Mark Morris, P.E.

#126, 1317-M, Summerville, SC 29483 843 209-5784, Fax (866)-213-4614

The truss drawing(s) listed below have been prepared by **Atlantic Building Components** under my direct supervision based on the parameters provided by the truss designers.

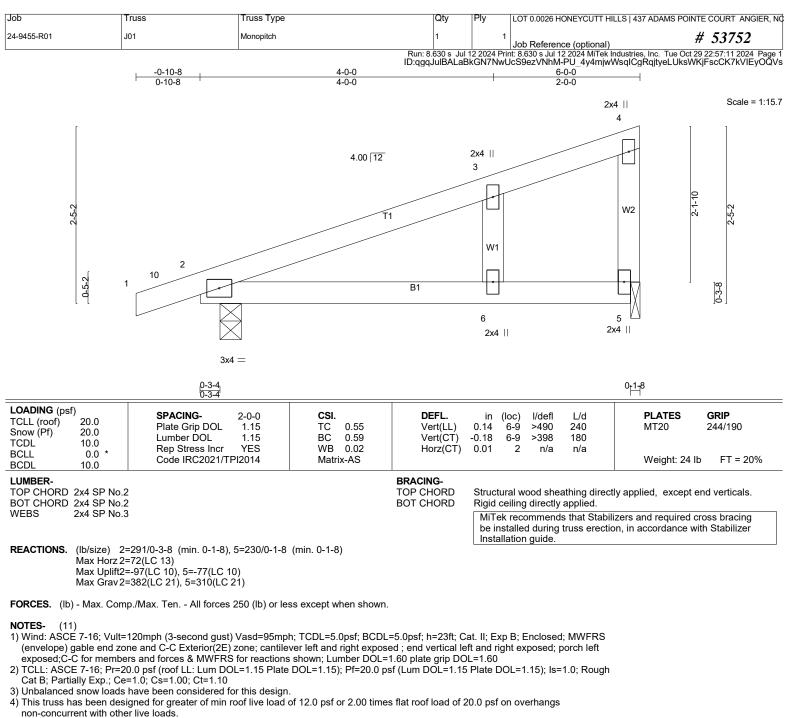
AST #: 53752 JOB: 24-9455-R01 JOB NAME: LOT 0.0026 HONEYCUTT HILLS Wind Code: ASCE7-16 Wind Speed: Vult= 120mph Exposure Category: B Mean Roof Height (feet): 23 These truss designs comply with IRC 2018 as well as IRC 2021. *41 Truss Design(s)*

Trusses:

J01, J01A, J02, J04, J05, J06, J08, J09, J10, J11, J12, P02, R01, R02, R05, R06, R06A, R07, R08, R09, R10, R11, R12, R13, R14, R14A, R15, R16, R17, V01, V02, V03, V04, V05, V06,



Warning !--- Verify design parameters and read notes before use.



5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

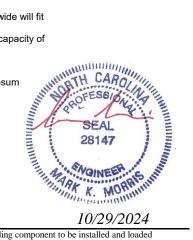
6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

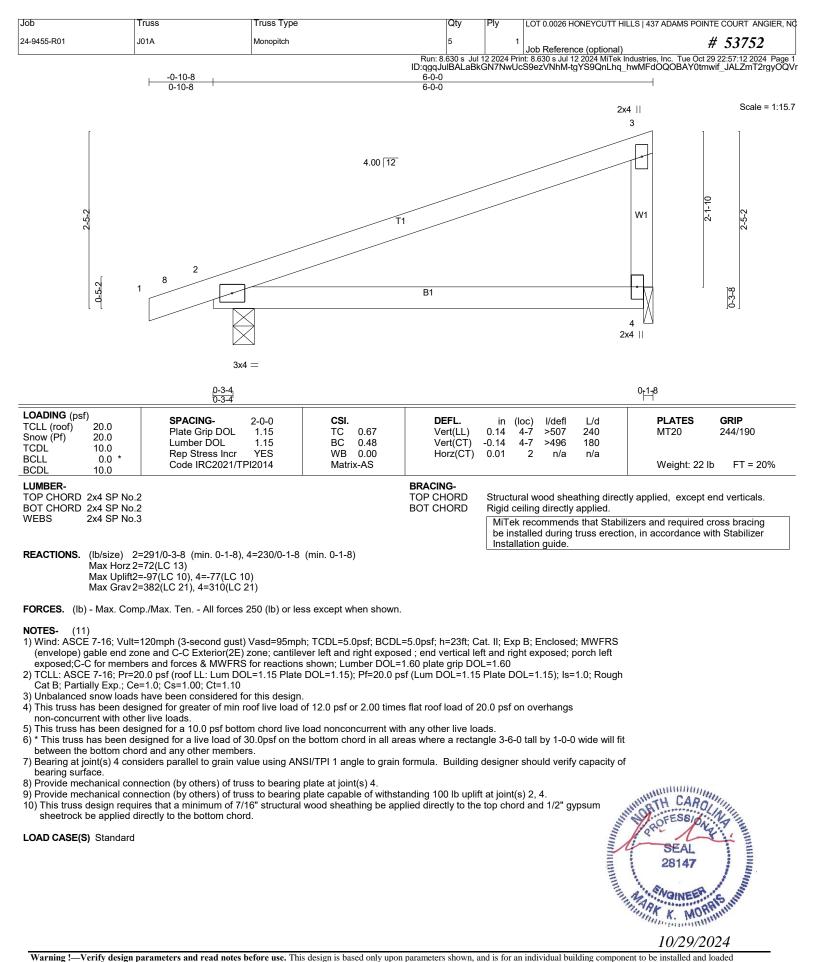
8) Provide mechanical connection (by others) of truss to bearing plate at joint(s) 5.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 5.

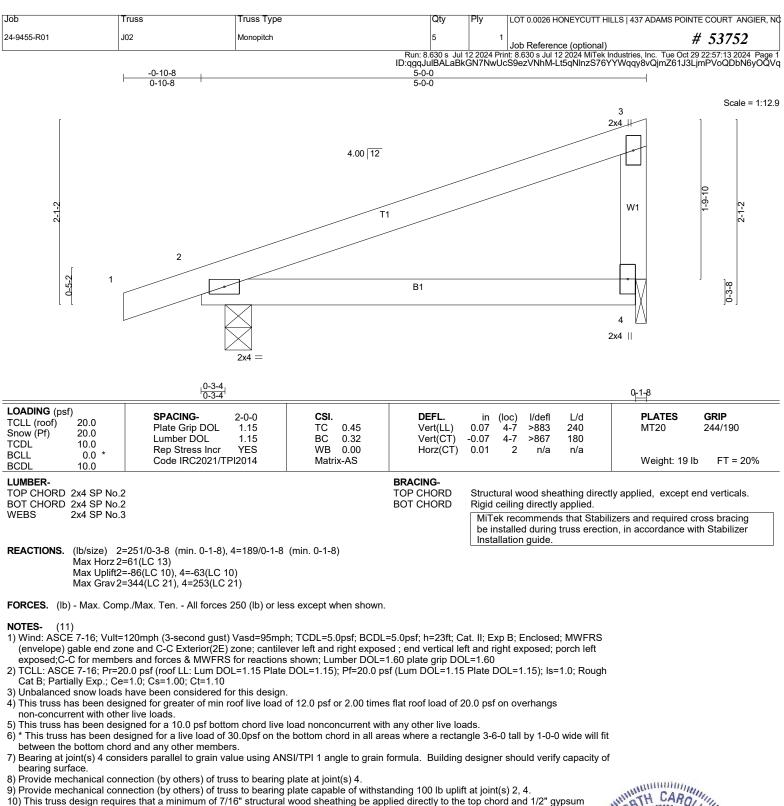
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.



LOAD CASE(S) Standard



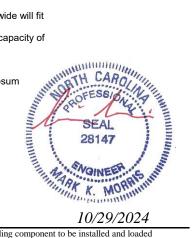
10/29/2024

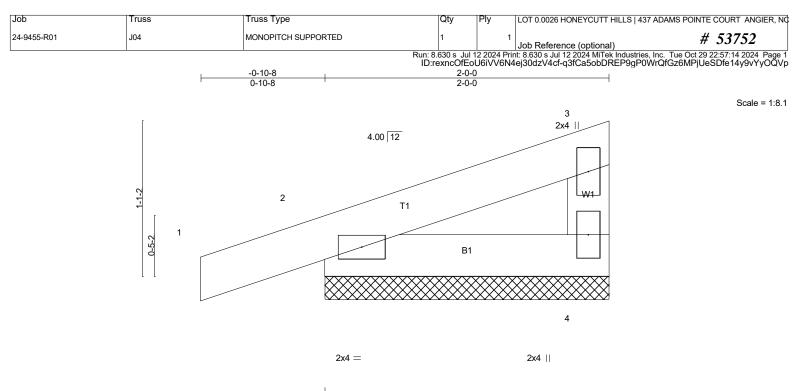


(10) This truss design requires that a minimum of 7/10 stru

LOAD CASE(S) Standard

sheetrock be applied directly to the bottom chord.





LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.09 BC 0.06 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d 0.00 1 n/r 180 0.00 1 n/r 80 0.00 4 n/a n/a	PLATES GRIP MT20 244/190 Weight: 8 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing direc end verticals. Rigid ceiling directly applied or	tly applied or 2-0-0 oc purlins,except 10-0-0 oc bracing.
					lizers and required cross bracing on, in accordance with Stabilizer

REACTIONS. (lb/size) 4=62/2-0-0 (min. 0-1-8), 2=139/2-0-0 (min. 0-1-8) Max Horz 2=27(LC 11) Max Uplift4=-9(LC 14), 2=-41(LC 10) Max Grav 4=76(LC 21), 2=180(LC 21)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

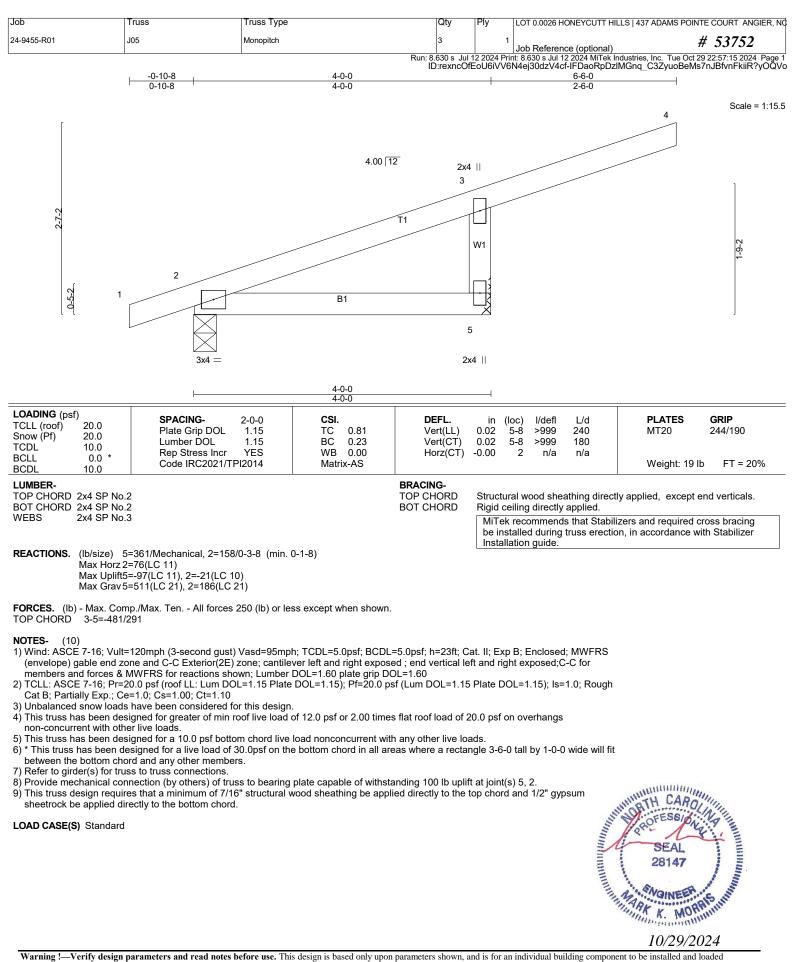
NOTES- (11)

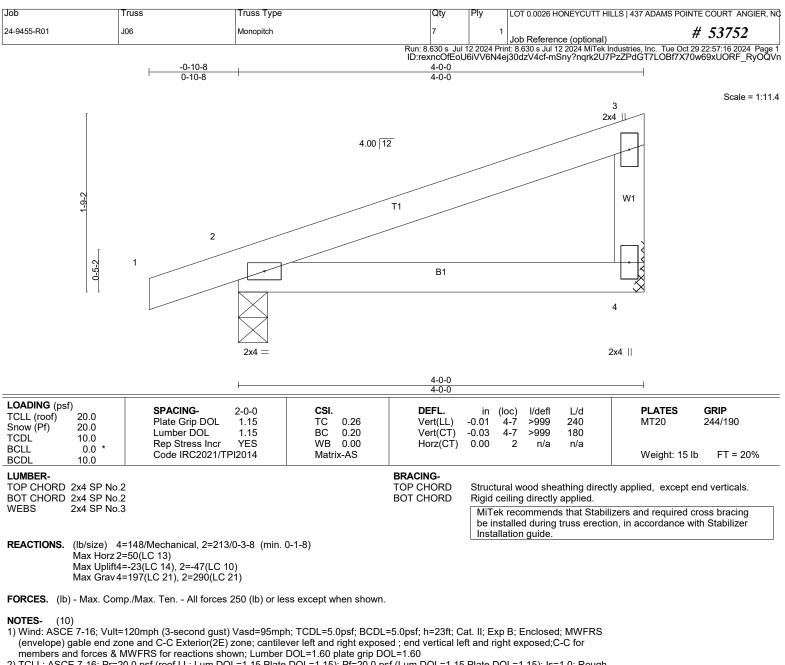
- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Corner(3E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 6) Gable requires continuous bottom chord bearing.
- 7) Gable studs spaced at 2-0-0 oc.
- 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 9) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.

