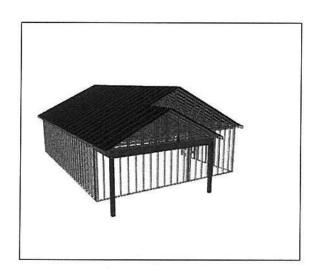


Carter Sanford Component Plant 298 Harvey Faulk Rd Sanford, NC 27332

Phone #:919-775-1450

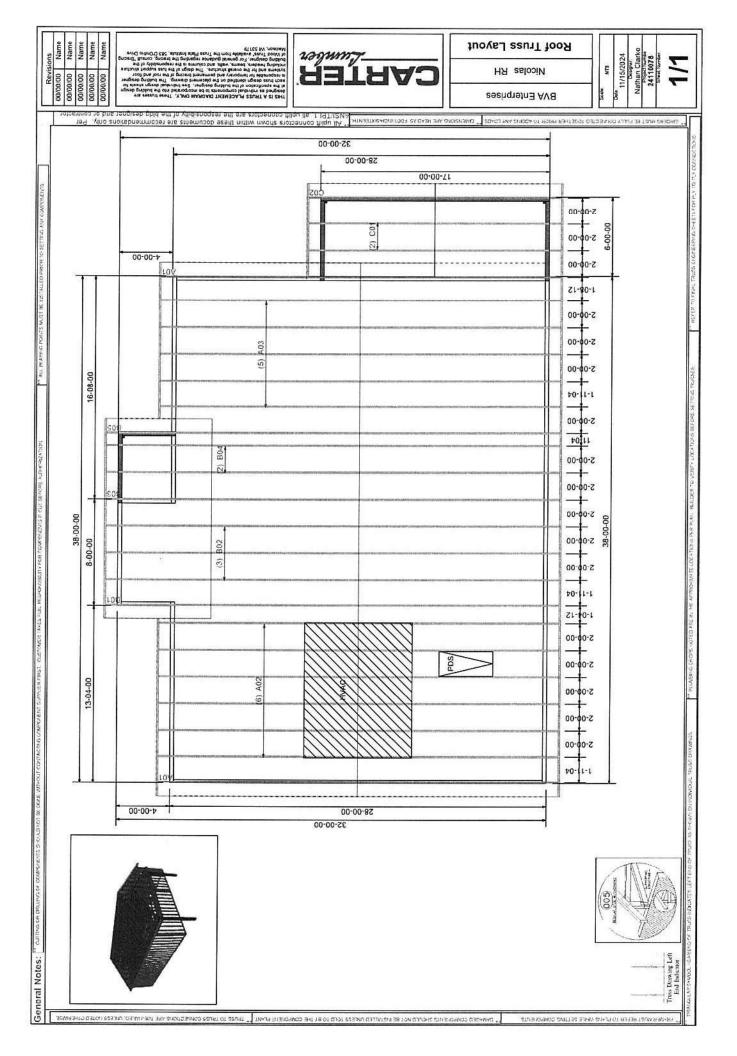
Builder: BVA Enterprises Model: Nicolas



THE PLACEMENT PLAN NOTES:

- 1. The Placement Plan is a diagram for truss installation. It is not an engineered drawing and has not been reviewed by an engineer. The Owner/Building Designer is responsible for obtaining an engineer's review if one is required by the local jurisdiction.
- 2. The responsibilities of the Owner, Contractor, Building Designer, Component Designer and Component Manufacturer shall be as set forth in ANSI/TPI 1. Capitalized terms shall be as defined in ANSI/TP 1 unless otherwise indicated.
- 3. Each Component is designed as an individual component utilizing information provided by others. The Owner/Building Designer is responsible for reviewing all Component Submittal Packages and individual Component Design Drawings for compliance with the Construction Documents and compatibility with the overall Building design.
- 4. Contractor will not proceed with component installation until the Owner/Building Designer has reviewed the Component Submittal Package. Questions on the suitability of any Component will be resolved by the Building Designer.
- 5. The Building Designer and Contractor are responsible for all temporary and permanent bracing.
- 6. The Placement Plan assumes the building is dimensionally correct, structurally sound, and in a suitable condition to support each Component during installation and thereafter, including but not limited to installation of all bearing points. Proper design and construction of all structural components, including foundations, headers, beams, walls and columns are the responsibility of the Owner, Building Designer and Contractor.
- 7. Do not cut, drill, or modify any Component without first consulting the Component Manufacturer or Building Designer. Damaged Components shall not be installed unless directed by the Building Designer or approved by the Component Manufacturer.
- 8. Components must be handled and installed following all applicable safety standards and best practices, including but not limited to BCSI, OSHA, TPI and local codes. Failure to properly handle, brace or otherwise install Component can result in serious injury or death.

Apprved by:	Date:





RE: 24110078

0 Rainbow-Roof-Nicolas - GRH

Trenco

818 Soundside Rd Edenton, NC 27932

Site Information:

Customer: BVA Enterprises Project Name: 24110078

Lot/Block:

Model:

Address: 0 Rainbow

Subdivision:

City: Dunn

State: NC

General Truss Engineering Criteria & Design Loads (Individual Truss Design Drawings Show Special Loading Conditions):

Design Code: IRC2018/TPI2014

Design Program: MiTek 20/20 8.6

Wind Code: ASCE 7-16

Wind Speed: 130 mph

Roof Load: 40.0 psf

Floor Load: N/A psf

This package includes 10 individual, dated Truss Design Drawings and 0 Additional Drawings.

