

Job 72434515	Truss A1	Truss Type Truss	Qty 2	Ply 2	SD - AVERY CFI Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

Run: 8.81 S Sep 13 2024 Print: 8.810 S Sep 13 2024 MiTek Industries, Inc. Fri Nov 08 13:48:22

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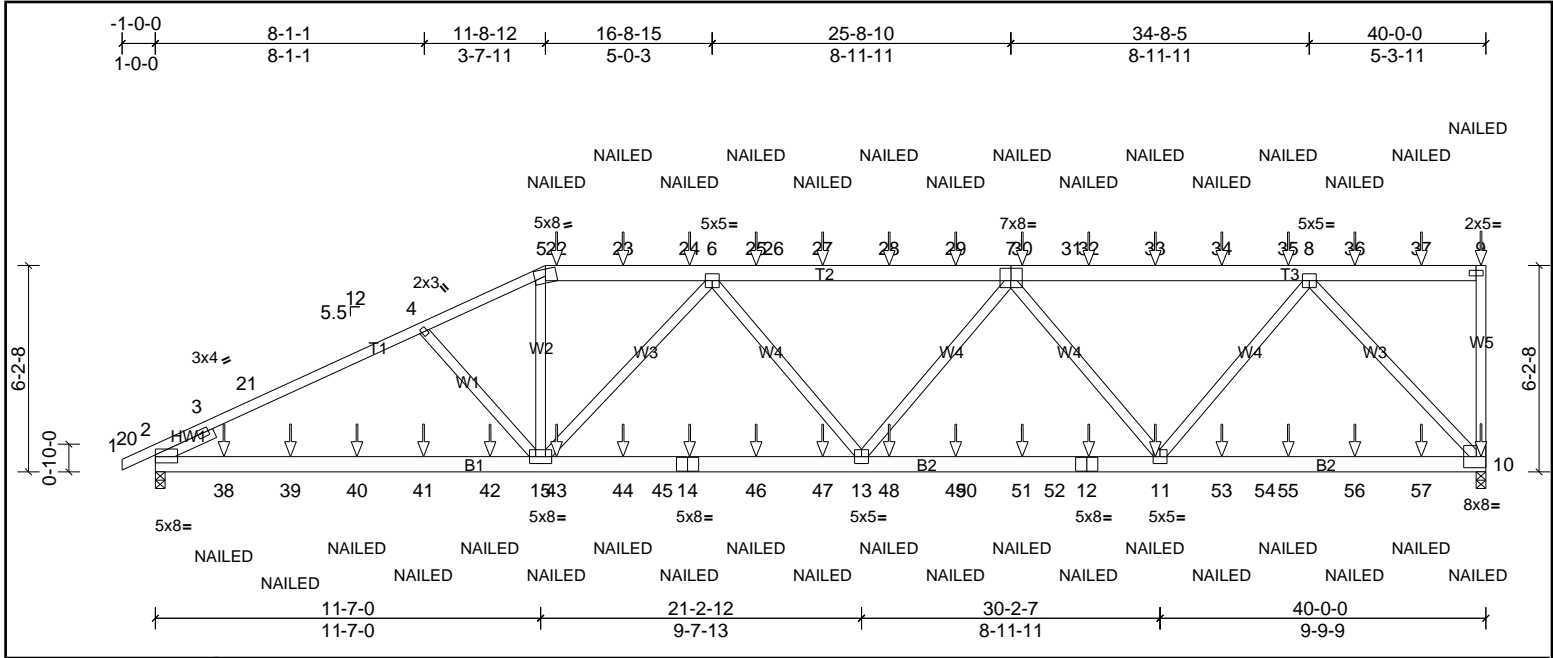


Plate Offsets (X, Y): [2:Edge,0-2-14], [7:0-4-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.79	Vert(LL)	0.18	13-15	>999	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.86	Vert(CT)	-0.33	13-15	>999	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.96	Horz(CT)	0.08	10	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											

Weight: 540 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x6 SP No.2 *Except* T1:2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied or 4-6-12 oc purlins, except end
BOT CHORD 2x6 SP No.2	VERTICALS and 2-0-0 oc purlins (6-0-0 max.): 5-9.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
SLIDER Left 2x4 SP No.3 -- 1-11-0	

REACTIONS	(lb/size)
Max Horiz	2=197 (LC 53)
Max Uplift	2=508 (LC 12), 10=1070 (LC 9)
Max Grav	2=2825 (LC 29), 10=3548 (LC 29)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-2957/205, 3-21=-5166/1083, 4-21=-5135/1112, 4-5=-4998/1128, 5-22=-4476/1035, 22-23=-4479/1036, 23-24=-4482/1037, 6-24=-4485/1039, 6-25=-5692/1516, 25-26=-5692/1516, 26-27=-5692/1516, 27-28=-5692/1516, 28-29=-5692/1516, 7-29=-5692/1516, 7-30=-4299/1150, 30-31=-4299/1150, 31-32=-4299/1150, 32-33=-4299/1150, 33-34=-4299/1150, 34-35=-4299/1150, 35-36=-4299/1150, 9-10=-297/208
BOT CHORD	2-38=-1097/4584, 38-39=-1097/4584, 39-40=-1097/4584, 40-41=-1097/4584, 41-42=-1097/4584, 15-42=-1097/4584, 15-43=-1549/5499, 43-44=-1549/5499, 44-45=-1549/5499, 14-45=-1549/5499, 14-46=-1549/5499, 46-47=-1549/5499, 13-47=-1549/5499, 13-48=-1626/5408, 48-49=-1626/5408, 49-50=-1626/5408, 50-51=-1626/5408, 51-52=-1626/5408, 12-52=-1626/5408, 11-12=-1626/5408, 11-53=-870/2666, 53-54=-870/2666, 54-55=-870/2666, 55-56=-870/2666, 56-57=-870/2666, 10-57=-870/2666
WEBS	4-15=-412/264, 5-15=-272/1779, 6-15=-1569/783, 6-13=0/367, 7-13=0/545, 7-11=-1806/775, 8-11=-456/2654, 8-10=-3953/1306

- NOTES**
- 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2x4 - 1 row at 0-9-0 oc, 2x6 - 2 rows staggered at 0-9-0 oc.
Bottom chords connected as follows: 2x6 - 2 rows staggered at 0-9-0 oc.
Web connected as follows: 2x4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1070 lb uplift at joint 10 and 508 lb uplift at joint 2.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - "NAILED" indicates Girder: 3-10d (0.148" x 3") toe-nails per NDS guidelines.
- LOAD CASE(S)** Standard
- Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss A1	Truss Type Truss	Qty 2	Ply 2	SD - AVERY CFI Job Reference (optional)
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Uniform Loads (lb/ft)

Vert: 1-5=-51, 5-9=-61, 10-16=-20

Concentrated Loads (lb)

Vert: 9=-96 (B), 10=-53 (B), 14=-44 (B), 12=-44 (B), 11=-44 (B), 22=-72 (B), 23=-68 (B), 24=-68 (B), 25=-68 (B), 27=-68 (B), 28=-68 (B), 29=-68 (B), 30=-68 (B), 32=-68 (B), 33=-68 (B), 34=-68 (B), 35=-68 (B), 36=-68 (B), 37=-68 (B), 38=-45 (B), 39=-48 (B), 40=-48 (B), 41=-47 (B), 42=-45 (B), 43=-44 (B), 44=-44 (B), 46=-44 (B), 47=-44 (B), 48=-44 (B), 49=-44 (B), 51=-44 (B), 53=-44 (B), 55=-44 (B), 56=-44 (B), 57=-44 (B)



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Job 72434515	Truss A2	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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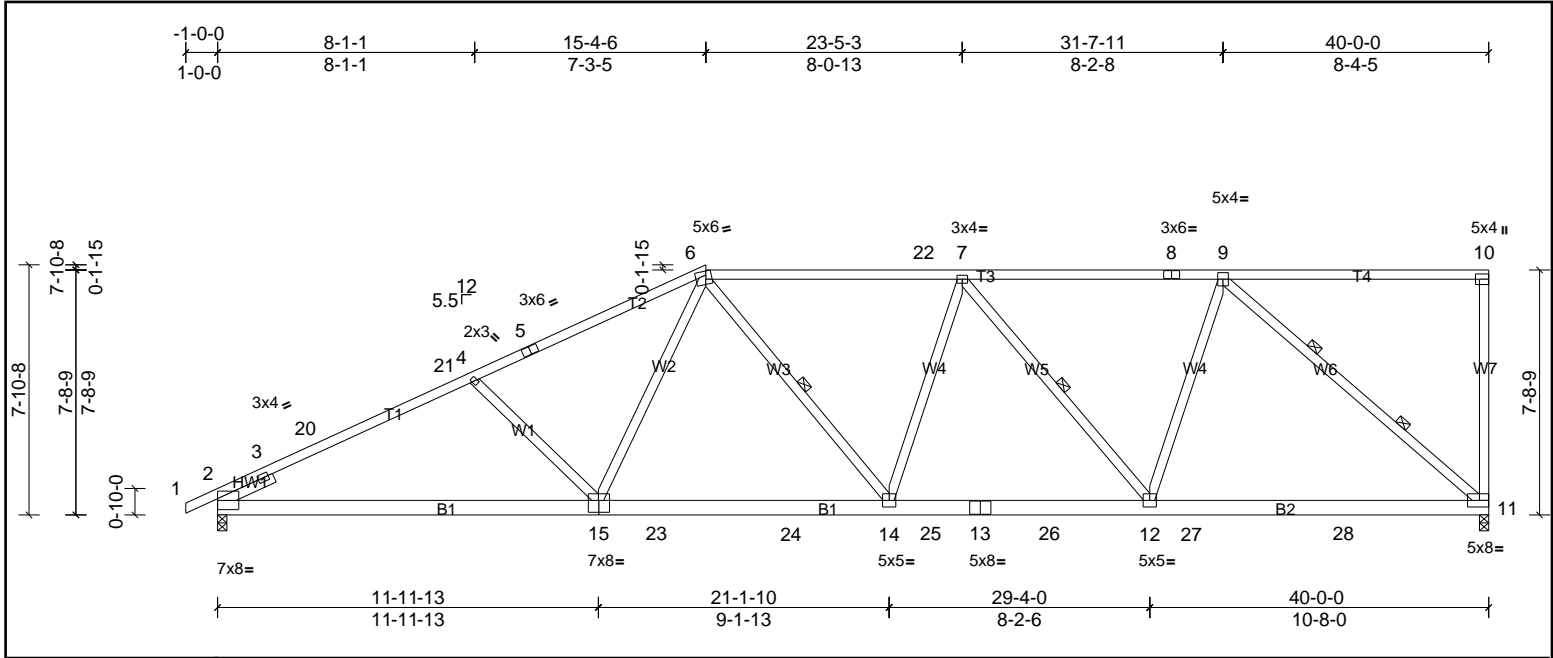


Plate Offsets (X, Y): [2:Edge,0-4-2], [6:0-2-0,Edge], [10:Edge,0-3-8], [15:0-4-0,0-4-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.99	Vert(LL)	-0.21	14-15	>999	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.81	Vert(CT)	-0.35	14-15	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.98	Horz(CT)	0.09	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											

Weight: 255 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP SS *Except* T1,T4:2x4 SP No.1	TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 6-10.
BOT CHORD 2x6 SP No.1 *Except* B2:2x6 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3 *Except* W6:2x4 SP No.1	WEBS 1 Row at midpt 6-14, 7-12
SLIDER Left 2x4 SP No.3 -- 1-11-0	WEBS 2 Rows at 1/3 pts 9-11

REACTIONS

(lb/size)	2=1538/0-3-8, (min. 0-2-14), 11=1580/0-3-8, (min. 0-3-4)
Max Horiz	2=252 (LC 14)
Max Uplift	2=90 (LC 14), 11=185 (LC 11)
Max Grav	2=1817 (LC 3), 11=2073 (LC 39)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1655/0, 3-20=-3461/127, 20-21=-3390/157, 4-21=-3327/160, 4-5=-3199/131, 5-6=-3127/163, 6-22=-2946/213, 7-22=-2949/212, 7-8=-2415/168, 8-9=-2415/168, 10-11=-302/78
BOT CHORD	2-15=-307/3128, 15-23=-199/2651, 23-24=-199/2651, 14-24=-199/2651, 14-25=-242/2956, 13-25=-242/2956, 13-26=-242/2956, 12-26=-242/2956, 12-27=-181/2065, 27-28=-181/2065, 11-28=-181/2065
WEBS	4-15=-585/225, 6-15=-35/827, 7-14=-185/349, 6-14=-81/520, 7-12=-862/119, 9-12=0/1170, 9-11=-2728/238

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 185 lb uplift at joint 11 and 90 lb uplift at joint 2.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-6=-51, 6-10=-61, 11-16=-20



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Job 72434515	Truss A3	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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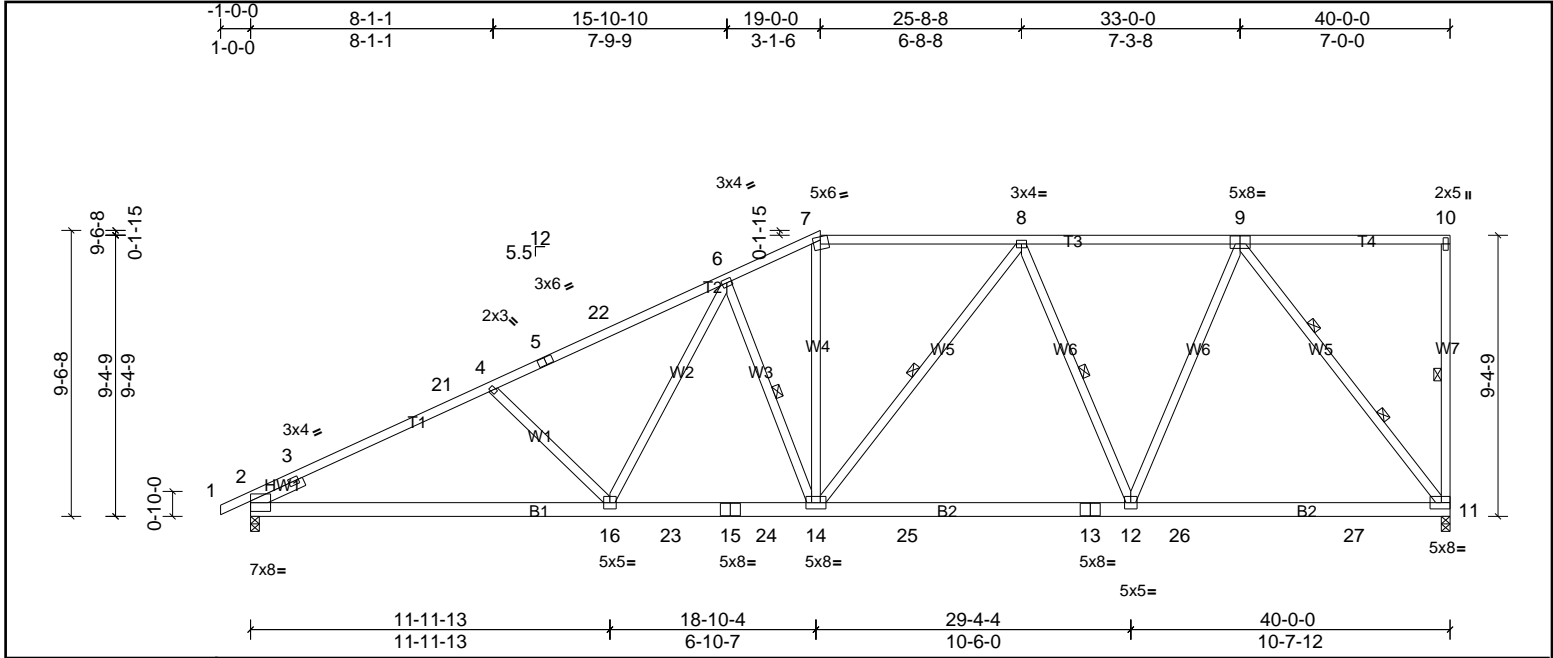


Plate Offsets (X, Y): [2:Edge,0-4-2], [9:0-4-0,0-3-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.95	Vert(LL)	-0.18	12-14	>999	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.83	Vert(CT)	-0.32	12-14	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.99	Horz(CT)	0.08	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											

