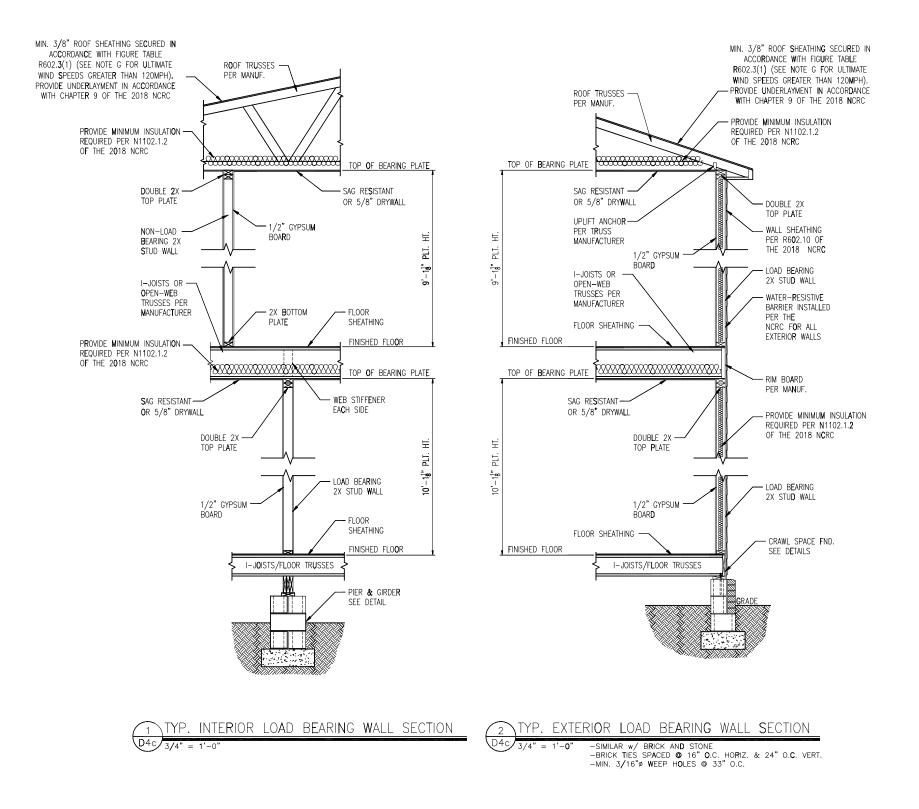
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE. . SEE ARCH. DWGS. FOR ALL TOP OF THE SLAB ELEVATIONS, SLOPES AND DEPRESSIONS.
- 4. REFER TO STRUCTURAL PLANS AND FRAMING DETAILS FOR BRACED WALL PANEL LAYOUT, DIMENSIONS, ATTACHMENT AND CONNECTIONS
- 5. REFER TO LOCAL AND STATEWIDE CODES FOR ADDITIONAL AMENDMENTS AND REQUIREMENTS NOT SHOWN
- 6. PERIMETER INSULATION SHOWN AS REQUIRED BY LOCAL CLIMATE ZONE. INSTALL PER TABLE N1102.1.2 OF THE 2018 NCRC