LOAD CASE(S) Standard



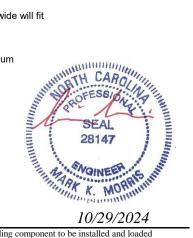


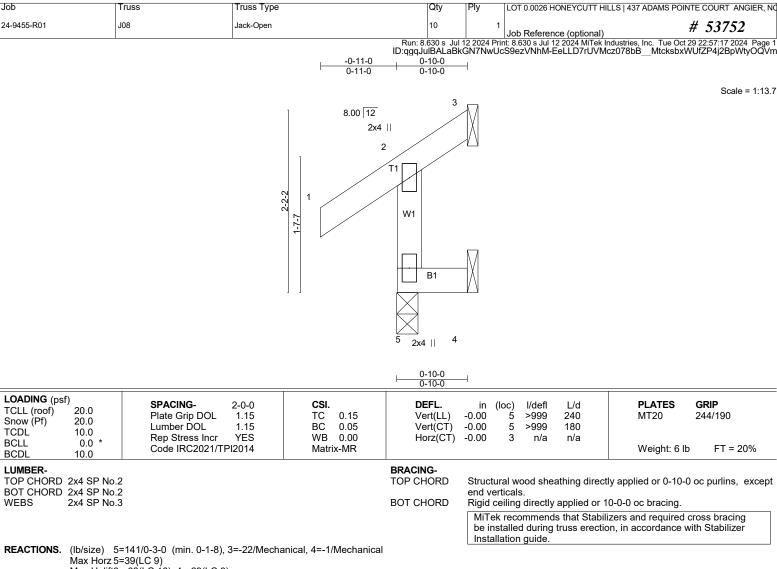


- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.

LOAD CASE(S) Standard

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Refer to girder(s) for truss to truss connections.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 9) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.





Max Uplift3=-63(LC 18), 4=-29(LC 9) Max Grav 5=203(LC 18), 3=12(LC 10), 4=33(LC 10)

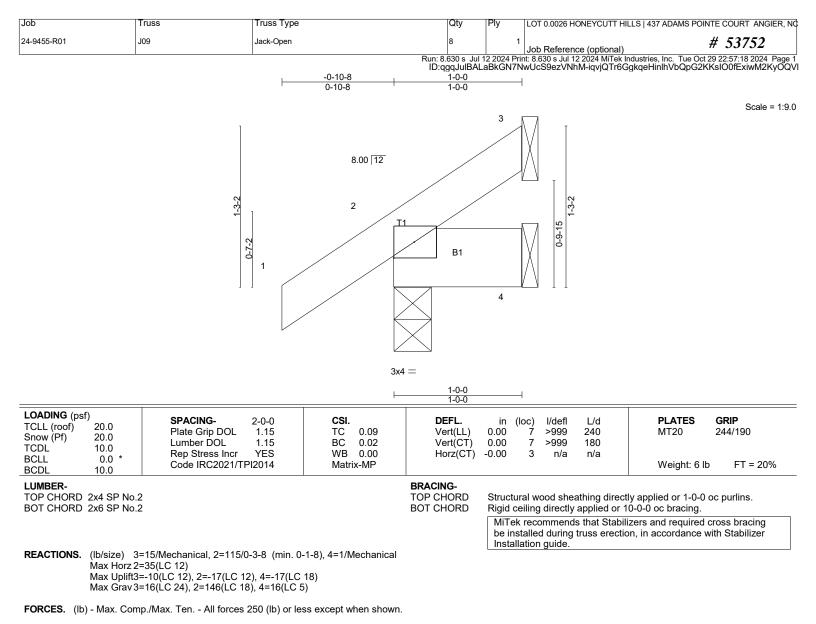
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 4.

LOAD CASE(S) Standard



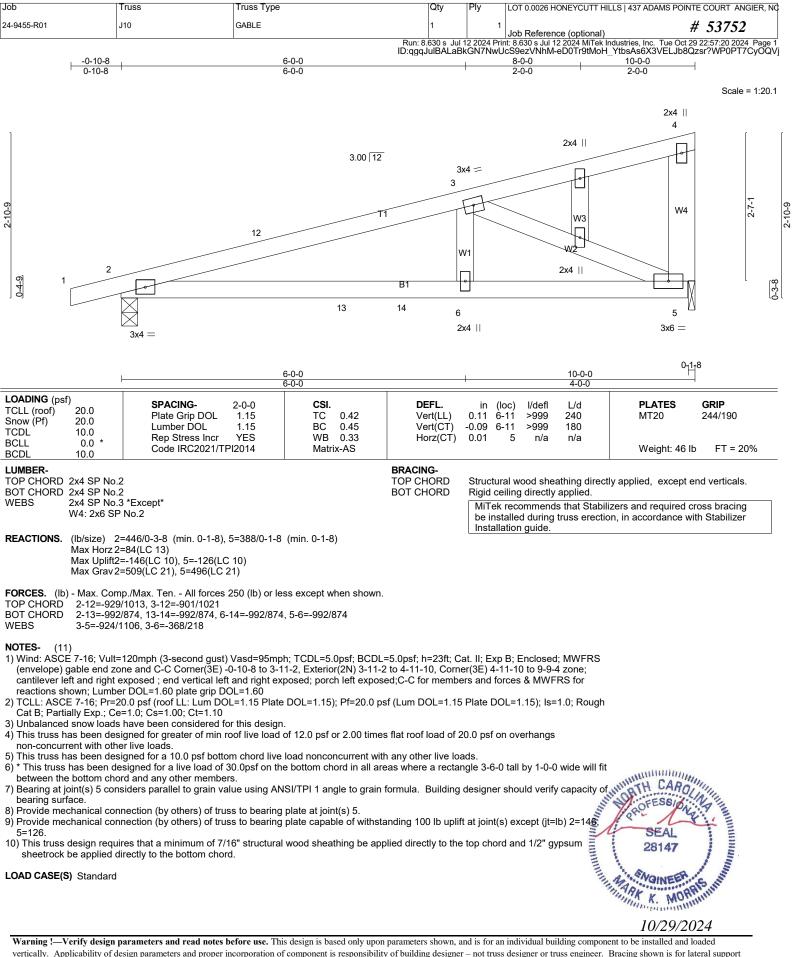


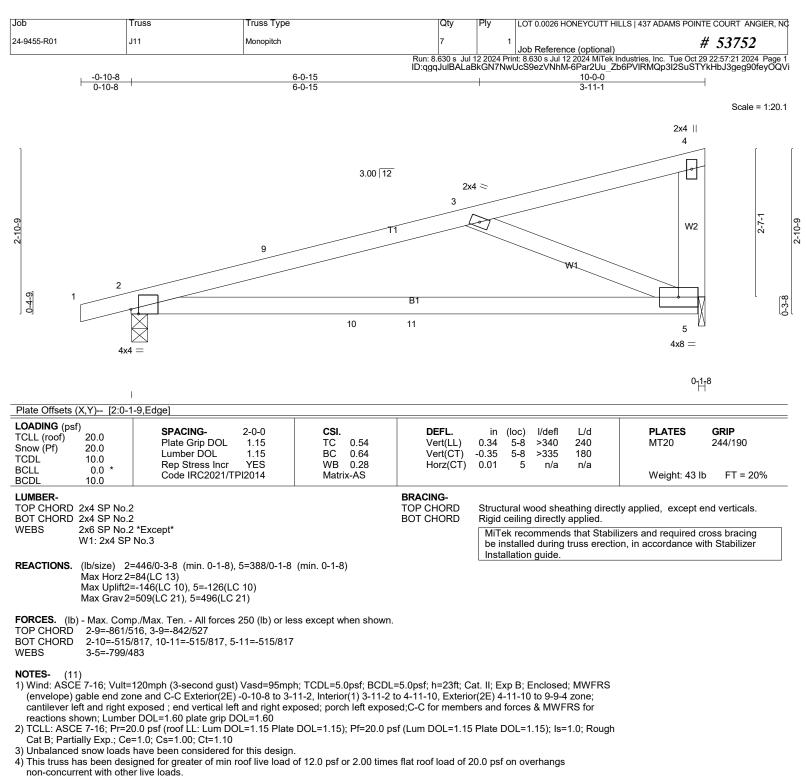
NOTES- (8)

- Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs
- non-concurrent with other live loads. 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 6) Refer to girder(s) for truss to truss connections.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 3, 2, 4.

LOAD CASE(S) Standard







5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

between the bottom chord and any other members.
7) Bearing at joint(s) 5 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of truss to bearing plate at joint(c) 5.
8) Provide mechanical connection (by others) of truss to bearing plate at joint(c) 5.

9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=146 5=126

10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

LOAD CASE(S) Standard

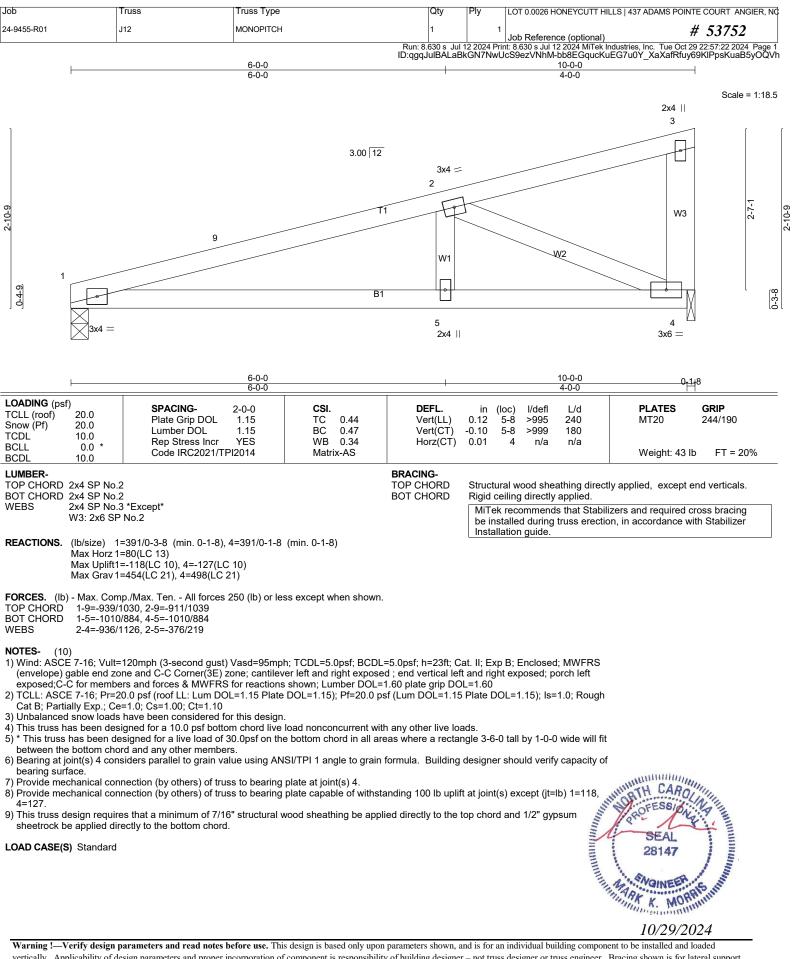
29/20.° Warning !-- Verify design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

