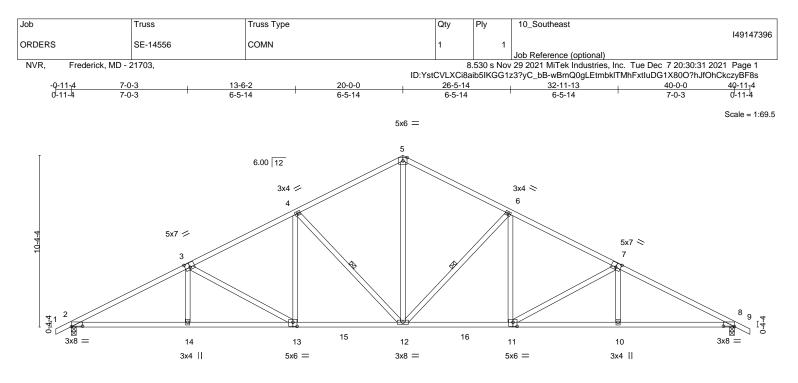




<ol> <li>STUDDED GABLE BRACING DETAIL I/RF-IC TO BE UTILIZED FOR TRUSSES 6'-9" IN HEIGHT OR GREATER.</li> <li>PARTIALLY SHEATHED GABLES, SEE 5/RF-IC FOR "L" BRACING WHEN REQUIRED.</li> </ol>		
SHEET NO. <b>DOMINICA SPRING</b> DRAWING TITLE TRUSS BRACING DETAILS		v rights are not add. or manner to be MM-LOT KIPLING VILLAGE - 0094
22 OPTION	OPTION 5285 Westview Drive, Suite 100 first obtaining the expressed written consent of NVR, Inc.	written IG BRAZAN COURT ICITY FUQUAY-VARINA MC 27526



	L	7-0-3	13-6-2		20-0-0		26-5-14		32-11-13	40-0-	0
	1	7-0-3	6-5-14	1	6-5-14	1	6-5-14	1	6-5-14	7-0-3	3
Plate Offs	sets (X,Y)	[2:0-8-0,0-0-5], [3:0-3-8,	0-3-4], [7:0-3-8	3,0-3-4], [8:0-8	3-0,0-0-5], [1	11:0-3-0,0-3-0], [13	:0-3-0,0-3-0]				
LOADING	G (psf)	SPACING-	2-0-0	CSI.		DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	20.Ó	Plate Grip DOL	1.15	тс	0.99	Vert(LL)	-0.19 11-12	>999	360	MT20	197/144
TCDL	10.0	Lumber DOL	1.15	BC	0.91	Vert(CT)	-0.39 11-12	>999	240		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.66	Horz(CT)	0.16 8	n/a	n/a		
BCDL	10.0	Code IBC2021/T	PI2014	Matrix	<-S	Wind(LL)	0.16 12-13	>999	240	Weight: 221 lb	FT = 5%

BRACING-

WEBS

TOP CHORD

BOT CHORD

Structural wood sheathing directly applied.

1 Row at midpt

Rigid ceiling directly applied or 9-0-13 oc bracing.

4-12, 6-12

## LUMBER-

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2
WEBS	2x4 SP No.3 or 2x4 SPF Stud

- REACTIONS. (size) 2=0-3-8, 8=0-3-8 Max Horz 2=-180(LC 15) Max Uplift 2=-229(LC 10), 8=-229(LC 11) Max Grav 2=1653(LC 1), 8=1653(LC 1)
- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown. TOP CHORD 2-3=-3030/371, 3-4=-2441/332, 4-5=-1866/308, 5-6=-1866/308, 6-7=-2441/332, 7-8=-3030/372 BOT CHORD 2-14=-410/2615, 13-14=-412/2612, 12-13=-240/2105, 11-12=-90/2105, 10-11=-233/2612, 8-10=-231/2615 WEBS 3-14=0/293, 4-13=-19/465, 5-12=-135/1224, 6-11=-19/465, 7-10=0/293, 3-13=-587/198,
- 4-12=-764/256, 6-12=-764/256, 7-11=-587/198