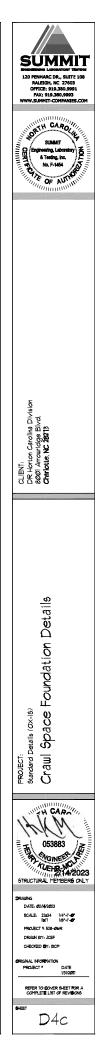




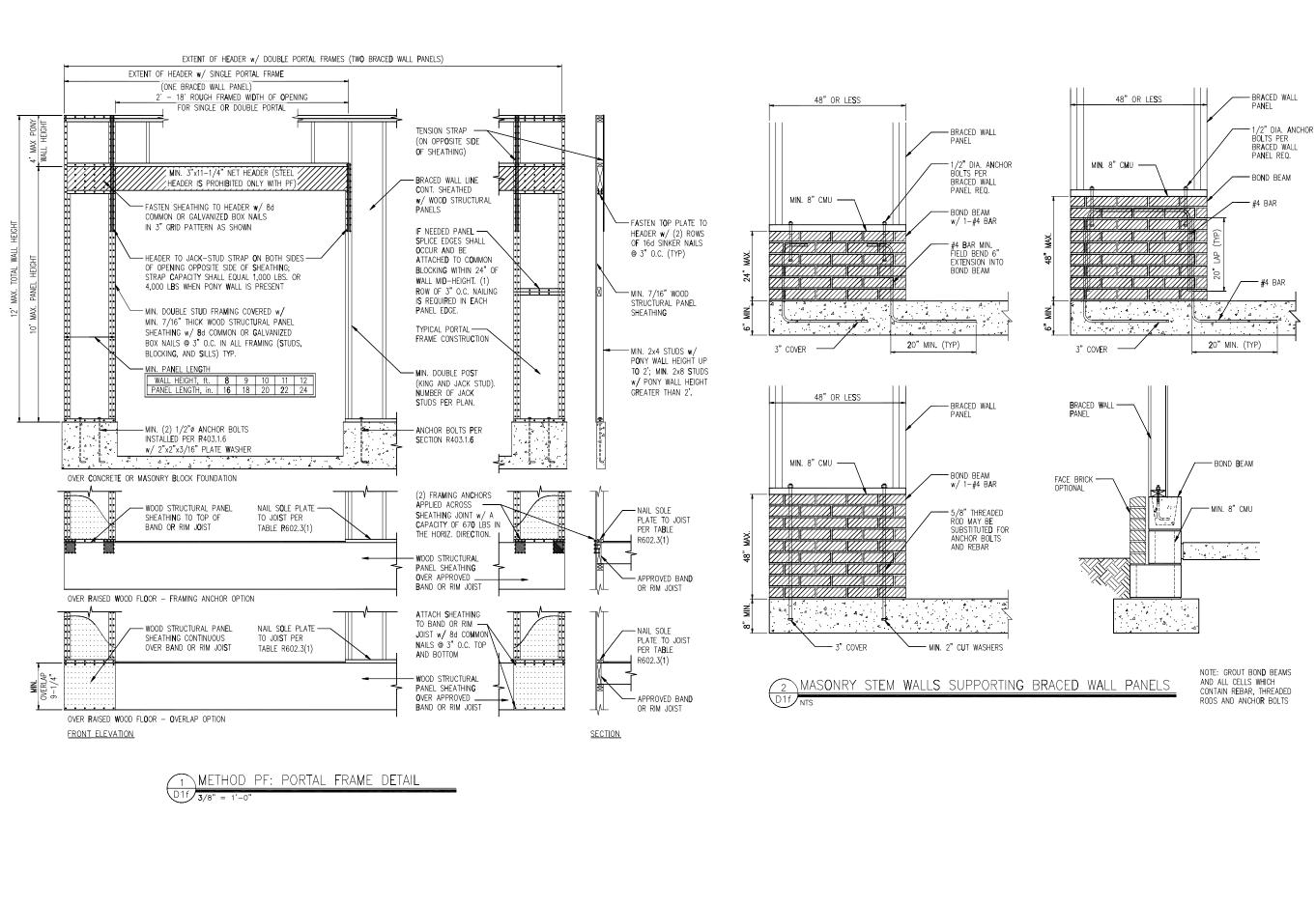


- NOTES: 1. REFER TO GENERAL NOTES & SPECIFICATIONS ON COVERSHEET FOR ADDITIONAL INFORMATION.
- PROVIDE 6 MIL VAPOR BARRIER UNDER ALL SLABS-ON-GRADE.
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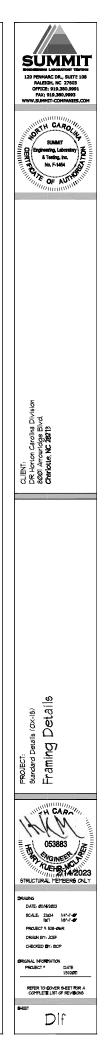