SEAL

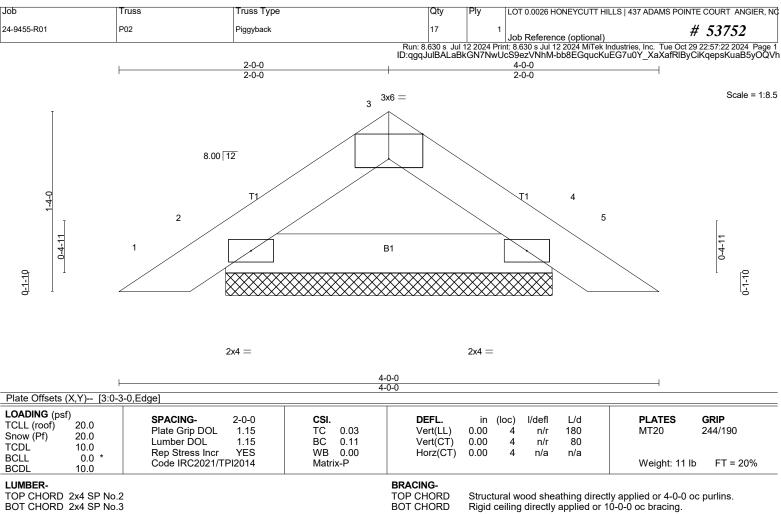
28147

NOINEE K. MORRIS

10/29/2024



vertically. Applicability of design parameters and read notes before use. This design is based only upon parameters shown, and is for an individual building component to be instanted and toaded vertically. Applicability of design parameters and read notes before use. This design is obased only upon parameters shown, and is for an individual building component to be instanted and toaded of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

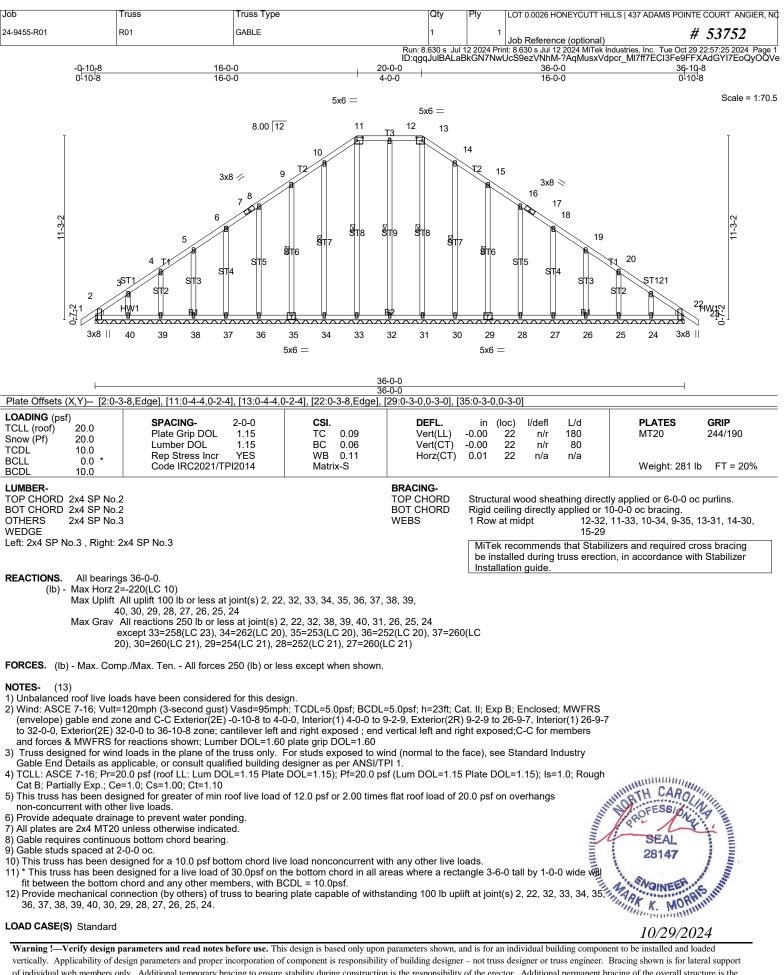
- REACTIONS. (lb/size) 2=128/2-5-0 (min. 0-1-8), 4=128/2-5-0 (min. 0-1-8) Max Horz 2=-23(LC 10) Max Uplift2=-16(LC 12), 4=-16(LC 13)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.

NOTES- (10)

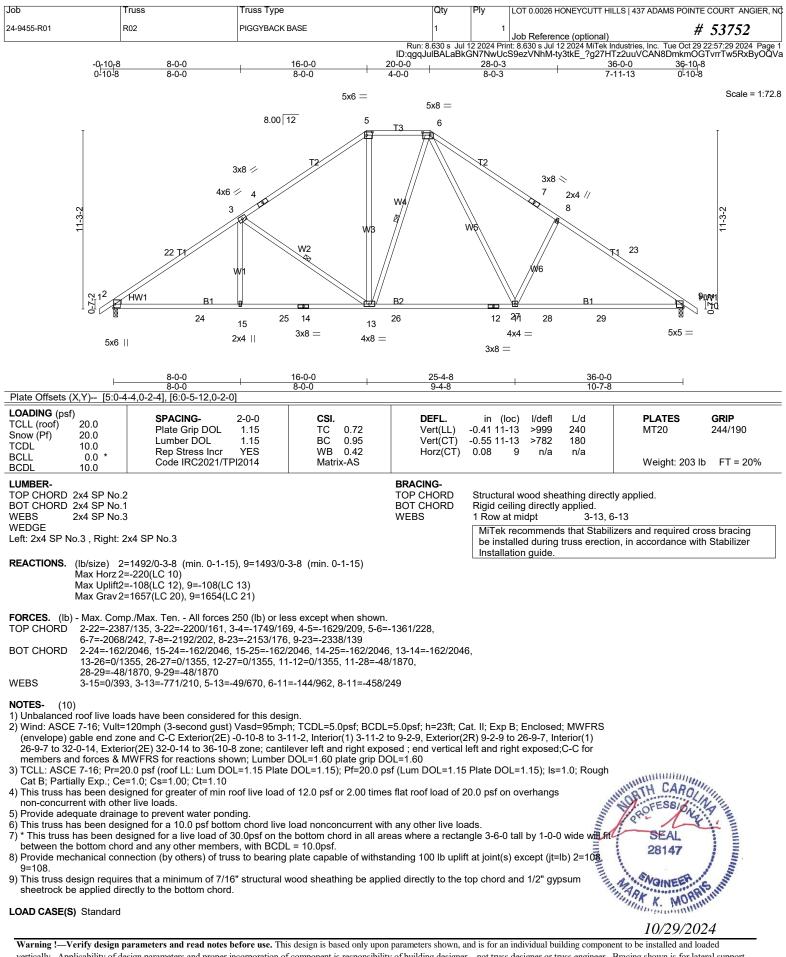
- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.0 psf on overhangs non-concurrent with other live loads.
- 5) Gable requires continuous bottom chord bearing.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 4.
- See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.

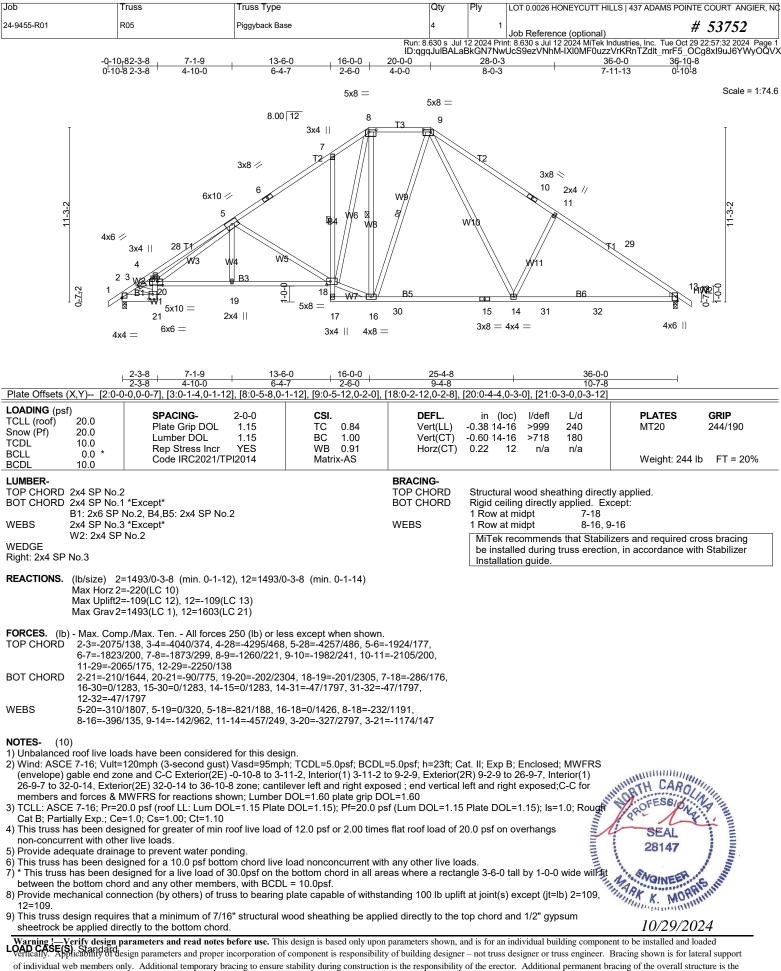
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LOAD CASE(S) Standard
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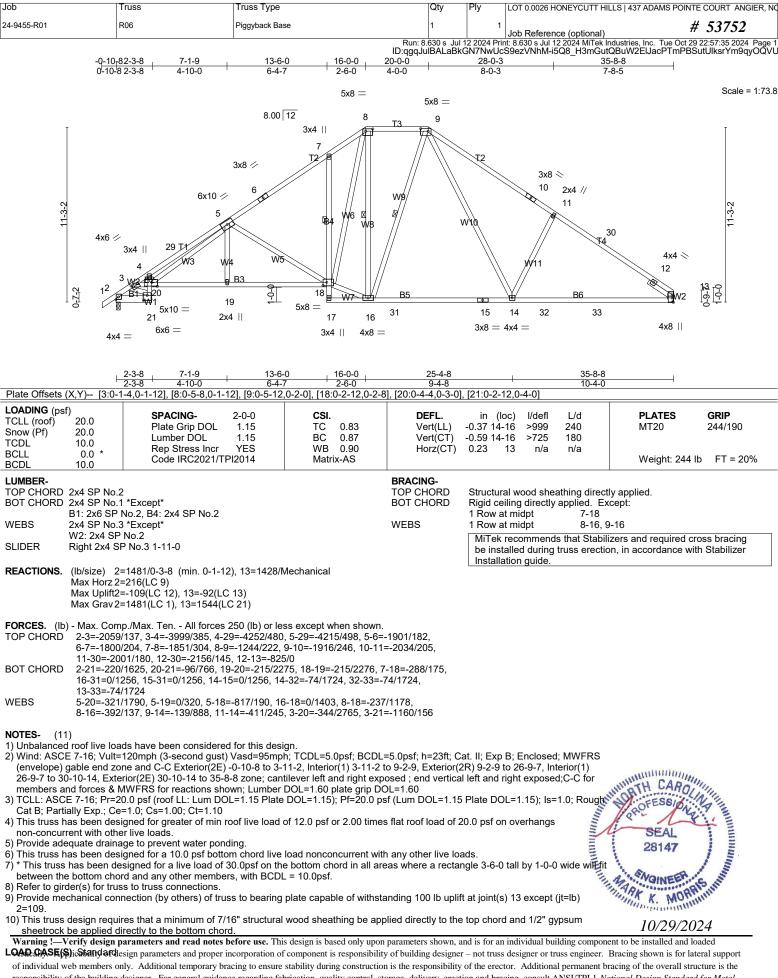


of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 *National Design Standard for Metal Plate Connected Wood Truss Construction* and BCSI 1-03 Guide to *Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses* from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