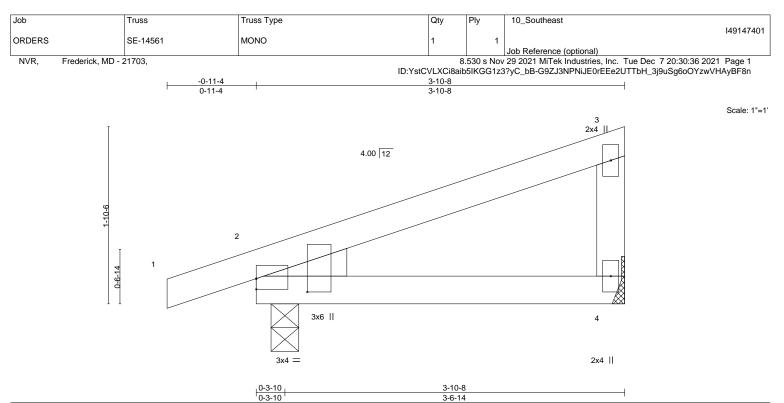
#### NOTES-(6-8)

1) Unbalanced roof live loads have been considered for this design.

- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=229 8=229
- 6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.







OADING (psf)	SPACING- 2	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plate Grip DOL	1.15	тс	0.20	Vert(LL)	-0.01	2-4	>999	360	MT20	197/144
TCDL 10.0	Lumber DOL	1.15	BC	0.21	Vert(CT)	-0.02	2-4	>999	240		
3CLL 0.0 '	Rep Stress Incr	YES	WB	0.00	Horz(CT)	0.00		n/a	n/a		
BCDL 10.0	Code IBC2021/TPI20	014	Matrix	(-P	Wind(LL)	0.00	2	****	240	Weight: 17 lb	FT = 5%

TOP CHORD	2x4 SP No.2 or 2x4 SPF No.2	TOP CHORD	Structural wood sheathing directly applied or 3-10-8 oc purlins,
BOT CHORD	2x4 SP No.2 or 2x4 SPF No.2		except end verticals.
WEBS	2x4 SP No.3 or 2x4 SPF Stud	BOT CHORD	Rigid ceiling directly applied or 10-0-0 oc bracing.
WEDGE			

Left: 2x4 SP or SPF No.3 or Stud

REACTIONS. (size) 4=Mechanical, 2=0-3-8

Max Horz 2=62(LC 6) Max Uplift 4=-37(LC 10), 2=-66(LC 6) Max Grav 4=134(LC 1), 2=218(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

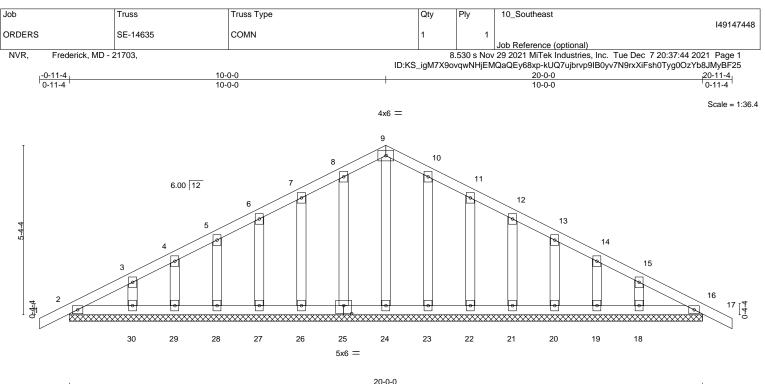
## NOTES- (6-8)

- Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 3) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 4) Refer to girder(s) for truss to truss connections.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 4, 2.
- 6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



January 12,2022





# 20-0-0

OADING (psf)	SPACING- 2-0-0	CSI.	DEFL. in (loc) I/defl L/d	PLATES GRIP
CLL 20.0	Plate Grip DOL 1.15	TC 0.06	Vert(LL) -0.00 16 n/r 120	MT20 197/144
CDL 10.0	Lumber DOL 1.15	BC 0.03	Vert(CT) -0.00 17 n/r 120	
CLL 0.0 *	Rep Stress Incr YES	WB 0.05	Horz(CT) 0.00 16 n/a n/a	
CDL 10.0	Code IBC2021/TPI2014	Matrix-S		Weight: 118 lb FT = 5%

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.3 or 2x4 SPF Stud OTHERS

TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 20-0-0.