No.	Seal#	Truss Name	Date
1	163763038	A01	2/22/2024
2	163763039	A02	2/22/2024
3	163763040	B01	2/22/2024
4	163763041	B02	2/22/2024
5	163763042	B03	2/22/2024
6	163763043	B04	2/22/2024
7	163763044	B05	2/22/2024
8	163763045	A03	2/22/2024
9	163763046	C02	2/22/2024
10	163763047	C01	2/22/2024

The truss drawing(s) referenced above have been prepared by Truss Engineering Co. under my direct supervision based on the parameters provided by Carter Components (Sanford, NC)).

Truss Design Engineer's Name: Gilbert, Eric

My license renewal date for the state of North Carolina is December 31, 2024

North Carolina COA: C-0844

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to TRENCO. Any project specific information included is for TRENCO customers file reference purpose only, and was not taken into account in the preparation of these designs. TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.



February 22, 2024

Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	10-10-10-10-10-10-10-10-10-10-10-10-10-1
24110078	A01	Common Supported Gable	2	1	Job Reference (optional)	163763038

Run: 8.63 S Nov 1 2023 Print: 8.630 S Nov 1 2023 MiTek Industries. Inc. Wed Feb 21 08:29:23 ID:dhy??BgWMufZFdTXIPW?dbziAn1-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

Page: 1

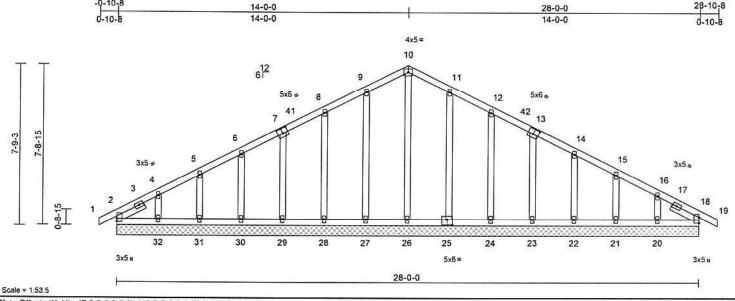


Plate Offsets (X, Y): [7:0-3-0,0-3-0], [13:0-3-0,0-3-0], [25:0-3-0,0-3-0]

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	тс	0.07	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	вс	0.04	Vert(CT)	n/a	_	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.16	Horz(CT)	0.01	18	n/a	n/a	0	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		110.2(01)	0.01	10	100	1110		
BCDL	10.0	(5) 77		mann morr							Weight: 173 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 **OTHERS** 2x4 SP No.3

SLIDER Left 2x4 SP No.3 - 1-6-0, Right 2x4 SP No.3

BRACING TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 2=28-0-0, 18=28-0-0, 20=28-0-0, 21=28-0-0, 22=28-0-0, 23=28-0-0, 24=28-0-0, 25=28-0-0, 26=28-0-0, 27=28-0-0, 28=28-0-0, 29=28-0-0,

30=28-0-0, 31=28-0-0, 32=28-0-0, 33=28-0-0, 37=28-0-0

Max Horiz 2=-114 (LC 15), 33=-114 (LC 15) Max Uplift 2=-17 (LC 15), 20=-73 (LC 15), 21=-36 (LC 15), 22=-42 (LC 15),

23=-42 (LC 15), 24=-45 (LC 15). 25=-40 (LC 15), 27=-41 (LC 14), 28=-45 (LC 14), 29=-41 (LC 14), 30=-43 (LC 14), 31=-33 (LC 14),

32=-85 (LC 14), 33=-17 (LC 15) 2=138 (LC 21), 18=139 (LC 22), Max Grav 20=154 (LC 35), 21=157 (LC 22), 22=152 (LC 35), 23=168 (LC 22), 24=226 (LC 22), 25=235 (LC 22),

26=163 (LC 27), 27=236 (LC 21), 28=225 (LC 21), 29=169 (LC 21), 30=152 (LC 34), 31=156 (LC 21), 32=154 (LC 34), 33=138 (LC 21),

37=139 (LC 22) **FORCES** (lb) - Maximum Compression/Maximum

TOP CHORD 1-2=0/23, 2-4=-135/57, 4-5=-92/63, 5-6=-72/80, 6-8=-55/129, 8-9=-69/175, 9-10=-86/216, 10-11=-86/216, 11-12=-69/176, 12-14=-52/130,

14-15=-46/44, 15-16=-59/20, 16-18=-86/33,

18-19=0/23

BOT CHORD 2-32=-23/110, 31-32=-23/110, 30-31=-23/110, 29-30=-23/110, 28-29=-24/111,

27-28=-24/111, 26-27=-24/111, 24-26=-24/111, 23-24=-23/110, 22-23=-22/109, 21-22=-22/109,

20-21=-22/109, 18-20=-22/109 10-26=-130/19, 9-27=-197/70, 8-28=-186/80, 7-29=-130/74, 6-30=-113/74, 5-31=-119/71, 4-32=-111/107, 11-25=-196/69, 12-24=-186/80, 13-23=-130/74,

14-22=-113/74, 15-21=-119/71, 16-20=-111/107

NOTES

WEBS

1) Unbalanced roof live loads have been considered for this design.

Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-0-0, Exterior(2N) 2-0-0 to 11-0-0, Corner(3R) 11-0-0 to 17-0-0, Exterior (2N) 17-0-0 to 25-10-8, Corner(3E) 25-10-8 to 28-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

Truss designed for wind loads in the plane of the truss 3) only. For study exposed to wind (normal to the face). see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated,
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.