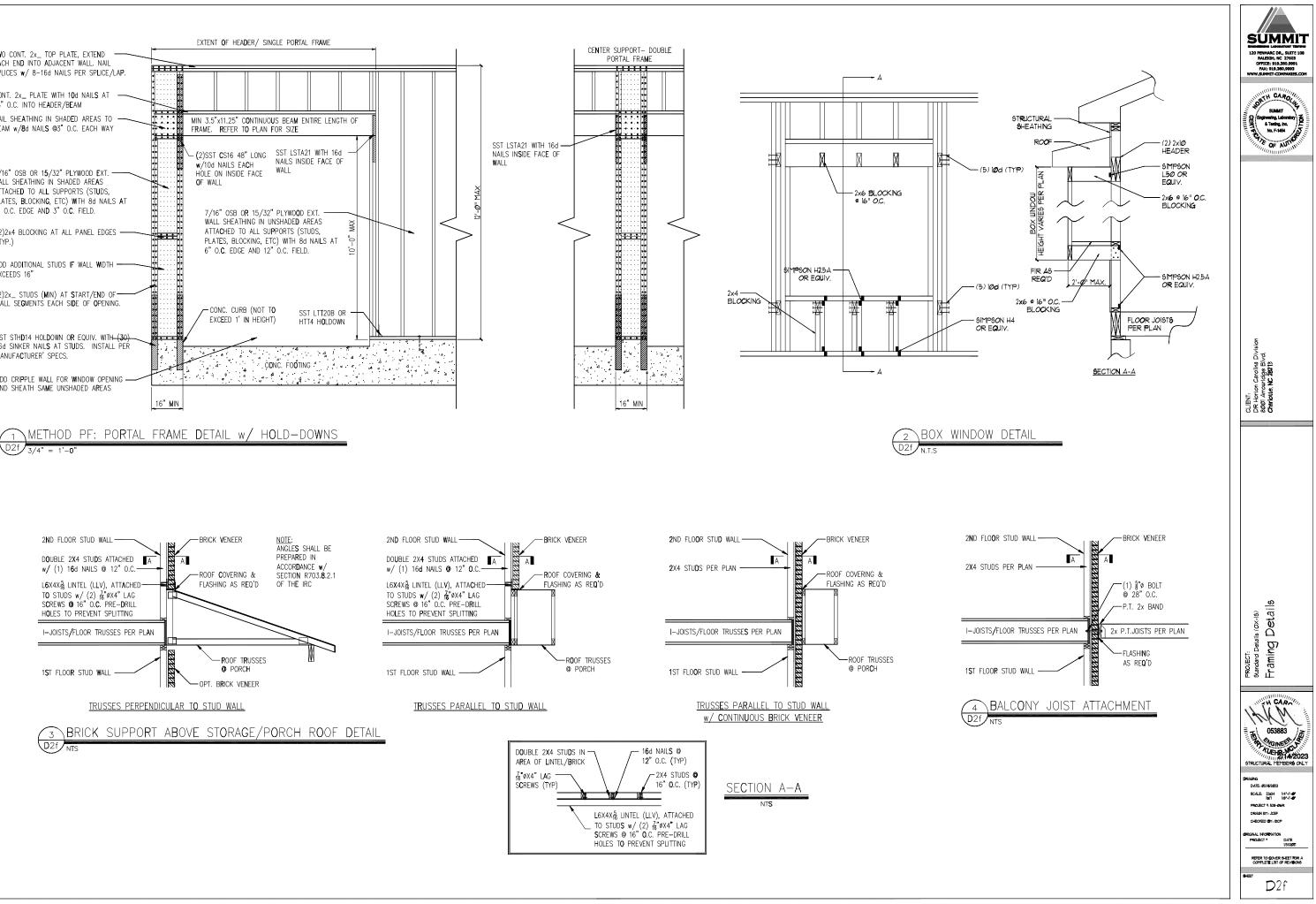


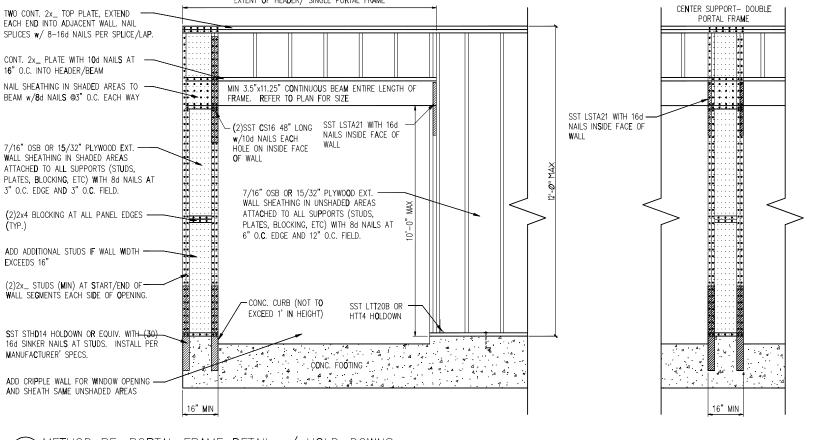
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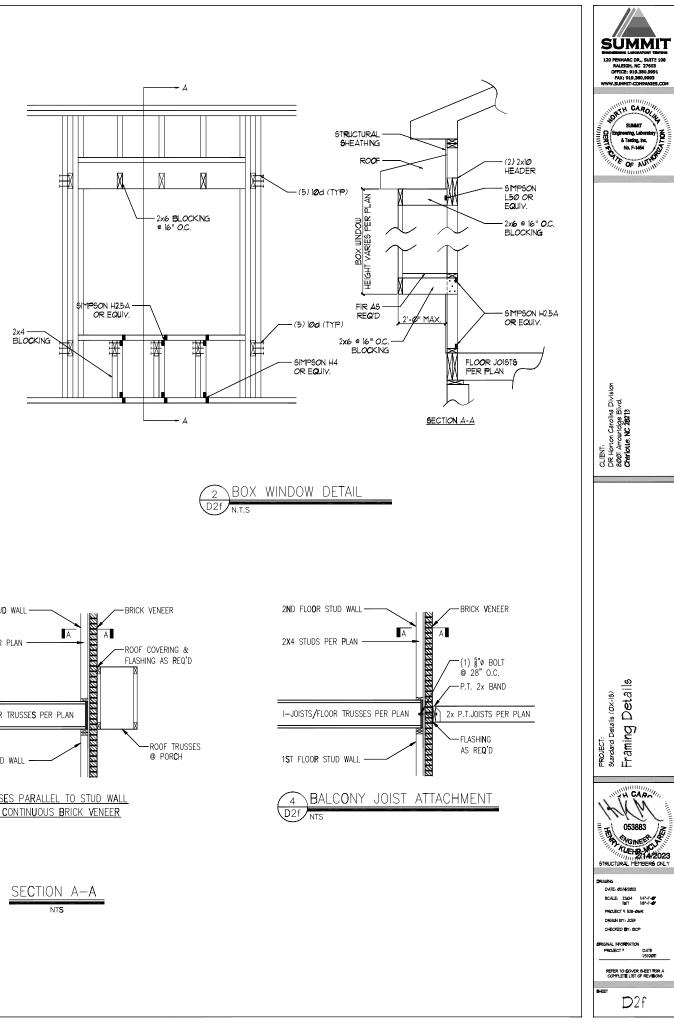