(lb) - Max Horz 2=94(LC 10)

Max Uplift All uplift 100 lb or less at joint(s) 2, 25, 26, 27, 28, 29, 30, 23, 22, 21, 20, 19, 18, 16 Max Grav All reactions 250 lb or less at joint(s) 2, 24, 25, 26, 27, 28, 29, 30, 23, 22, 21, 20, 19, 18, 16

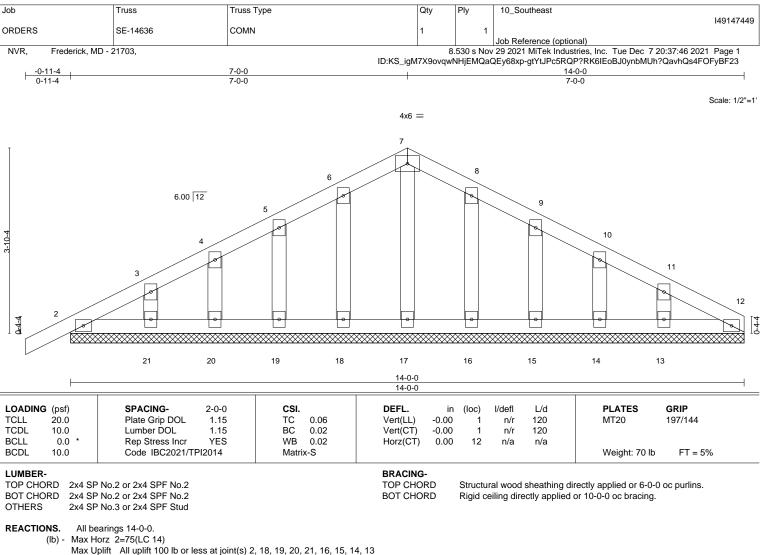
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

#### NOTES-(10-12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 4) All plates are 3x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 25, 26, 27, 28, 29. 30. 23. 22. 21. 20. 19. 18. 16.
- 10) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 11) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 12) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.







Max Grav All reactions 250 lb or less at joint(s) 2, 12, 17, 18, 19, 20, 21, 16, 15, 14, 13

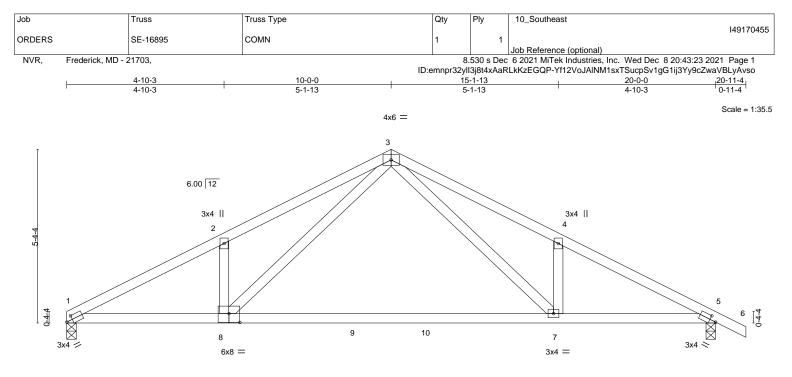
FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

### NOTES-(10-12)

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- All plates are 3x4 MT20 unless otherwise indicated.
- 5) Gable requires continuous bottom chord bearing.
- 6) Gable studs spaced at 1-4-0 oc.
- 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 8) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 18, 19, 20, 21, 16, 15, 14, 13.
- 10) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 11) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 12) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.