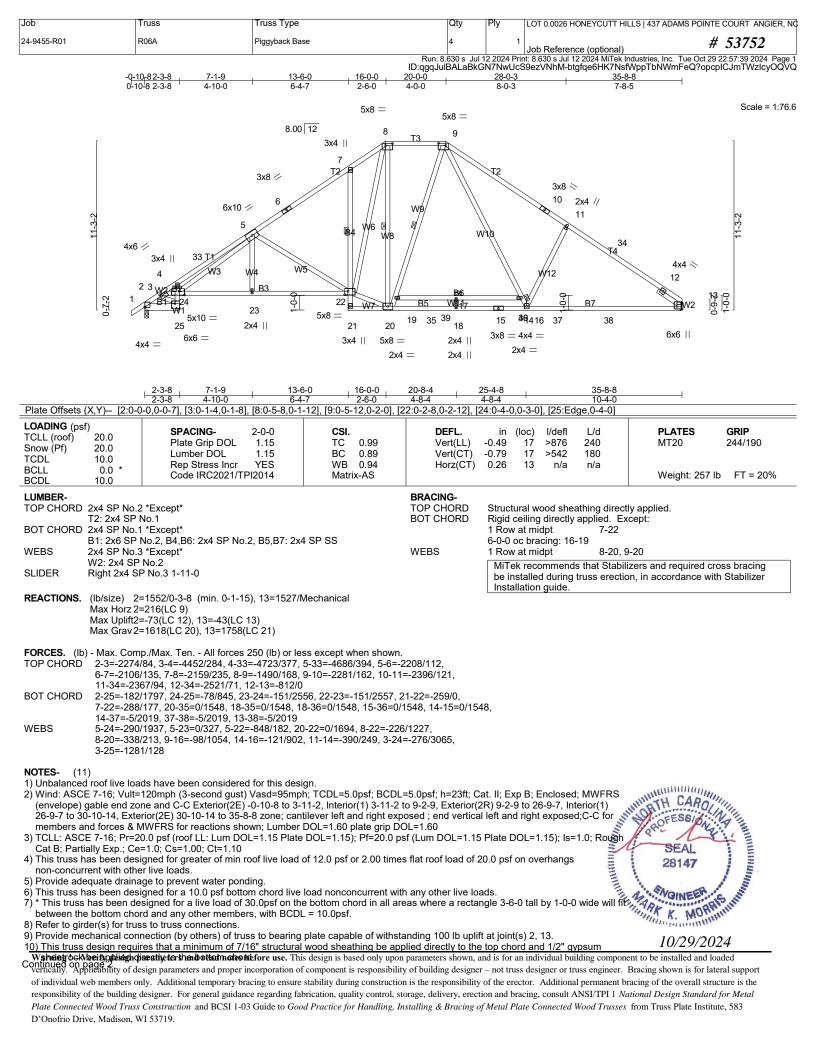




of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



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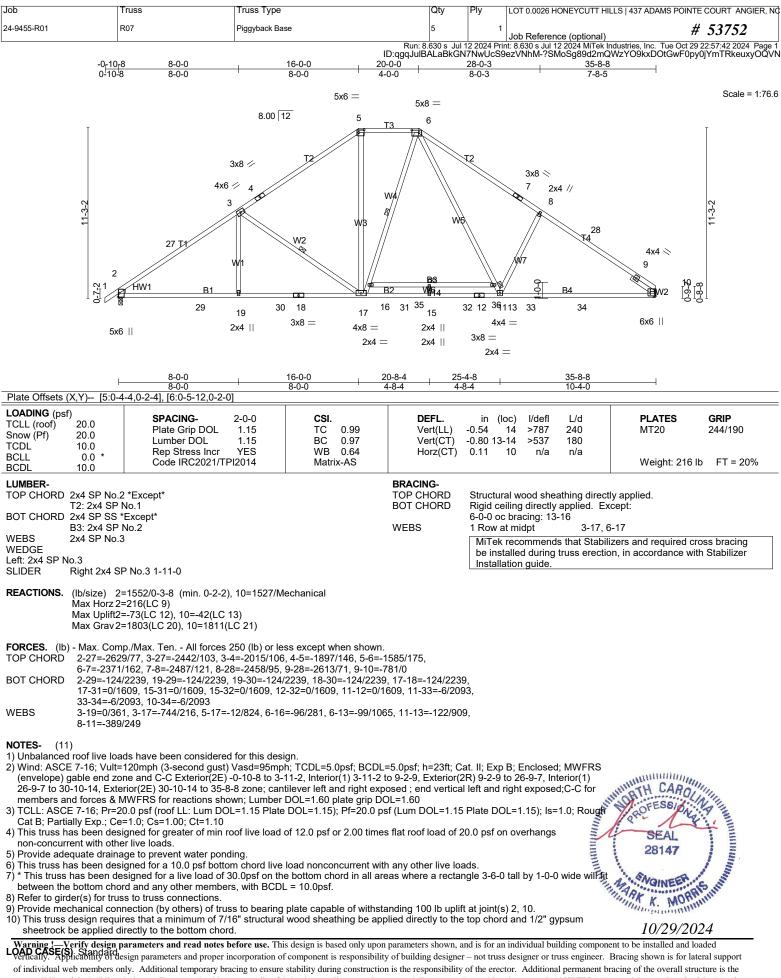


Job	Truss	Truss Type	Qty	Ply	LOT 0.0026 HONEYCUTT HILLS 437 AD	AMS POINTE COURT ANGIER, NC
24-9455-R01	R06A	Piggyback Base	4	1	Job Reference (optional)	# 53752
			Pup: 8,630 c Jul 1	2 2024 Drin	at: 8 630 c. Jul 12 2024 MiTok Industrias Inc.	Tuo Oct 20 22:57:30 2024 Page 2

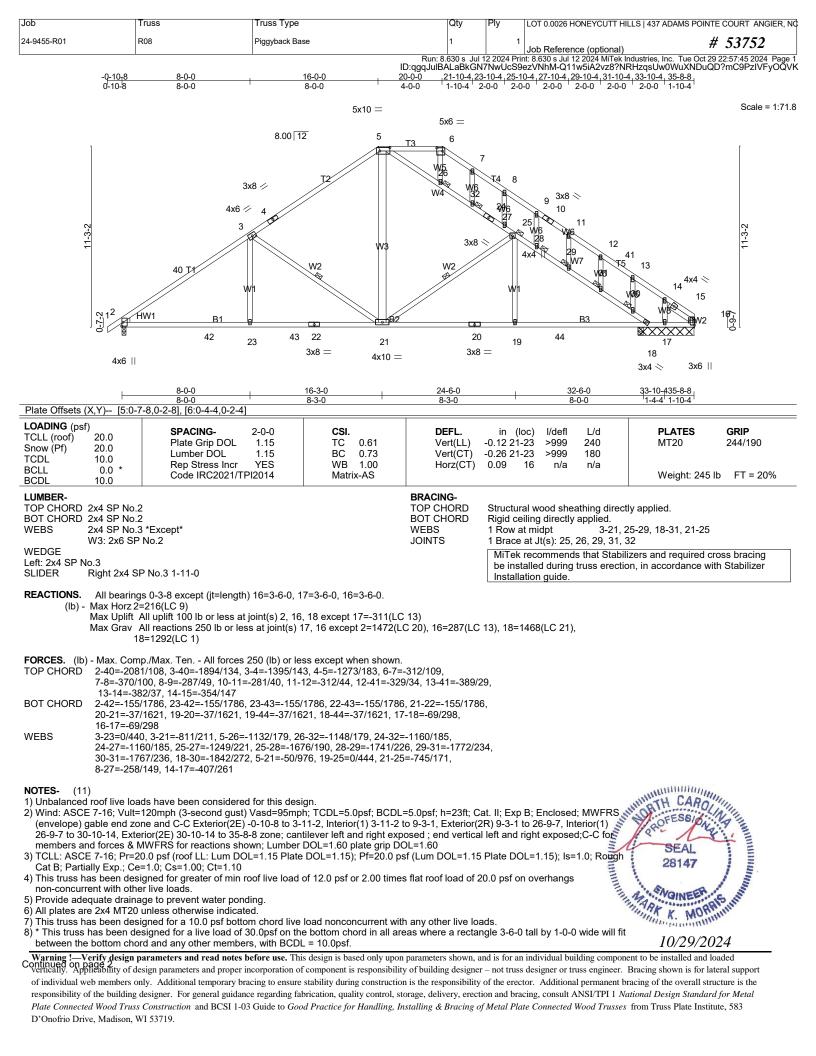
Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 29 22:57:39 2024 Page 2 ID:qgqJulBALaBkGN7NwUcS9ezVNhM-btgfqe6HK7NsfWppTbNWmFeQ?opcpICJmTWzIcyOQVQ

LOAD CASE(S) Standard





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.



Job	Truss	Truss Type	Qty	Ply	LOT 0.0026 HONEYCUTT HILLS 437 ADAMS POINTE COURT ANGIER, NC
24-9455-R01	R08	Piggyback Base	1	1	Job Reference (optional) # 53752
Run: 8.630 s_Jul 12 2024 Print: 8.630 s_Jul 12 2024 MiTek Industries, IncTue Oct 29 22:57:46 2024 P ID:qqqJulBALaBkGN7NwUcS9ezVNhM-uDbIl2BggGGs?arAOZ?9ZjQi6dEfyS0LO3ir1iy0					

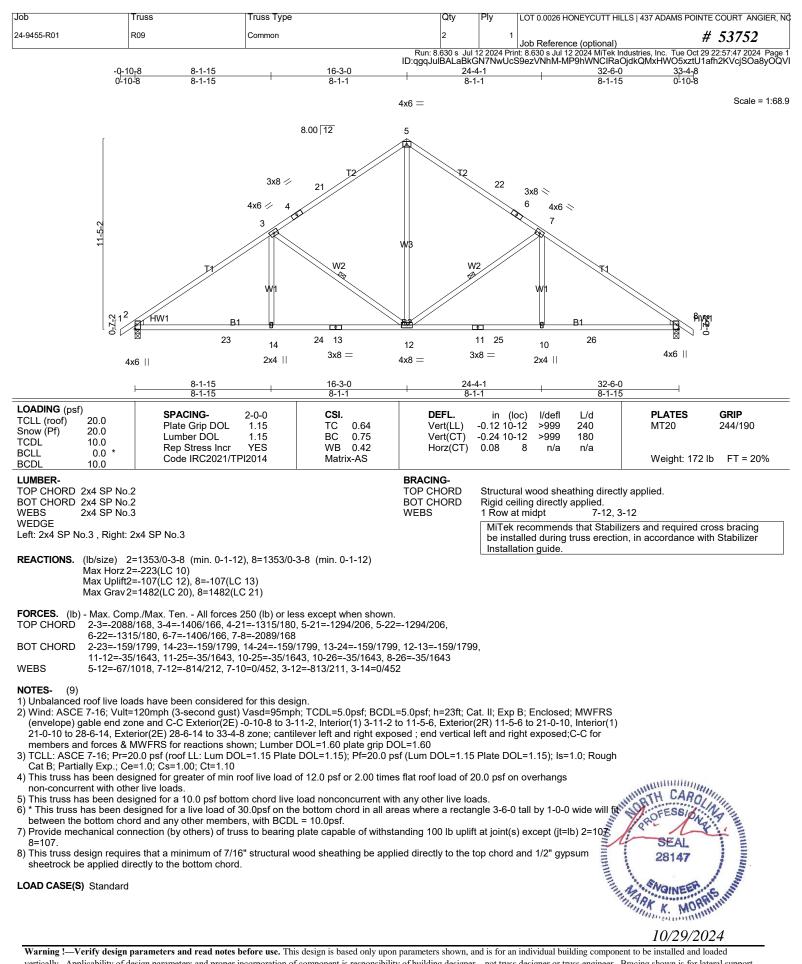
NOTES- (11)

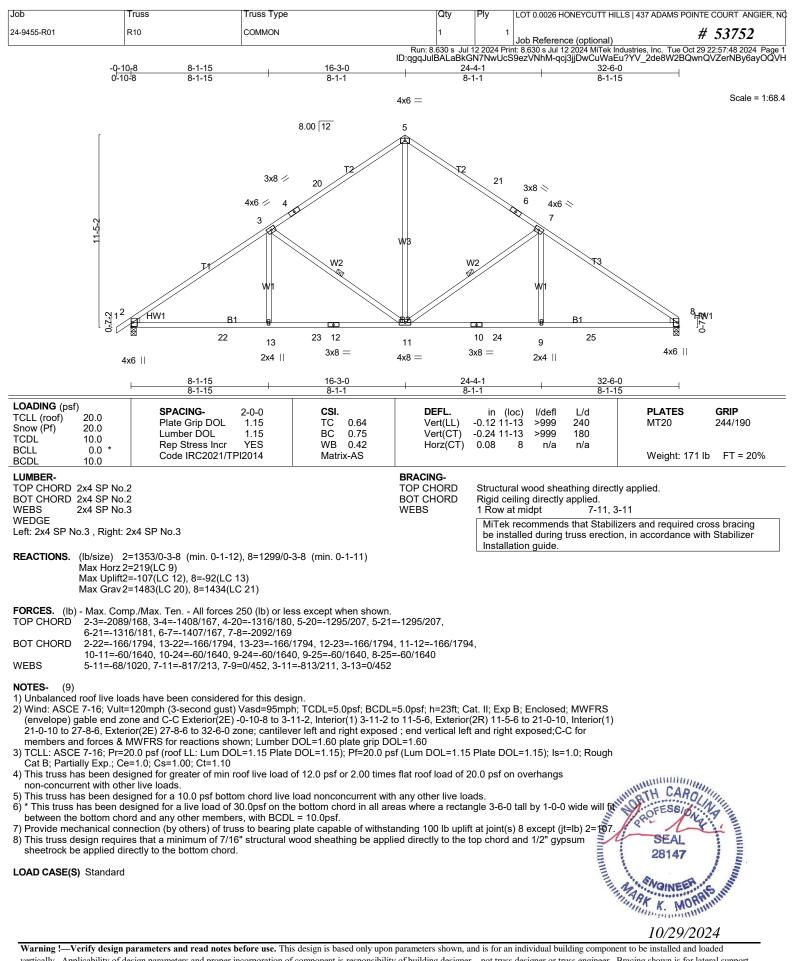
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 16, 18, 16 except (jt=lb) 17=311.