Weight: 279 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.1 *Except* T3,T4:2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied, except end verticals, and 2-0-0 oc purlins (2-2-0 max.): 7-10.
BOT CHORD 2x6 SP No.2 *Except* B1:2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3 *Except* W5:2x4 SP No.2	WEBS 1 Row at midpt 10-11, 8-14, 8-12, 6-14
SLIDER Left 2x4 SP No.3 -- 1-11-0	WEBS 2 Rows at 1/3 pts 9-11

REACTIONS (lb/size) 2=1517/0-3-8, (min. 0-2-14), 11=1564/0-3-8, (min. 0-3-3)

Max Horiz 2=309 (LC 14)

Max Uplift 2=109 (LC 14), 11=173 (LC 11)

Max Grav 2=1821 (LC 3), 11=2038 (LC 39)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD 2-3=-1825/0, 3-21=-3497/162, 4-21=-3388/194, 4-5=-3194/123, 5-22=-3093/138, 6-22=-3092/158, 6-7=-2542/155, 7-8=-2279/146, 8-9=-1916/116

BOT CHORD 2-16=-387/3192, 16-23=-209/2547, 15-23=-209/2547, 15-24=-209/2547, 14-24=-209/2547, 14-25=-171/2197, 13-25=-171/2197, 12-13=-171/2197, 12-26=-122/1417, 26-27=-122/1417, 11-27=-122/1417

WEBS 7-14=-31/841, 8-14=-91/464, 8-12=-759/144, 9-12=0/1340, 9-11=-2313/200, 4-16=-529/222, 6-16=-52/704, 6-14=-896/208

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 173 lb uplift at joint 11 and 109 lb uplift at joint 2.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (lb/ft)

Vert: 1-7=-51, 7-10=-61, 11-17=-20



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Job 72434515	Truss A4	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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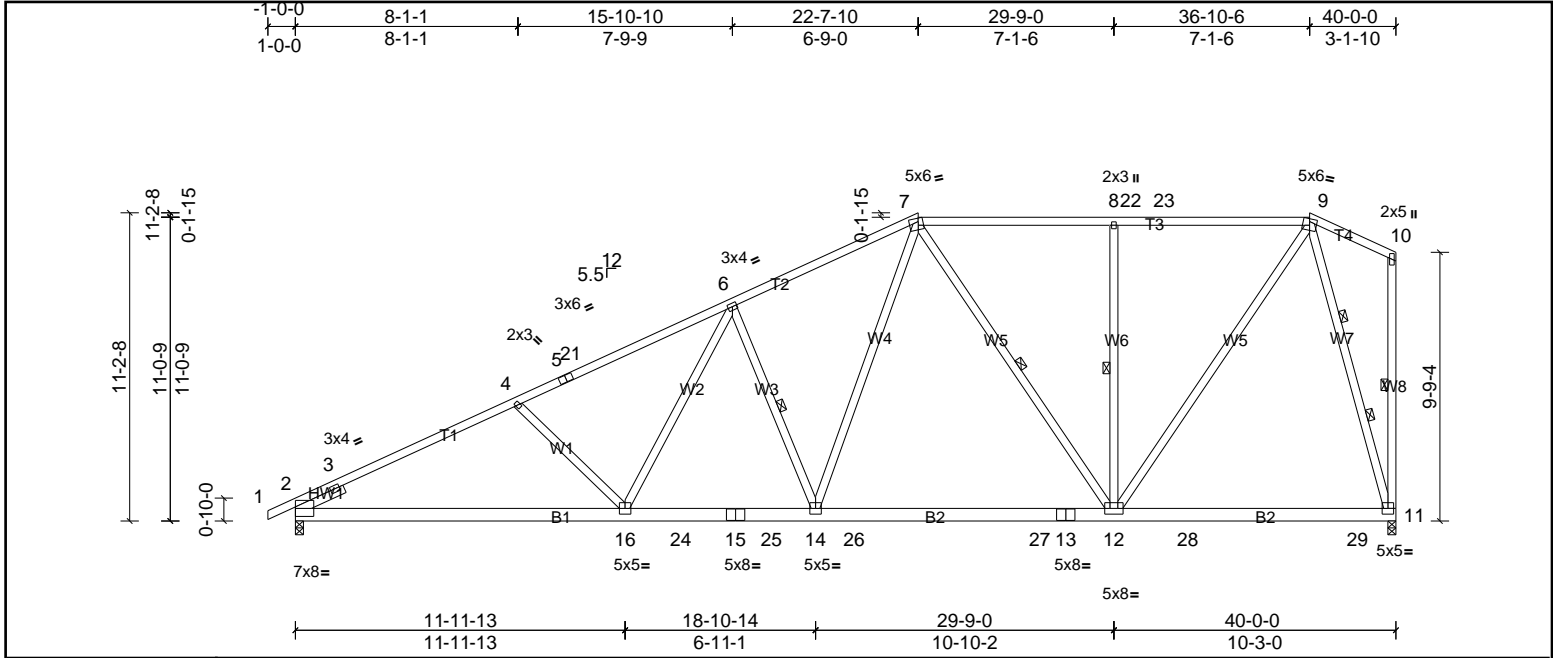


Plate Offsets (X, Y): [2:Edge,0-4-2], [7:0-2-0,Edge], [9:0-2-12,0-1-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.83	Vert(LL)	-0.21	12-14	>999	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-0.36	12-14	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.90	Horz(CT)	0.07	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											

Weight: 292 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.1 *Except* T4:2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 2-2-0 oc purlins, except end verticals, and 2-0-0 oc purlins (4-5-4 max.): 7-9.
BOT CHORD 2x6 SP No.2 *Except* B1:2x6 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 8-12, 10-11, 7-12, 6-14
SLIDER Left 2x4 SP No.3 -- 1-11-0	WEBS 2 Rows at 1/3 pts 9-11
REACTIONS	
(lb/size) 2=1498/0-3-8, (min. 0-2-14), 11=1516/0-3-8, (min. 0-2-15)	
Max Horiz 2=343 (LC 14)	
Max Uplift 2=-126 (LC 14), 11=-104 (LC 11)	
Max Grav 2=1826 (LC 3), 11=1855 (LC 3)	
FORCES	
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD 2-3=-1791/0, 3-4=-3503/230, 4-5=-3250/161, 5-21=-3208/169, 6-21=-3167/197, 6-7=-2618/207, 7-8=-1556/108, 8-22=-1556/107, 22-23=-1555/107, 9-23=-1553/108	
BOT CHORD 2-16=-453/3200, 16-24=-282/2637, 15-24=-282/2637, 15-25=-282/2637, 14-25=-282/2637, 14-26=-149/1862, 26-27=-149/1862, 13-27=-149/1862, 12-13=-149/1862, 12-28=-41/520, 28-29=-41/520, 11-29=-41/520	
WEBS 4-16=-418/214, 6-16=-46/638, 8-12=-734/187, 9-12=-83/1847, 9-11=-1913/156, 7-12=-734/166, 7-14=-137/1380, 6-14=-960/242	

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 126 lb uplift at joint 2 and 104 lb uplift at joint 11.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-7=-51, 7-9=-61, 9-10=-51, 11-17=-20



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Job 72434515	Truss A5	Truss Type Truss	Qty 11	Ply 1	SD - AVERY CFI Job Reference (optional)
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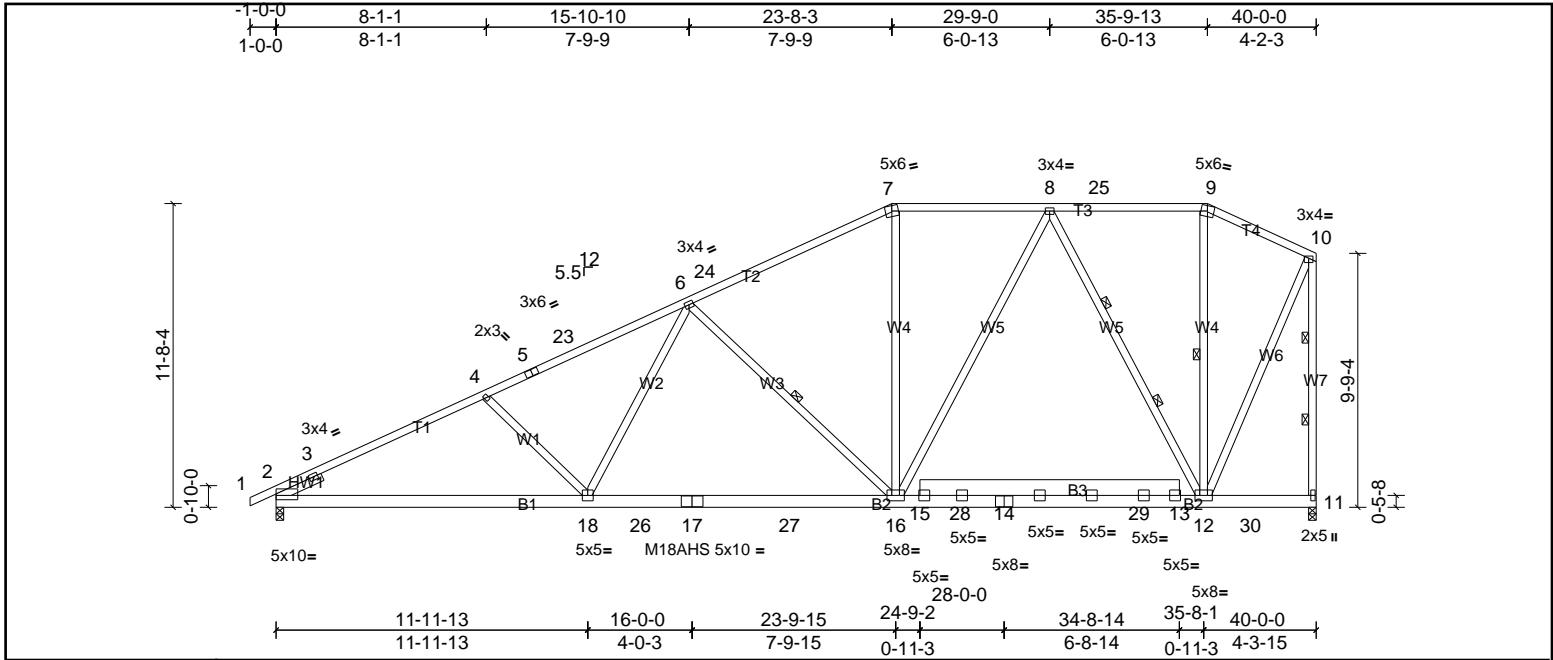


Plate Offsets (X, Y): [2:Edge,0-2-10], [10:0-2-0,0-1-0], [11:0-2-8,0-1-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.85	Vert(LL)	-0.27	16-18	>999	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.81	Vert(CT)	-0.45	16-18	>999	180	M18AHS	186/179
TCDL	10.0	Rep Stress Incr	NO	WB	0.84	Horz(CT)	0.07	11	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 327 lb	FT = 20%

LUMBER	BRACING
TOP CHORD: 2x4 SP SS *Except* T3,T4:2x4 SP No.2	TOP CHORD: Structural wood sheathing directly applied or 2-10-9 oc purlins, except end verticals, and 2-0-0 oc purlins (4-1-2 max.): 7-9.
BOT CHORD: 2x6 SP No.1 *Except* B3:2x8 SP No.2	BOT CHORD: Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS: 2x4 SP No.3	WEBS: 1 Row at midpt 6-16, 9-12
SLIDER: Left 2x4 SP No.3 -- 1-11-0	WEBS: 2 Rows at 1/3 pts 8-12, 10-11

REACTIONS	(lb/size)
Max Horiz	2=351 (LC 14)
Max Uplift	2=-131 (LC 14), 11=-81 (LC 11)
Max Grav	2=1821 (LC 3), 11=1841 (LC 3)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-1649/0, 3-4=-3524/230, 4-5=-3287/163, 5-23=-3226/176, 6-23=-3136/199, 6-24=-2082/104, 7-24=-1959/139, 7-8=-1800/163, 8-25=-709/61, 9-25=-709/61, 9-10=-807/58, 10-11=-1980/93
BOT CHORD	2-18=-462/3213, 18-26=-311/2625, 17-26=-311/2625, 17-27=-311/2625, 16-27=-311/2625, 15-16=-90/1341, 15-28=-95/1323, 14-28=-92/1329, 14-29=-90/1342, 13-29=-92/1336, 12-13=-90/1341
WEBS	4-18=-394/210, 6-18=-8/784, 6-16=-1192/256, 7-16=0/451, 8-16=-92/1075, 8-12=-1403/139, 10-12=-73/1695

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 131 lb uplift at joint 2 and 81 lb uplift at joint 11.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S)	Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
Vert: 1-7=-51, 7-9=-61, 9-10=-51, 11-19=-20	



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Job 72434515	Truss AJ1	Truss Type Truss	Qty 30	Ply 1	SD - AVERY CFI Job Reference (optional)
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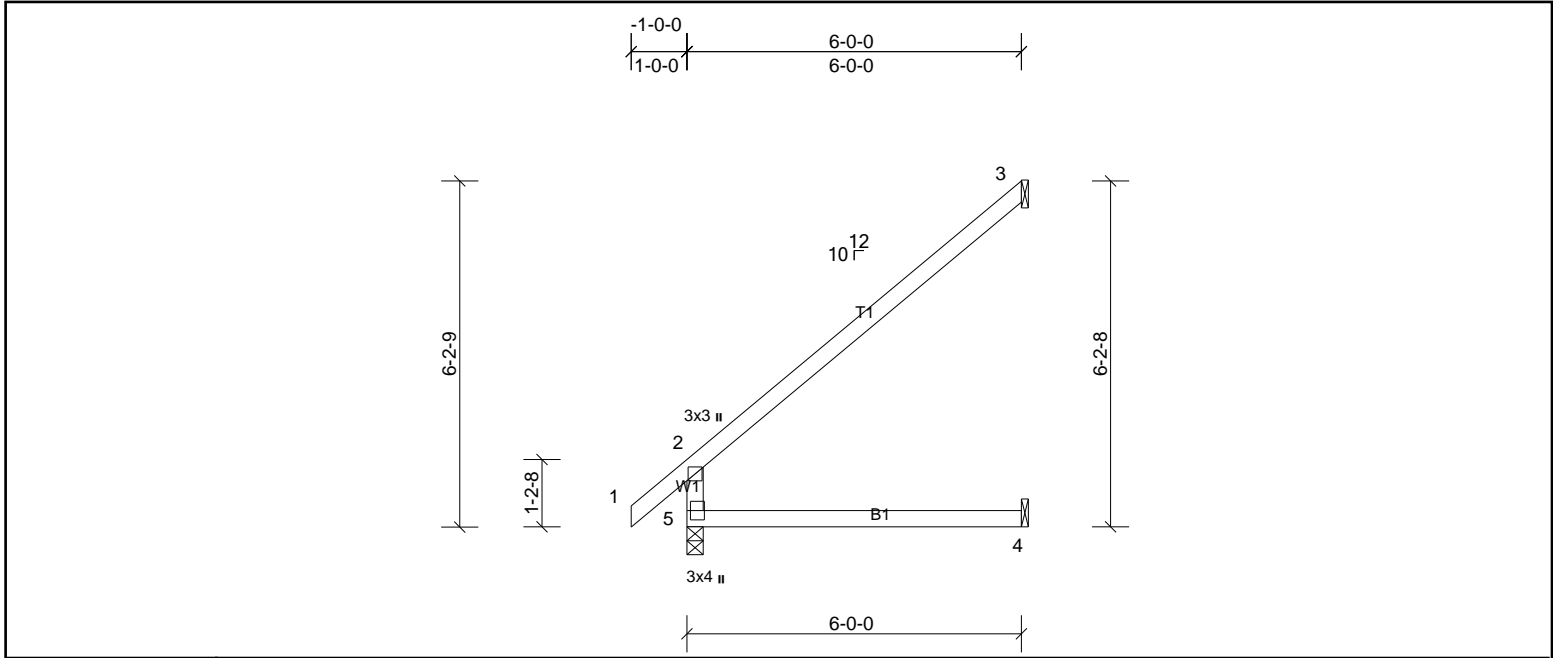