Hate Offsets (X,Y) [1:0-2-5,0-1-8], [5:0-2-5,0-1-8]		<u>10-0-0</u> <u>5-1-13</u> , [8:0-4-0,0-3-4]	<u>15-1-13</u> 5-1-13	20-0-0 4-10-3	
LOADING (psf) TCLL 20.0 TCDL 10.0 3CLL 0.0 * 3CDL 10.0	SPACING- 2-0- Plate Grip DOL 1.1 Lumber DOL 1.1 Rep Stress Incr YE Code IBC2021/TPI2014	TC         0.32           15         BC         0.81           ES         WB         0.30	Vert(LL) -0.26 7-8 > Vert(CT) -0.54 7-8 > Horz(CT) 0.03 5	/defi L/d PLATES 925 360 MT20 439 240 n/a 1/a 999 240 Weight: 1	197/144
UMBER-	No.2 or 2x4 SPF No.2	· · ·	BRACING- TOP CHORD Structural	wood sheathing directly applied or	4-5-10 oc purlins.

BOT CHORD

Rigid ceiling directly applied or 10-0-0 oc bracing.

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 WEBS 2x4 SP No.3 or 2x4 SPF Stud

(size) 1=0-3-8, 5=0-3-8 Max Horz 1=-101(LC 15) Max Uplift 1=-101(LC 10), 5=-126(LC 11)

Max Grav 1=787(LC 1), 5=855(LC 1)

- FORCES. (Ib) Max. Comp./Max. Ten. All forces 250 (Ib) or less except when shown.
- TOP CHORD 1-2=-1466/148, 2-3=-1469/273, 3-4=-1456/267, 4-5=-1458/143

BOT CHORD 1-8=-157/1248, 7-8=-46/758, 5-7=-57/1238

WEBS 4-7=-298/211, 2-8=-307/215, 3-8=-155/706, 3-7=-149/691

NOTES-(6-8)

REACTIONS.

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.

5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=101, 5=126.

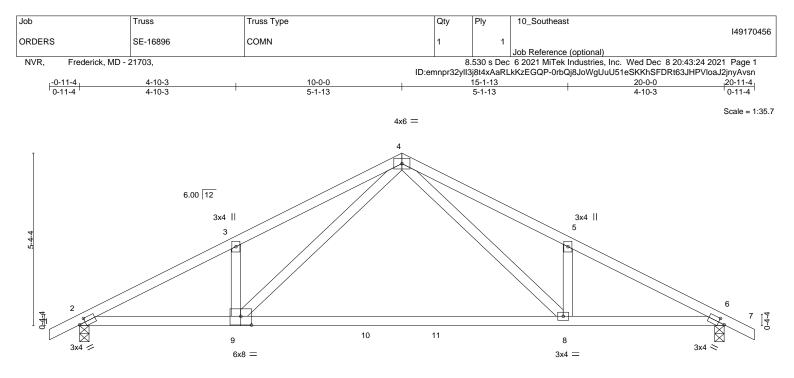
6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.

7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.