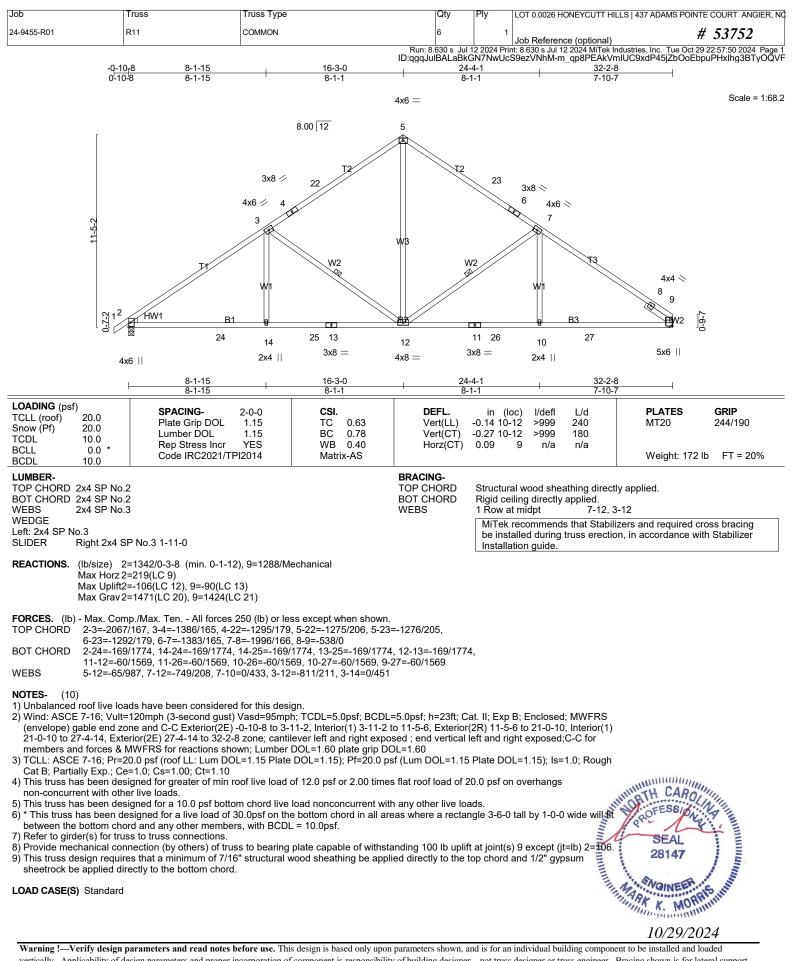
10) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