Plate Offsets (X, Y): [5:0-2-0,0-0-12]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.61	Vert(LL)	0.08	4-5	>912	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.13	4-5	>553	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.14	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 24 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS

(lb/size)	3=129/ Mechanical, (min. 0-1-8), 4=64/ Mechanical, (min. 0-1-8), 5=260/0-3-8, (min. 0-1-8)
Max Horiz	5=182 (LC 12)
Max Uplift	3=136 (LC 12)
Max Grav	3=179 (LC 26), 4=112 (LC 5), 5=307 (LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-5=-258/14

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 136 lb uplift at joint 3.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss AJ2	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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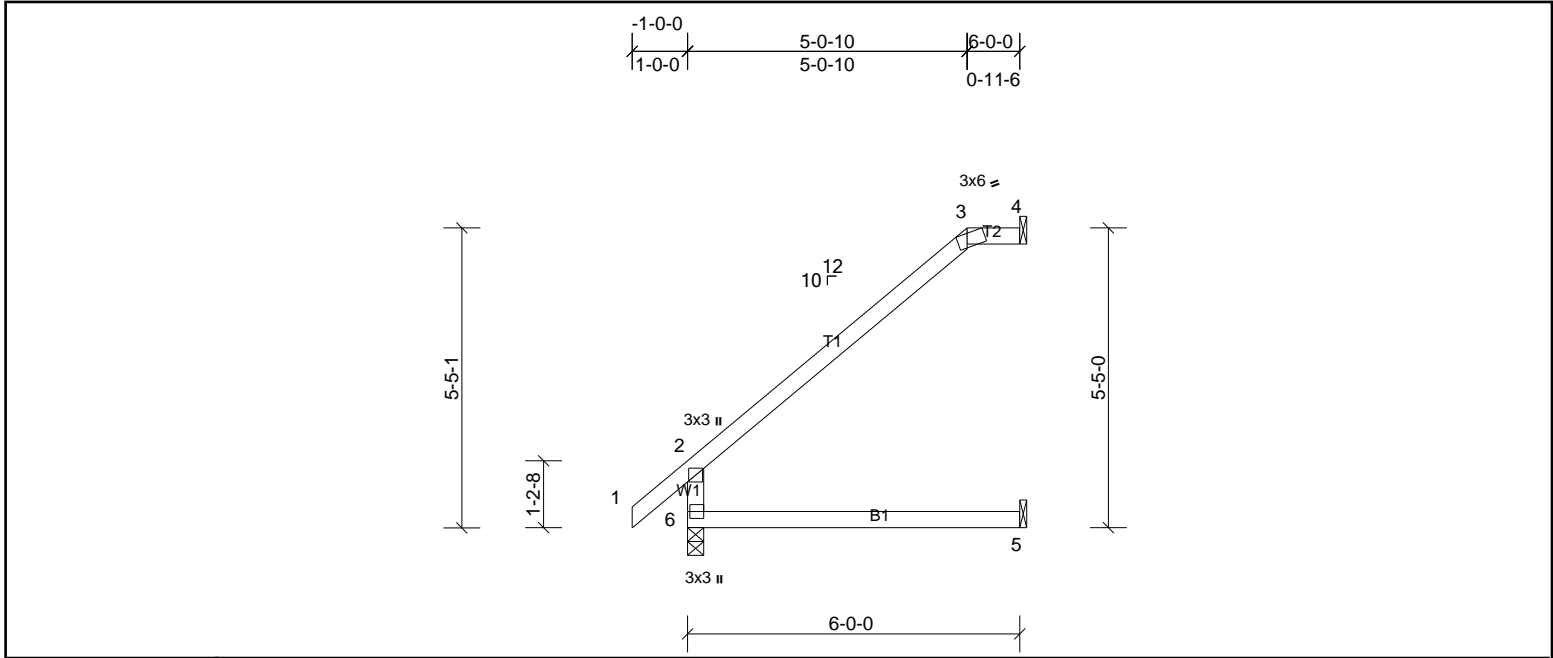


Plate Offsets (X, Y): [3:0-1-13,Edge], [6:0-1-8,0-0-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.57	Vert(LL)	0.07	5-6	>963	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.13	5-6	>550	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.26	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 24 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	
(lb/size)	4=138/ Mechanical, (min. 0-1-8), 5=65/ Mechanical, (min. 0-1-8), 6=261/0-3-8, (min. 0-1-8)
Max Horiz	6=158 (LC 12)
Max Uplift	4=103 (LC 12)
Max Grav	4=158 (LC 2), 5=113 (LC 5), 6=307 (LC 2)

FORCES	
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	2-6=-259/34

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) ** TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (14.3 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93, 1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 6) Provide adequate drainage to prevent water ponding.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 103 lb uplift at joint 4.
 - 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S)	
Standard	
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
Vert: 1-2=-49, 2-3=-49, 3-4=-61, 5-6=-20	



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Job 72434515	Truss AJ3	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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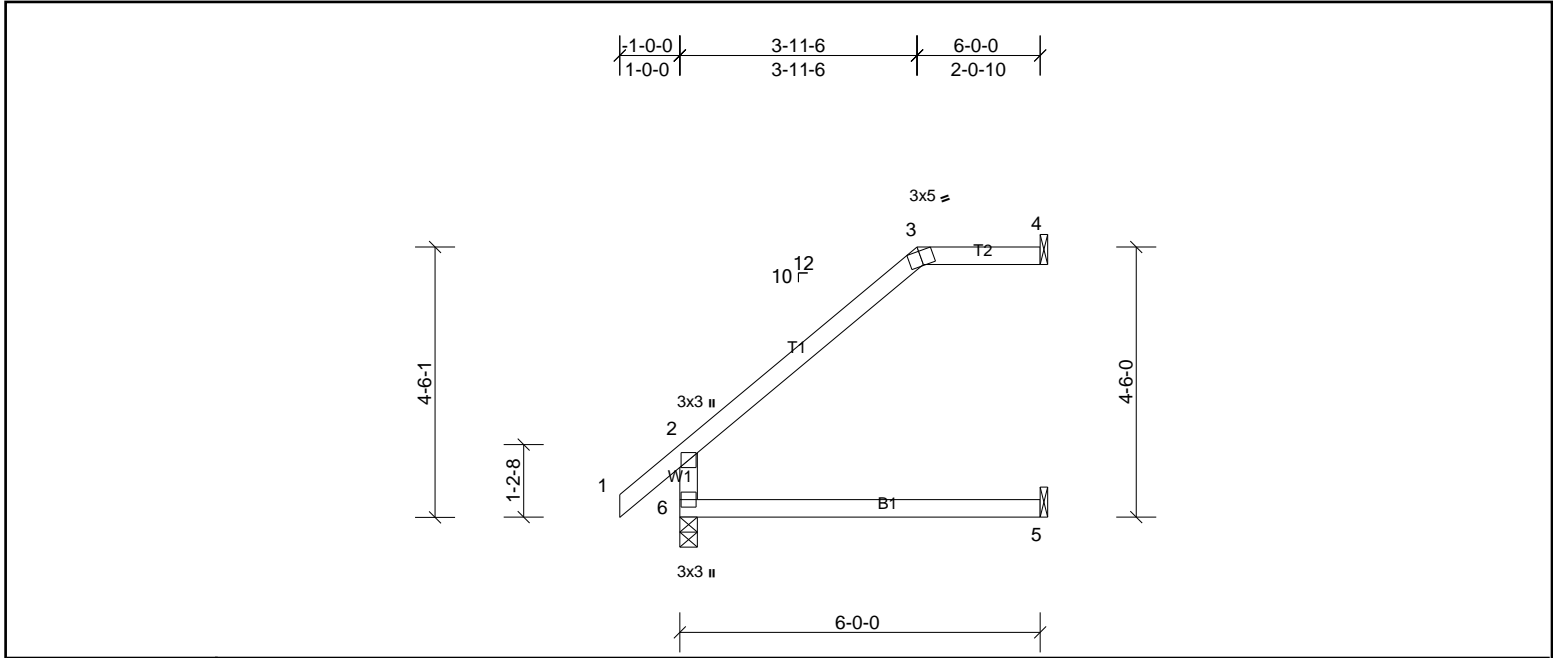


Plate Offsets (X, Y): [3:0-2-8,Edge]

Loading	(psf)	Spacing		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	2-0-0	TC	0.53	Vert(LL)	0.06	5-6	>999	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.13	5-6	>543	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	-0.32	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 23 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	
(lb/size)	4=146/ Mechanical, (min. 0-1-8), 5=67/ Mechanical, (min. 0-1-8), 6=264/0-3-8, (min. 0-1-8)
Max Horiz	6=127 (LC 12)
Max Uplift	4=69 (LC 12)
Max Grav	4=156 (LC 2), 5=113 (LC 5), 6=307 (LC 2)

FORCES	
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	2-6=-260/52

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (14.3 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93, 1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 6) Provide adequate drainage to prevent water ponding.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 69 lb uplift at joint 4.
 - 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S)	
Standard	
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
Vert: 1-2=-49, 2-3=-49, 3-4=-61, 5-6=-20	



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Job 72434515	Truss AJ4	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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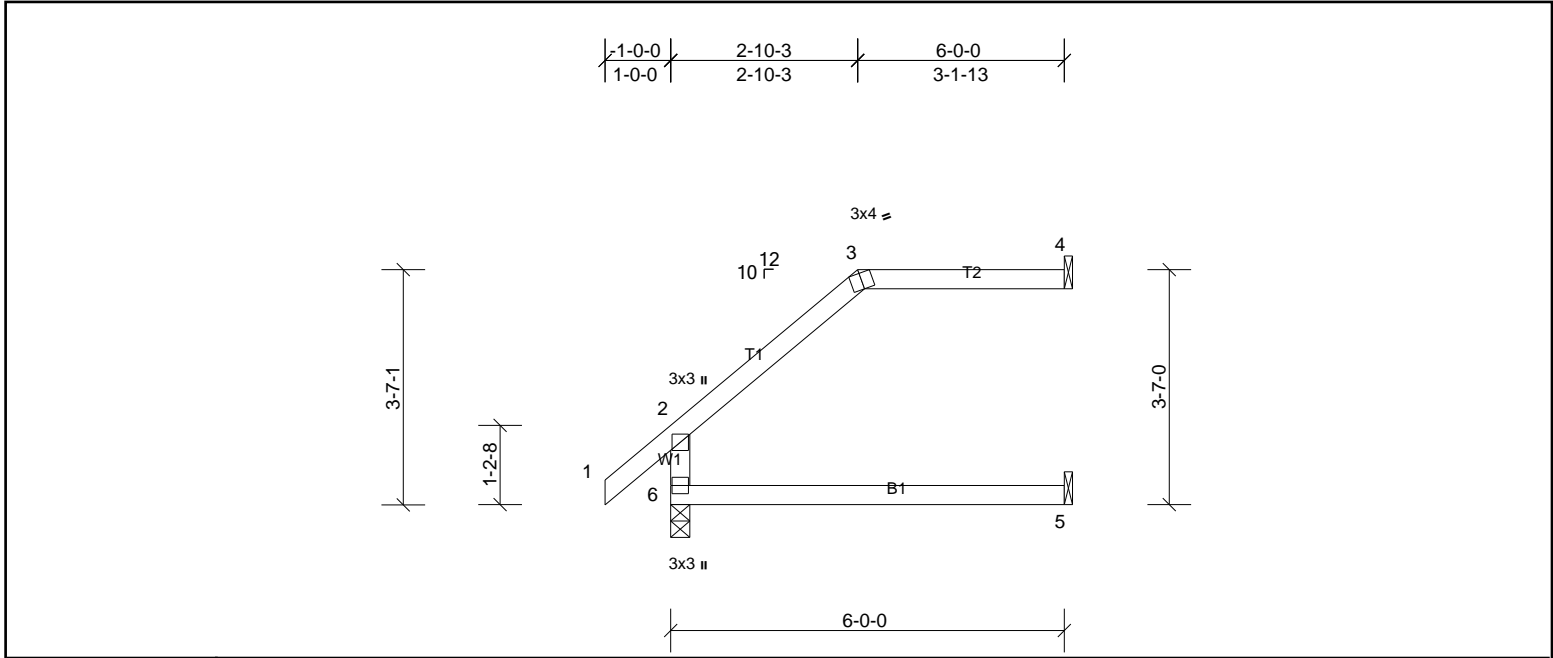


Plate Offsets (X, Y): [3:0-2-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.51	Vert(LL)	-0.06	5-6	>999	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.13	5-6	>538	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.33	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 23 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	

REACTIONS	(lb/size)	4=152/ Mechanical, (min. 0-1-8), 5=68/ Mechanical, (min. 0-1-8), 6=270/0-3-8, (min. 0-1-8)
Max Horiz	6=95 (LC 12)	
Max Uplift	4=61 (LC 9), 6=-10 (LC 12)	
Max Grav	4=155 (LC 2), 5=114 (LC 5), 6=307 (LC 2)	

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-6=-261/61

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (14.3 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93, 1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 10 lb uplift at joint 6 and 61 lb uplift at joint 4.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S)	Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
	Vert: 1-2=-49, 2-3=-49, 3-4=-61, 5-6=-20



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



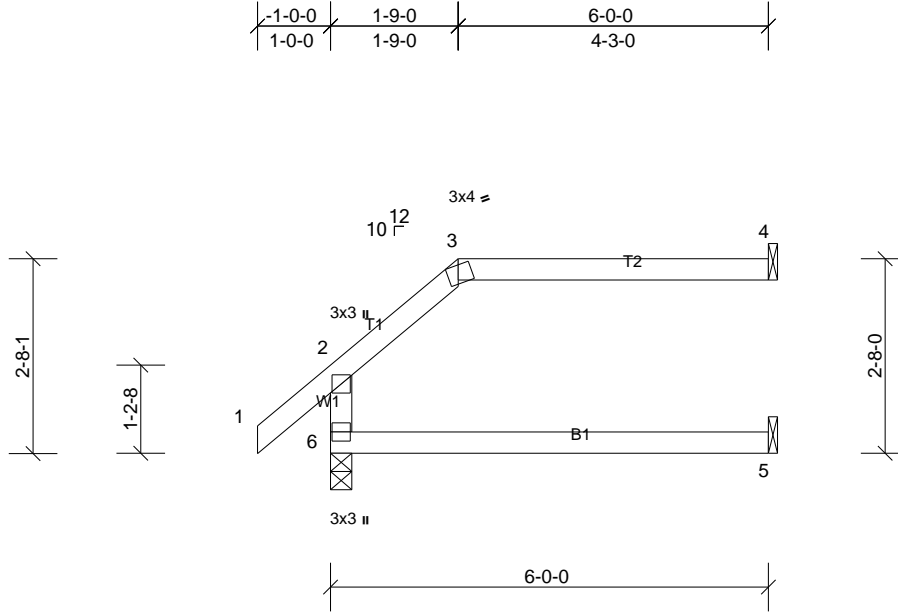
Job 72434515	Truss AJ5	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.54	Vert(LL)	-0.06	5-6	>999	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.41	Vert(CT)	-0.13	5-6	>541	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.24	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 22 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	(lb/size)	4=158/ Mechanical, (min. 0-1-8), 5=68/ Mechanical, (min. 0-1-8), 6=278/0-3-8, (min. 0-1-8)
Max Horiz		6=64 (LC 12)
Max Uplift		4=60 (LC 9), 6=14 (LC 12)
Max Grav		4=158 (LC 1), 5=113 (LC 5), 6=307 (LC 2)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-6=-259/60

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCCL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (14.3 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93,1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 14 lb uplift at joint 6 and 60 lb uplift at joint 4.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S)	Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
	Vert: 1-2=-49, 2-3=-49, 3-4=-61, 5-6=-20