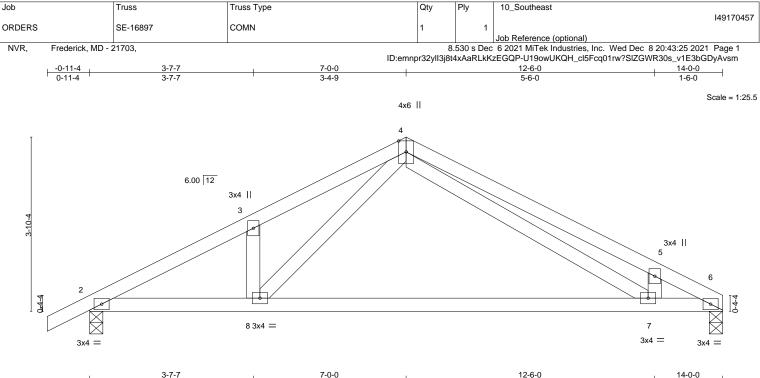
L	4-10-3	10-0-0		15-1-13			20-0-0	
	4-10-3	5-1-13	l	5-1-13		1	4-10-3	I
Plate Offsets (X,Y)	[2:0-2-5,0-1-8], [6:0-2-5,0-1-8], [9:0-4-0	,0-3-4]	1					
LOADING (psf)	SPACING- 2-0-0	CSI.	DEFL.	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0 TCDL 10.0	Plate Grip DOL 1.15 Lumber DOL 1.15	TC 0.31 BC 0.81	( )	0.26 8-9 0.54 8-9	>926 >439	360 240	MT20	197/144
BCLL 0.0 *	Rep Stress Incr YES	WB 0.30		0.54 6-9	>439 n/a	240 n/a		
BCDL 10.0	Code IBC2021/TPI2014	Matrix-S		0.05 8-9		11/a 240	Weight: 94 lb	FT = 5%
DODE 10.0		Matrix-0	VVIIId(LL)	0.05 0-5	2333	240	Weight. 94 lb	11 = 576
LUMBER-       BRACING-         TOP CHORD       2x4 SP No.2 or 2x4 SPF No.2       TOP CHORD       Structural wood sheathing directly applied or 4-5-12 oc purlins.         BOT CHORD       2x4 SP No.2 or 2x4 SPF No.2       BOT CHORD       Structural wood sheathing directly applied or 10-0-0 oc bracing.         WEBS       2x4 SP No.3 or 2x4 SPF Stud       BOT CHORD       Rigid ceiling directly applied or 10-0-0 oc bracing.         REACTIONS.       (size)       2=0-3-8, 6=0-3-8 Max Horz       2=94(LC 14) Max Uplit(2=-126(LC 10), 6=-126(LC 11) Max Grav       Structural wood sheathing directly applied or 10-0-0 oc bracing.								
FORCES.       (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown.         TOP CHORD       2-3=-1454/142, 3-4=-1452/266, 4-5=-1452/266, 5-6=-1454/142         BOT CHORD       2-9=-151/1234, 8-9=-45/754, 6-8=-57/1234         WEBS       5-8=-297/211, 3-9=-297/211, 4-9=-149/691								
<ol> <li>Wind: ASCE 7-16; W MWFRS (envelope) MWFRS for reaction</li> <li>This truss has been</li> <li>* This truss has bee will fit between the b</li> </ol>	e loads have been considered for this d /ult=130mph (3-second gust) Vasd=100 gable end zone and C-C Exterior(2) zo is shown; Lumber DOL=1.60 plate grip designed for a 10.0 psf bottom chord li n designed for a live load of 20.0psf on vottom chord and any other members, v connection (by others) of truss to bear	mph; TCDL=6.0psf; BCD ne; cantilever left and righ DOL=1.60 ve load nonconcurrent with the bottom chord in all are vith BCDL = 10.0psf.	it exposed ;C-C for n h any other live load eas where a rectang	nembers and s. le 3-6-0 tall I	d forces &	le		

- 6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1** Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

818 Soundside Road Edenton, NC 27932



	3-7-7	7-0-0 3-4-9	+ <u>12-6-0</u> 5-6-0	<u> </u>
LOADING (psf)	<b>SPACING-</b> 2-0-0	CSI. DEFI	in (loc) l/defl L/d PL	LATES GRIP
TCLL 20.0 TCDL 10.0 BCLL 0.0 *	Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	TC         0.46         Vert(           BC         0.65         Vert(           WB         0.28         Horz	CT) -0.30 7-8 >555 240	T20 197/144
BCDL 10.0	Code IBC2021/TPI2014	Matrix-S Wind		/eight: 65 lb FT = 5%

BRACING-

TOP CHORD

BOT CHORD

# LUMBER-

TOP CHORD 2x4 SP No.2 or 2x4 SPF No.2 BOT CHORD 2x4 SP No.2 or 2x4 SPF No.2 2x4 SP No.3 or 2x4 SPF Stud WEBS

REACTIONS. (size) 2=0-3-8, 6=0-3-8 Max Horz 2=75(LC 14) Max Uplift 2=-95(LC 10), 6=-70(LC 11) Max Grav 2=616(LC 1), 6=546(LC 1)

FORCES. (Ib) - Max. Comp./Max. Ten. - All forces 250 (Ib) or less except when shown.