February 22,2024

WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a fruss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Griteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	163763038
24110078	A01	Common Supported Gable	2	1	Job Reference (optional)	100700000

Run: 8.63 S Nov 1 2023 Print: 8.630 S Nov 1 2023 MiTek Industries, Inc. Wed Feb 21 08:29:23 ID:dhy??BgWMufZFdTXIPW?dbztAn1-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 2

- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 2, 41 lb uplift at joint 27, 45 lb uplift at joint 28, 41 lb uplift at joint 29, 43 lb uplift at joint 30, 33 lb uplift at joint 31, 85 lb uplift at joint 32, 40 lb uplift at joint 25, 45 lb uplift at joint 24, 42 lb uplift at joint 23, 42 lb uplift at joint 22, 36 lb uplift at joint 21, 73 lb uplift at joint 20 and 17 lb uplift at joint 20.
- Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 2, 33.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	and the real state
24110078	A02	Common	6	1	Job Reference (optional)	163763039

Run: 8.63 S. Nov. 1 2023 Print: 8.630 S. Nov. 1 2023 MiTek Industries, Inc. Wed Feb 21 08:29:29 ID:wsSwA1NuijzKCYvh9bZczpzlAm7-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWrCDoi7J4zJC?f

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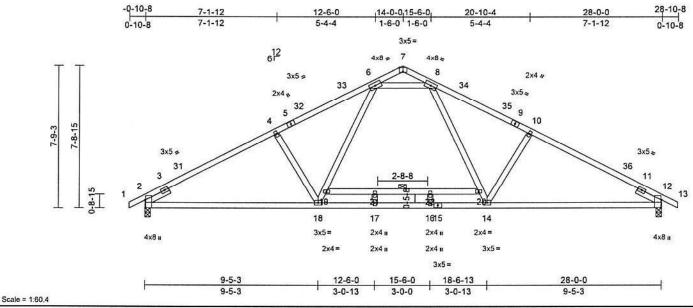


Plate Offsets (X, Y):	[2:0-5-0,Edge],	[7:0-2-8,Edge],	[12:0-5-0,Edge]
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Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.24	14-29	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	ВС	0.85	Vert(CT)	-0.32	14-29	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.35	Horz(CT)	0.07	12	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 -						
BCDL	10.0	(3)				Į.					Weight: 149 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Left 2x4 SP No.3 - 1-6-0, Right 2x4 SP No.3

1-6-0

BRACING TOP CHORD

Structural wood sheathing directly applied or

2-2-0 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

WEBS 1 Row at midpt 19-20 REACTIONS (size) 2=0-3-8, 12=0-3-8

Max Horiz 2=-118 (LC 15) Max Uplift 2=-124 (LC 14), 12=-124 (LC 15)

Max Grav 2=1188 (LC 21), 12=1188 (LC 22)

FORCES (lb) - Maximum Compression/Maximum

Tension

TOP CHORD 1-2=0/23, 2-4=-1813/242, 4-6=-1623/261,

6-7=-53/127, 7-8=-53/127, 8-10=-1623/261, 10-12=-1813/242, 12-13=0/23

BOT CHORD 2-18=-236/1552, 17-18=-43/1173, 16-17=-43/1173, 14-16=-43/1173,

12-14=-141/1552

WEBS 8-20=-83/543, 14-20=-87/521,

10-14=-406/220, 18-19=-88/521, 6-19=-83/543, 4-18=-406/220, 19-21=-19/0,

21-22=-19/0, 20-22=-19/0, 17-21=0/29,

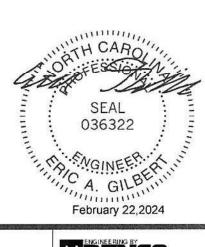
16-22=0/29, 6-8=-1213/301

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 11-0-0, Exterior(2R) 11-0-0 to 17-0-0, Interior (1) 17-0-0 to 25-10-8, Exterior(2E) 25-10-8 to 28-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12,0 psf or 1,00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 12. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



February 22,2024

WARNING - Verily design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

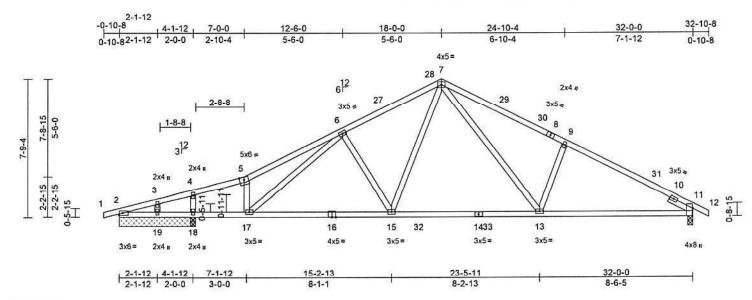
Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent localipse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSUTPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	
24110078	B01	Roof Special	1	1	Job Reference (optional)	163763040

Run: 8.63 S Nov 1 2023 Print: 8.630 S Nov 1 2023 MiTek Industries, Inc. Wed Feb 21 08:29:30 ID:RVg7QBiyzdimGUwMdJcUwwzlAoG-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:62.1

Loading	(psf)	Spacing	1-11-4	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	1.00	Vert(LL)	-0.27	15-17	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.54	15-17	>623	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.38	Horz(CT)	0.07	11	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		30 50					***************************************	
BCDL	10.0	7,500,000		SSELFRAMMONAMEN							Weight: 159 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1 *Except* 5-7,8-12:2x4 SP No.2 2x4 SP 2400F 2.0E *Except* 16-14:2x4 SP **BOT CHORD**

2x4 SP No.3 WEBS

Right 2x4 SP No.3 -- 1-6-0 SLIDER

BRACING

TOP CHORD Structural wood sheathing directly applied. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing.