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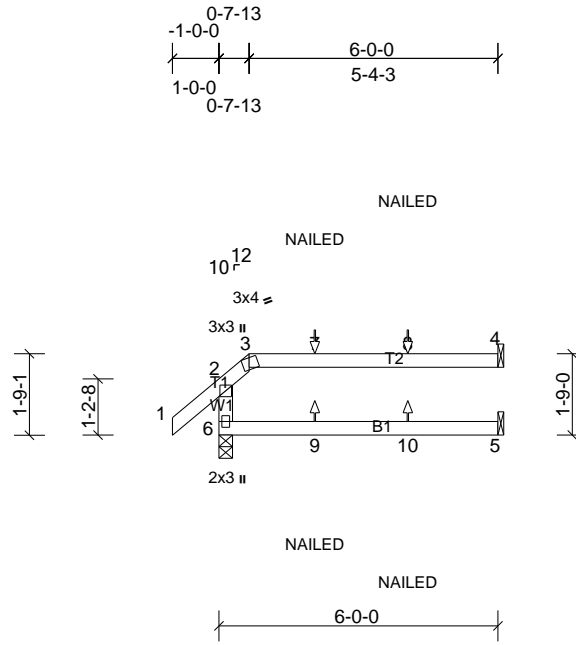
Job 72434515	Truss AJ6	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.67	Vert(LL)	-0.06	5-6	>999	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.44	Vert(CT)	-0.13	5-6	>553	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.00	Horz(CT)	0.12	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							Weight: 22 lb	FT = 20%
BCDL	10.0											

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins: 3-4.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	(lb/size)	4=163/ Mechanical, (min. 0-1-8), 5=64/ Mechanical, (min. 0-1-8), 6=289/0-3-8, (min. 0-1-8)
Max Horiz		6=41 (LC 7)
Max Uplift		4=61 (LC 7), 6=25 (LC 10)
Max Grav		4=163 (LC 1), 5=113 (LC 5), 6=305 (LC 2)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-6=-256/66

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (14.3 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; ls=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93,1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 6) Provide adequate drainage to prevent water ponding.
 - 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 61 lb uplift at joint 4 and 25 lb uplift at joint 6.
 - 10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
 - 11) "NAILED" indicates Girder: 3-10d (0.148" x 3") toe-nails per NDS guidelines.
 - 12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S)	Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
	Vert: 1-2=-49, 2-3=-49, 3-4=-61, 5-6=-20
Concentrated Loads (lb)	
	Vert: 9=0 (F), 10=0 (F)



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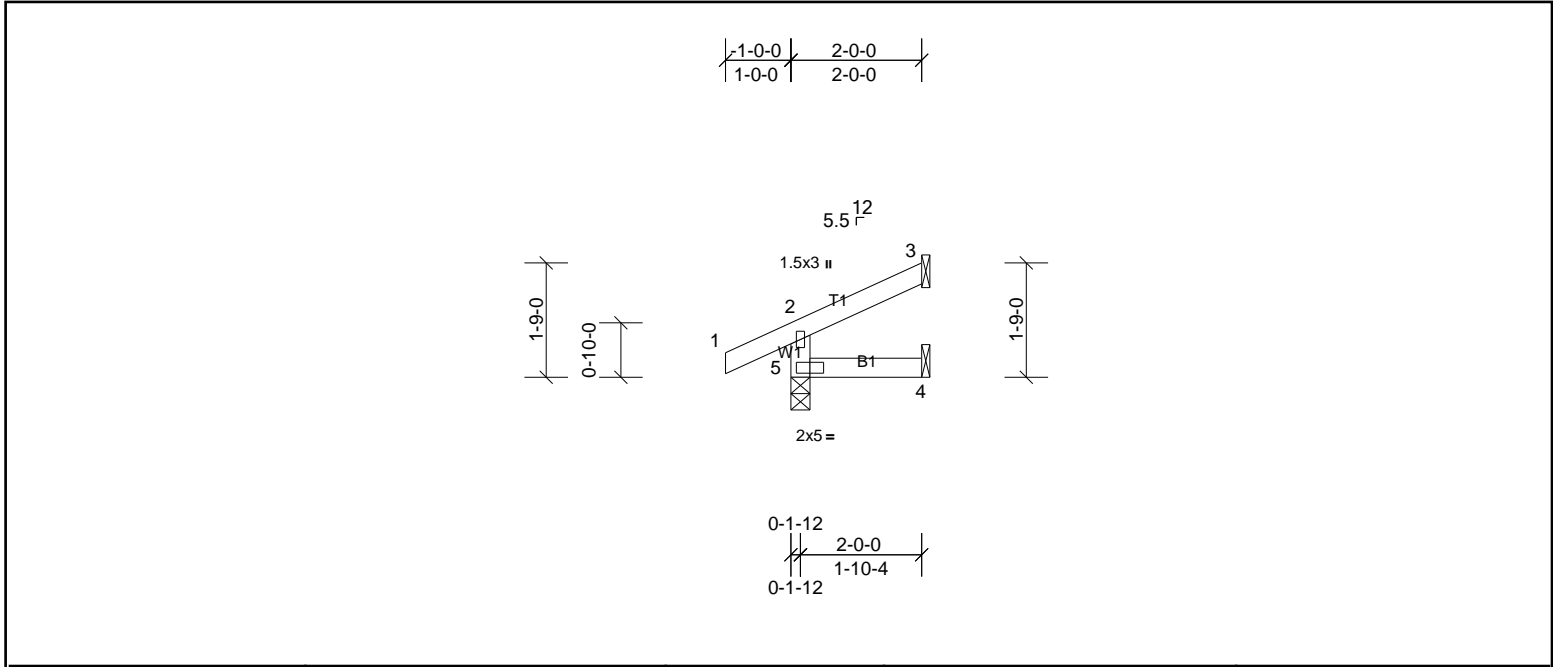
Job 72434515	Truss AJ7	Truss Type Truss	Qty 4	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.11	Vert(LL)	0.00	4-5	>999	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.03	Vert(CT)	0.00	4-5	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 9 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	
(lb/size)	3=32/ Mechanical, (min. 0-1-8), 4=15/ Mechanical, (min. 0-1-8), 5=142/0-3-8, (min. 0-1-8)
Max Horiz	5=38 (LC 11)
Max Uplift	3=25 (LC 14), 5=18 (LC 14)
Max Grav	3=45 (LC 21), 4=33 (LC 7), 5=193 (LC 21)

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 25 lb uplift at joint 3 and 18 lb uplift at joint 5.



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Job 72434515	Truss B1	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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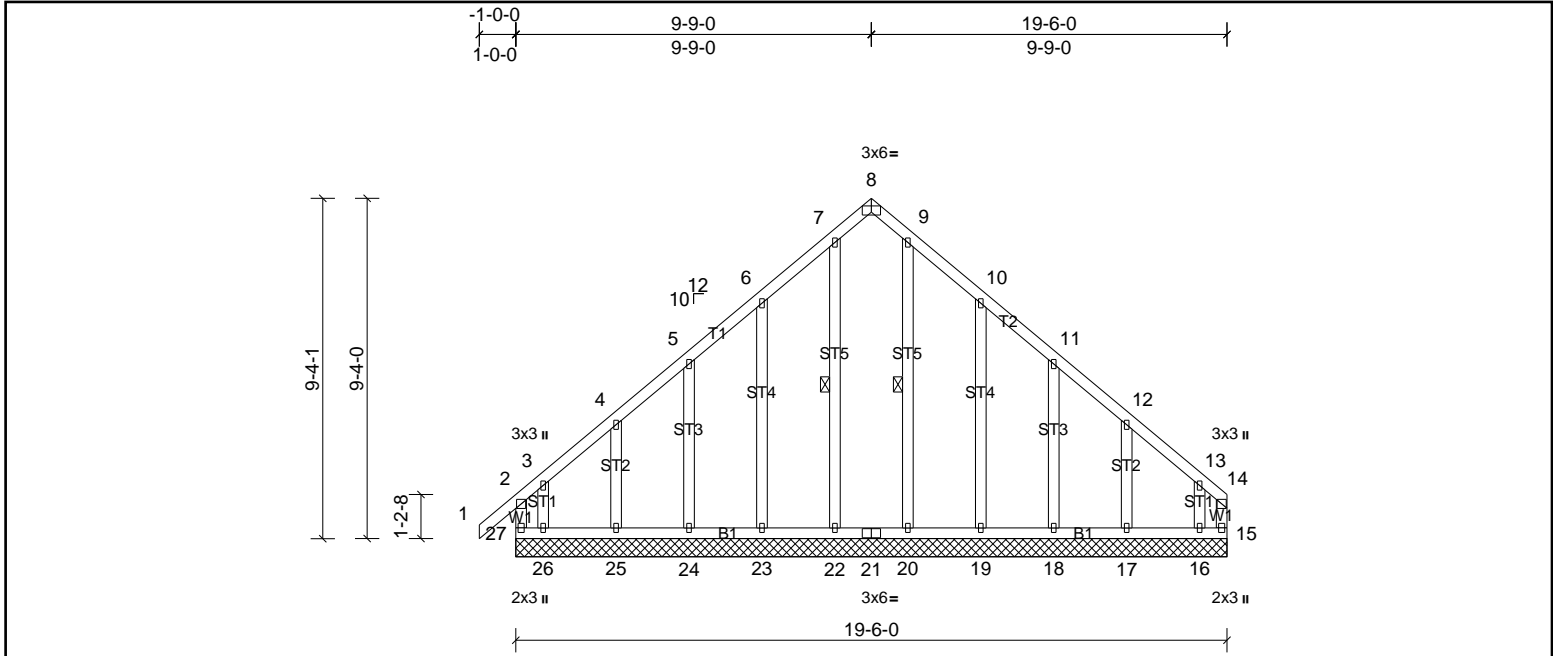


Plate Offsets (X, Y): [8:0-3-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.22	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.13	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.00	15	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-R								
BCDL	10.0										Weight: 140 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3	WEBS	1 Row at midpt
OTHERS	2x4 SP No.3		7-22, 9-20

REACTIONS	
All bearings	19-6-0.
(lb) - Max Horiz	27=222 (LC 9)
Max Uplift	All uplift 100 (lb) or less at joint(s) 17, 18, 23, 24, 25 except 15=236 (LC 11), 16=-290 (LC 13), 19=-101 (LC 13), 26=-310 (LC 12), 27=-225 (LC 10)
Max Grav	All reactions 250 (lb) or less at joint(s) 17, 18, 19, 20, 22, 23, 24, 25 except 15=341 (LC 13), 16=270 (LC 11), 26=276 (LC 10), 27=347 (LC 12)

FORCES	
TOP CHORD	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown. 2-3=-251/177

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - All plates are 1.5x3 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 23, 24, 25, 18, 17 except (jt=lb) 27=224, 15=236, 26=310, 19=101, 16=289.



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Job 72434515	Truss B2	Truss Type Truss	Qty 3	Ply 1	SD - AVERY CFI Job Reference (optional)
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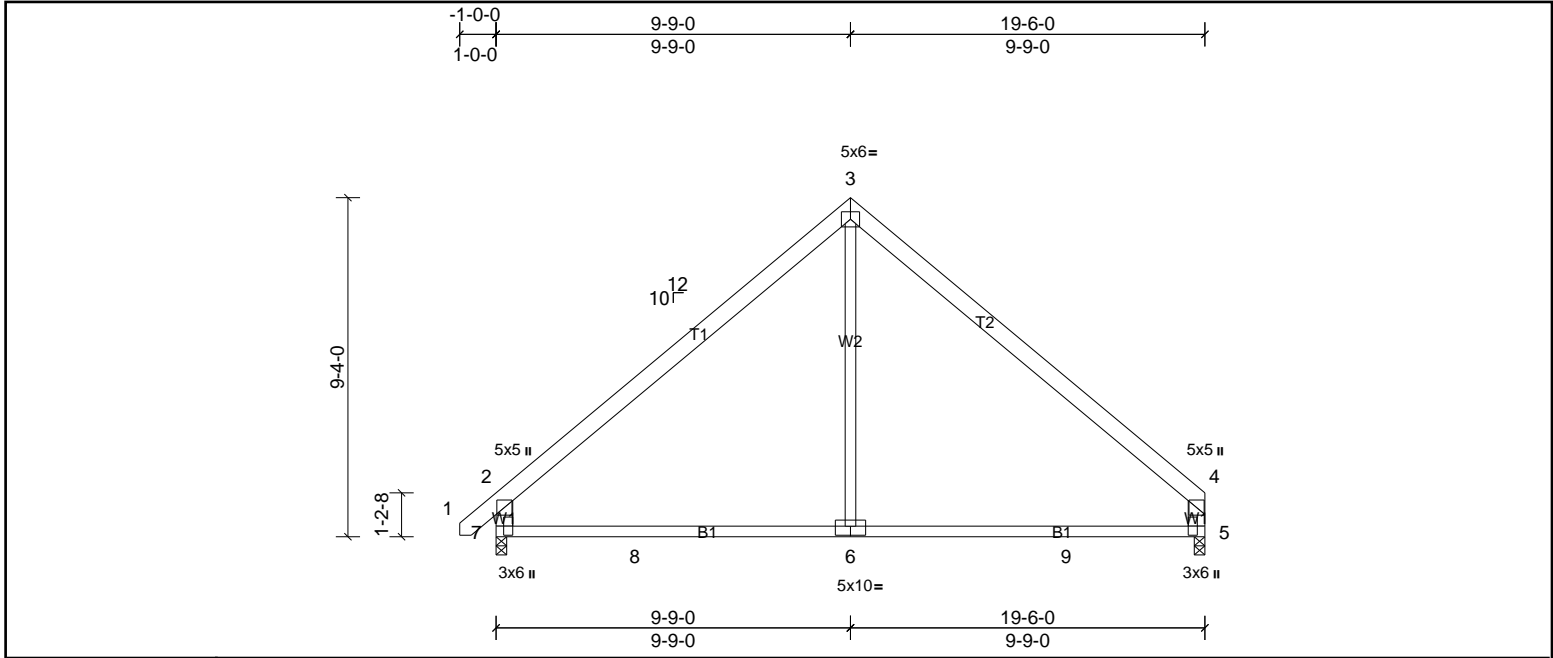


Plate Offsets (X, Y): [5:0-3-0,Edge], [6:0-5-0,0-3-0], [7:0-3-0,0-2-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	Vert(LL)	-0.23	6-7	>978	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	Vert(CT)	-0.38	6-7	>599	180		
TCDL	10.0	Rep Stress Incr	YES	WB	Horz(CT)	0.02	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR							
BCDL	10.0									Weight: 109 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 5-9-14 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x6 SP No.2 *Except* W2:2x4 SP No.3	
REACTIONS	
(lb/size) 5=652/0-3-8, (min. 0-1-8), 7=707/0-3-8, (min. 0-1-8)	
Max Horiz 7=219 (LC 11)	
Max Uplift 5=34 (LC 13), 7=52 (LC 12)	
Max Grav 5=912 (LC 27), 7=974 (LC 26)	
FORCES	
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD 2-3=-1024/120, 3-4=-1020/120, 2-7=-913/116, 4-5=-864/97	
BOT CHORD 7-8=0/643, 6-8=0/643, 6-9=0/643, 5-9=0/643	
WEBS 3-6=0/603	

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - 4) Roof design snow load has been reduced to account for slope.
 - 5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 52 lb uplift at joint 7 and 34 lb uplift at joint 5.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss B3	Truss Type Truss	Qty 7	Ply 1	SD - AVERY CFI Job Reference (optional)
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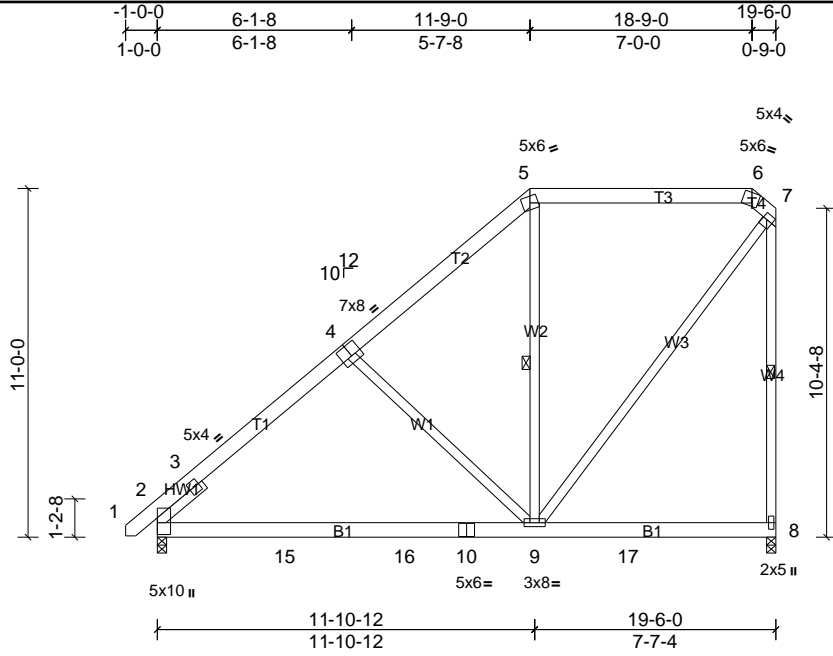