- TOP CHORD 2-3=-994/78, 3-4=-966/161, 4-5=-1248/219, 5-6=-1203/87
- BOT CHORD 2-8=-87/830, 7-8=-38/524, 6-7=-92/1050

WEBS 4-8=-70/469, 4-7=-129/657

### NOTES-(6-8)

1) Unbalanced roof live loads have been considered for this design.

2) Wind: ASCE 7-16; Vult=130mph (3-second gust) Vasd=103mph; TCDL=6.0psf; BCDL=6.0psf; h=33ft; Cat. II; Exp B; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

- 4) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 6. 6) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction
- to a wind speed of 115 mph.

7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

8) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.



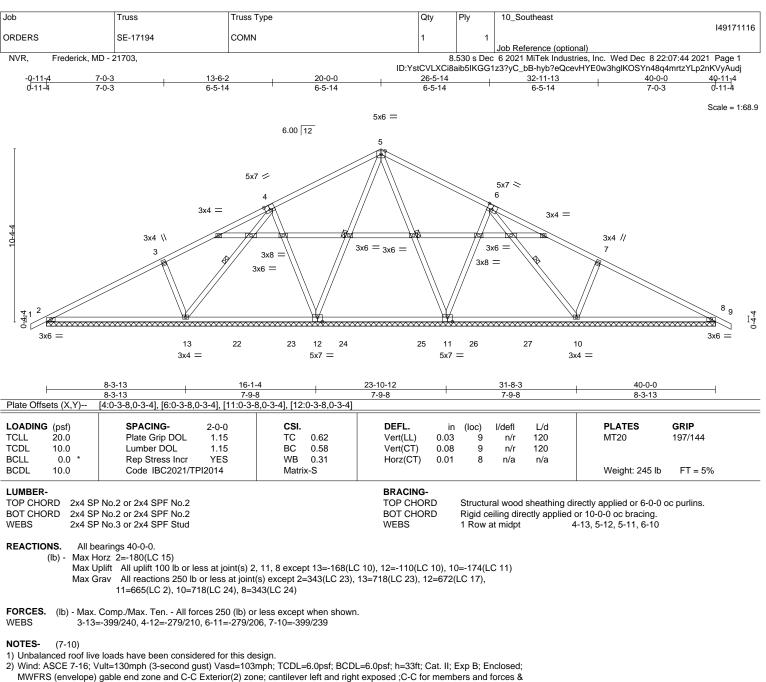
Structural wood sheathing directly applied or 4-4-12 oc purlins.

Rigid ceiling directly applied or 10-0-0 oc bracing.