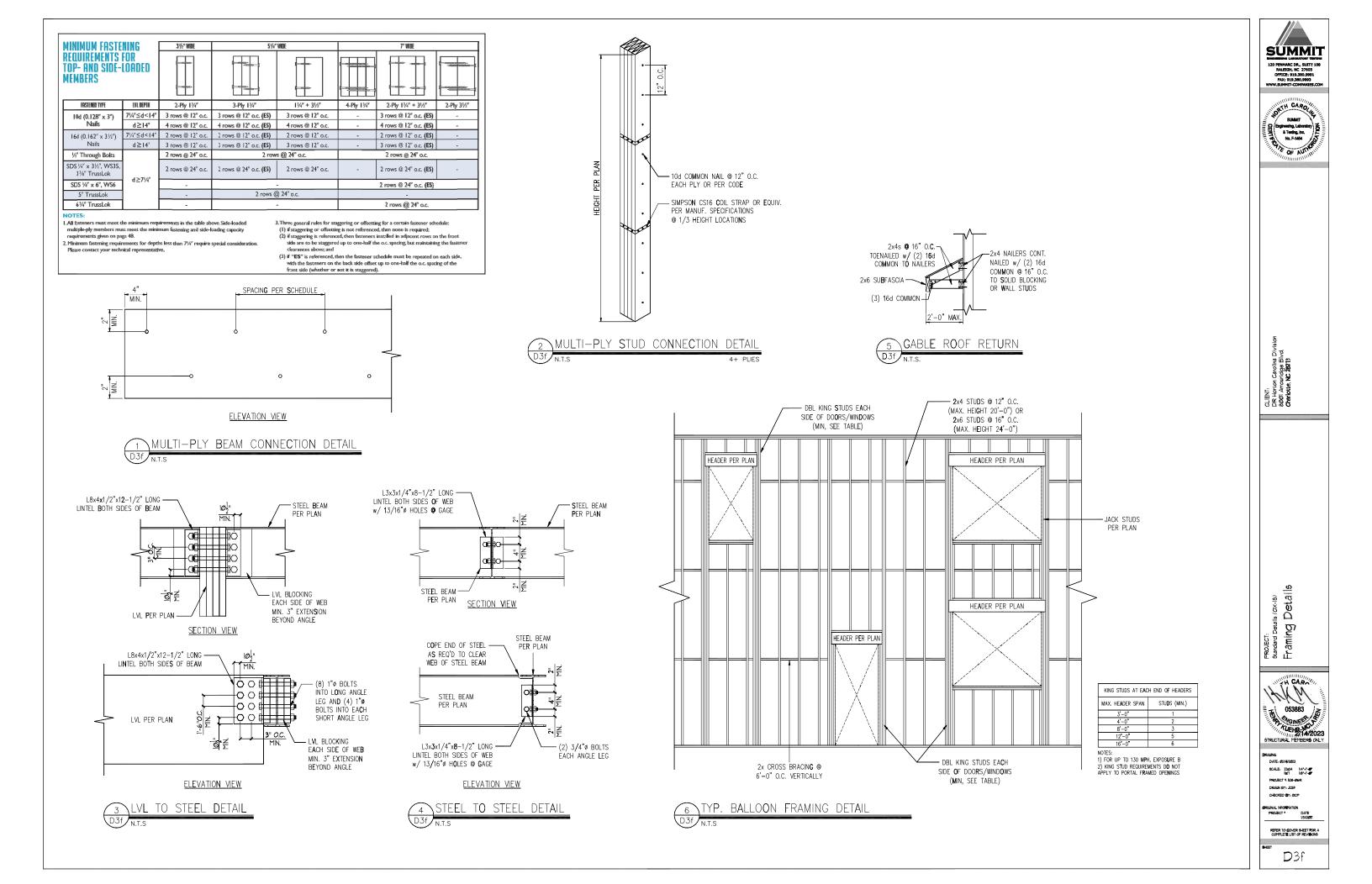


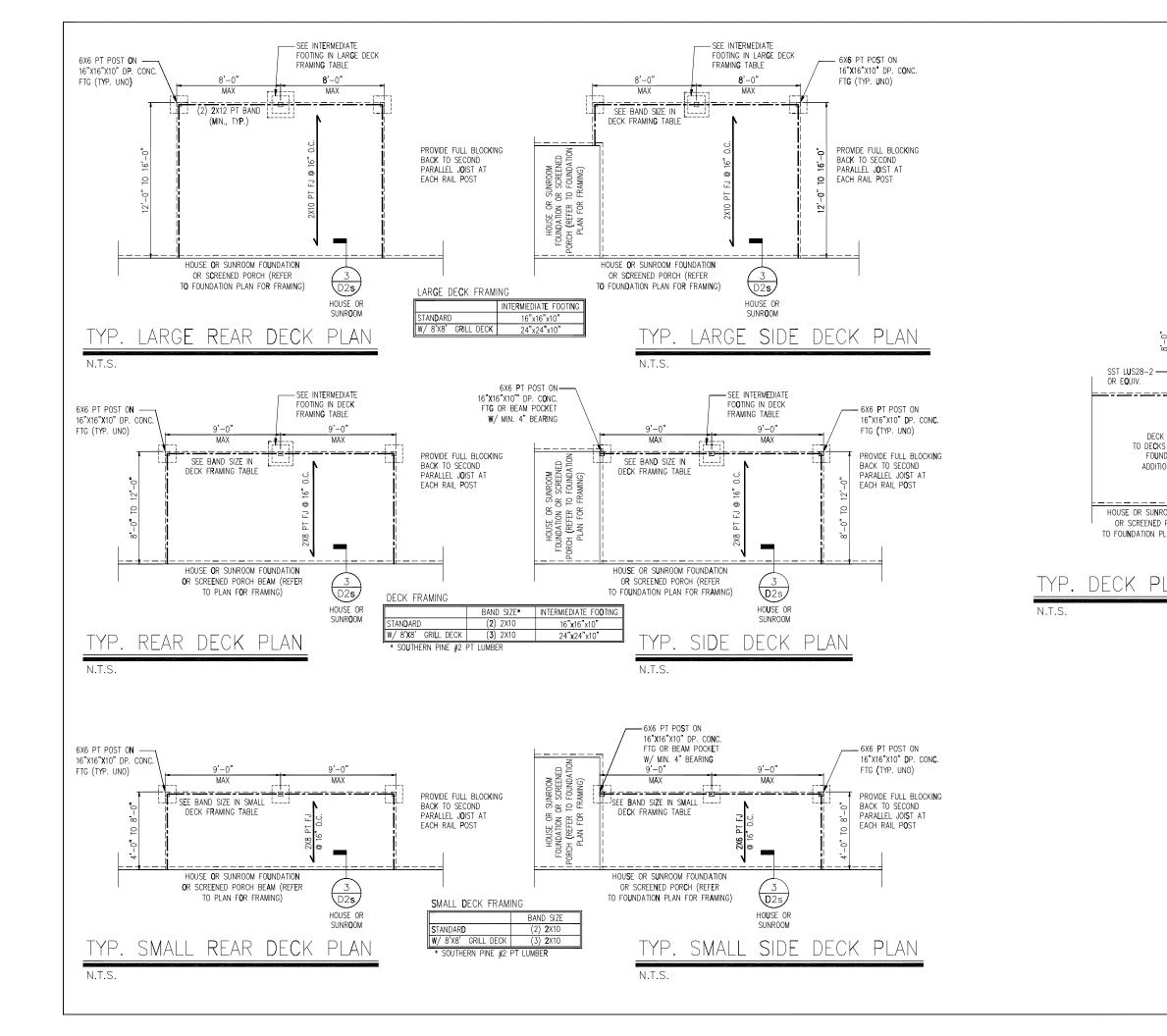


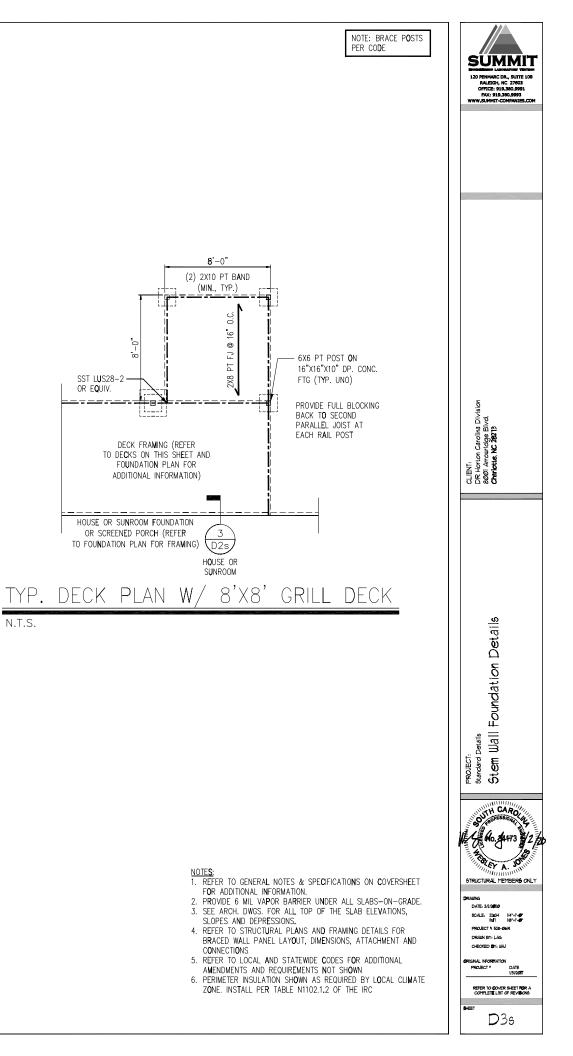




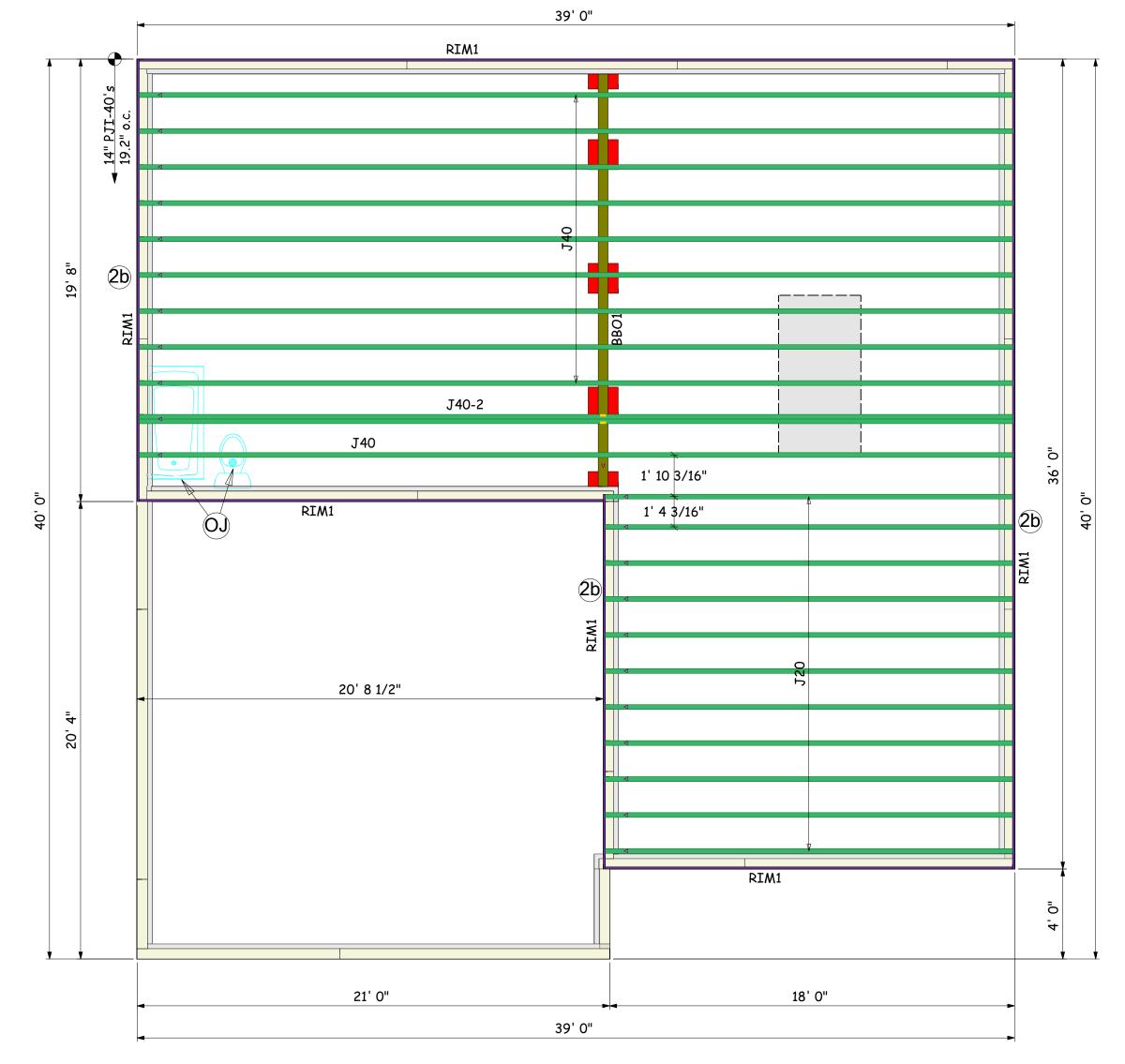


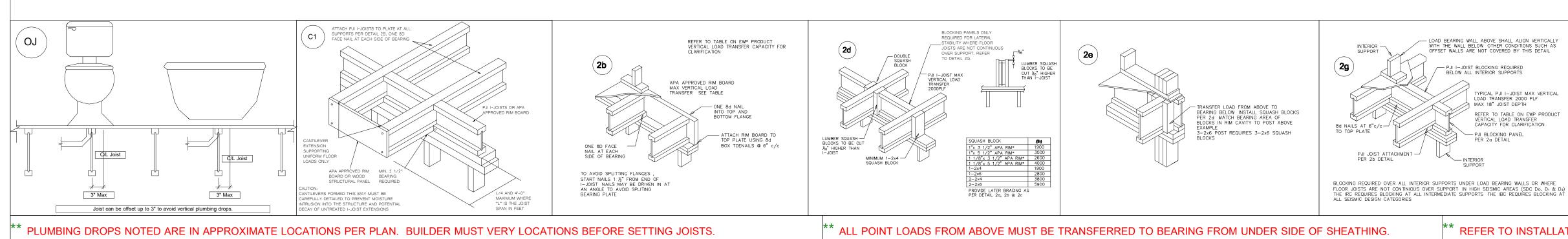












** PLUMBING DROPS NOTED ARE IN APPROXIMATE LOCATIONS PER PLAN. BUILDER MUST VERY LOCATIONS BEFORE SETTING JOISTS.

PlotID	Length	Product	
J40	40' 0"	14" PJI-40	
J40-2	40' 0"	14" PJI-40	
J20	20' 0"	14" PJI-40	
RIM1	12' 0"	1 1/8" × 14" APA	A Rim Bo
		Accessories	
PlotID	Length	Product	Plies
		3/4" 4×8 OSB	1
K	EMPSVILLE	BUILDING MATER	IALS IS
FOR T	HE DESIGN	NOR CALCULATION	OF ANY
LVL/PSI	. BEAM MA	TERIAL. ALL ENGIN	VEERING