Plate Offsets (X, Y): [4:0-4-0,0-4-8], [7:0-1-8,0-2-4]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	-0.15	9-13	>999	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.62	Vert(CT)	-0.25	9-13	>913	180		
TCDL	10.0	Rep Stress Incr	NO	WB	0.44	Horz(CT)	0.02	2	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 166 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x6 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end
BOT CHORD 2x6 SP No.2	verticals, and 2-0-0 oc purlins (6-0-0 max.): 5-6.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
SLIDER Left 2x4 SP No.3 -- 1-11-0	WEBS 1 Row at midpt 5-9, 7-8

REACTIONS

(lb/size)	2=724/0-3-8, (min. 0-1-8), 8=730/0-3-8, (min. 0-1-8)
Max Horiz	2=349 (LC 12)
Max Uplift	2=-1 (LC 12), 8=-102 (LC 12)
Max Grav	2=943 (LC 26), 8=899 (LC 3)

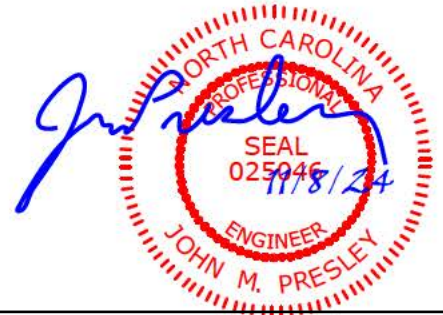
FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	2-3=-1018/0, 3-4=-870/23, 4-5=-719/42, 5-6=-493/88, 6-7=-523/81, 7-8=-897/133
BOT CHORD	2-15=-367/740, 15-16=-246/740, 10-16=-246/740, 9-10=-246/740
WEBS	4-9=-360/233, 7-9=-129/809

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (14.3 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93, 1.00, 0.93; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 2 and 102 lb uplift at joint 8.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-5=-49, 5-6=-61, 6-7=-49, 8-11=-20



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Job 72434515	Truss C1	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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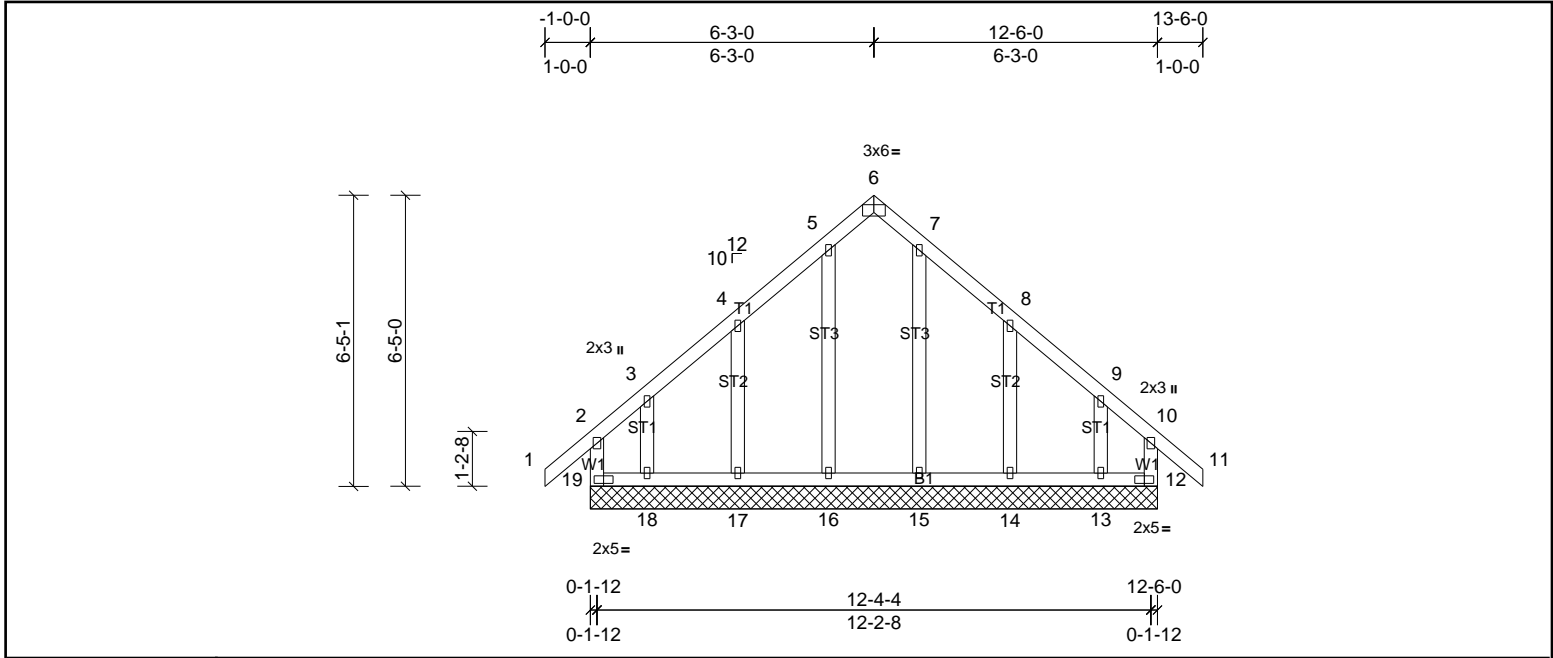


Plate Offsets (X, Y): [6:0-3-0,Edge]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.14	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.00	12	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-R							
BCDL	10.0									Weight: 80 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

REACTIONS All bearings 12-6-0.
 (lb) - Max Horiz 19=-165 (LC 10)
 Max Uplift All uplift 100 (lb) or less at joint(s) 12, 14, 17, 19 except 13=-135 (LC 13), 18=-138 (LC 12)
 Max Grav All reactions 250 (lb) or less at joint(s) 12, 13, 14, 15, 16, 17, 18, 19

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - All plates are 1.5x3 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
 - Gable studs spaced at 2-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 19, 12 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 19, 12, 17, 14 except (jt=lb) 18=138, 13=134.



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Job 72434515	Truss C2	Truss Type Truss	Qty 9	Ply 1	SD - AVERY CFI Job Reference (optional)
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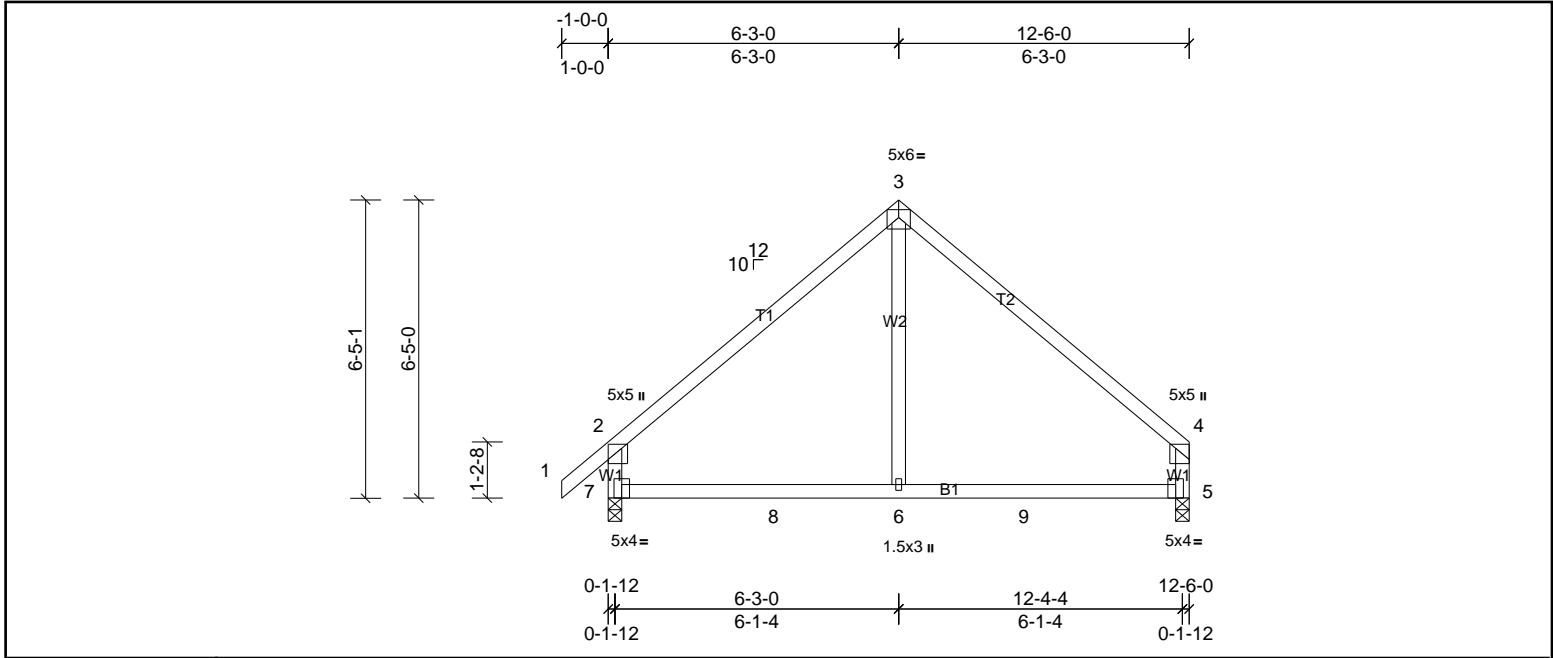


Plate Offsets (X, Y): [2:0-2-8,0-1-12]

Loading	(psf)	Spacing		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	2-0-0	TC	0.73	Vert(LL)	-0.05	6-7	>999	240	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.40	Vert(CT)	-0.10	6-7	>999	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.13	Horz(CT)	0.01	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MR								
BCDL	10.0										Weight: 56 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	
(lb/size)	5=416/0-3-8, (min. 0-1-8), 7=477/0-3-8, (min. 0-1-8)
Max Horiz	7=158 (LC 9)
Max Uplift	5=21 (LC 13), 7=40 (LC 12)
Max Grav	5=563 (LC 26), 7=632 (LC 26)

FORCES	
(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.	
TOP CHORD	2-3=-617/86, 3-4=-610/85, 2-7=-591/92, 4-5=-538/71
BOT CHORD	7-8=-4/368, 6-8=-4/368, 6-9=-4/368, 5-9=-4/368
WEBS	3-6=0/330

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Bearing at joint(s) 7, 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 7 and 21 lb uplift at joint 5.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss D4	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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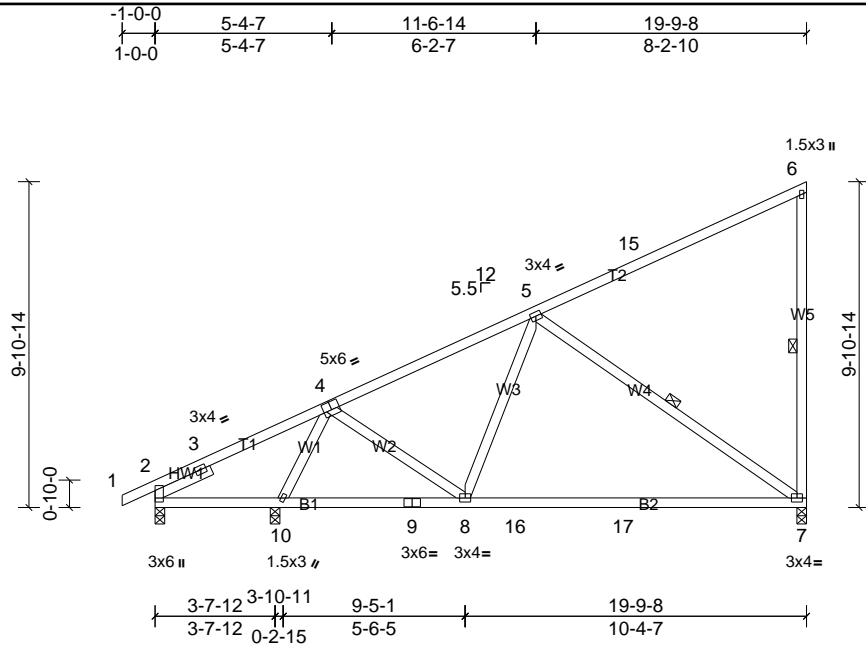


Plate Offsets (X, Y): [4:0-3-0,0-3-0]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.84	Vert(LL)	-0.45	7-8	>420	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.89	Vert(CT)	-0.78	7-8	>243	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.41	Horz(CT)	0.01	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 115 lb	FT = 20%

LUMBER	BRACING
TOP CHORD	TOP CHORD
BOT CHORD	BOT CHORD
WEBS	WEBS
SLIDER	

REACTIONS	(lb/size)
Max Horiz	2=323 (LC 14)
Max Uplift	2=40 (LC 14), 7=181 (LC 14), 10=1 (LC 14)
Max Grav	2=328 (LC 2), 7=760 (LC 5), 10=715 (LC 3)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	4-5=-832/0
BOT CHORD	2-10=-338/241, 9-10=-291/585, 8-9=-291/585, 8-16=-185/656, 16-17=-185/656, 7-17=-185/656
WEBS	6-7=-253/97, 5-8=0/373, 5-7=-809/229, 4-8=0/275, 4-10=-766/13

- NOTES**
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 2, 181 lb uplift at joint 7 and 1 lb uplift at joint 10.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss D5	Truss Type Truss	Qty 6	Ply 1	SD - AVERY CFI Job Reference (optional)
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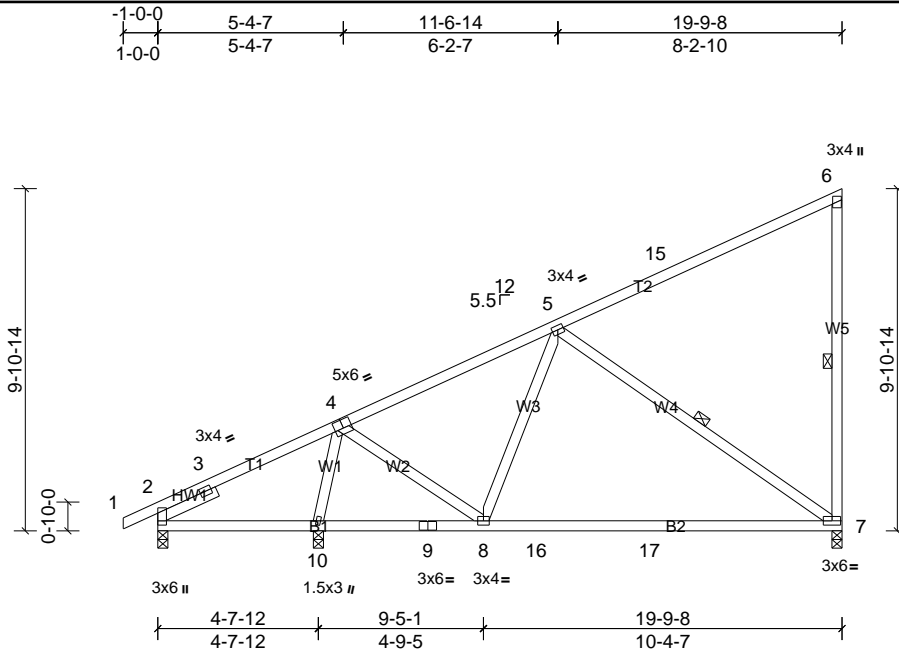


Plate Offsets (X, Y): [4:0-3-0,0-3-0]