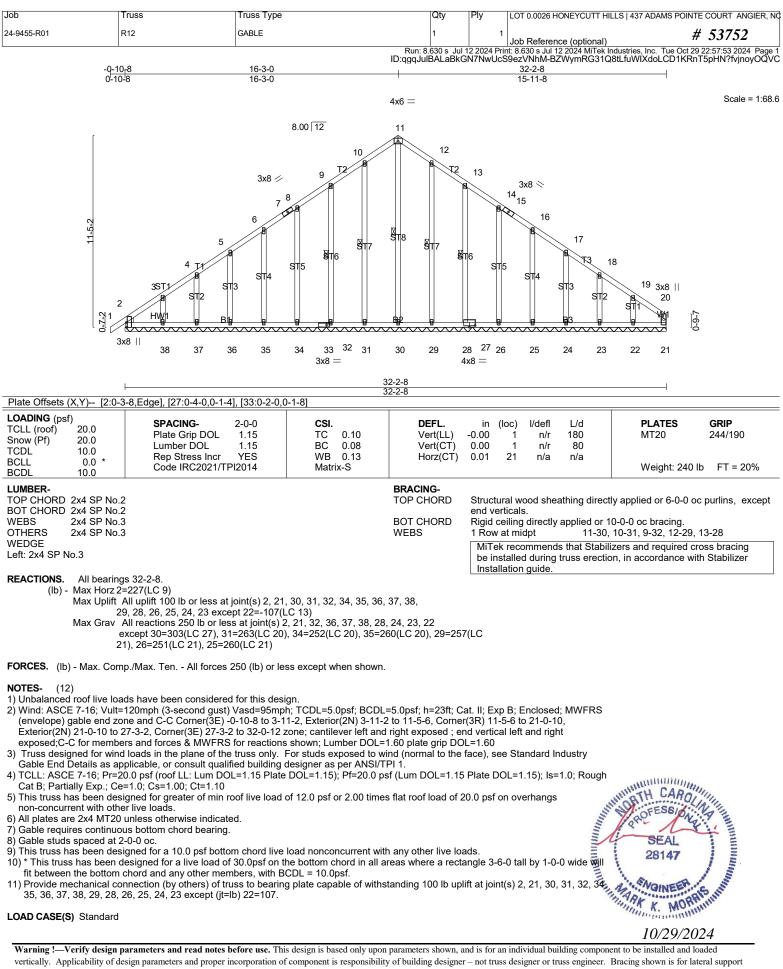
LOAD CASE(S) Standard

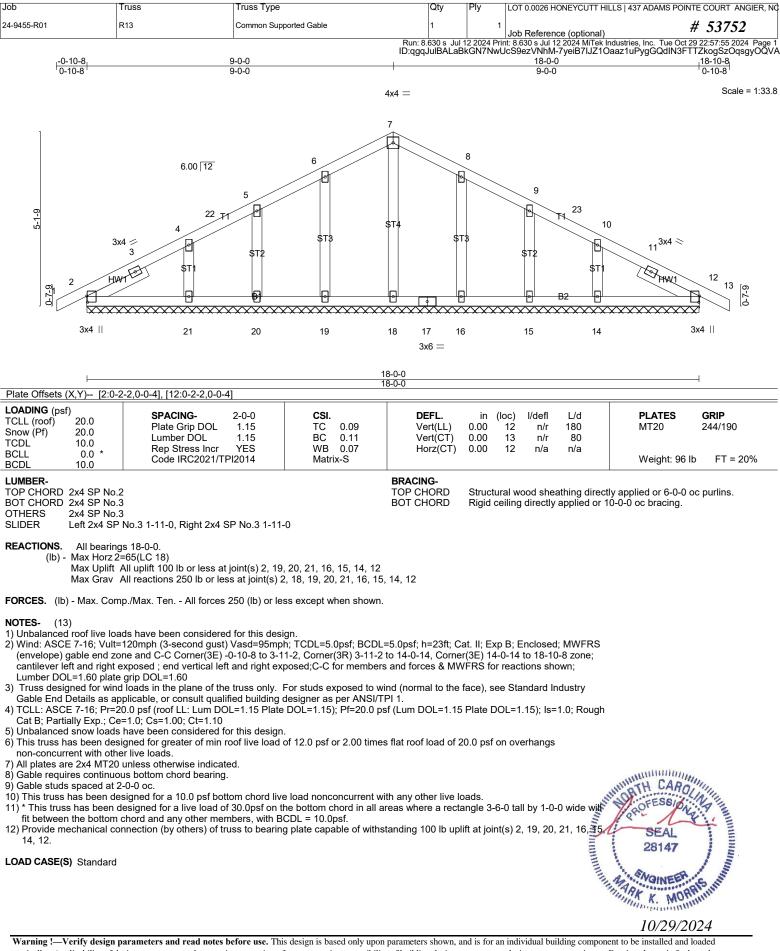


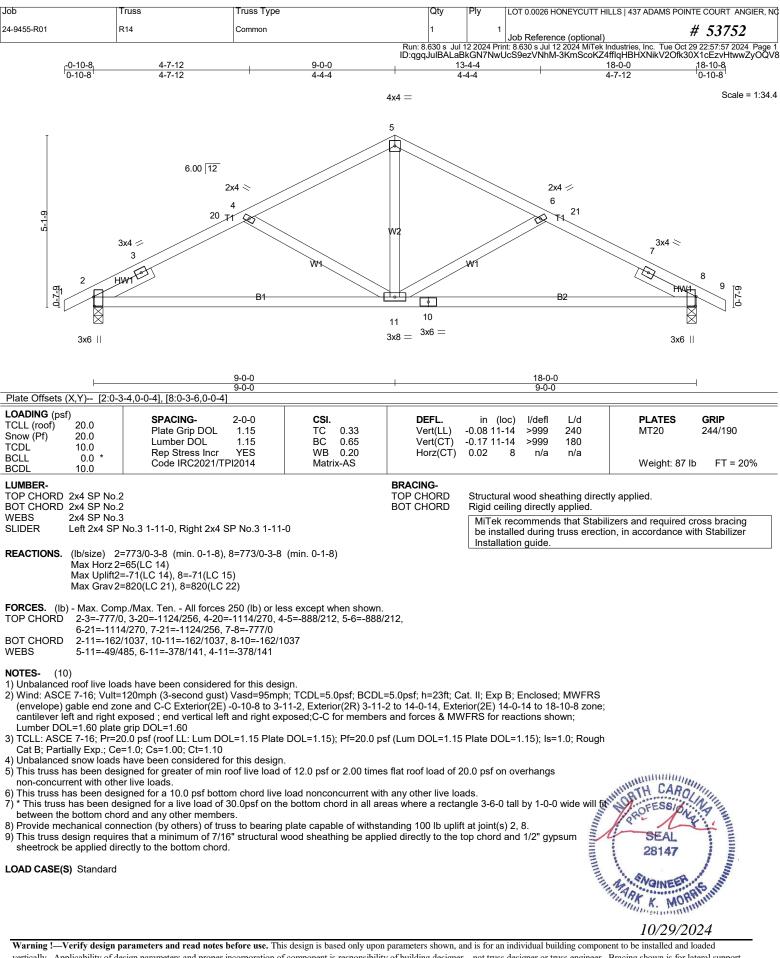


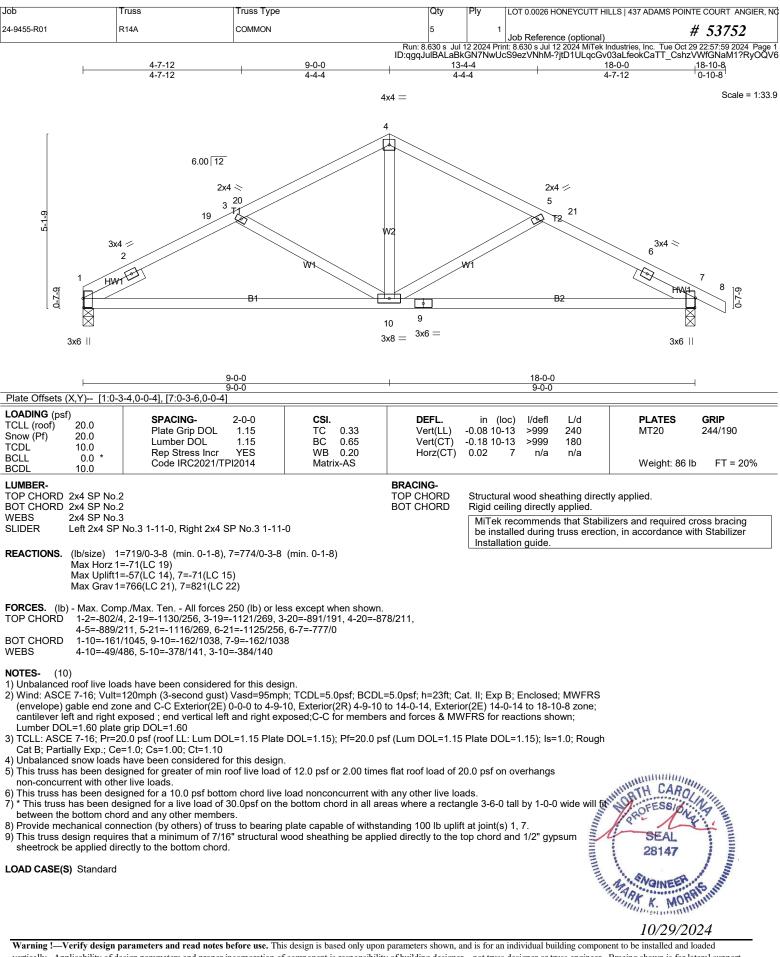












Job	Tr	russ	Truss Type	Qty	Ply LOT 0.0026 HOM		ADAMS POINTE COURT ANGIER, N
24-9455-R01	R1		COMMON GIRDER	1			# 53752
				Run: 8.630 s Jul	12 2024 Print: 8.630 s Jul 12 2	2024 MiTek Industries,	Inc. Tue Oct 29 22:58:04 2024 Page LKNHWAIJtRvAc6?Ws3ogfyOQV
				3-0 11-3-0 13-3-1 0-0 2-0-0 2-0-0	0 15-3-0 18-3-8 18 _т 6-0		
			0-2-0 0-0-0 2-0-0 2-0-0 2-		2-0-0 0-0-0 0-2-0		Scale = 1:70.9
				4x6			Scale - 1.70.
		ſ		10			
			/				
			12.00 12 7 J				
			5		14		
		0	TT B		BT 1		
		11-2-10	2 W7	WB W7	17		
			$w_{3} 4x4 = 4x$		W6 4x4 =W3	:	
		6X	$0 = \frac{W_3 4x4}{1} + \frac{4}{6} + \frac{4}{6} + \frac{4}{1} + \frac{4}$	'	16 19		
		01-				-11-10 2-2-2	
		1-11-10	W2 B1 W5	10x10 W5	W2	<u>1-11-10</u> 2-2-2	
			⊠ 26 27 28 2 25 26 24	²⁹ 23 ²² 30	21 ^{31 32} [⊠] 20		
			5x6 = 8x8 =	4x6 = 7x8 =	8x8 = 5x6 =	:	
			4-8-6 9-3-0	13-9-10	18-6-0		
Plate Offsets ()	X.Y) [1:0-6-	8.0-1-71. [19:0-6-8.0-1-7]	<u>4-8-6</u> <u>4-6-10</u> , [21:0-3-8,0-4-8], [23:0-4-0,0-4-12],	4-6-10	4-8-6		
LOADING (psf))	SPACING-	2-0-0 CSI .	DEFL.	in (loc) l/defl	L/d	PLATES GRIP
TCLL (roof) Snow (Pf)	20.0 20.0	Plate Grip DOL Lumber DOL	1.15 TC 0.76 1.15 BC 0.47	Vert(LL) Vert(CT)	-0.06 21-23 >999		MT20 244/190
TCDL BCLL	10.0 0.0 *	Rep Stress Incr	NO WB 0.96	Horz(CT)	0.02 20 n/a	n/a	
BCDL	10.0	Code IRC2021/TP	2014 Matrix-MSH				Weight: 649 lb FT = 20%
LUMBER- TOP CHORD :				BRACING- TOP CHORD		ning directly appli	ed or 6-0-0 oc purlins, except
BOT CHORD	T2,T3: 2x6 SI 2x6 SP DSS	P No.2		BOT CHORD	end verticals. Rigid ceiling directly a	oplied or 10-0-0 o	c bracing.
WEBS	2x4 SP No.3			JOINTS	1 Brace at Jt(s): 11, 16	8, 4, 13, 8	
	(lb/size) 25 Max Horz 25		, 20=6773/0-3-8 (min. 0-1-12)				
	Max Uplift25	=-511(LC 11), 20=-414(L =9063(LC 3), 20=7682(L					
			50 (lb) or less except when shown.				
	1-2=-5314/2	264, 2-5=-5106/314, 5-7=	-5115/365, 7-10=-4957/399, 10-12=		.		
	1-3=-4012/4	495, 3-4=-4012/495, 4-6=	17-19=-5314/266, 1-25=-6685/388 -1635/120, 6-8=-1635/120, 8-9=-16	35/120,			
	18-19=-417	6/503	13-15=-1635/120, 15-16=-1635/120		,		
BOT CHORD			24-27=-261/1332, 24-28=-617/755 22-30=-547/7755, 21-30=-547/775				
WEBS		1315, 20-32=-92/1315 /6289, 16-23=-2716/409	16-21=-162/1522, 4-23=-2557/411	4-24=-161/1434			
			17-18=-145/253, 10-11=-498/6663,				
NOTES- (14)		ed together with 10d (0.1	a1"v2") pails as follows:				
Top chords of	connected as	follows: 2x4 - 1 row at 0	9-0 oc, 2x6 - 2 rows staggered at 0				
Wohe conne	acted as follow	l as follows: 2x6 - 2 rows ws: 2x4 - 1 row at 0-9-0 c	<u> </u>			1111	
 All loads are connections 	considered e have been p	equally applied to all plies rovided to distribute only	s, except if noted as front (F) or bac loads noted as (F) or (B), unless ot	k (B) face in the L nerwise indicated	OAD CASE(S) section.	Ply to ply	HUAROLIN
 Unbalanced Wind: ASCE 	roof live load 5 7-16; Vult=1	ls have been considered 20mph (3-second gust)	for this design. /asd=95mph; TCDL=5.0psf; BCDL=	=5.0psf; h=23ft; C	at. II; Exp B; Enclosed; I	WFRS	Nation
(envelope) g 5) TCLL: ASCE	jable end zon E 7-16; Pr=20	e; cantilever left and righ .0 psf (roof LL: Lum DOL	t exposed ; end vertical left and righ =1.15 Plate DOL=1.15): Pf=20.0 ps	t exposed; Lumb f (Lum DOL=1.15	er DOL=1.60 plate grip I Plate DOL=1.15): ls=1	OOL=1.60	SEAL
Cat B; Partia	ally Exp.; Ce=	1.0; Cs=1.00; Ct=1.10	staggered at 0-4-0 oc. c. s, except if noted as front (F) or back loads noted as (F) or (B), unless oth for this design. /asd=95mph; TCDL=5.0psf; BCDL= t exposed ; end vertical left and righ =1.15 Plate DOL=1.15); Pf=20.0 ps ng.	,		11111	28147
7) All plates are	e 2x4 MT20 u	inless otherwise indicate	d. a chord live load ponconcurrent with	any other live lo	ads	THE AS	NOINEER &
9) * This truss I	has been des	signed for a live load of 3	c. s, except if noted as front (F) or back loads noted as (F) or (B), unless oth for this design. /asd=95mph; TCDL=5.0psf; BCDL= t exposed ; end vertical left and righ =1.15 Plate DOL=1.15); Pf=20.0 ps ng. d. n chord live load nonconcurrent with 0.0psf on the bottom chord in all are s. ain value using ANSI/TPI 1 angle to effore use. This design is based only upon corporation of component is responsibility cing to ensure stability during construction e regarding fabrication, quality control, ste Guide to <i>Good Practice for Handling, In</i>	as where a rectai	ngle 3-6-0 tall by 1-0-0 v	vide will fit	K. MORRAUM
10) Bearing at j	joint(s) 25, 20	d and any other members) considers parallel to gra	s. ain value using ANSI/TPI 1 angle to	grain formula. Bu	uilding designer should v	verify	10/20/2024
capacity of	bearing surfa	ace. Trameters and read notes by	fore use. This design is based only upon	narameters shown	nd is for an individual build	ing component to be	10/29/2024
Continued on p	age 2 des	ign parameters and proper in	corporation of component is responsibility	of building designer	r – not truss designer or trus	s engineer. Bracing	shown is for lateral support
of individual we responsibility of	eb members only of the building do	 Additional temporary bra esigner. For general guidanc 	cing to ensure stability during construction e regarding fabrication, quality control. sto	i is the responsibility prage, delivery, erect	of the erector. Additional p ion and bracing, consult AN	ermanent bracing of SI/TPI 1 National D	the overall structure is the Design Standard for Metal
Plate Connecte	d Wood Truss (Construction and BCSI 1-03	Guide to Good Practice for Handling, In	stalling & Bracing o	f Metal Plate Connected We	ood Trusses from Tr	uss Plate Institute, 583

D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0026 HONEYCUTT HILLS 437 ADAMS POINTE COURT ANGIER, NC
24-9455-R01	R15	COMMON GIRDER	1	3	Job Reference (optional) # 53752
					nt: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 29 22:58:04 2024 Page 2 S9ezVNhM-Mhh64BPyRoXJ9LDdRLKNHWAIJtRvAc6?Ws3ogfyOQV1

NOTES- (14)

- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 25=511, 20=414.
- 12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.
- 13) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 27 lb down and 60 lb up at 0-4-11, and 27 lb down and 60 lb up at 18-1-5 on top chord, and 1512 lb down and 104 lb up at 0-1-12, 29 lb down and 31 lb up at 2-3-4, 1717 lb down and 63 lb up at 2-3-12, 29 lb down and 31 lb up at 4-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1717 lb down and 63 lb up at 6-3-4, 1717 lb down and 63 lb up at 4-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 6-3-4, 1717 lb down and 63 lb up at 12-3-4, 1717 lb down and 63 lb up at 4-3-12, 29 lb down and 31 lb up at 8-3-12, 29 lb down and 63 lb up at 10-3-12, 29 lb down and 31 lb up at 12-3-4, 1771 lb down and 62 lb up at 12-3-12, 29 lb down and 31 lb up at 14-3-4, 1771 lb down and 62 lb up at 14-3-12, and 29 lb down and 31 lb up at 16-3-4, and 1771 lb down and 62 lb up at 16-3-12 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

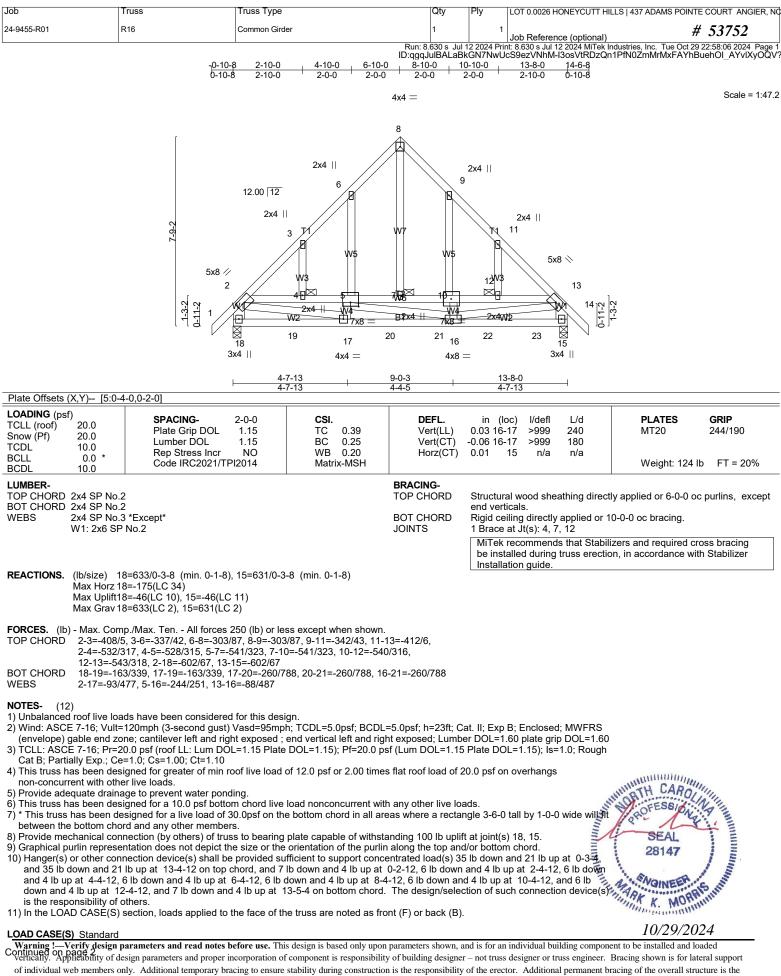
Uniform Loads (plf)

Vert: 1-10=-60, 10-19=-60, 20-25=-20, 1-19=-6(F)

Concentrated Loads (lb)

Vert: 22=-1503(F=4, B=-1507) 1=20(F) 19=20(F) 25=-1416(B) 26=-1503(F=4, B=-1507) 27=-1503(F=4, B=-1507) 28=-1503(F=4, B=-1507) 29=-1503(F=4, B=-1507) 30=-1503(F=4, B=-1507) 31=-1503(F=4, B=-1507) 32=-1503(F=4, B=-1507) 32=-1503(F





responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

Job	Truss	Truss Type	Qty	Ply	LOT 0.0026 HONEYCUTT HILLS 437 ADAM	IS POINTE COURT ANGIER, NO
24-9455-R01	R16	Common Girder	1	1	Job Reference (optional)	# 53752
		Dup: 9	620 a Jul 1	2 2024 Driv	at: 9,620 a Jul 12,2024 MiTak Industrias Inc. T	up Opt 20 22:59:06 2024 Dogo 2

Run: 8.630 s Jul 12 2024 Print: 8.630 s Jul 12 2024 Milek Industries, Inc. Tue Oct 29 22:58:06 2024 Page 2 ID:qgqJulBALaBkGN7NwUcS9ezVNhM-I3osVtRDzQn1PfN0ZmMrMxFAYhBuehOI_AYvIXyOQV?

