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The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

AST #: 46673 JOB: 24-2119-R01 JOB NAME: LOT 0.0105 BLAKE POND Wind Code: 37 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2015 as well as IRC 2018. 29 Truss Design(s)

Trusses:

J01, R01, R02, R02A, R03, R04, R05, R06, R07, R08, R09, R10, R11, R12, R13, R14, R15, R16, R17, R18, R19, R20, R21, SP01, SP02, VT01, VT02, VT03, VT04



## Warning !--- Verify design parameters and read notes before use.

This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for* 



Max Uplift3=-21(LC 14), 2=-46(LC 10)

Max Grav 3=63(LC 21), 4=41(LC 7), 2=209(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### **NOTES-** (9)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

non-concurrent with other live loads.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Refer to girder(s) for truss to truss connections.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 21 lb uplift at joint 3 and 46 lb uplift at joint 2.

LOAD CASE(S) Standard





Job	Truss	Truss Type		Qty	Ply	LOT 0.0105 BL	AKE POND   92 FR	OST MEADOW WAY	LILLINGTON, NC
24-2119-R01	R02	Roof Special		6	1	Job Reference	e (optional)	#	46673
		40.4.0		ID:WI8rkg6E	K5SaRYC	VGf9_0xywFJ	5-iMHUkVngZaL	dUyzk?8j59PJmy61	83gBrYTp8yfzZboc
-	0-10-8 2-3-8 7-10-0 0-10-8 2-3-8 5-6-8	5-6-8		6-2-11	5-11-1	14	6-3-2	———————————————————————————————————————	
				6x8 /	5				Scale = 1:70.5
				0,0 >	-				
J		6.0	0 12	6					
			20						
			3x4    5 TO			21	9.00 12		
						2 22 4	x6 ≪		
10	6	5x10 =				13 <sup>3</sup>	000		
10-5		4		W3 W5			23		
	5x8 = 18		В3		W6				
	I1	W1 W2			)/	wz		4x0 <> 8	
	2 1	B2							
2.12		15 -	14 9	W4 B4 6		a le	B5 W8	1-0-1 1-0-1	
Ċ	$_{3x4}^{\boxtimes} = 16$	2x4	13	12		<sup>11</sup> 10		9	
	2x4    2x4		3x4	4x8 =	= 3	3x8 = 4x4	=	3x4	
	<u>1-8-52-3-8 7-10-0</u> 1-8-50-7-3 5-6-8	<u>13-4-8</u> 5-6-8		<u>19-7-3</u> 6-2-11	<u>25-6-1</u> 5-11-1	14 10	<u>31-10-0</u> 6-3-2	—	
Plate Offsets (X,Y) [3:0	<u>)-9-8,0-3-10], [3:0-1-10,0-0-</u>	<u>4], [3:0-8-8,0-0-0], [6</u>	:0-6-0,0-3-0], [1	<u>4:0-2-8,0-3-4]</u>					
TCLL (roof) 20.0	SPACING- Plate Grip DOI	2-0-0 <b>C</b>	SI.	DEFL.	in (lo	c) l/defl 16 >999	L/d 240	PLATES MT20	GRIP 244/190
Snow (Pf) 20.0 TCDL 10.0	Lumber DOL	1.25 B	C 0.67	Vert(CT)	-0.40	16 >945	180	WIT20	244/100
BCLL 0.0 * BCDI 10.0	Code IRC2021/TF	12014 M	atrix-SH	Horz(CT)	0.22	9 n/a	n/a	Weight: 219 lb	FT = 20%
LUMBER-				BRACING-					
TOP CHORD 2x4 SP No T1: 2x6 SI	D.2 *Except* P DSS			TOP CHORD	Structura Rigid cei	al wood shea	thing directly ap	oplied, except en	d verticals.
BOT CHORD 2x4 SP No	p.2 *Except*			WEBS	1 Row at	midpt	7-12	e ee blaeing.	
WEBS 2x4 SP No	Р NO.2, B2: 2x6 SP DSS, B 0.3	3: 284 SP NO.3			MiTek r	recommends alled during t	s that Stabilizers russ erection, ir	s and required cro n accordance with	ss bracing Stabilizer
WEDGE Left: 2x4 SP No.3					Installat	tion guide.	,		
<b>REACTIONS</b> (Ib/size)	2-1318/0.3.8 (min 0.1.0)	0-1262/Mechanical							
Max Horz	2=1310/0-3-0 (mm. 0-1-3) 2=210(LC 11)								
Max Uplit Max Grav	t2=-120(LC 14), 9=-66(LC 1 2=1332(LC 21), 9=1262(LC	5) C 1)							
FORCES. (lb) - Max Co	mp /Max Ten - All forces :	250 (lb) or less excer	t when shown						
TOP CHORD 2-3=-686	6/108, 3-18=-2856/267, 4-1	3=-2794/285, 4-5=-19	942/255, 5-19=-	-1990/328,					
7-23=-13	334/199, 8-23=-1532/178, 8	-9=-1202/165	221232/232,	7-221290/223,					
BOT CHORD 3-15=-32 WEBS 4-15=0/2	24/2542, 14-15=-324/2561, 170, 4-14=-1027/190, 12-14	5-14=-447/177, 11-1 =-11/816, 6-14=-250	2=-87/1147, 10 /1330, 6-12=-69	-11=-87/1147 9/285,					
7-12=-38	36/166, 8-10=-57/1033								
NOTES- (10)		for this desire							
<ol> <li>Onbalanced root live loads have been considered for this design.</li> <li>Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS</li> </ol>									
(envelope) gable end zone and C-C Exterior(2E) -0-7-2 to 4-2-8, Interior(1) 4-2-8 to 14-9-10, Exterior(2R) 14-9-10 to 24-4-13, Interior(1) 24-4-13 to 26-10-10, Exterior(2E) 26-10-10 to 31-8-4 zone;C-C for members and forces & MWFRS for reactions shown: Lumber									
DOL=1.60 plate grip DOL=1.60 3) TCL - ASCE 7-16: Pr=20.0 pcf (roof L - Lum DOL=1.25 Plate DOL=1.25): Pf=20.0 pcf (Lum DOL=1.15 Plate DOL=1.15): lo=1.0: Pouron									
Cat B; Partially Exp.; (	Ce=1.0; Cs=1.00; Ct=1.10		20), 1 1–20.0 p3			L=1.10), 13-	intering interior	TH CAHOLA	1111
<ul><li>4) Unbalanced show load</li><li>5) This truss has been de</li></ul>	as have been considered for esigned for greater of min re	r this design. oof live load of 12.0 p	sf or 2.00 times	s flat roof load of 2	20.0 psf on	overhangs	Inni	ROPAN	henry
non-concurrent with of 6) This truss has been de	her live loads. esigned for a 10.0 psf botto	m chord live load nor	concurrent with	any other live loa	ads			SEAL	
7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit 28147									
8) Refer to girder(s) for truss to truss connections.									
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 9 except (jt=lb) 2=120									
LOAD CASE(S) Standar	d						"III	Manager Monum	30
								3/18/2024	