January 12,2022

818 Soundside Road

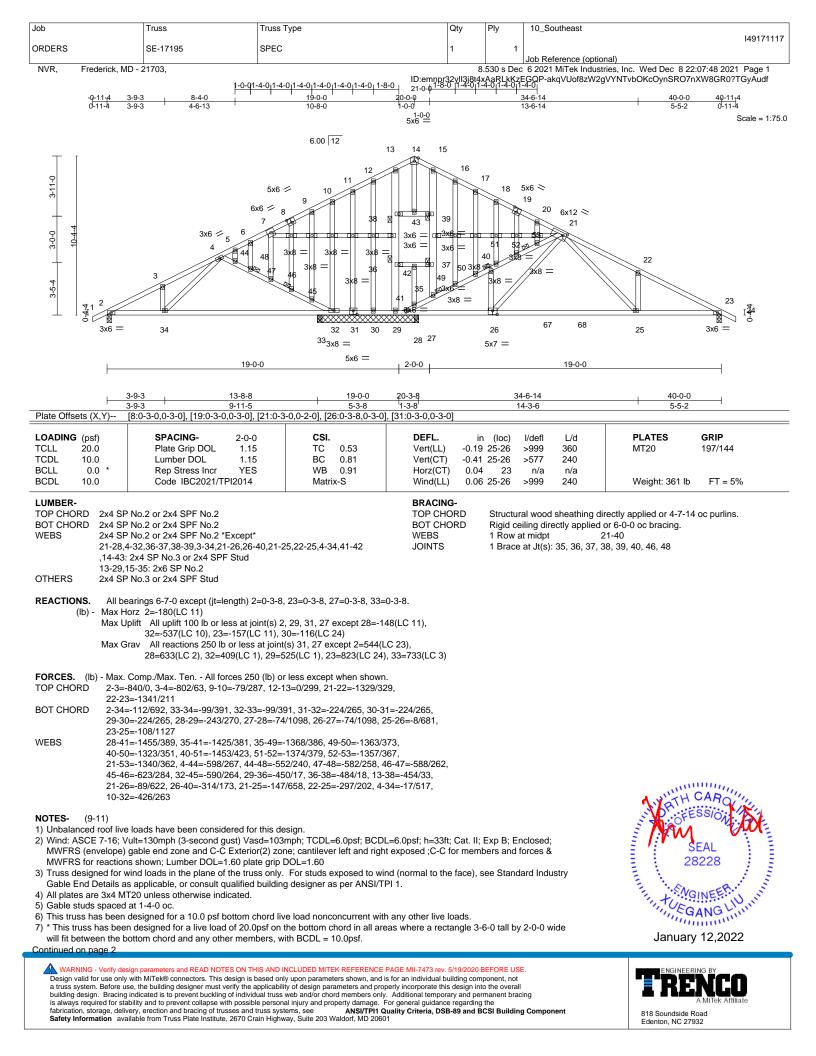
Edenton, NC 27932



- MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) Gable requires continuous bottom chord bearing.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) \* This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
- 6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 11, 8 except (jt=lb) 13=168, 12=110, 10=174.
- 7) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
- 8) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.
- 9) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.
- 10) Framing and bracing of the gable end frame shall be provided by the building designer.







Job		Truss	Truss Type	Qty	Ply	10_Southeast	
ORDERS		SE-17195	SPEC	1	1	I49171117	
						Job Reference (optional)	
NVR,	NVR,         Frederick, MD - 21703,         8.530 s Dec 6 2021 MiTek Industries, Inc. Wed Dec 8 22:07:48 2021 Pa						
	ID:emnpr32yll3j8t4xAaRLkKzEGQP-akqVUof8zW2gVYNTvbOKcOynSRO7nXW8GR0?T						

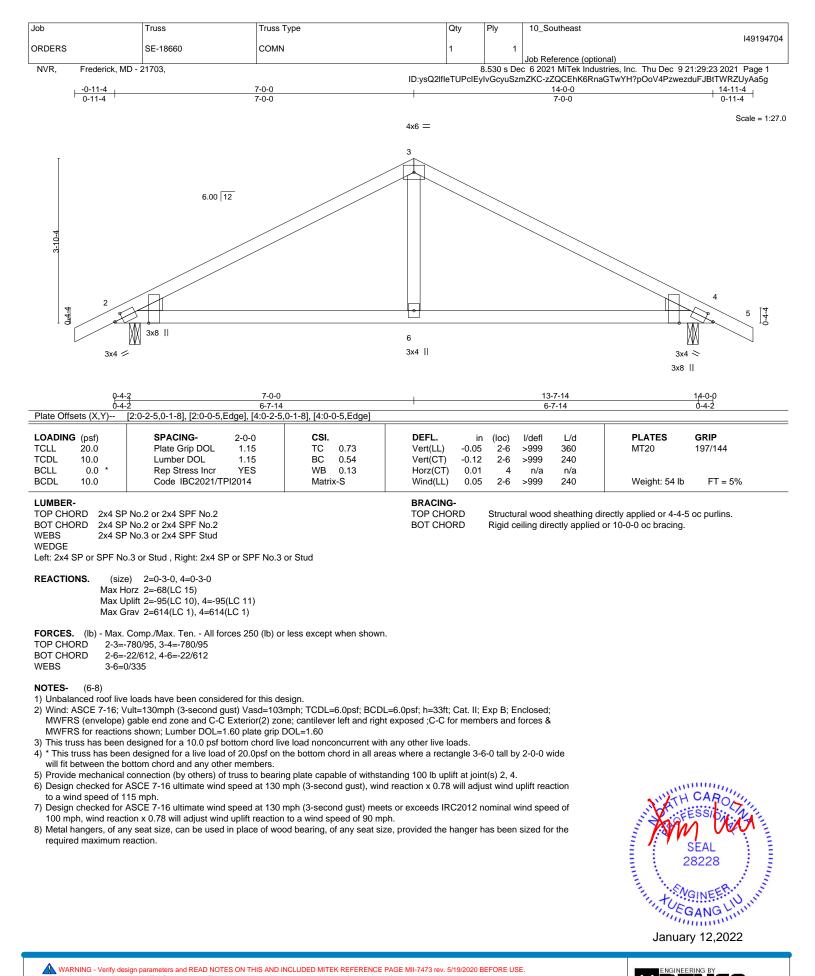
NOTES- (9-11)