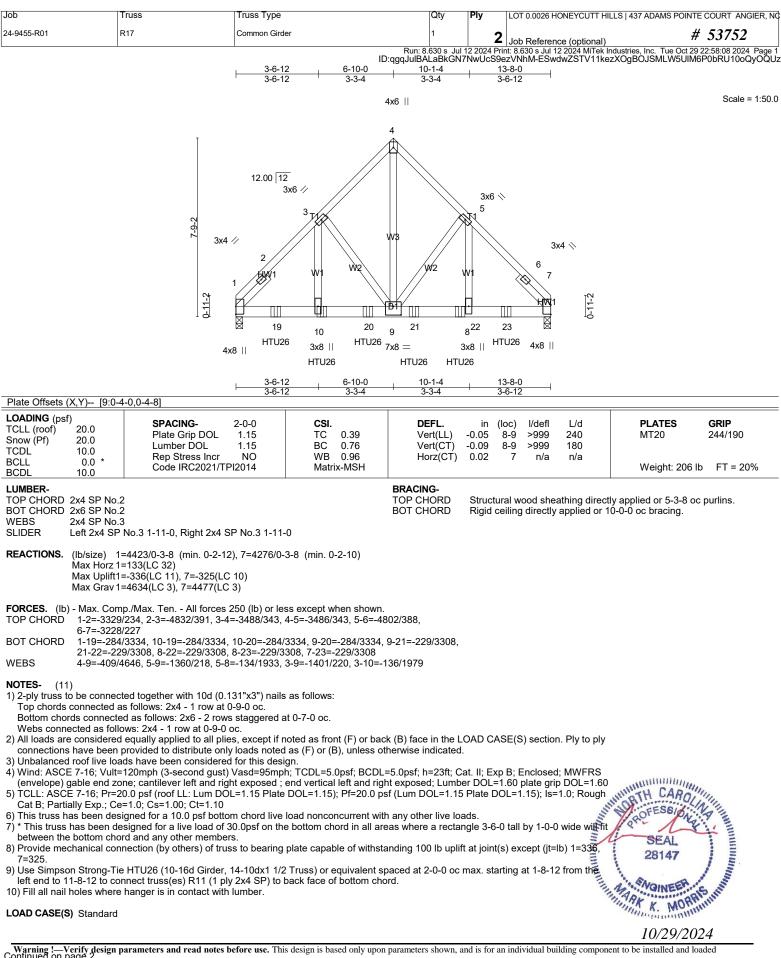
LOAD CASE(S) Standard

1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15 Uniform Loads (plf)

Vert: 1-2=-60, 2-8=-60, 8-13=-60, 13-14=-60, 15-18=-20, 2-13=-8(F) Concentrated Loads (lb)

Vert: 18=4(F) 15=4(F) 17=4(F) 19=4(F) 20=4(F) 21=4(F) 22=4(F) 23=4(F)





[Job	Truss	Truss Type	Qty	Ply	LOT 0.0026 HONEYCUTT HILLS 437 ADAMS POINTE	ECOURT ANGIER, NC
	24-9455-R01	R17	Common Girder	1	2	Job Reference (optional) #	53752
						it: 8.630 s Jul 12 2024 MiTek Industries, Inc. Tue Oct 29 JcS9ezVNhM-jeU?7vT5GL9bG76aEuwY_athru5b	

LOAD CASE(S) Standard

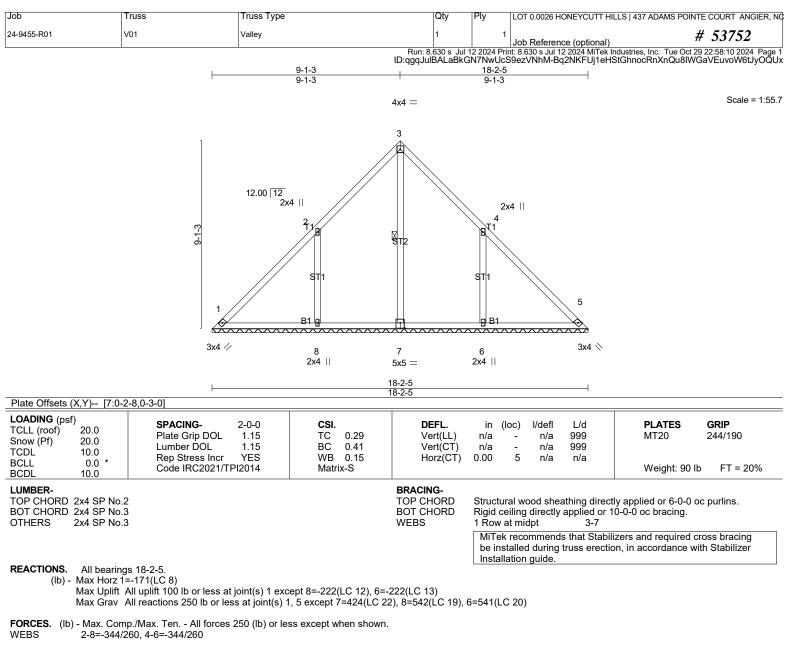
1) Dead + Snow (balanced): Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (plf) Vert: 1-4=-60, 4-7=-60, 11-15=-20

Concentrated Loads (lb)

Vert: 10=-1268(B) 19=-1268(B) 20=-1268(B) 21=-1268(B) 22=-1268(B) 23=-1268(B)





NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

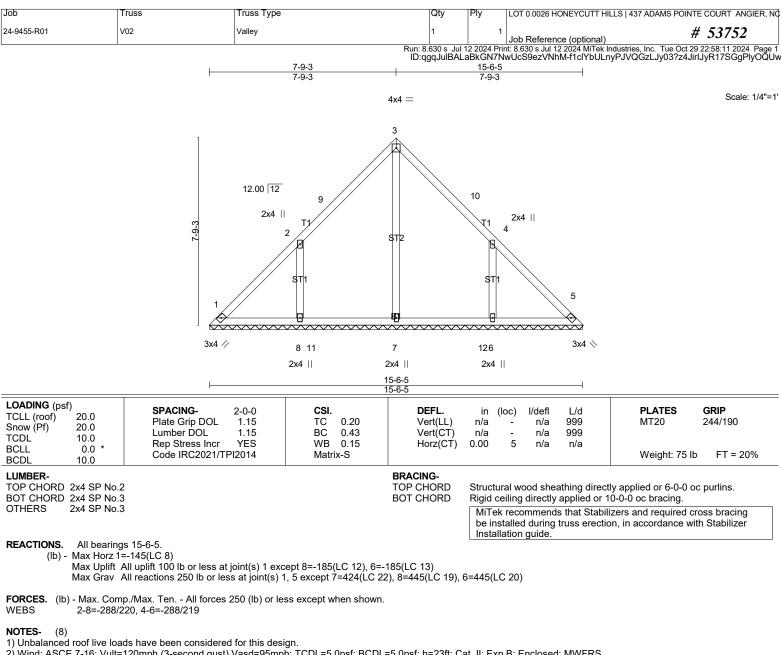
- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-3, Exterior(2R) 5-1-3 to 13-1-3, Exterior(2E) 13-1-3 to 17-10-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 4) Gable requires continuous bottom chord bearing.
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=222, 6=222.

LOAD CASE(S) Standard





- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 10-4-8, Exterior(2E) 10-4-8 to 15-2-2 zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

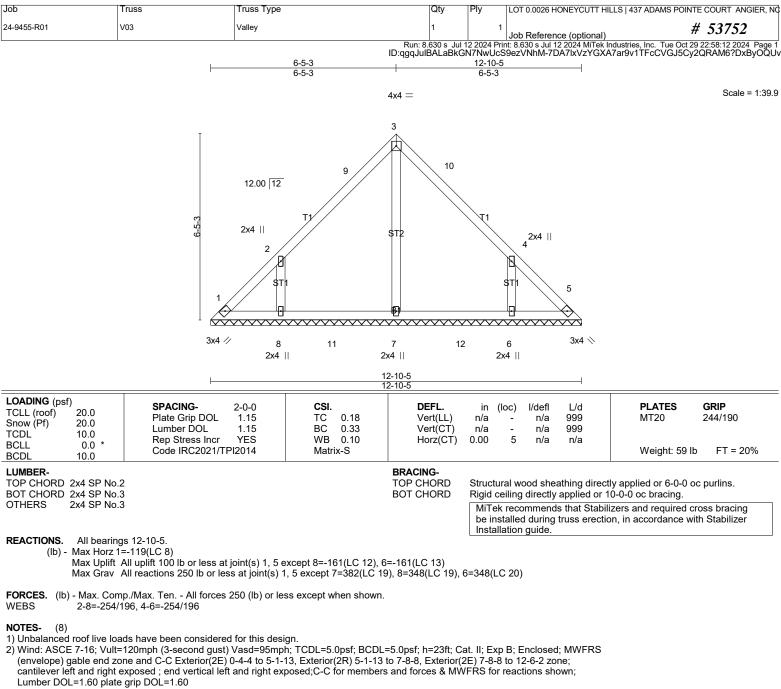
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 8=185, 6=185.

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LOAD CASE(S) Standard
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3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

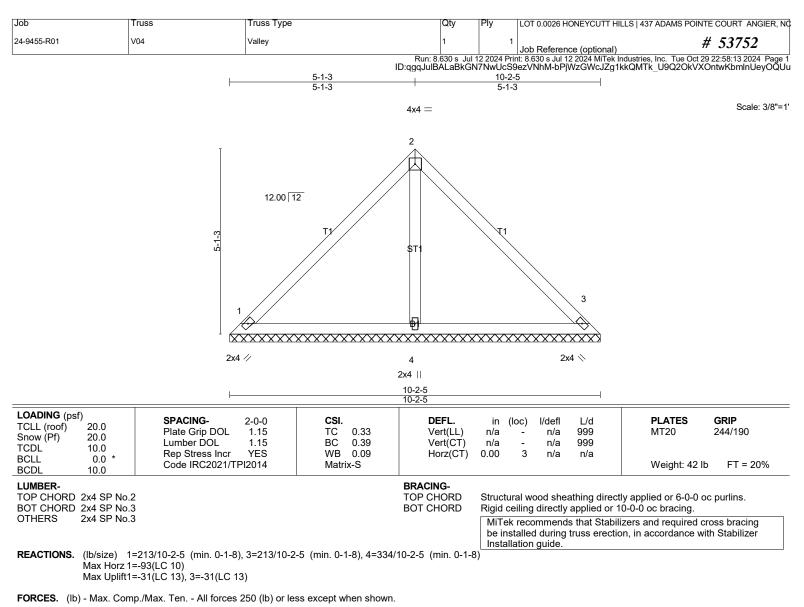
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 8=161, 6=161.