No.1

REACTIONS (size) 2=4-3-8, 11=0-3-8, 18=4-3-8,

19=4-3-8, 20=4-3-8

Max Horiz 2=116 (LC 14), 20=116 (LC 14) Max Uplift 2=-77 (LC 14), 11=-121 (LC 15)

18=-135 (LC 14), 19=-842 (LC 36),

20=-77 (LC 14)

2=1081 (LC 5), 11=1291 (LC 6), Max Grav 18=1230 (LC 5), 19=44 (LC 14),

20=1081 (LC 5)

(lb) - Maximum Compression/Maximum **FORCES**

TOP CHORD

1-2=0/17, 2-3=-2473/292, 3-4=-2371/299, 4-5=-2488/334, 5-6=-2615/411, 6-7=-1832/373, 7-9=-1937/409,

9-11=-2027/349, 11-12=0/23 2-19=-234/2354, 18-19=-234/2354,

17-18=-234/2354, 15-17=-181/1823,

13-15=-54/1253, 11-13=-193/1755

5-17=-556/158, 6-17=-55/687,

6-15=-613/201, 7-15=-87/921 7-13=-144/721, 9-13=-428/221,

4-18=-580/111, 3-19=0/364

NOTES

WEBS

BOT CHORD

Unbalanced roof live loads have been considered for 1) this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-12, Interior (1) 2-1-12 to 14-9-10, Exterior(2R) 14-9-10 to 21-2-6, Interior (1) 21-2-6 to 29-8-2, Exterior(2E) 29-8-2 to 32-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 842 lb uplift at joint 19
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at joint 11. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



February 22,2024

is and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE Design valid for use only with MTTeKs connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/THI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSB-Wildon Component Safety Information available from the Storage Building Component Assets Information available from the Storage Building Component Assets Information available from the Storage Building Component Safety Information.

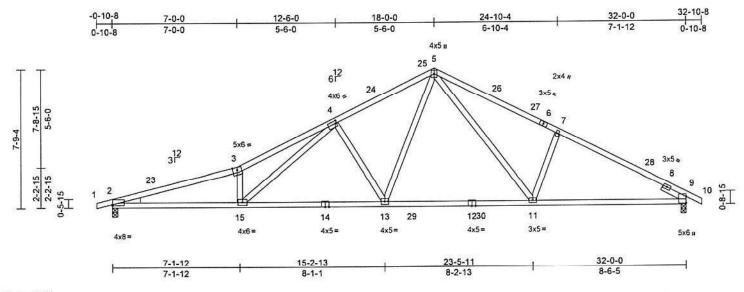
and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	163763041
24110078	B02	Roof Special	3	1	Job Reference (optional)	103703041

Run: 8.63 S Nov 1 2023 Print: 8.630 S Nov 1 2023 MiTek Industries, Inc. Wed Feb 21 08:29:31 ID:6bneuflm5RCIFv2PGq563fzIAkM-RfC?PsB70Hq3NSgPqnL8w3uITXbGKWfCDoi7J4zJC?f

Page: 1



Scale = 1:62.1

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defi	0.555 (55)	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.97	Vert(LL)	-0.31	13-15	>999	240	MT20	244/190
Snow (Pf)	20,0	Lumber DOL	1,15	BC	0.80	Vert(CT)	-0.59	13-15	>647	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.84	Horz(CT)	0.10	9	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		60 10					a District of the Court of the	
BCDL	10.0	98.200 W.C.		85500V							Weight: 158 lb	FT = 20%

LUMBER

2x4 SP No.1 *Except* 5-6:2x4 SP 2400F TOP CHORD

2x4 SP 2400F 2.0E *Except* 14-12:2x4 SP BOT CHORD

No.1

WEBS 2x4 SP No.3 WEDGE Left: 2x4 SP No.3

SLIDER Right 2x4 SP No.3 -- 1-6-0 BRACING

TOP CHORD

Structural wood sheathing directly applied.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 2=0-3-8, 9=0-3-8

Max Horiz 2=120 (LC 14)

Max Uplift 2=-158 (LC 14), 9=-126 (LC 15)

Max Grav 2=1440 (LC 5), 9=1437 (LC 3)

(lb) - Maximum Compression/Maximum **FORCES**

Tension

1-2=0/17, 2-3=-4112/594, 3-4=-4335/701, TOP CHORD

4-5=-2216/441, 5-7=-2187/458, 7-9=-2289/397, 9-10=0/23

2-15=-496/3938, 13-15=-262/2335,

BOT CHORD 11-13=-90/1494, 9-11=-230/1978

3-15=-1150/282, 4-15=-282/2022, WEBS

4-13=-916/252, 5-13=-131/1205,

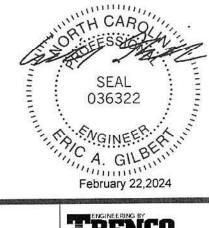
5-11=-147/732, 7-11=-429/227

NOTES

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-3-14, Interior (1) 2-3-14 to 14-9-10, Exterior(2R) 14-9-10 to 21-2-6, Interior (1) 21-2-6 to 29-8-2, Exterior(2E) 29-8-2 to 32-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 9. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



MARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE MARNING: -Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1,272023 BEFORE USE.