Job	Truss	Truss Type	Qty	Ply	LOT 0.0105 BLAKE POND   92 FROST MEA	ADOW WAY LILLINGTON, NC
24-2119-R01	R07	Common Girder	1	2	Job Reference (optional)	# 46673
		Run: 8.43	0 s Feb 12 D:Wl8rkg6	2021 Print: BK5SaRY	8.430 s Feb 12 2021 MiTek Industries, Inc. M /CYGf9_0xywFJ5-tURe1GvZzyj3IfJr8yF	/on Mar 18 20:46:25 2024 Page 2 Pg5jHf6Ypt8ebS3gzDrWzZboS

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 3-5=-60, 1-5=-20

Concentrated Loads (lb)

Vert: 6=-1242(B) 9=-1247(B) 10=-1242(B) 11=-1242(B) 12=-1242(B) 13=-1242(B) 14=-1242(B) 15=-1348(B) 16=-1348(B)



3/18/2024



3/18/2024







Max Horz 2=61(LC 10) Max Uplift6=-20(LC 10), 2=-32(LC 10), 7=-32(LC 14)

Max Grav 6=112(LC 21), 2=206(LC 21), 7=276(LC 21)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 6) Gable studs spaced at 2-0-0 oc
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 6, 2, 7. 10) Non Standard bearing condition. Review required.