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2, 29, 31, 27 except (jt=lb) 28=148, 32=537, 23=157, 30=116.

9) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust), wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 115 mph.
 10) Design checked for ASCE 7-16 ultimate wind speed at 130 mph (3-second gust) meets or exceeds IRC2012 nominal wind speed of 100 mph, wind reaction x 0.78 will adjust wind uplift reaction to a wind speed of 90 mph.

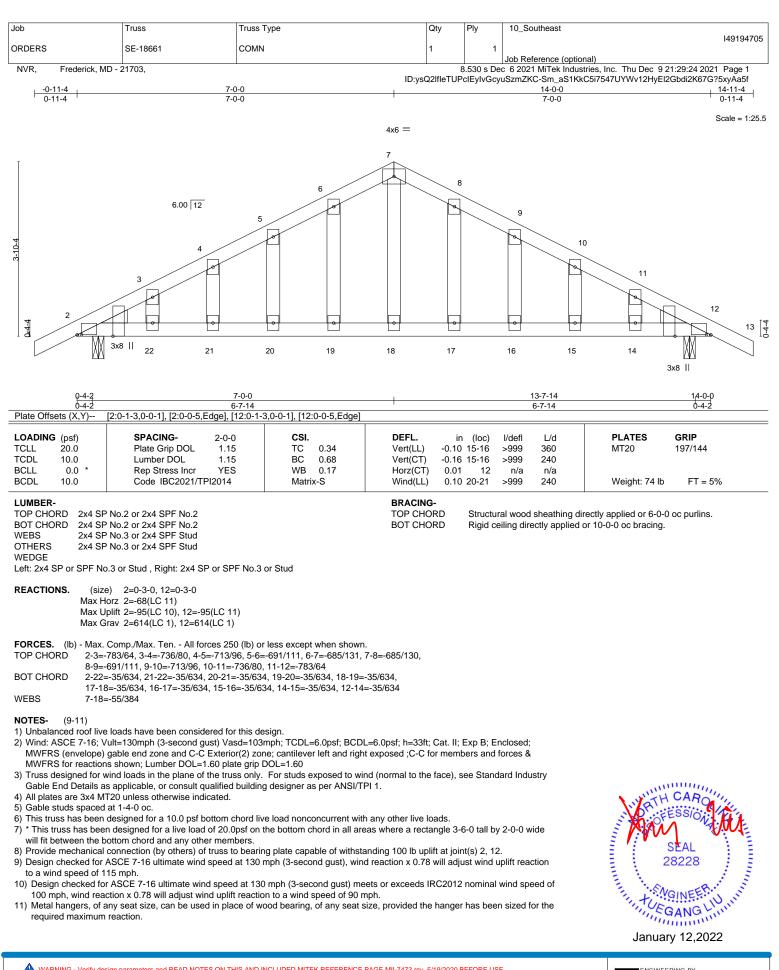
11) Metal hangers, of any seat size, can be used in place of wood bearing, of any seat size, provided the hanger has been sized for the required maximum reaction.







Edenton, NC 27932



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 5/19/2020 BEFORE USE. Design valid for use only with MITek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 2670 Crain Highway, Suite 203 Waldorf, MD 20601

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