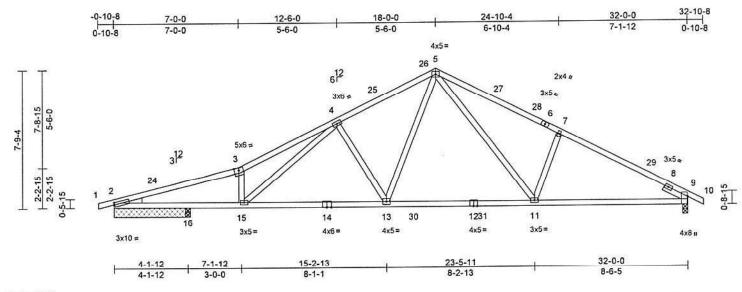
Dosign valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly manage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	163763042
24110078	B03	Roof Special	1	1	Job Reference (optional)	100/000/12

Run: 8.63 S Nov 1 2023 Print: 8,630 S Nov 1 2023 MiTek Industries, Inc., Wed Feb 21 08:29:33 ID:IF?fL2H?VGtv9q_2PmOftN8zlAjf-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



Scale = 1:62.1

.oading CLL (roof) Snow (Pf) CDL	(psf) 20,0 20,0 10,0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	2-0-0 1.15 1.15 YES	TC BC WB	0.92 0.93 0.51	Vert(LL) Vert(CT) Horz(CT)	-0.33 -0.65 0.09	(loc) 13-15 13-15	l/defl >999 >518 n/a	2753270	MT20	GRIP 244/190
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		12 48					Weight: 158 lb	ET 000/

LUMBER

TOP CHORD 2x4 SP 2400F 2.0E *Except* 3-5,6-10:2x4 SP

BOT CHORD 2x4 SP 2400F 2.0E *Except* 14-12:2x4 SP No.1

WEBS

2x4 SP No 3 Left: 2x4 SP No.3 WEDGE Right 2x4 SP No.3 -- 1-6-0 SLIDER

BRACING TOP CHORD Structural wood sheathing directly applied. **BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc

bracing, Except: 2-2-0 oc bracing: 13-15.

REACTIONS (size) 2=4-3-8, 9=0-3-8, 16=0-3-8

Max Horiz 2=120 (LC 14)

Max Uplift 2=-118 (LC 14), 9=-127 (LC 15), 16=-46 (LC 14)

2=1036 (LC 5), 9=1378 (LC 3), Max Grav

16=465 (LC 5)

(lb) - Maximum Compression/Maximum FORCES

Tension

TOP CHORD 1-2=0/17, 2-3=-3221/482, 3-4=-3380/581,

4-5=-2039/419, 5-7=-2071/442, 7-9=-2166/381, 9-10=0/23

BOT CHORD 2-16=-387/3067, 15-16=-387/3067,

13-15=-230/2077, 11-13=-76/1384,

9-11=-216/1874

WEBS 3-15=-916/252, 4-15=-183/1236, 4-13=-741/229, 5-13=-113/1065,

5-11=-147/731, 7-11=-433/226

NOTES

Unbalanced roof live loads have been considered for 1) this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-3-14, Interior (1) 2-3-14 to 14-9-10, Exterior(2R) 14-9-10 to 21-2-6, Interior (1) 21-2-6 to 29-8-2, Exterior(2E) 29-8-2 to 32-10-8 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00: Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One RT8A MiTek connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2. This connection is for uplift only and does not consider lateral
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 9. This connection is for uplift only and does not consider lateral forces.
- 10) H10A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 16. This connection is for uplift only and does not consider lateral forces.

11) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



eters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MILTATA tov. 1.27023 BEFORE USE.

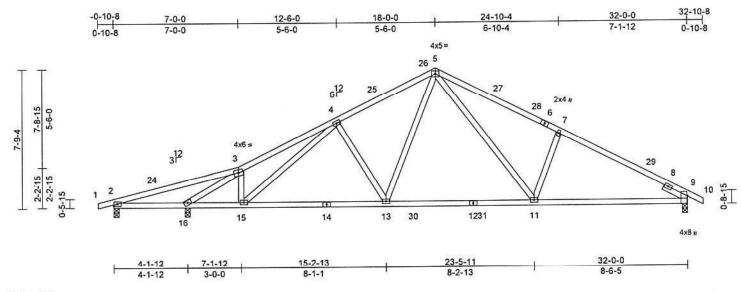
Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly manage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	163763043
24110078	B04	Roof Special	2	1	Job Reference (optional)	103700040

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Page: 1



Scale = 1:62.1

Loading (psf FCLL (roof) 20.0 Snow (Pf) 20.0	Spacing Plate Grip DOL Lumber DOL	2-0-0 1.15 1.15	CSI TC BC	1.00 0.98	DEFL Vert(LL) Vert(CT) Horz(CT)	-0.22 -0.36 0.07	(loc) 11-13 11-13	l/defl >999 >945 n/a	(2000)	PLATES MT20	GRIP 244/190
TCDL 10.0 BCLL 0.0	Rep Stress Incr Code	YES IRC2018/TPI2014	WB Matrix-MSH	0.73	Horz(C1)	0.07	3	IVa	108.400	Weight: 162 lb	CT - 20%

LUMBER

TOP CHORD 2x4 SP No.2 *Except* 5-6:2x4 SP No.1

BOT CHORD 2x4 SP No.2 WEBS 2x4 SP No.3

SLIDER Right 2x4 SP No.3 -- 1-6-0

BRACING

FORCES

BOT CHORD

TOP CHORD Structural wood sheathing directly applied. **BOT CHORD** Rigid ceiling directly applied or 2-2-0 oc

REACTIONS (size) 2=0-3-8, 9=0-3-8, 16=0-3-8

Max Horiz 2=120 (LC 14)