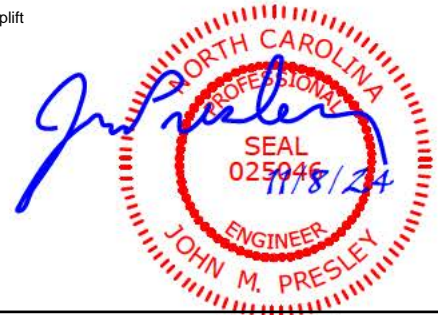
Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.99	Vert(LL)	-0.39	7-8	>459	240	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.80	Vert(CT)	-0.68	7-8	>267	180		
TCDL	10.0	Rep Stress Incr	YES	WB	0.36	Horz(CT)	0.01	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-MSH								
BCDL	10.0											
											Weight: 115 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD 2x4 SP No.2 *Except* B2:2x4 SP No.1	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	WEBS 1 Row at midpt 6-7, 5-7
SLIDER Left 2x4 SP No.3 -- 1-11-0	

REACTIONS	(lb/size)
Max Horiz	2=323 (LC 14)
Max Uplift	2=-38 (LC 11), 7=-177 (LC 14), 10=-18 (LC 14)
Max Grav	2=357 (LC 3), 7=734 (LC 5), 10=713 (LC 3)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	4-5=-745/0, 6-7=-263/102
BOT CHORD	2-10=-334/264, 9-10=-273/428, 8-9=-273/428, 8-16=-178/603, 16-17=-178/603, 7-17=-178/603
WEBS	4-10=-722/29, 4-8=0/340, 5-8=0/318, 5-7=-717/218

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; porch left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 177 lb uplift at joint 7, 38 lb uplift at joint 2 and 18 lb uplift at joint 10.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss PB1	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

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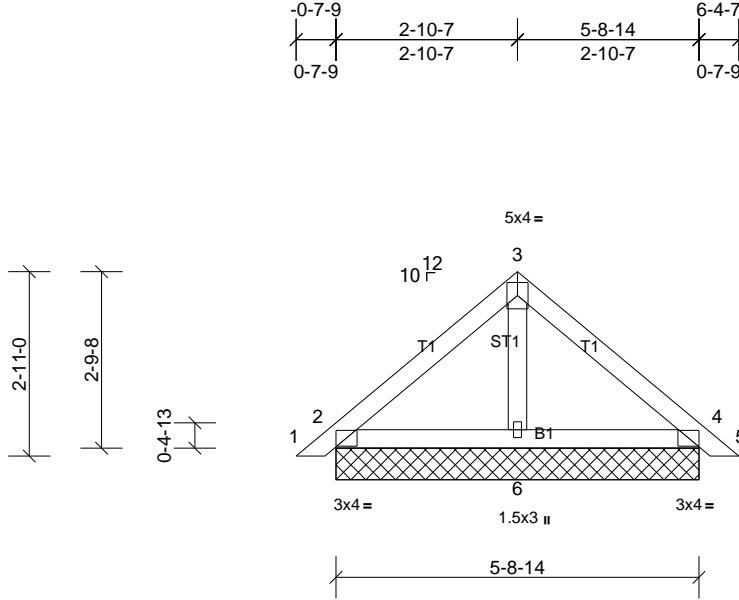


Plate Offsets (X, Y): [2:0-2-1,0-1-8], [4:0-2-1,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.08	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.03	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 25 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=122/5-8-14, (min. 0-1-8), 4=122/5-8-14, (min. 0-1-8), 6=189/5-8-14, (min. 0-1-8)
 Max Horiz 2=60 (LC 10)
 Max Uplift 2=19 (LC 12), 4=27 (LC 13)
 Max Grav 2=145 (LC 2), 4=145 (LC 2), 6=217 (LC 2)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- Truss designed for wind loads in the plane of the truss only.
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
- Roof design snow load has been reduced to account for slope.
- This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 4-0-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 2 and 27 lb uplift at joint 4.
- See standard piggyback truss connection detail for connection to base truss.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



Job 72434515	Truss PB2	Truss Type Truss	Qty 7	Ply 1	SD - AVERY CFI Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

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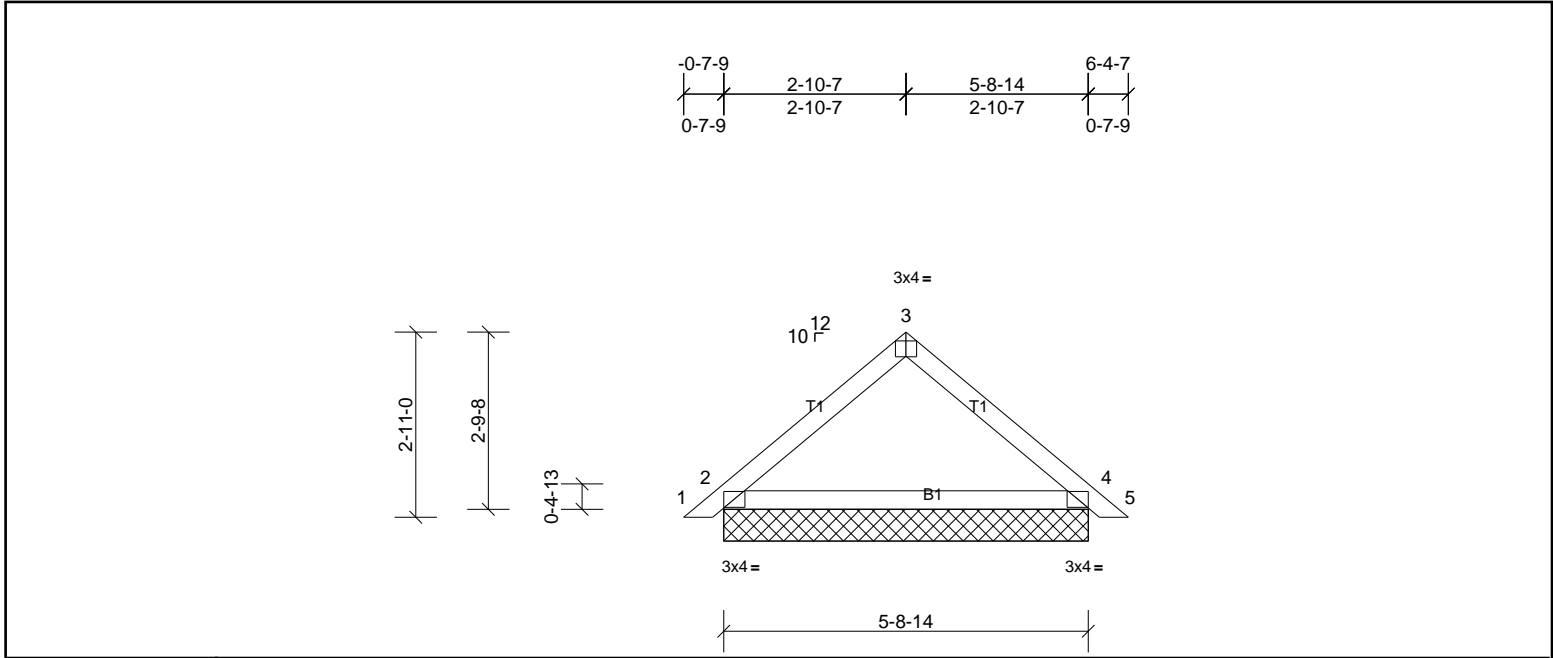


Plate Offsets (X, Y): [2:0-2-1,0-1-8], [3:0-2-0,Edge], [4:0-2-1,0-1-8]

Loading	(psf)	Spacing	2-0-0	CSI	DEFLL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.17	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.24	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 22 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(lb/size)	2=216/5-8-14, (min. 0-1-8), 4=216/5-8-14, (min. 0-1-8)
	Max Horiz	2=-60 (LC 10)
	Max Uplift	2=-19 (LC 12), 4=-19 (LC 13)
	Max Grav	2=254 (LC 2), 4=254 (LC 2)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only.
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 4-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 19 lb uplift at joint 2 and 19 lb uplift at joint 4.
 - See standard piggyback truss connection detail for connection to base truss.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



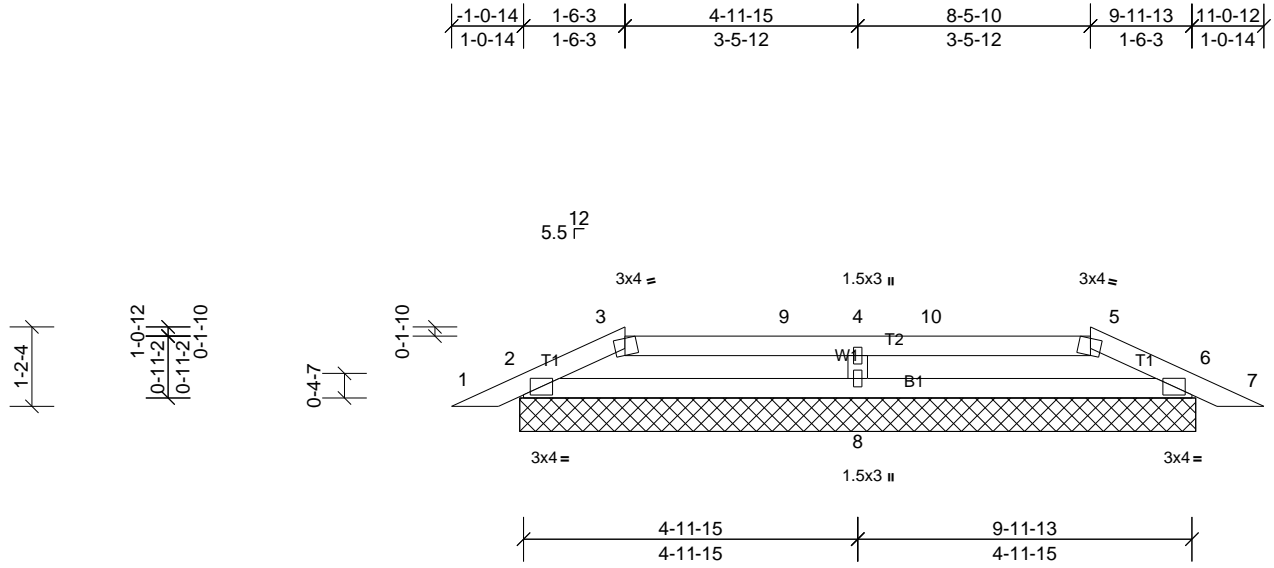
Job 72434515	Truss PB3	Truss Type Truss	Qty 2	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.30	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.23	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horz(CT)	0.01	6	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 34 lb	FT = 20%

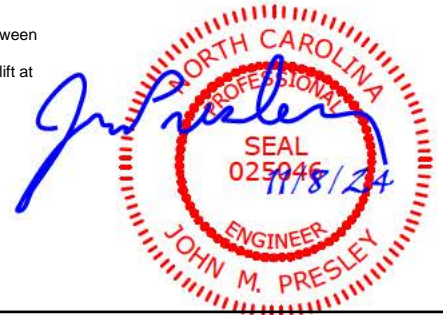
LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except
BOT CHORD 2x4 SP No.2	2-0-0 oc purlins (6-0-0 max.): 3-5.
WEBS 2x4 SP No.3	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS	(lb/size)
	2=231/10-1-3, (min. 0-1-8), 6=231/10-1-3, (min. 0-1-8), 8=388/10-1-3, (min. 0-1-8)
Max Horiz	2=14 (LC 14)
Max Uplift	2=26 (LC 14), 6=27 (LC 15), 8=34 (LC 10)
Max Grav	2=268 (LC 41), 6=268 (LC 41), 8=497 (LC 40)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	2-3=-356/61, 3-9=-326/54, 4-9=-328/54, 4-10=-328/54, 5-10=-326/54, 5-6=-356/61
BOT CHORD	2-8=-35/320, 6-8=-35/320
WEBS	4-8=-380/98

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - Truss designed for wind loads in the plane of the truss only.
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
 - Provide adequate drainage to prevent water ponding.
 - Gable requires continuous bottom chord bearing.
 - Gable studs spaced at 4-0-0 oc.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 26 lb uplift at joint 2, 27 lb uplift at joint 6 and 34 lb uplift at joint 8.
 - See standard piggyback truss connection detail for connection to base truss.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S)	Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15	
Uniform Loads (lb/ft)	
	Vert: 1-3=-51, 3-5=-61, 5-7=-51, 2-6=-20



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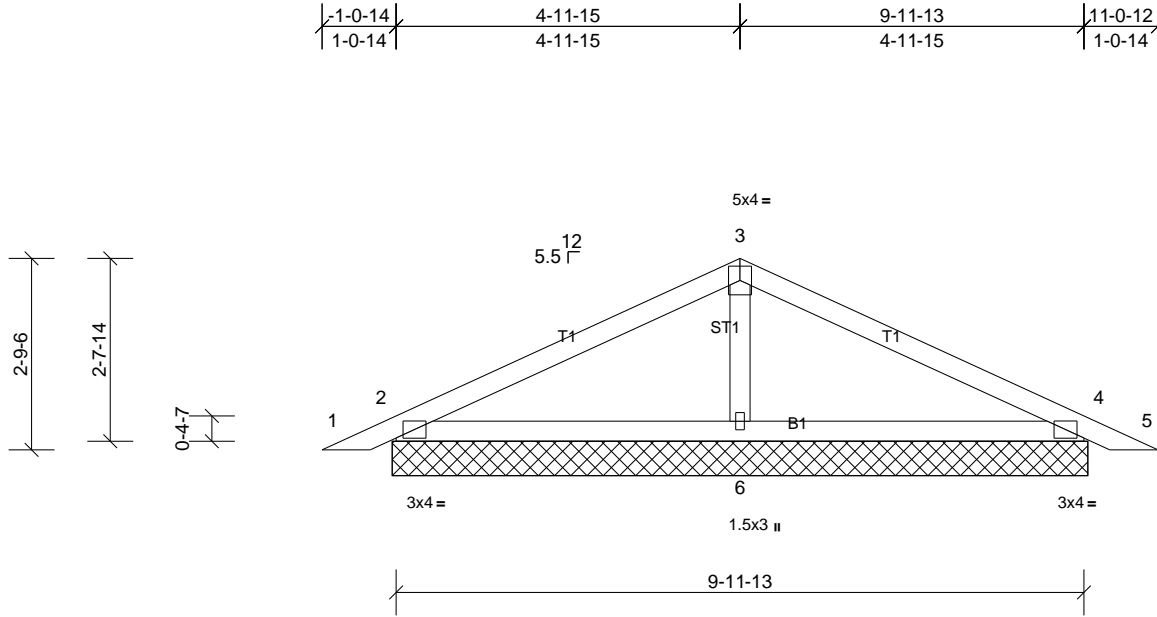
Job 72434515	Truss PB4	Truss Type Truss	Qty 9	Ply 1	SD - AVERY CFI Job Reference (optional)
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UFPI Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.40	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.27	Vert(CT)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horz(CT)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 38 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 OTHERS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=196/10-1-3, (min. 0-1-8), 4=196/10-1-3, (min. 0-1-8), 6=389/10-1-3, (min. 0-1-8)
 Max Horiz 2=39 (LC 19)
 Max Uplift 2=40 (LC 14), 4=47 (LC 15), 6=5 (LC 14)
 Max Grav 2=264 (LC 21), 4=264 (LC 22), 6=437 (LC 2)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

WEBS

3-6=-290/56

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only.
- 4) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 5) Roof design snow load has been reduced to account for slope.
- 6) Unbalanced snow loads have been considered for this design.
- 7) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 15.4 psf on overhangs non-concurrent with other live loads.
- 8) Gable requires continuous bottom chord bearing.
- 9) Gable studs spaced at 4-0-0 oc.
- 10) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 11) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 40 lb uplift at joint 2, 47 lb uplift at joint 4 and 5 lb uplift at joint 6.
- 13) See standard piggyback truss connection detail for connection to base truss.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