THIS	MATERIAL	IS TO BE PROVIDE	ED BY TH
MA	rked on a	PPROVED SET OF P	LANS. AL
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		Produc	ts
PlotID	Length	Product	
J40	40' 0"	14" PJI-40	
J40-2	40' 0"	14" PJI-40	
J20	20' 0"	14" PJI-40	
RIM1	12' 0"	1 1/8" × 14" APA	Rim B
		Accessories	
PlotID	Length	Product	Plies
		3/4" 4x8 OSB	1
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3/4" 4x8 OSB	1	
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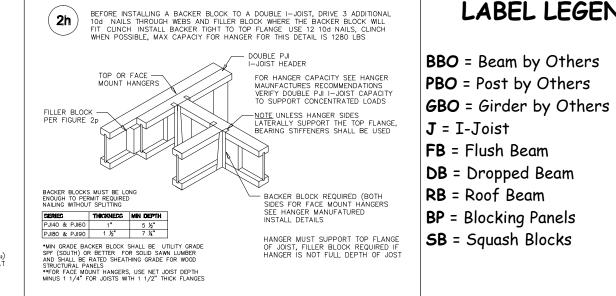
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** DAMAGED FLOOR JOISTS SHOULD NOT BE INSTALLED UNLE	INSTALLED UNLESS APPROVED BY COMPONENT PLANT.	