Max Uplift 2=-123 (LC 10), 9=-126 (LC 15),

16=-130 (LC 14) 2=259 (LC 39), 9=1256 (LC 6), Max Grav

16=1571 (LC 5) (lb) - Maximum Compression/Maximum

Tension

1-2=0/17, 2-3=-102/622, 3-4=-1810/381, TOP CHORD

4-5=-1617/364, 5-7=-1849/412,

7-9=-1942/350, 9-10=0/23

2-16=-577/97, 15-16=-224/1564, 13-15=-160/1536, 11-13=-44/1136,

9-11=-189/1677

3-16=-2433/318, 3-15=-134/185,

4-15=-109/291, 4-13=-441/205, 5-13=-84/707, 5-11=-149/766, 7-11=-441/228

NOTES

WEBS

Unbalanced roof live loads have been considered for 1) this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-3-14, Interior (1) 2-3-14 to 14-9-10, Exterior(2R) 14-9-10 to 21-2-6, Interior (1) 21-2-6 to 29-8-2, Exterior(2E) 29-8-2 to 32-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 3x5 MT20 unless otherwise indicated.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2, 9, and 16. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

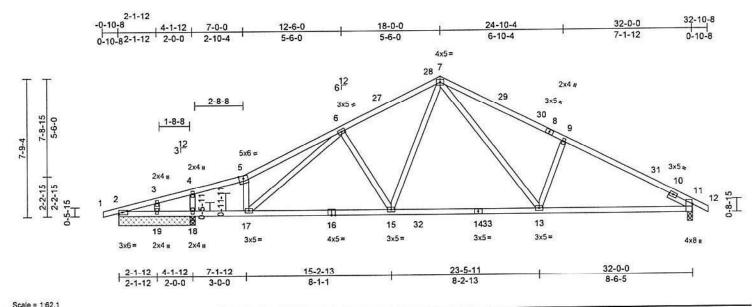
LOAD CASE(S) Standard



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	163763044
24110078	B05	Roof Special	1	1	Job Reference (optional)	

Run; 8.63 S Nov 1 2023 Print; 8.630 S Nov 1 2023 MiTek Industries, Inc. Wed Feb 21 08:29:34 ID:vVpnLyLajbGOVUvLgEXg1uzlAnS-RfC?PsB70Hq3NSgPqnL8w3ulTXbGKWrCDoi7J4zJC?f

Page: 1



PCU 0.0° Code IRC2018/TPI2014 Matrix-MSH	Loading TCLL (roof) Snow (Pf) TCDL	(psf) 20.0 20.0 10.0	Spacing Plate Grip DOL Lumber DOL Rep Stress Incr	1-11-4 1.15 1.15 YES	CSI TC BC WB	1.00 0.83 0.38	DEFL Vert(LL) Vert(CT) Horz(CT)	-0.27 -0.54 0.07	(loc) 15-17 15-17 11	l/defl >999 >623 n/a	00000000	PLATES MT20	GRIP 244/190
Weight: 159 lb FT = 2	BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		0. 76				1	ROUNG-POORSELINE	

LUMBER

TOP CHORD 2x4 SP No.1 *Except* 5-7,8-12:2x4 SP No.2 BOT CHORD

2x4 SP 2400F 2.0E *Except* 16-14:2x4 SP No.1

WEBS 2x4 SP No.3

Right 2x4 SP No.3 -- 1-6-0 SLIDER

BRACING

TOP CHORD Structural wood sheathing directly applied. BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing

REACTIONS (size) 2=4-3-8, 11=0-3-8, 18=4-3-8,

19=4-3-8, 20=4-3-8

2=116 (LC 14), 20=116 (LC 14) Max Horiz Max Uplift 2=-77 (LC 14), 11=-121 (LC 15), 18=-135 (LC 14), 19=-842 (LC 36),

20=-77 (LC 14)

Max Grav 2=1081 (LC 5), 11=1291 (LC 6), 18=1230 (LC 5), 19=44 (LC 14),

20=1081 (LC 5)

FORCES (lb) - Maximum Compression/Maximum

TOP CHORD 1-2=0/17, 2-3=-2473/292, 3-4=-2371/299,

4-5=-2488/334, 5-6=-2615/411, 6-7=-1832/373, 7-9=-1937/409,

9-11=-2027/349, 11-12=0/23 2-19=-234/2354, 18-19=-234/2354, 17-18=-234/2354, 15-17=-181/1823, BOT CHORD

13-15=-54/1253, 11-13=-193/1755 5-17=-556/158, 6-17=-55/687,

6-15=-613/201, 7-15=-87/921, 7-13=-144/721, 9-13=-428/221,

4-18=-580/111, 3-19=0/364

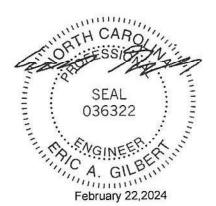
NOTES

WEBS

Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-12, Interior (1) 2-1-12 to 14-9-10, Exterior(2R) 14-9-10 to 21-2-6, Interior (1) 21-2-6 to 29-8-2, Exterior(2E) 29-8-2 to 32-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown: Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00: Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 842 lb uplift at joint 19
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at joint 11. This connection is for uplift only and does not consider lateral forces.
- 10) This truss is designed in accordance with the 2018 International Residential Code sections R502,11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



WAPNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE,
Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not
a truss system. Before use, the building designer must verify the applicability of design parameters and property incorporate this design into the overall
building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing
is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the
fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPH Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org)
and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	TOURS OF A TAX
24110078 A03 Comm		5	1	Job Reference (optional)	163763045
ford, NC), Sanford, NC - 2733	2,	Run: 8.63 S Nov 1 2023 Print	8.630 S No	v 1 2023 MiTek Industries, Inc. Wed Feb 21 08:29:30	Page:
•	A03	A03 Common	A03 Common 5	A03 Common 5 1	A03 Common 5 1 Job Reference (optional)