LOAD CASE(S) Standard



3/18/2024



REACTIONS. (lb/size) 4=182/0-1-8 (min. 0-1-8), 2=256/0-3-8 (min. 0-1-8) Max Horz 2=61(LC 10) Max Uplift4=-32(LC 14), 2=-51(LC 10) Max Grav 4=243(LC 21), 2=351(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (10)

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard



3/18/2024



Job	Truss	Truss Type	Qty	Ply	LOT 0.0105 BLAKE POND   92 FROST ME	EADOW WAY LILLINGTON, NC
24-2119-R01	R12	GABLE	1	1	Job Reference (optional)	# 46673
			Run: 8.430 s Feb 12	2021 Print:	8.430 s Feb 12 2021 MiTek Industries, Inc.	Mon Mar 18 20:46:31 2024 Page 2

10) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 11) Bearing symbols are not considered in the structural design of the truss to support the

loads indicated. 12) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate Composited Wead Truesso for additional bracing guidelings, including diagonal bracing

12) Web blacking shown is to hater support of individual web individual web individual to be of a boot a basic of boot a basic of a basic of

LOAD CASE(S) Standard







- 6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
- 8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 4.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

10) Use Simpson Strong-Tie LUS24 (4-10d Girder, 2-10d Truss, Single Ply Girder) or equivalent spaced at 2-0-0 oc max. starting at 2-4-0 from the left end to 4-4-0 to connect truss(es) R15 (1 ply 2x4 SP), R16 (1 ply 2x4 SP) to front face of bottom chord.

- 11) Fill all nail holes where hanger is in contact with lumber.
- 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

# LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-3=-60, 2-4=-20 Concentrated Loads (lb) Vert: 5=-366(F) 6=-315(F)











6) Bearing at joint(s) 3 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of

bearing surface. 7) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 3.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





*rule Connectea wood Truss Construction* and BCSI 1-05 Guide to *Good Practice for Handling, Inst* D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0105 BLAKE POND   92 FROST ME	EADOW WAY LILLINGTON, NC
24-2119-R01	R19	GABLE	1	1	Job Reference (optional)	# 46673
		F	Run: 8.430 s Feb 12	2021 Print:	8.430 s Feb 12 2021 MiTek Industries, Inc.	Mon Mar 18 20:46:42 2024 Page 2

ID:Wl8rkg6BK5SaRYCYGf9\_0xywFJ5-ulz3c46EzAsfqG66e0DflJUkROkndUPy\_qbdx1zZboB

12) Graphical bracing representation does not depict the size, type or the orientation of the brace on the member. Symbol only indicates that the member must be braced. 13) Bearing symbols are only graphical representations of a possible bearing condition. Bearing symbols are not considered in the structural design of the truss to support the loads indicated.

14) Web bracing shown is for lateral support of individual web members only. Refer to BCSI - Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal Plate

Connected Wood Trusses for additional bracing guidelines, including diagonal bracing. 15) SEE BCSI-B3 SUMMARY SHEET- PERMANENT RESTRAING/BRACING OF CHORDS & WEB MEMBERS FOR RECOMMENDED MINIMUM BRACING REQUIREMENTS OF TOP CHORD, BOTTOM CHORD, AND WEB PLANES. IN ADDITION TO THESE MINIMUM GUIDELINES, ALWAYS CONSULT THE PROJECT ARCHITECT OR ENGINEER FOR ADDITIONAL BRACING CONSIDERATIONS.

LOAD CASE(S) Standard













- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-13 to 5-2-7, Exterior(2R) 5-2-7 to 9-1-13, Exterior(2E) 9-1-13 to 13-11-7 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough
- Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=134, 6=133.

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LOAD CASE(S) Standard
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- Unbalanced roof live loads have been considered for this design.
   Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS
- (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=137, 6=136.

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LOAD CASE(S) Standard
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FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

#### NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)\* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (Ib/size) 1=63/3-6-10 (min. 0-1-8), 3=63/3-6-10 (min. 0-1-8), 4=94/3-6-10 (min. 0-1-8) Max Horz 1=22(LC 9) Max Uplift1=-10(LC 13), 3=-13(LC 13)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-(8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.25 Plate DOL=1.25); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) \* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