** DIMENSIONS ARE READ AS: FOOT-INCH-SIXTEENTH.	** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONENTS.	ENTS.
Date: Desig	DR Horton	G		00/00 00/00 00/00 00/00
: 1/4" = // 11/22 ner: DW ct #: 241 Sheet Numb	15 Mason Ridge Havden B			D/00 D/00 D/00
2/24 / 10143	FLOOR JOIST LAYOUT		flanges on joists without prior approval from the manufacturing Representative unless following hole guidlines in the installation guide of product. Builder takes full responsibility for doing so and NO Back charge will be accepted.	ns Name Name Name Name

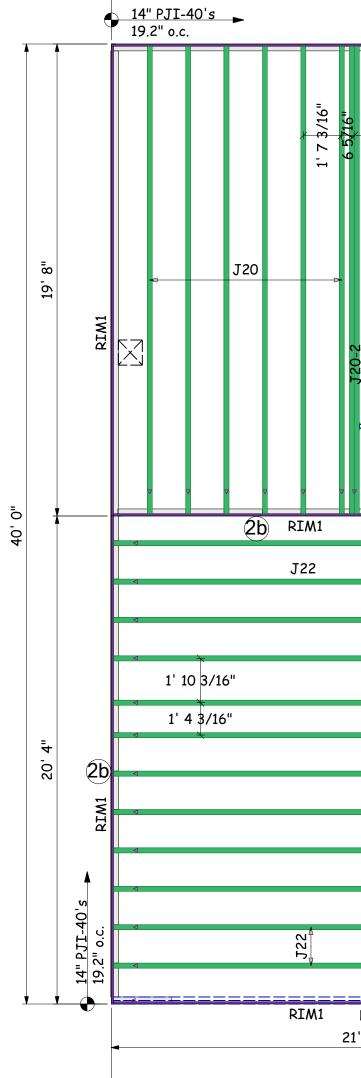
Plies Net Qty 10 2 2 11 13 oard 1 Net Qty 34