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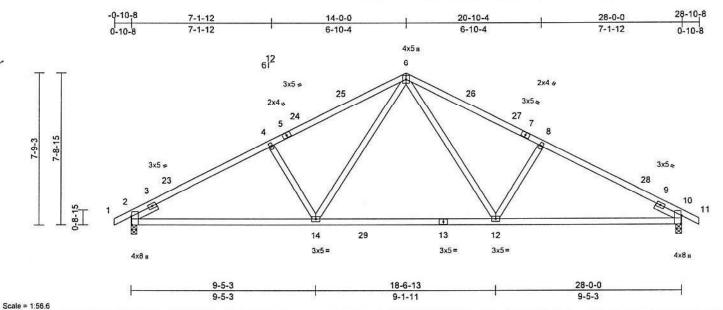


Plate Offsets (X, Y): [2:0-5-0,Edge], [10:0-5-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	l/defi	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1,15	TC	0.98	Vert(LL)	-0.29	12-14	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.74	Vert(CT)	-0.45	12-14	>744	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.31	Horz(CT)	0.07	10	n/a	n/a		
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH		1000 - WH 80				-		
BCDL	10.0	58-25/S20(40%)		59650-52504390G458654							Weight: 134 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.1 *Except* 1-5,7-11:2x4 SP No.2 2x4 SP No.1 **BOT CHORD**

2x4 SP No.3 WEBS

Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 SLIDER

-- 1-6-0

BRACING

TOP CHORD Structural wood sheathing directly applied.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size) 2=0-3-8, 10=0-3-8

Max Horiz 2=-118 (LC 15)

Max Uplift 2=-124 (LC 14), 10=-124 (LC 15) Max Grav 2=1276 (LC 5), 10=1275 (LC 6)

FORCES (lb) - Maximum Compression/Maximum

Tension

TOP CHORD 1-2=0/23, 2-4=-1972/248, 4-6=-1813/273, 6-8=-1812/273, 8-10=-1971/248, 10-11=0/23

BOT CHORD 2-14=-236/1705, 12-14=-29/1160,

10-12=-139/1704

WEBS 6-12=-101/753, 8-12=-437/224,

6-14=-101/755, 4-14=-437/224

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 11-0-0, Exterior(2R) 11-0-0 to 17-0-0, Interior (1) 17-0-0 to 25-10-8, Exterior(2E) 25-10-8 to 28-10-8 zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20,0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 8) One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 10. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK PEFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE.

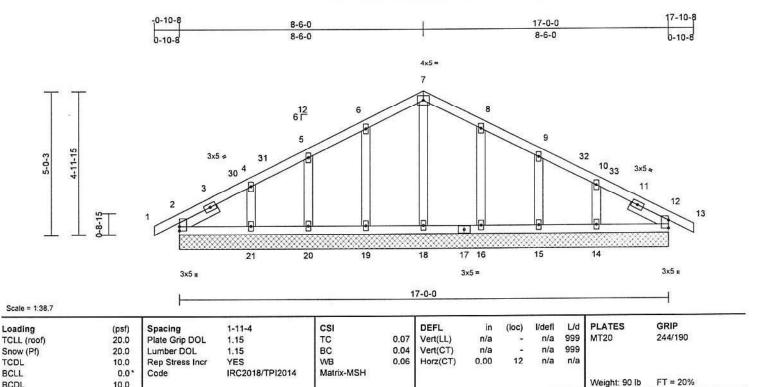
Design valid for use only with MITEK® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord mehors only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPI1 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org). and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	163763046
24110078	C02	Common Supported Gable	1	1	Job Reference (optional)	103703040

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_	1	2	10			_	

BCDL

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2

2x4 SP No.3 **OTHERS**

SLIDER Left 2x4 SP No.3 -- 1-6-0, Right 2x4 SP No.3 -- 1-6-0

10.0

BRACING

TOP CHORD Structural wood sheathing directly applied or

6-0-0 oc purlins

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size)

2=17-0-0, 12=17-0-0, 14=17-0-0, 15=17-0-0, 16=17-0-0, 18=17-0-0, 19=17-0-0, 20=17-0-0, 21=17-0-0,

22=17-0-0, 26=17-0-0 Max Horiz 2=72 (LC 14), 22=72 (LC 14)

Max Uplift 2=-20 (LC 15), 12=-8 (LC 15),

14=-66 (LC 15), 15=-36 (LC 15), 16=-46 (LC 15), 19=-47 (LC 14), 20=-33 (LC 14), 21=-72 (LC 14),

22=-20 (LC 15), 26=-8 (LC 15) 2=163 (LC 1), 12=163 (LC 1), 14=195 (LC 22), 15=213 (LC 22), Max Grav

16=239 (LC 22), 18=126 (LC 27), 19=239 (LC 21), 20=213 (LC 21), 21=195 (LC 21), 22=163 (LC 1),

26=163 (LC 1)

FORCES (lb) - Maximum Compression/Maximum

Tension

1-2=0/23, 2-4=-61/46, 4-5=-65/74, TOP CHORD

5-6=-70/122, 6-7=-77/173, 7-8=-77/173,

8-9=-70/122, 9-10=-63/74, 10-12=-45/19,

12-13=0/23 **BOT CHORD**

2-21=0/60, 20-21=0/60, 19-20=0/60,

18-19=0/60, 16-18=0/60, 15-16=0/60,

14-15=0/60, 12-14=0/60 7-18=-87/7, 6-19=-200/92, 5-20=-179/84. WEBS

> 4-21=-143/116, 8-16=-200/92, 9-15=-179/84, 10-14=-143/116

NOTES

- Unbalanced roof live loads have been considered for 1) this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Corner(3E) -0-10-8 to 2-1-8, Exterior(2N) 2-1-8 to 5-6-0, Comer(3R) 5-6-0 to 11-6-0, Exterior(2N) 11-6-0 to 14-10-8, Corner(3E) 14-10-8 to 17-10-8 zone; cantilever left and right exposed; end vertical left and right exposed: C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- All plates are 2x4 MT20 unless otherwise indicated.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 2-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.

- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 20 lb uplift at joint 2, 8 lb uplift at joint 12, 47 lb uplift at joint 19, 33 lb uplift at joint 20, 72 lb uplift at joint 21, 46 lb uplift at joint 16, 36 lb uplift at joint 15, 66 lb uplift at joint 14, 20 lb uplift at joint 2 and 8 lb uplift at joint 12.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802,10,2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



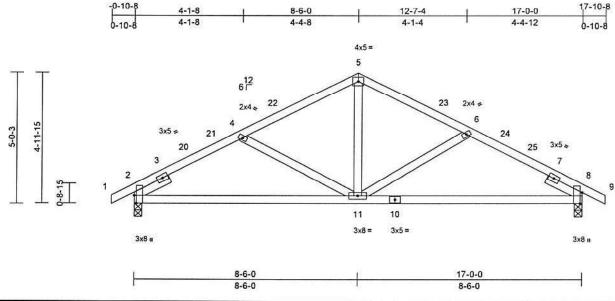
ters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 1/2/2023 BEFORE USE Design valid for use only with MiTeks connectors. This design is based only upon parameters and properly incorporate this design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and properly damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TPHI Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org). and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcacomponents.com)



Job	Truss	Truss Type	Qty	Ply	0 Rainbow-Roof-Nicolas - GRH	
24110078	C01	Common	2	1	Job Reference (optional)	163763047

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Scale = 1:42,3 Plate Offsets (X, Y): [2:0-3-8,Edge], [8:0-5-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI		DEFL	in	(loc)	I/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.45	Vert(LL)	-0.07	11-18	>999	240	MT20	244/190
Snow (Pf)	20.0	Lumber DOL	1.15	BC	0.58	Vert(CT)	-0.14	11-18	>999	180	111111111111111111111111111111111111111	
TCDL	10.0	Rep Stress Incr	YES	WB	0.18	Horz(CT)	0.03	8	n/a	n/a	8	
BCLL	0.0*	Code	IRC2018/TPI2014	Matrix-MSH	1276		23.0	100	1010000	03///07/6		
BCDL	10.0	2000									Weight: 82 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2 **BOT CHORD** 2x4 SP No.2 WEBS 2x4 SP No.3

Left 2x4 SP No.3 - 1-6-0, Right 2x4 SP No.3 SLIDER

BRACING TOP CHORD

Structural wood sheathing directly applied or

5-0-6 oc purlins.

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc

bracing.

REACTIONS (size)

2=0-3-8, 8=0-3-8 Max Horiz 2=-74 (LC 15)

Max Uplift 2=-81 (LC 14), 8=-81 (LC 15)

Max Grav 2=798 (LC 21), 8=798 (LC 22)

FORCES Tension

(lb) - Maximum Compression/Maximum

TOP CHORD 1-2=0/23, 2-4=-1128/278, 4-5=-833/218,

5-6=-826/220, 6-8=-1114/274, 8-9=0/23

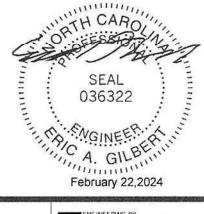
BOT CHORD 2-11=-161/970, 8-11=-154/954 WEBS 5-11=-33/432, 6-11=-348/143, 4-11=-355/147

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior(2E) -0-10-8 to 2-1-8, Interior (1) 2-1-8 to 5-6-0, Exterior(2R) 5-6-0 to 11-6-0, Interior (1) 11-6-0 to 14-10-8, Exterior(2E) 14-10-8 to 17-10-8 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- Unbalanced snow loads have been considered for this design.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 20.0 psf on overhands non-concurrent with other live loads.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- One H2.5A Simpson Strong-Tie connectors recommended to connect truss to bearing walls due to UPLIFT at jt(s) 2 and 8. This connection is for uplift only and does not consider lateral forces.
- This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



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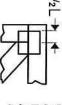
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818 Soundside Road

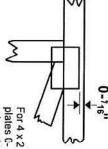
Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated.

Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 116" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek software or upon request.

PLATE SIZE

4 × 4

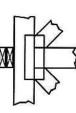
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number/letter where bearings occur Min size shown is for crushing only.

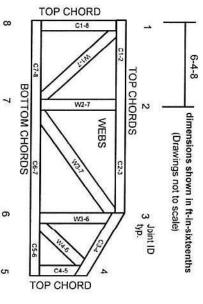
Industry Standards:

ANSI/TPI1: National Design

DSB-22

National Design Specification for Metal Plate Connected Wood Truss Construction. Design Standard for Bracing. Building Component Safety Information, Guide to Good Practice for Handling, Installing, Restraining & Bracing of Metal

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

Product Code Approvals

CC-ES Reports:

ESR-1988, ESR-2362, ESR-2685, ESR-3282 ESR-4722, ESL-1388

Design General Notes

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MIG.



MiTek Engineering Reference Sheet: MII-7473 rev. 1/2/2023

▲ General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.

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Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.

4.

Cut members to bear tightly against each other

5

- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft, spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or after truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.