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LOAD CASE(S) Standard
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NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

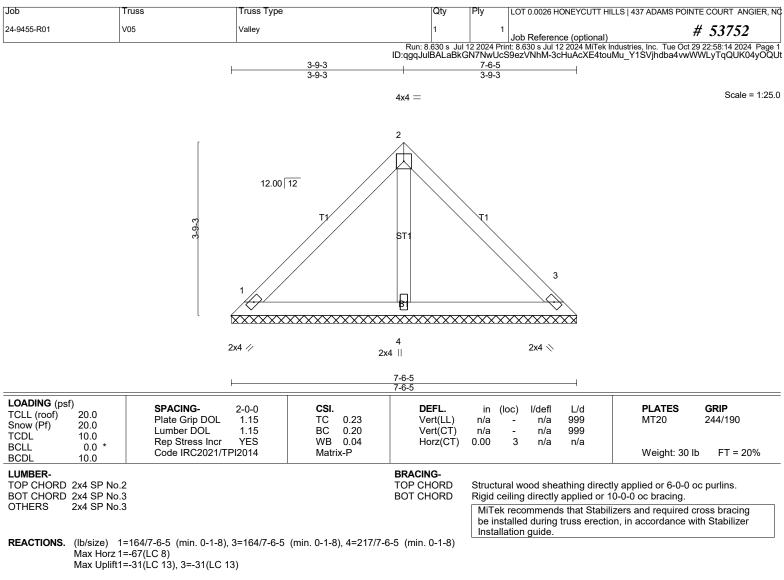
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

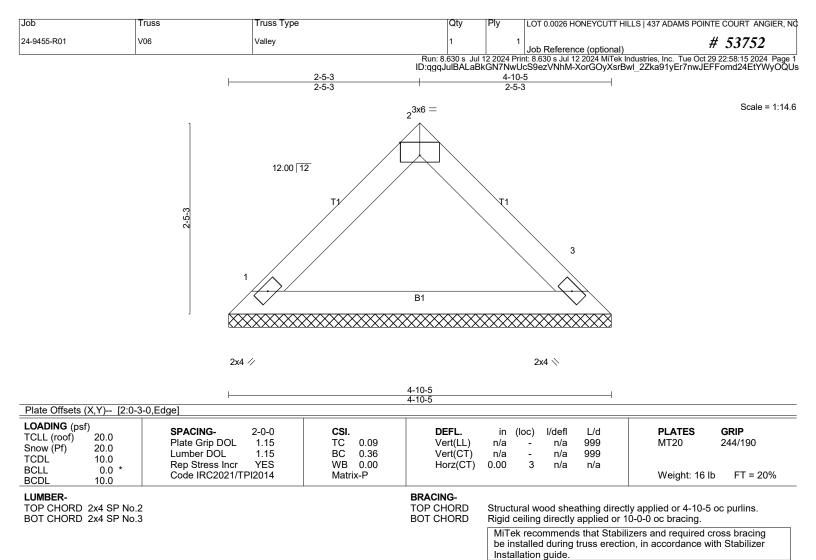
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=166/4-10-5 (min. 0-1-8), 3=166/4-10-5 (min. 0-1-8) Max Horz 1=-41(LC 10) Max Uplift1=-8(LC 12), 3=-8(LC 12)

NOTES- (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

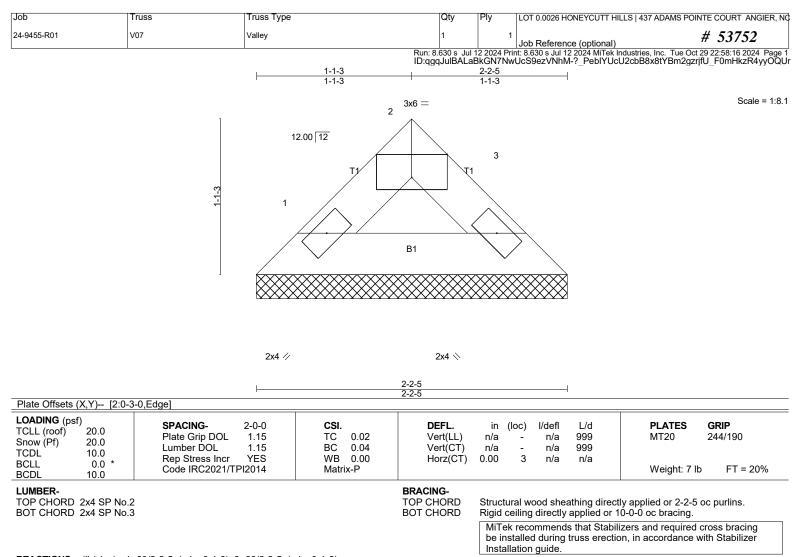
LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.

²⁾ Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



REACTIONS. (Ib/size) 1=60/2-2-5 (min. 0-1-8), 3=60/2-2-5 (min. 0-1-8) Max Horz 1=15(LC 11) Max Uplift1=-3(LC 12), 3=-3(LC 13)

NOTES- (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

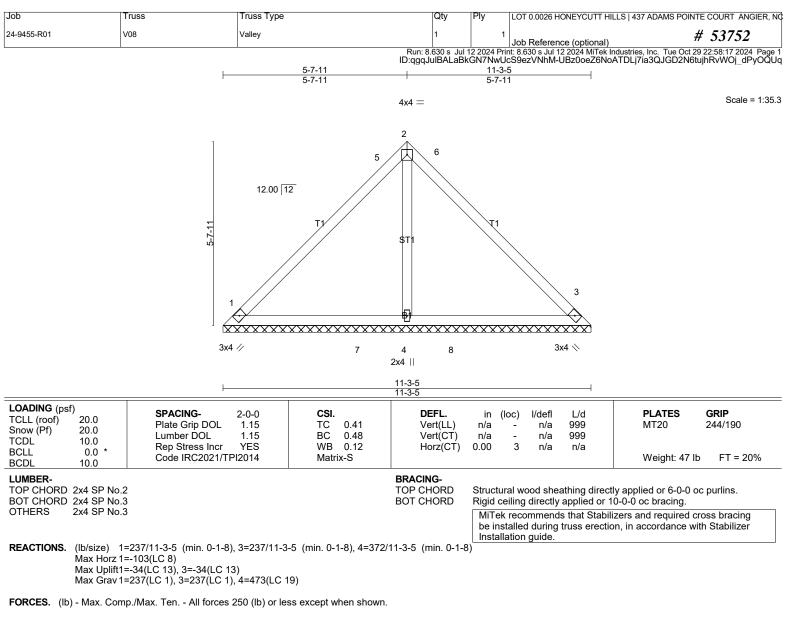
- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.



- NOTES-(8)
- 1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) 0-4-4 to 5-1-13, Exterior(2R) 5-1-13 to 6-1-8, Exterior(2E) 6-1-8 to 10-11-2 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough

Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10 4) Gable requires continuous bottom chord bearing.

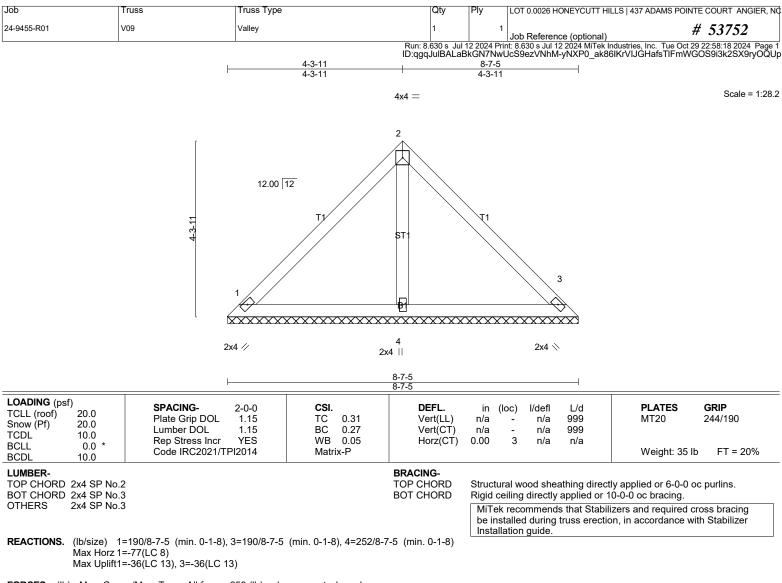
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES- (8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

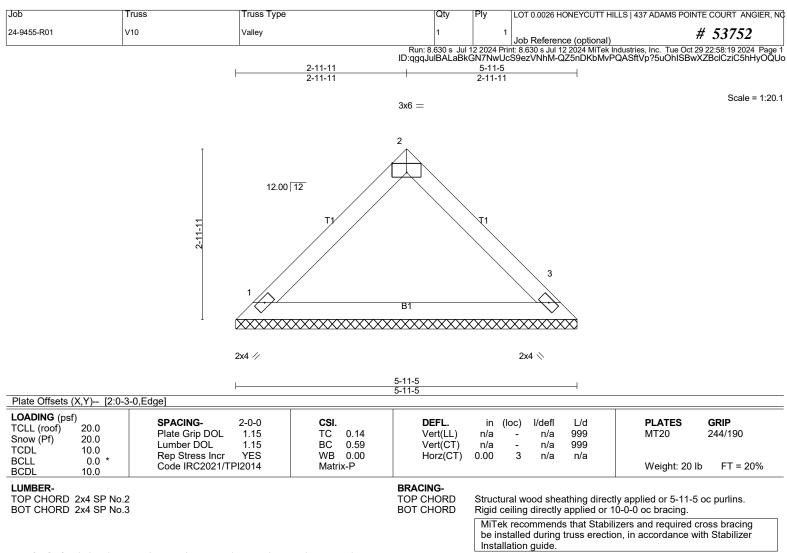
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard





REACTIONS. (lb/size) 1=210/5-11-5 (min. 0-1-8), 3=210/5-11-5 (min. 0-1-8) Max Horz 1=-51(LC 8) Max Uplift1=-10(LC 12), 3=-10(LC 12)

NOTES- (8)

3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

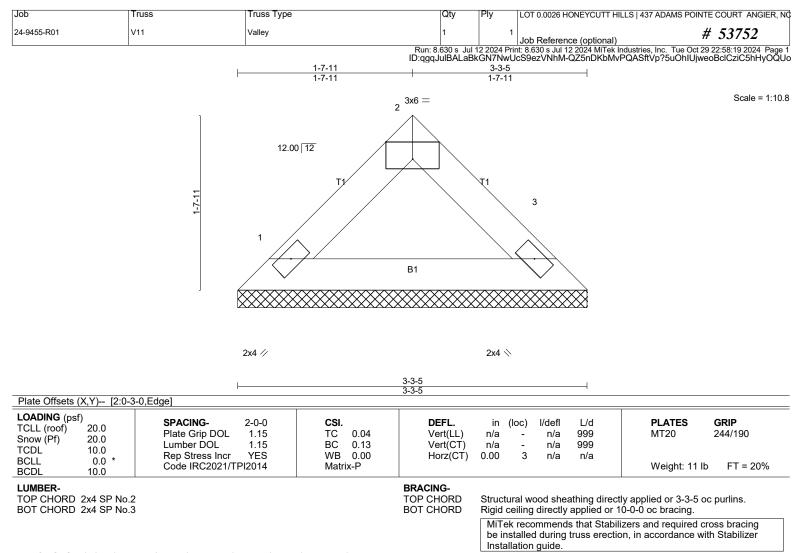
LOAD CASE(S) Standard



FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.

²⁾ Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60



REACTIONS. (lb/size) 1=103/3-3-5 (min. 0-1-8), 3=103/3-3-5 (min. 0-1-8) Max Horz 1=25(LC 11) Max Uplift1=-5(LC 12), 3=-5(LC 13)

NOTES- (8)

- 2) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10

4) Gable requires continuous bottom chord bearing.

5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

6) * This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit between the bottom chord and any other members.

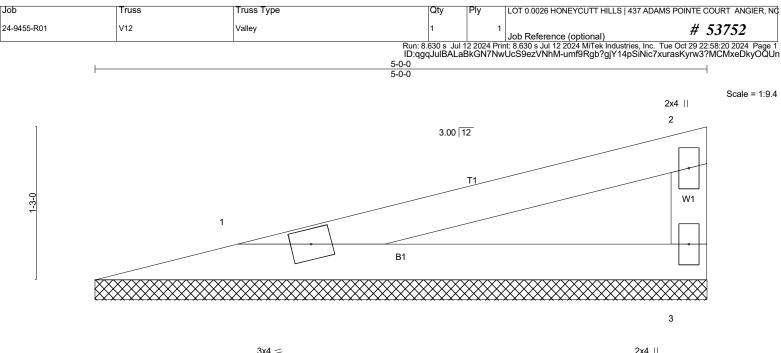
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard



FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

¹⁾ Unbalanced roof live loads have been considered for this design.



2x4 ||

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LOADING (psf) TCLL (roof) 20.0 Snow (Pf) 20.0 TCDL 10.0 BCLL 0.0 * BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2021/TPI2014	CSI. TC 0.33 BC 0.27 WB 0.00 Matrix-P	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (loc) l/defl L/d n/a - n/a 999 n/a - n/a 999 0.00 3 n/a n/a	PLATES GRIP MT20 244/190 Weight: 14 lb FT = 20%
LUMBER- TOP CHORD 2x4 SP No.2 BOT CHORD 2x4 SP No.3 WEBS 2x4 SP No.3			BRACING- TOP CHORD BOT CHORD	Structural wood sheathing dire end verticals. Rigid ceiling directly applied or	ctly applied or 5-0-0 oc purlins, except 10-0-0 oc bracing.
					ilizers and required cross bracing tion, in accordance with Stabilizer

REACTIONS. (lb/size) 1=147/5-0-0 (min. 0-1-8), 3=147/5-0-0 (min. 0-1-8) Max Horz 1=29(LC 11) Max Uplift1=-18(LC 10), 3=-21(LC 14) Max Grav 1=184(LC 20), 3=184(LC 20)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

NOTES-

- 1) Wind: ASCE 7-16; Vult=120mph (3-second gust) Vasd=95mph; TCDL=5.0psf; BCDL=5.0psf; h=23ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=20.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pf=20.0 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.10
- 3) Unbalanced snow loads have been considered for this design.

4) Gable requires continuous bottom chord bearing.

- 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 6)* This truss has been designed for a live load of 30.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 1-0-0 wide will fit
- between the bottom chord and any other members.
- 7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 3.

LOAD CASE(S) Standard