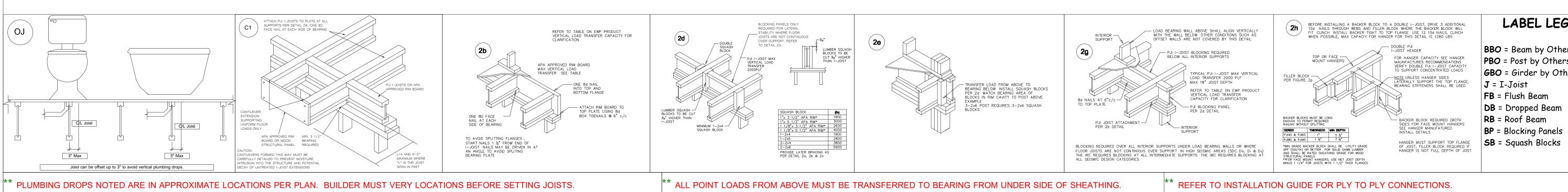
<u>1ST FLOOR LAYOUT</u>

LABEL LEGEND









** PLUMBING DROPS NOTED ARE IN APPROXIMATE LOCATIONS PER PLAN. BUILDER MUST VERY LOCATIONS BEFORE SETTING JOISTS.

					39	9' 0"												-	
		RIM1	2b			DE	33-2			(2b		D	B2-2					.
$\begin{array}{c c} & \bullet \\ & &$	FB6 H4		1' 10 3/16"	J16	1' 10 3/16"		H3 IH 2 J20-2 1' 11 7/8"	1, 5, 1/2"	*	V		7	J20				×	36' 0" 36' 0"	0_
		J40							J20		FB1-4	99	/16" 3/16"						40' 0"
						4			F.	V			/8"		0.)			
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							IWIN 2b				RIM1							4 "0"	
	DB4-2 ' 0"											18' ()"					•	<u>v V</u>
					39	9' 0"												•	