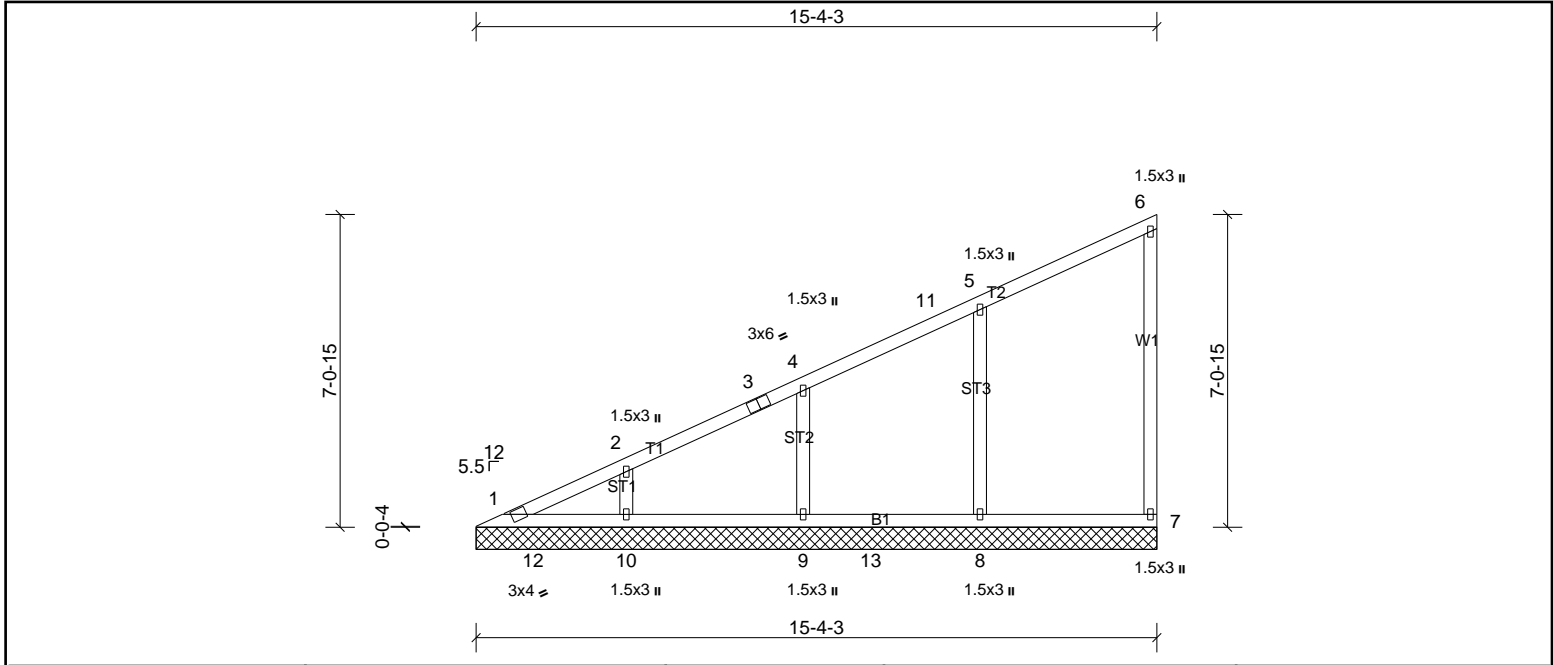
Job 72434515	Truss V1	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

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Loading	(psf)	Spacing		CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	2-0-0	TC	0.23	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.18	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.15	Horiz(TL)	0.00	7	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 69 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		
OTHERS	2x4 SP No.3		

REACTIONS	
	All bearings 15-4-12.
(lb) - Max Horiz	1=231 (LC 14)
Max Uplift	All uplift 100 (lb) or less at joint(s) 7, 8, 9, 10
Max Grav	All reactions 250 (lb) or less at joint(s) 1, 7 except 8=468 (LC 5), 9=348 (LC 3), 10=318 (LC 3)

FORCES	
	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS	5-8=-309/127, 4-9=-252/119

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed ; end vertical left exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - All plates are 1.5x3 MT20 unless otherwise indicated.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 7, 8, 9, 10.



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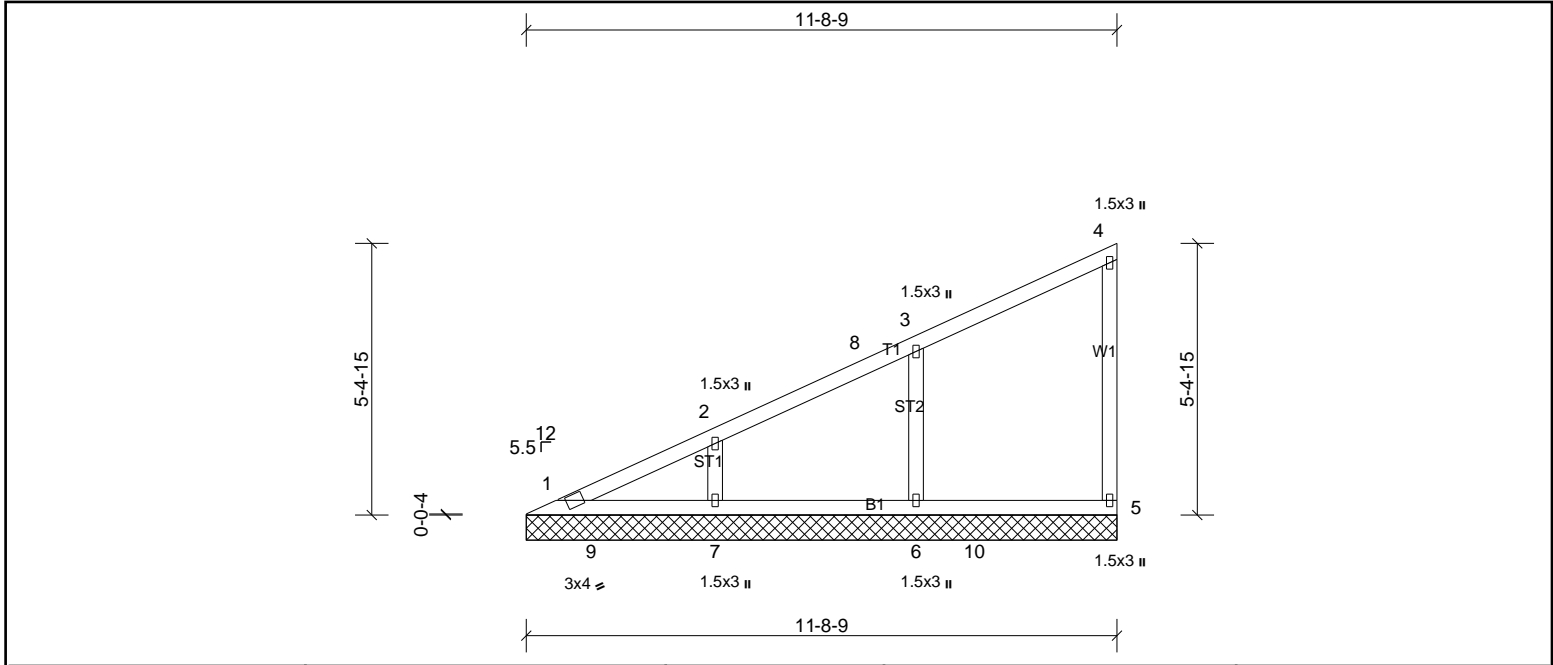
Job 72434515	Truss V2	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.23	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.15	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.08	Horiz(TL)	0.00	5	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH							
BCDL	10.0									Weight: 49 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

REACTIONS	FORCES
All bearings 11-9-2.	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
(lb) - Max Horiz 1=173 (LC 14)	
Max Uplift All uplift 100 (lb) or less at joint(s) 5, 6, 7	
Max Grav All reactions 250 (lb) or less at joint(s) 1, 5 except 6=424 (LC 5), 7=325 (LC 3)	

WEBS
3-6=312/130

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL; Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=15.4 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 5, 6, 7.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



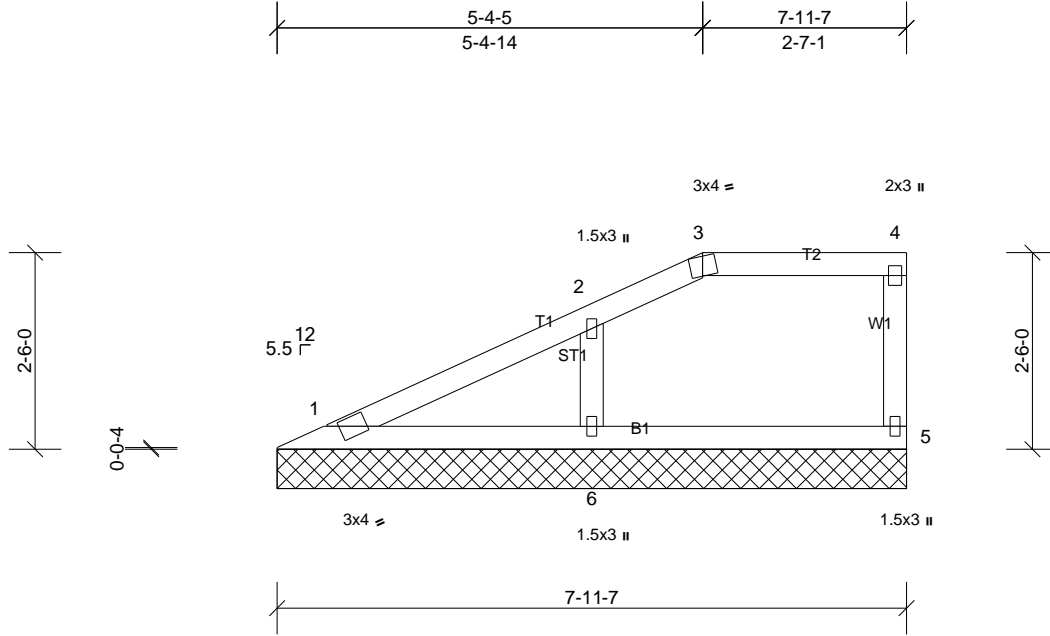
Job 72434515	Truss V3	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.20	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	15.4/20.0	Lumber DOL	1.15	BC	0.12	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.06	Horiz(TL)	0.00	5	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 28 lb	FT = 20%

LUMBER
TOP CHORD 2x4 SP No.2
BOT CHORD 2x4 SP No.2
WEBS 2x4 SP No.3
OTHERS 2x4 SP No.3

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals, and 2-0-0 oc purlins (6-0-0 max.): 3-4.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size)
1=96/8-0-0, (min. 0-1-8), 5=133/8-0-0, (min. 0-1-8), 6=306/8-0-0, (min. 0-1-8)
Max Horiz 1=76 (LC 14)
Max Uplift 5=18 (LC 10), 6=55 (LC 14)
Max Grav 1=144 (LC 37), 5=175 (LC 36), 6=401 (LC 37)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS 2-6=318/100

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - ** TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps= varies (15.4 psf Lum DOL=1.15 Plate DOL=1.15) see load cases; Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10, Lu=50-0-0; Min. flat roof snow load governs. Rain surcharge applied to all exposed surfaces with slopes less than 0.500/12 in accordance with IBC 1608.3.4.
 - Roof design snow load has been reduced to account for slope.
 - Unbalanced snow loads have been considered for this design.
 - Provide adequate drainage to prevent water ponding.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 18 lb uplift at joint 5 and 55 lb uplift at joint 6.
 - Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (lb/ft)
Vert: 1-3=51, 3-4=61, 1-5=20



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



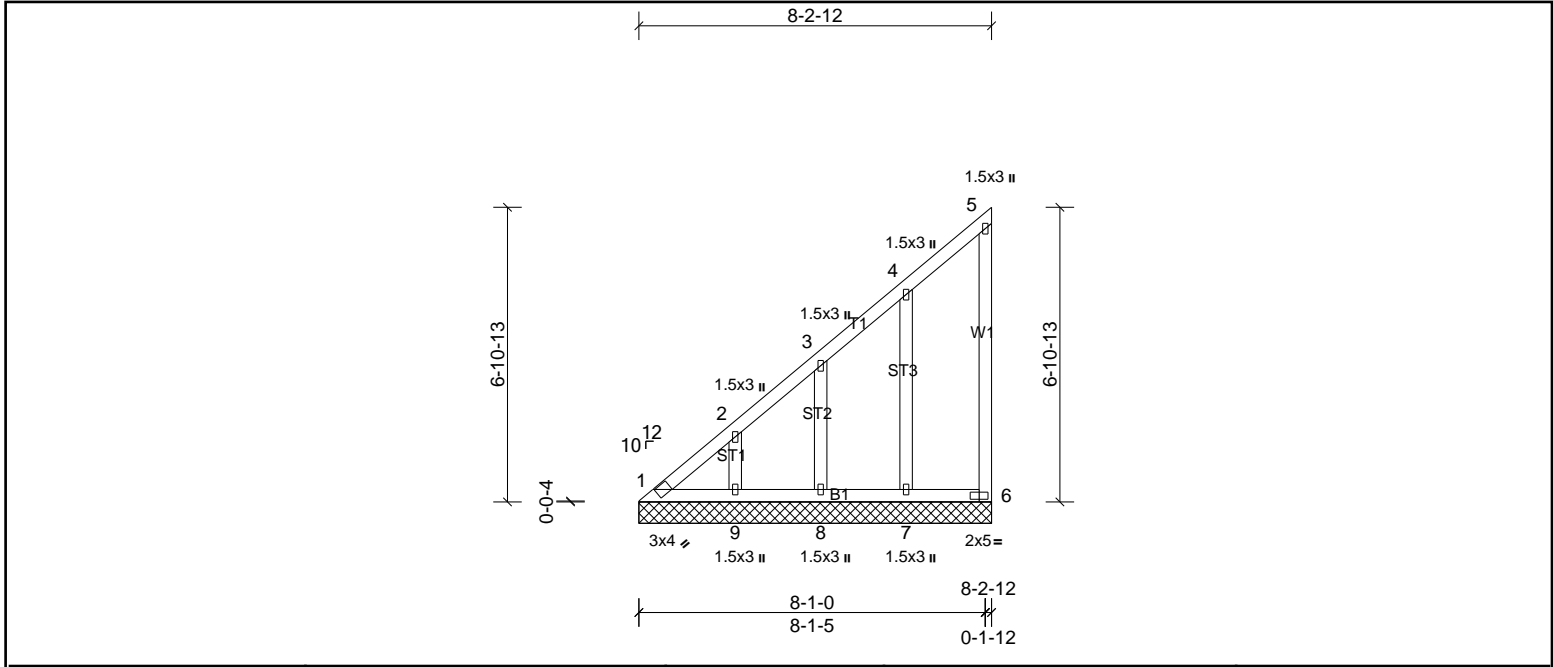
Job 72434515	Truss V4	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	n/a	-	n/a	999	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.03	Vert(TL)	n/a	-	n/a	999	
TCDL	10.0	Rep Stress Incr	YES	WB	0.07	Horiz(TL)	0.00	6	n/a	n/a	
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH							
BCDL	10.0									Weight: 50 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		
OTHERS	2x4 SP No.3		

REACTIONS	
	All bearings 8-3-1.
(lb) - Max Horiz	1=222 (LC 12)
Max Uplift	All uplift 100 (lb) or less at joint(s) 1, 6, 7, 8, 9
Max Grav	All reactions 250 (lb) or less at joint(s) 1, 6, 7, 8, 9

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD	1-2=262/158

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 6 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 6, 7, 8, 9.
 - Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



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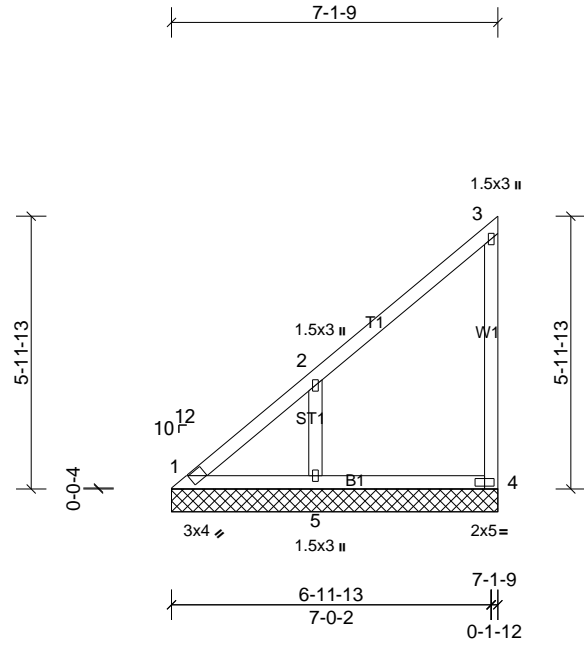
Job 72434515	Truss V5	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.18	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.11	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.05	Horiz(TL)	0.00	4	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 35 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	
OTHERS 2x4 SP No.3	