		Products		
PlotID	Length	Product	Plies	Net Qty
J40	40' 0"	14" PJI-40	1	9
J22	22' 0"	14" PJI-40	1	3
J20	20' 0"	14" PJI-40	1	18
J20-2	20' 0"	14" PJI-40	2	4
J16	16' 0"	14" PJI-40	1	6
J2	2' 0"	14" PJI-40	1	1
DB3-2	8' 0"	2.1 RigidLam SP LVL 1-3/4 × 9-1/4	2	2
DB2-2	6' 0"	2.1 RigidLam SP LVL 1-3/4 × 9-1/4	2	2
DB4-2	22' 0"	2.1 RigidLam SP LVL 1-3/4 x 11-7/8	2	2
FB1-4	20' 0"	2.1 RigidLam SP LVL 1-3/4 x 14	4	4
FB6	4' 0"	2.1 RigidLam SP LVL 1-3/4 x 14	1	1
RIM1	12' 0"	1 1/8" x 14" APA Rim Board	1	16
BP1	2' 0"	14" PJI-40	1	2

		Acces	sories							
PlotID	Length	n Produc	†	Plies	Net Qty					
		3/4" 4	x8 OSB	1	46					
Connector Summary										
PlotID	Qty	Manuf	Product	ł	Backer Block	ks Web Stiff				
H1	1	Simpson	IUS2.5	6/14	2 and Filler	No				
H2	10	Simpson	IUS2.5	6/14	No	No				
H3	1	Simpson	MIU5.1	2/14	No	No				
H4	1	Simpson	HUS1.8	1/10	No	No				
				-	5 NOT RESPON					

FOR THE DESIGN OR CALCULATION OF ANY AND ALL I-JOIST AND LVL/PSL BEAM MATERIAL. ALL ENGINEERING AND INFORMATION FOR THIS MATERIAL IS TO BE PROVIDED BY THE ENGINEER OF RECORD MARKED ON APPROVED SET OF PLANS. ALL BEAM PLACEMENTS ARE PER THE ENGINEERING RECEIVED. ALL CONNECTION DETAILS TO BE PROVIDED BY ENGINEER OF RECORD. REFER TO ENGINEER OR RECORD FOR ALL MULTI-PLY LVL/ I-JOIST CONNECTION PATTERNS. BUILDER TO VERIFY ALL MATERIAL LENGTHS, QUANTITIES, AND SIZES PRIOR TO ORDERING.

* BARGED FLOOR DATE READ BY COMPORT RATE READ AS: FOOLINGHALT Composition of the control of t					
00/00/00 Name 12 Mason Ridde Manutetrue scientus and manutetrue scientus scientus scientus scientus scientus scientus scientus and manutetrue and manutescientus and manutescientuscientus and manutescientus and manutescientus and manu		3E INSTALLED UNLESS APPROVED BY COMPONENT PLANT.		** FRAMER MUST REFER TO PLANS WHILE SETTING COMPONEN	NTS.
000 Name 000 Name 000 Name 12 Mason Ridge 12 Mason Ridge 13 Mason Ridge 15 Mason Ridge 14 Mayden B 15 Mason Ridge 15 Mason Ridge 15 Mason Ridge 16 Mason Ridge 15 Mason Ridge 17 Mason Ridge 16 Mason Ridge 18 Hayden B 18 Hayden B 19 Mayden B 19 Mayden B 19 Mayden B 19 Mayden B 11 Hayden B 10 Mayden B 11	Date: Design Projec	DR Horton		t of	00/00 00/00 00/00
Name Index in the installation Betweentative unless following hole guidlines in the installation Bride of product: Builder takes full responsibility for doing so and ND Back charge will be accepted. and ND Back charge will be accepted.	// 11/ ner: [ct #: 24 Sheet Nu	15 Mason Ridge)/00)/00)/00)/00
FLOOR JOIST LAYOUT and NO Back charge will be accepted. Builder takes full responsibility for doing so a a a a a a a a a a a a a a a a a a	22/24 DW 4110	Hayden B	Lumber	c	Na Na Na Na
	4	FLOOR JOIST LAYOUT			me me me

UST B	E FULLY	CONNECTED	TOGETHER	PRIOR TO	ADDING ANY	LOADS



J = I-Joist

FB = Flush Beam

RB = Roof Beam

DB = Dropped Beam

SB = Squash Blocks



BBO = Beam by Others

PBO = Post by Others

GBO = Girder by Others

** REFER TO INSTALLATION GUIDE FOR PLY TO PLY CONNECTIONS.