REACTIONS	(lb/size)
	1=65/7-1-14, (min. 0-1-8), 4=110/7-1-14, (min. 0-1-8), 5=280/7-1-14, (min. 0-1-8)
Max Horiz	1=190 (LC 12)
Max Uplift	4=56 (LC 12), 5=139 (LC 12)
Max Grav	1=125 (LC 12), 4=138 (LC 26), 5=349 (LC 26)

FORCES	(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
WEBS	2-5=306/184

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL); Lum DOL=1.15 Plate DOL=1.15; Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Bearing at joint(s) 4 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 56 lb uplift at joint 4 and 139 lb uplift at joint 5.
 - Gap between inside of top chord bearing and first diagonal or vertical web shall not exceed 0.500in.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



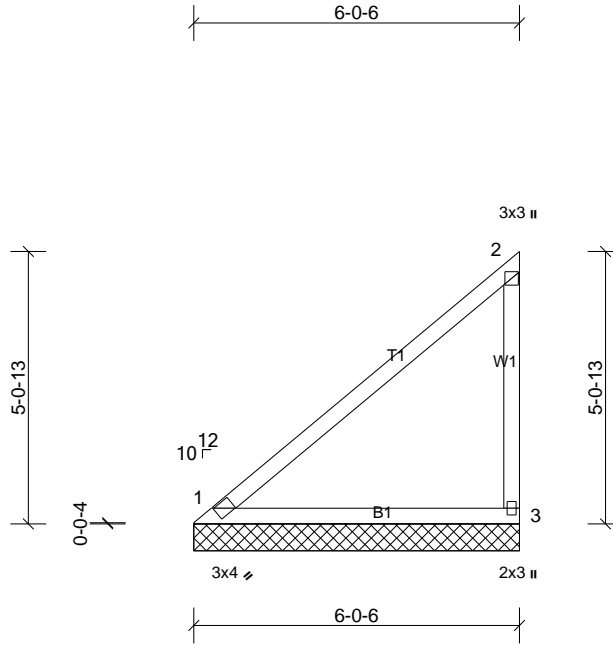
Job 72434515	Truss V6	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.42	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.31	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-SH								
BCDL	10.0										Weight: 27 lb	FT = 20%

LUMBER

TOP CHORD 2x4 SP No.2
 BOT CHORD 2x4 SP No.2
 WEBS 2x4 SP No.3

BRACING

TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 1=190/6-0-11, (min. 0-1-8), 3=190/6-0-11, (min. 0-1-8)
 Max Horiz 1=159 (LC 12)
 Max Uplift 3=95 (LC 12)
 Max Grav 1=221 (LC 2), 3=237 (LC 25)

FORCES

(lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
- Roof design snow load has been reduced to account for slope.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 95 lb uplift at joint 3.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



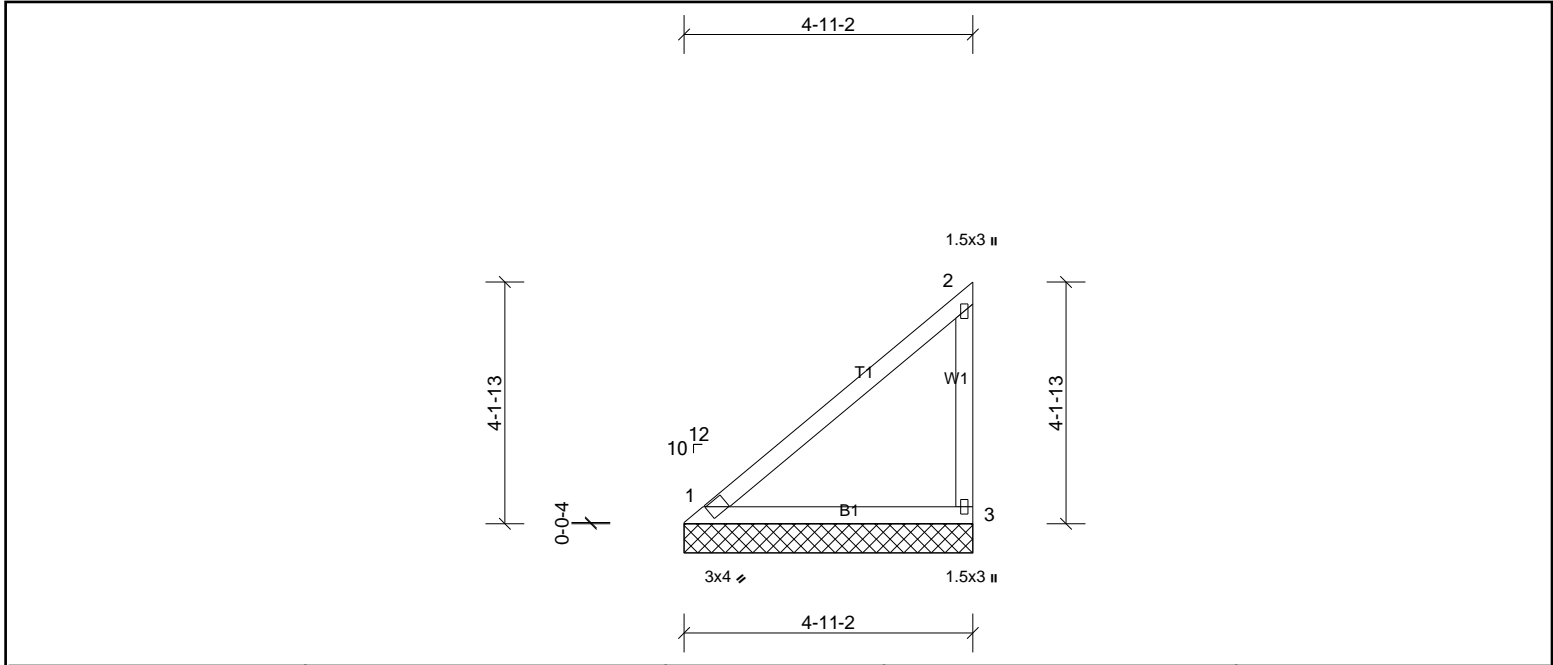
Job 72434515	Truss V7	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.39	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.24	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-P								
BCDL	10.0										Weight: 21 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 4-11-12 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	

REACTIONS	(lb/size)	1=152/4-11-7, (min. 0-1-8), 3=152/4-11-7, (min. 0-1-8)
Max Horiz	1=127 (LC 12)	
Max Uplift	3=-76 (LC 12)	
Max Grav	1=177 (LC 2), 3=190 (LC 25)	

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 76 lb uplift at joint 3.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



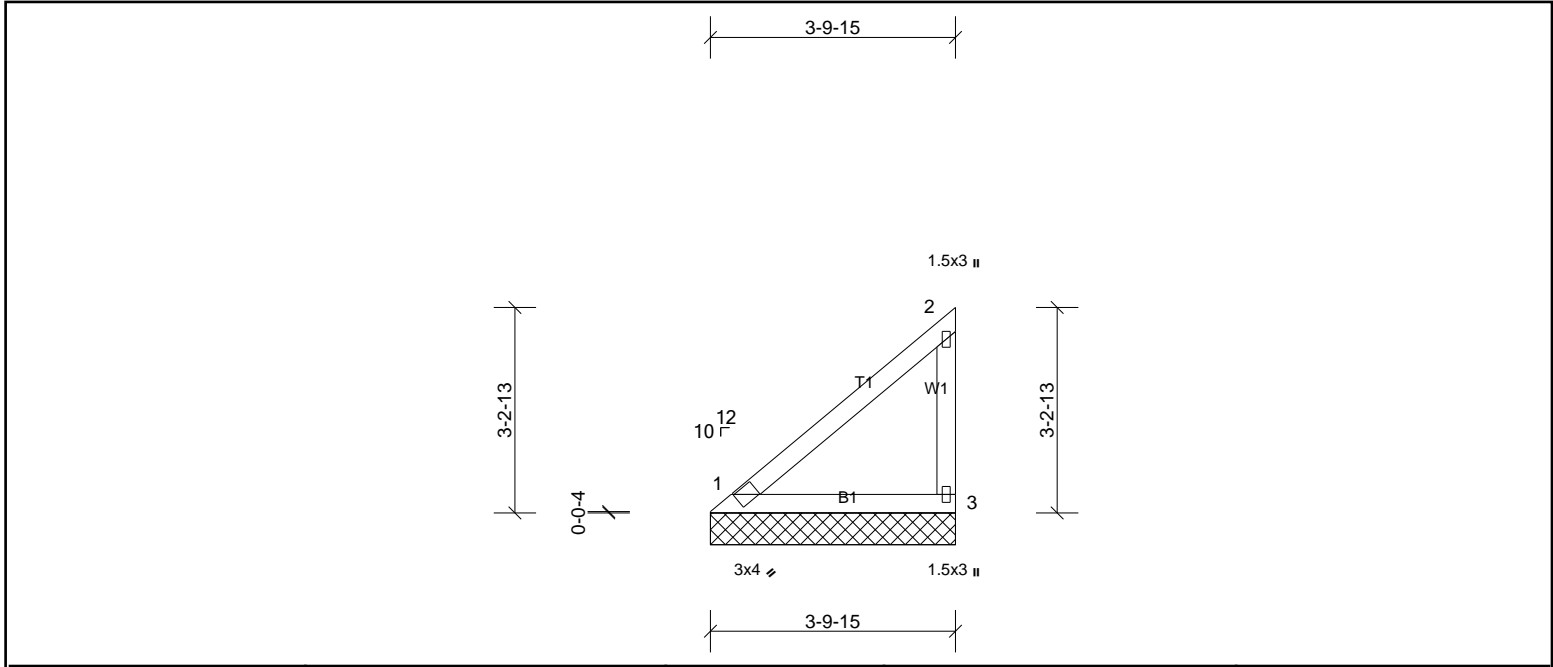
Job 72434515	Truss V8	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.13	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-P								
BCDL	10.0										Weight: 16 lb	FT = 20%

LUMBER	BRACING
TOP CHORD 2x4 SP No.2	TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins, except end verticals.
BOT CHORD 2x4 SP No.2	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2x4 SP No.3	

REACTIONS	(lb/size)	1=114/3-10-4, (min. 0-1-8), 3=114/3-10-4, (min. 0-1-8)
Max Horiz	1=96 (LC 12)	
Max Uplift	3=57 (LC 12)	
Max Grav	1=133 (LC 2), 3=143 (LC 25)	

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 57 lb uplift at joint 3.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.



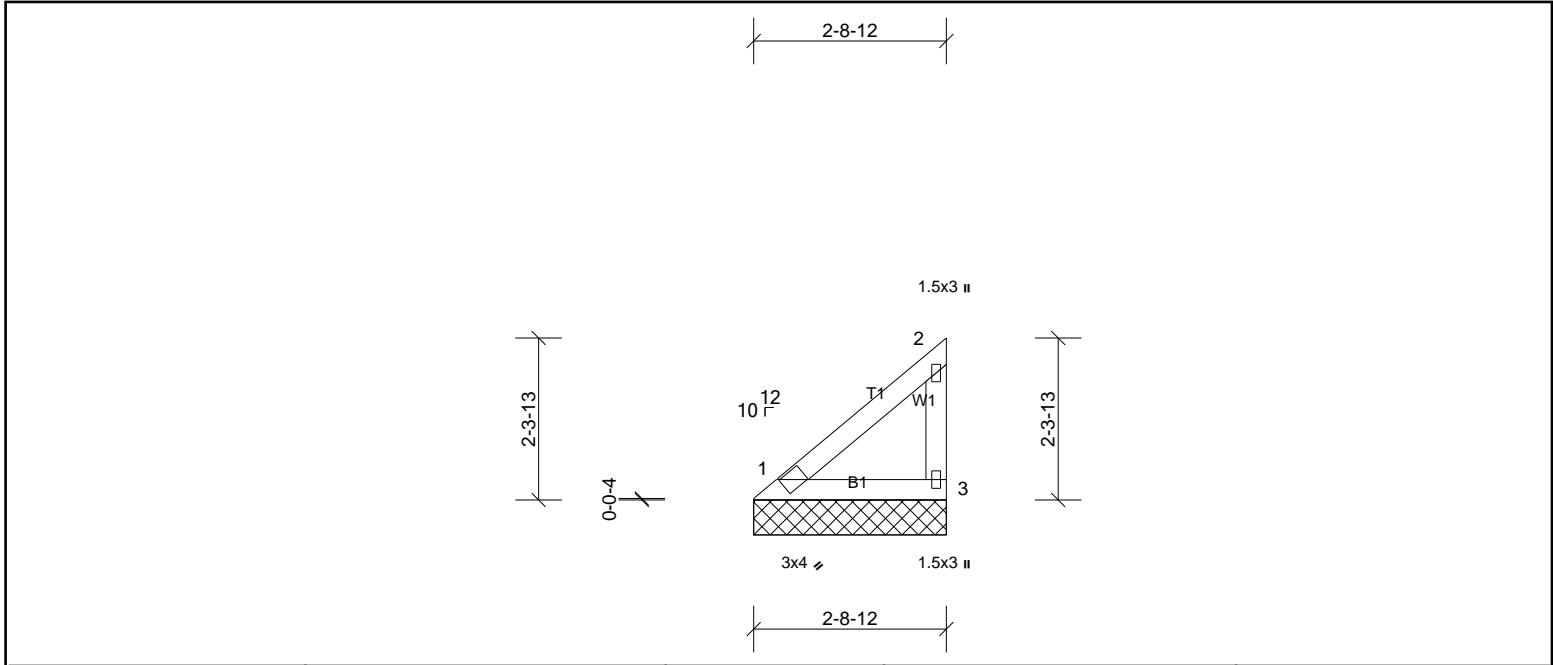
Job 72434515	Truss V9	Truss Type Truss	Qty 1	Ply 1	SD - AVERY CFI Job Reference (optional)
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UFP Mid Atlantic LLC, 5631 S. NC 62, Burlington, NC, Gina Tolley

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Loading	(psf)	Spacing	2-0-0	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP	
TCLL (roof)	20.0	Plate Grip DOL	1.15	TC	0.09	Vert(LL)	n/a	-	n/a	999	MT20	244/190
Snow (Ps/Pg)	14.3/20.0	Lumber DOL	1.15	BC	0.05	Vert(TL)	n/a	-	n/a	999		
TCDL	10.0	Rep Stress Incr	YES	WB	0.00	Horiz(TL)	0.00	3	n/a	n/a		
BCLL	0.0*	Code	IRC2021/TPI2014	Matrix-P								
BCDL	10.0										Weight: 11 lb	FT = 20%

LUMBER		BRACING	
TOP CHORD	2x4 SP No.2	TOP CHORD	Structural wood sheathing directly applied or 2-9-6 oc purlins, except end verticals.
BOT CHORD	2x4 SP No.2	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS	2x4 SP No.3		

REACTIONS	(lb/size)	1=77/2-9-1, (min. 0-1-8), 3=77/2-9-1, (min. 0-1-8)
Max Horiz	1=64 (LC 12)	
Max Uplift	3=-38 (LC 12)	
Max Grav	1=89 (LC 2), 3=96 (LC 25)	

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=6.0psf; BCDL=6.0psf; h=35ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) exterior zone and C-C Exterior (2) zone; cantilever left and right exposed; end vertical left exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
 - TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=20.0 psf; Ps=14.3 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=0.93; Ct=1.10
 - Roof design snow load has been reduced to account for slope.
 - Gable requires continuous bottom chord bearing.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-06-00 tall by 2-00-00 wide will fit between the bottom chord and any other members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 38 lb uplift at joint 3.



This design is based upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of the Building Designer. Building Designer shall verify all design information on this sheet for conformance with conditions and requirements of the specific building and governing codes and ordinances. Building Designer accepts responsibility for the correctness or accuracy of the design information as it may relate to a specific building. Certification is valid only when truss is fabricated by a UFPI plant. Bracing shown is for lateral support of truss members only and does not replace erection and permanent bracing. Refer to Building Component Safety Information (BCSI) for general guidance regarding storage, erection and bracing available from SBCA and Truss Plate Institute.

